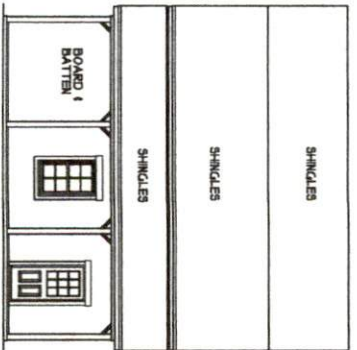
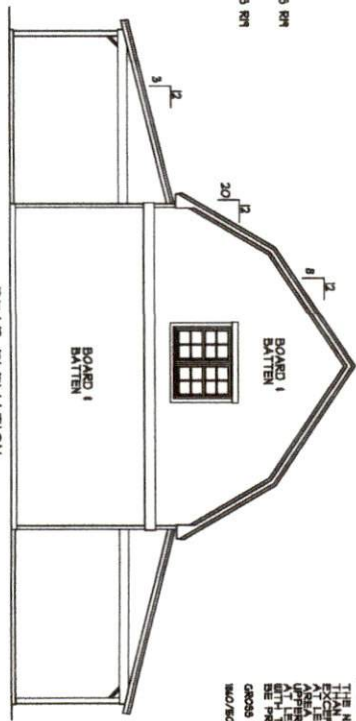


FRONT ELEVATION
SCALE: 1/8" = 1'-0"

ENERGY COMPLIANCE
ZONE 3 - MAX GLAZING U-FACTOR .25
R-VALUE = 5.0 PER SQUARE FOOT
ZONE 4 - MAX GLAZING U-FACTOR .25
R-VALUE = 5.0 PER SQUARE FOOT
FOR BAKE, DERRAH, ORANGE COUNTY

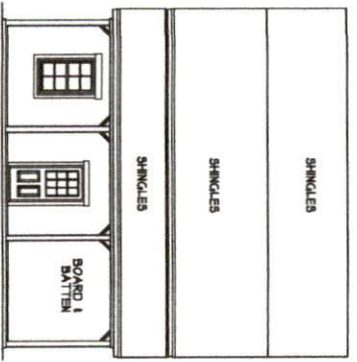


LEFT ELEVATION
SCALE 1/8" = 1'-0"



REAR ELEVATION
SCALE 1/8" = 1'-0"

ATTIC VENTILATION:
THE NET GROSS VENTILATED AREA SHALL BE NOT LESS THAN 1% OF THE UNFINISHED ATTIC SPACE PROVIDED EXCEPT THAT THE AREA MAY BE REDUCED TO 0.7% IF THE AREA IS PROVIDED BY VENTILATORS LOCATED IN THE UPPER PORTION OF AN INCLINED ROOF OR ROOF VENTS WITH THE BALANCE OF THE REQUIRED VENTILATION TO BE PROVIDED BY EAVE OR CORNER VENTS.
GROSS ATTIC AREA TO BE VENTILATED 140 SQ.FT.
140/100 = 1.4 SQ.FT. NET FREE AREA



RIGHT ELEVATION
SCALE 1/8" = 1'-0"

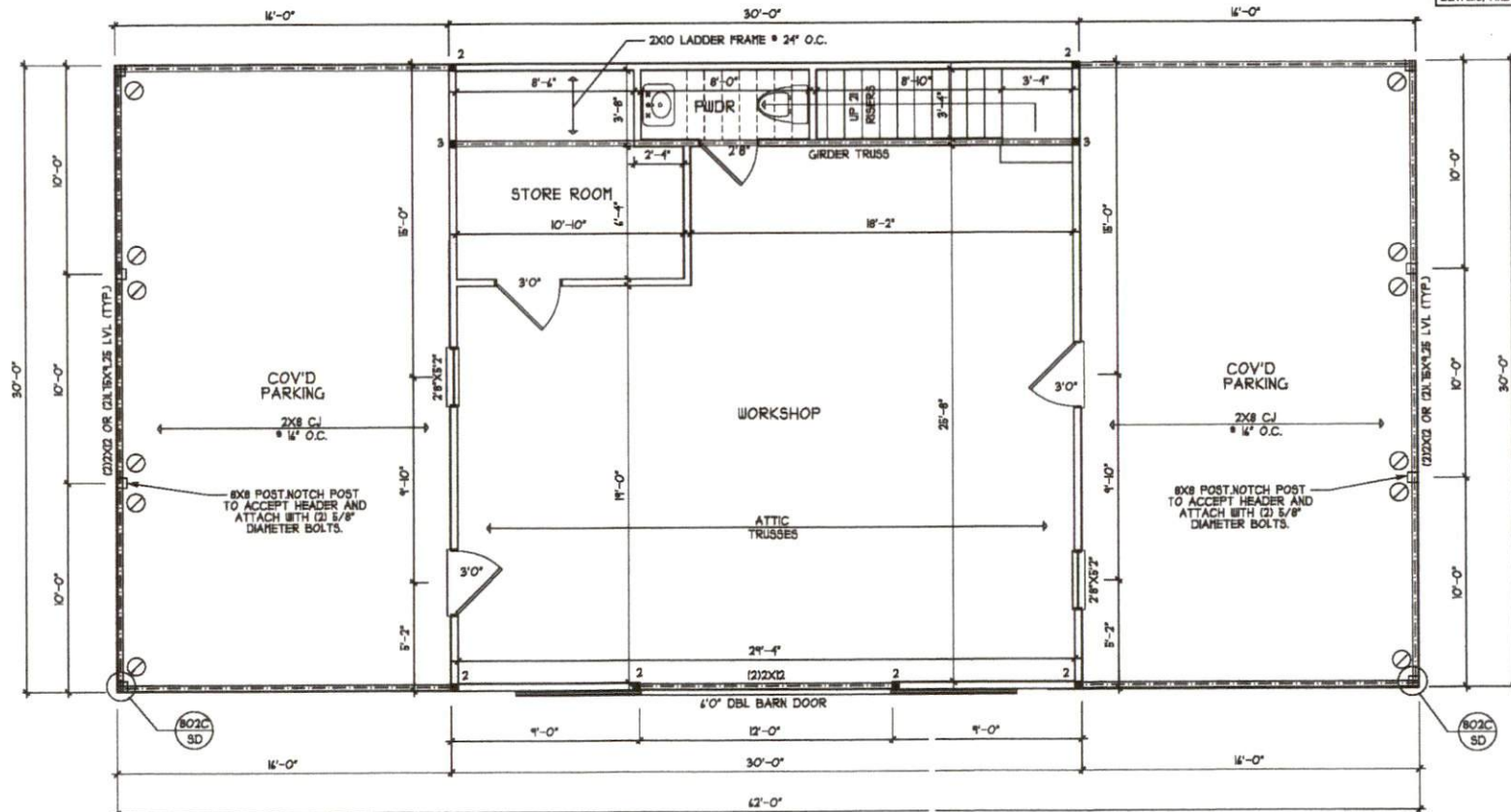
<p>H SQUARED HOME DESIGN, INC.</p>	<p>HEATHER HALL 185 HEATHERSTONE CT BENSON NC 27504 919 207-1403</p>	<p>SQUARE FOOTAGE:</p> <p>WORKSHOP = 900 each PARKING = 480 SECOND FLOOR = 610</p>	<p>FOOTAGE:</p> <p>#1510</p>	<p>CURT & ASHLEY HONEYCUTT GAR.</p>
	<p>THIS PLAN IS TO BE USED FOR PERMITS ONLY. IT IS THE RESPONSIBILITY OF THE SUBMITTER TO OBTAIN ALL NECESSARY PERMITS AND APPROVALS FROM THE LOCAL BUILDING DEPARTMENT. THE SUBMITTER SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE LOCAL BUILDING DEPARTMENT.</p>	<p>DATE: 02/18/2022</p>	<p>FILE: 081021.3</p>	<p>GARAGE</p>



STRUCTURAL DESIGN BY:
SOUTHERN ENGINEERS, P.A.
376 BENSON DR., RALEIGH, NC 27604
LICENSE: C-TT2, PHONE: 919-878-4671
PROJECT #: 22-1210

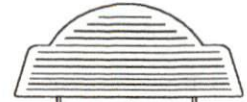
- Engineers seal applies only to structural components on this document. Seal does not include construction means, methods, techniques, sequences, procedures or safety precautions.
- Any deviations or discrepancies on plans are to be brought to the immediate attention of Southern Engineers. Failure to do so will void Southern Engineer's liability.
- Seal is valid for a project permitted one year from date of seal.
- Use of these plans constitutes approval of terms & conditions as defined in the customer agreement.

REFER TO "B0" SHEETS FOR STANDARD DETAILS, BRACING DETAILS, AND STRUCTURAL NOTES



⊙ DENOTES "Y" BRACE, SEE SHEET SD FOR DETAIL

FIRST FLOOR PLAN
SCALE 1/4" = 1'-0"



CURT & ASHLEY
HONEYCUTT GAR.

FOOTAGE
#1510

SQUARE FOOTAGE
WORKSHOP = 800
each PARKING = 480
SECOND FLOOR = 80

HEATHER HALL
185 HEATHERSTONE CT
BENSON NC 27504
(888) 207-1403



ANY DEVIATION OF THE SPECIFIED DIMENSIONS OR DIMENSIONED VIEWS IS SOQUARES HOME DESIGN, INC.'S LIABILITY.

THIS PLAN HAS BEEN DRAWN IN ACCORDANCE WITH NORTH CAROLINA STATE RESIDENTIAL BUILDING CODES 2008 EDITION.

THIS PLAN IS TO ONLY BE BUILT BY THE ABOVE CITED BUILDER OR HOMEOWNER. NOT FOR MULTIPLE BUILDS UNLESS APPROVED BY H SQUARED.

DATE: 02/16/2023

GARAGE

FILE: 081021.3

TRUSS SYSTEM REQUIREMENTS

NC (2018 NCRC) Wind 115-120 MPH

1. TRUSS SYSTEM LAYOUTS (PLACEMENT PLANS) SHALL BE DESIGNED IN ACCORDANCE WITH SEALED STRUCTURAL PLANS. ANY NEED TO CHANGE TRUSSES SHALL BE COORDINATED WITH SOUTHERN ENGINEERS.

2. TRUSS SCHEMATICS (PROFILES) SHALL BE PREPARED AND SEALED BY TRUSS MANUFACTURER.

3. ALL TRUSSES SHALL BE DESIGNED FOR BEARING ON SPP #2 OR #3 PLATES OR LEDGERS (UNO).

4. ALL REQUIRED ANCHORS FOR TRUSSES DUE TO UPLIFT OR BEARING SHALL MEET THE REQUIREMENTS AS SPECIFIED ON THE TRUSS SCHEMATICS.

HEADER/BEAM & COLUMN NOTES

1. ALL EXTERIOR AND LOAD BEARING HEADERS SHALL BE MIN. (2) 2x10 (4" WALL) OR (3) 2x10 (4" WALL) WITH (1) SUPPORT STUD, UNLESS NOTED OTHERWISE.

2. THE NUMBER SHOWN AT BEAM AND HEADER SUPPORTS INDICATES THE NUMBER OF SUPPORT STUDS REQUIRED IN STUD POCKET OR COLUMN. THE NUMBER OF KING STUDS AT EACH END OF HEADERS IN EXTERIOR WALLS SHALL BE ACCORDING TO ITEM #4 IN TABLE R402.3(5) OR AS BELOW PER MCD01 COMMENTARY 'KING STUDS AT WALL OPENINGS' REVISED 1-9-2020.

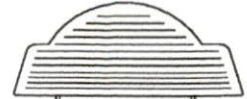
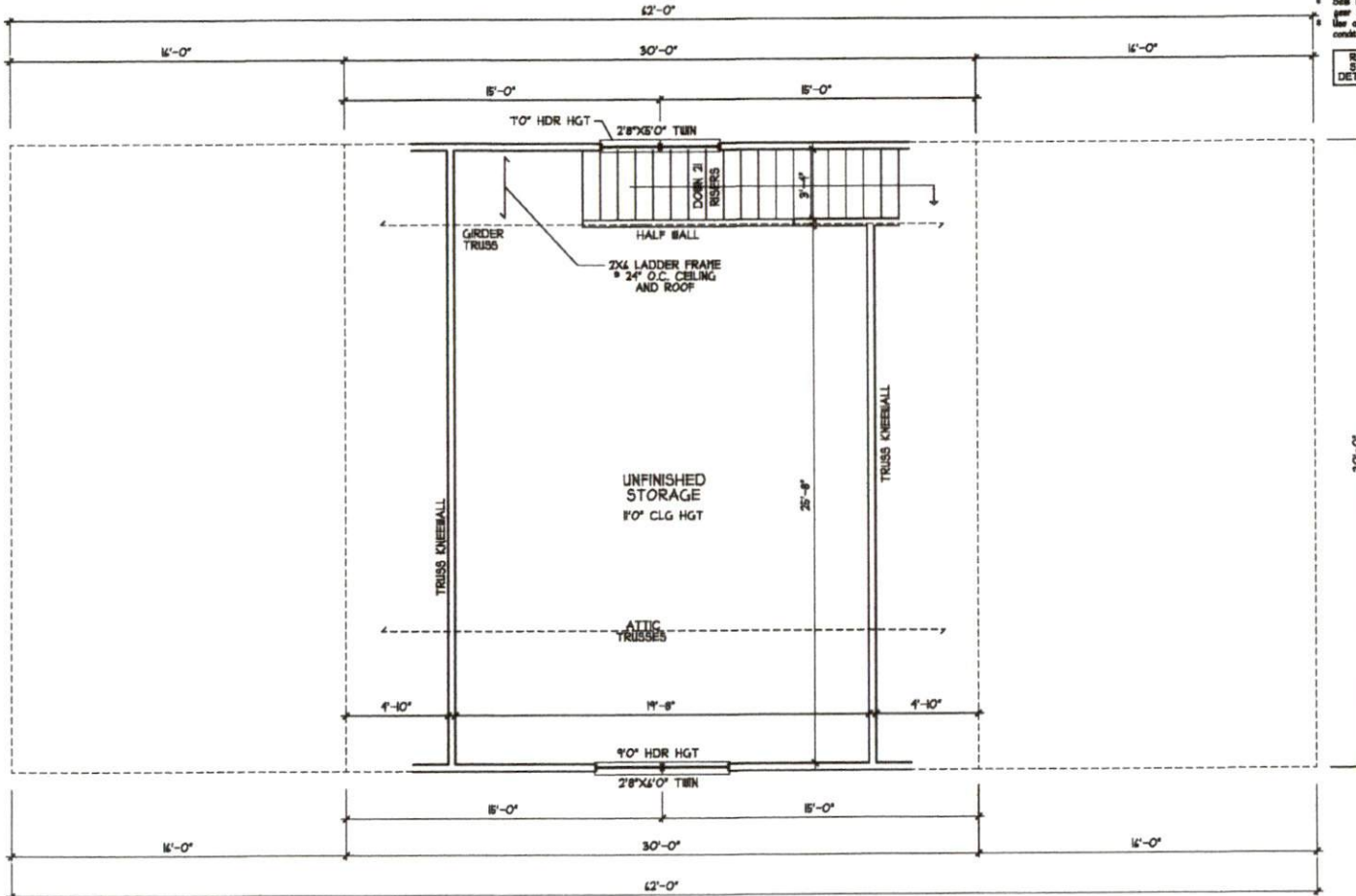
- UP TO 3' SPAN: (1) KING STUD
- OVER 3' UP TO 4' SPAN: (2) KING STUDS
- OVER 4' UP TO 7' SPAN: (3) KING STUDS
- OVER 7' UP TO 12' SPAN: (4) KING STUDS
- OVER 12' UP TO 15' SPAN: (5) KING STUDS



STRUCTURAL DESIGN BY:
SOUTHERN ENGINEERS, P.A.
374 BENSON DR., RALEIGH, NC 27604
LICENSE: C-TID, PHONE: 919-518-4071
PROJECT #: 22-1210

- 1. Engineers seal applies only to structural components on this document. Seal does not include construction means, methods, techniques, sequences, procedures or safety precautions.
- 2. Any deviations or discrepancies on plans are to be brought to the immediate attention of Southern Engineers. Failure to do so will void Southern Engineer's liability.
- 3. Seal is valid for a project permitted one year from date of seal.
- 4. Use of these plans constitutes approval of terms & conditions as defined in the customer agreement.

REFER TO '90' SHEETS FOR STANDARD DETAILS, BRACING DETAILS, AND STRUCTURAL NOTES

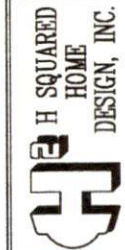


CURT & ASHLEY
HONEYCUTT GAR.

FOOTAGE: #1510

SQUARE FOOTAGE:
WORKSHOP = 800
with PARKING = 480
SECOND FLOOR = 80

HEATHER HALL
185 HEATHERSTONE CT
BENSON NC 27504
(881) 207-1403



ANY DEVIATION OF THE SPECIFIED REQUIREMENTS OR DIMENSIONS VOIDS H SQUARED HOME DESIGN, INC.'S LIABILITY.

THIS PLAN HAS BEEN DRAWN IN ACCORDANCE WITH NORTH CAROLINA STATE RESIDENTIAL BUILDING CODES 309 BOTTOM.

THIS PLAN IS TO ONLY BE BUILT BY THE ABOVE CITED BUILDER OR HOMEOWNER. NOT FOR MULTIPLE BUILDS UNLESS APPROVED BY H SQUARED.

DATE: 02/18/2023

GARAGE

FILE: 081021.3

FUTURE- UNFIN.
SECOND FLOOR PLAN
SCALE 1/4" = 1'-0"

ROOF FRAMING NOTES:

- NC 0208 NCRCCH 8048 16-120 11PM
- 1 2x6 RAFTERS @ 12" O.C. WITH 2x10 RIDGE, UNO.
 - 2 (2) 2x10 OR 1 1/2x12 LVL HP, (2) 2x10 HPS MAY BE REPLACED WITH A 12x16 2'-0" OVERLAP AT CENTER
 - 3 (2) 2x10 OR 1 1/2x12 LVL VALLEY, DO NOT SPICE VALLEYS
 - 4 1 1/2x12 LVL OR 2x12x12 LVL VALLEY.
 - 5 FALSE FRAME VALLEY ON 2x10 PLAT PLATE
 - 6 2x6 RAFTERS @ 12" O.C. @ 2x10 RIDGE, UNO.
 - 7 2x10 RAFTERS @ 12" O.C. @ 2x10 RIDGE, UNO.
 - 8 EXTEND RIDGE 12" BEYOND INTERSECTION
 - 'SR' = SINGLE RAFTER
 - 'DR' = DOUBLE RAFTER
 - 'TR' = TRIPLE RAFTER
 - 'RS' = ROOF SUPPORT
 - 'R' = (3) STUD OR 4x4 POST FOR ROOF SUPPORT (SEE 2x4 OR 2x6 FOR SUPPORT POSTS OVER 10'-0" IN HEIGHT)
 - ATTACH VAULTED RAFTERS WITH HURRICANE CLIPS, SIMPSON 'HCLM' OR EQUIVALENT. TIES TO BE INSTALLED ON THE OUTSIDE FACE FRAMING.
 - INSTALL RAFTER TIES AND COLLAR TIES PER SECTION REQ2.3.1 OF THE 2008 NC RESIDENTIAL CODE

TRUSS SYSTEM REQUIREMENTS

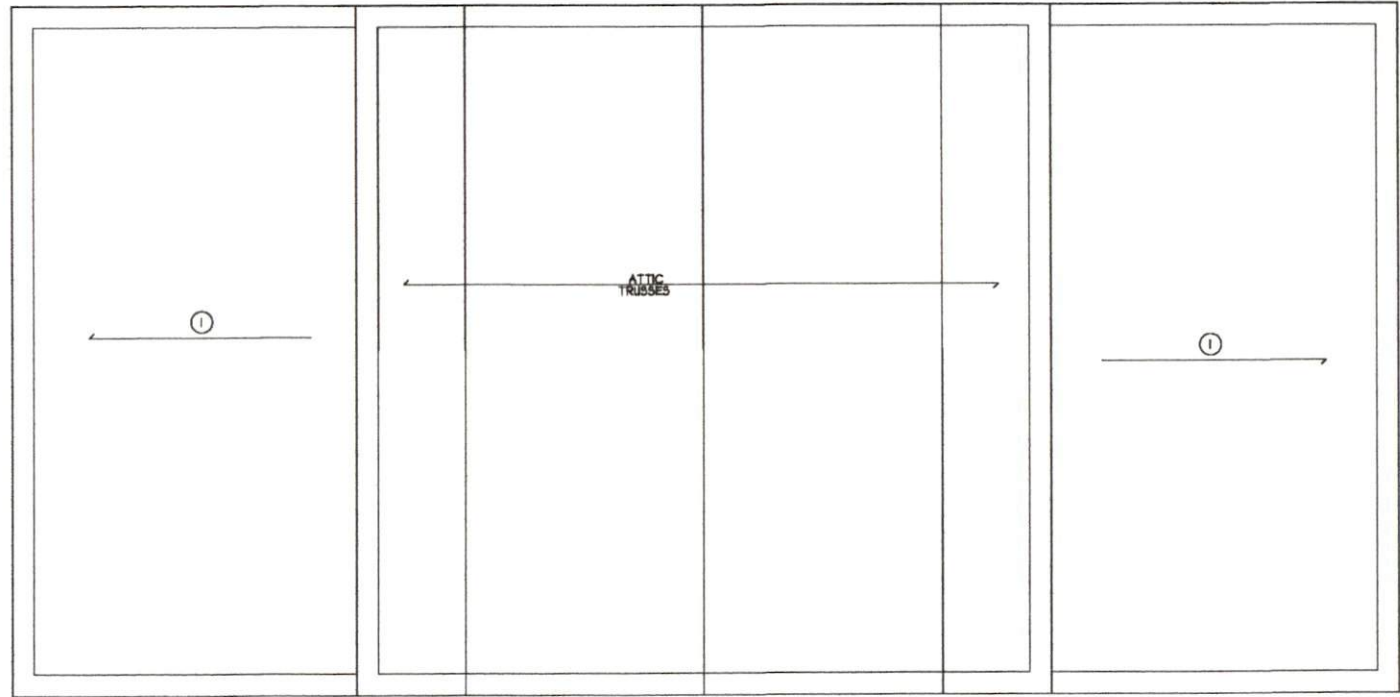
- NC 0208 NCRCCH 8048 16-120 11PM
1. TRUSS SYSTEM LAYOUTS (PLACEMENT PLANS) SHALL BE DESIGNED IN ACCORDANCE WITH SEALED STRUCTURAL PLANS. ANY NEED TO CHANGE TRUSSES SHALL BE COORDINATED WITH SOUTHERN ENGINEERS.
 2. TRUSS SCHEMATICS (PROFILES) SHALL BE PREPARED AND SEALED BY TRUSS MANUFACTURER.
 3. ALL TRUSSES SHALL BE DESIGNED FOR BEARING ON 2x12 OR 2x10 PLATES OR LEOGAINS (BIO).
 4. ALL REQUIRED ANCHORS FOR TRUSSES DUE TO UPLIFT OR BEARING SHALL MEET THE REQUIREMENTS AS SPECIFIED ON THE TRUSS SCHEMATICS.



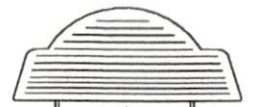
STRUCTURAL DESIGN BY:
SOUTHERN ENGINEERS, P.A.
374 BENSON DR. RALEIGH, NC 27604
LICENSE: C-4112, PHONE: 919-818-4211
PROJECT #: 22-1210

- Engineers seal applies only to structural components on this document. Seal does not include construction means, methods, techniques, sequences, procedures or safety precautions.
- Any deviations or discrepancies on plans are to be brought to the attention of Southern Engineers. Failure to do so will void Southern Engineer's liability.
- Seal is valid for a project permitted one year from date of seal.
- Use of these plans constitutes approval of terms & conditions as defined in the customer agreement.

REFER TO "SD" SHEET(S) FOR STANDARD DETAILS, BRACING DETAILS, AND STRUCTURAL NOTES



ROOF PLAN
SCALE 1/4" = 1'-0"



CURT & ASHLEY
HONEYCUTT GAR.

FOOTAGE
1510

SQUARE FOOTAGE	WORKSHOP	= 800
	each PARKING	= 480
	SECOND FLOOR	= 810

HEATHER HALL
185 HEATHERSTONE CT
BENSON NC 27504
(818) 207-4403



ANY DEVIATION OF THE SPECIFIED REARRANGEMENTS OR DIMENSIONS VOIDS H SQUARED HOME DESIGN, INC.'S LIABILITY.

THIS PLAN HAS BEEN DRAWN IN ACCORDANCE WITH NORTH CAROLINA STATE RESIDENTIAL BUILDING CODES JOB SECTION.

THIS PLAN IS TO ONLY BE BUILT BY THE ABOVE CITED BUILDER OR HOMEOWNER. NOT FOR MULTIPLE BUILDS UNLESS APPROVED BY H SQUARED.

DATE: 02/15/2022

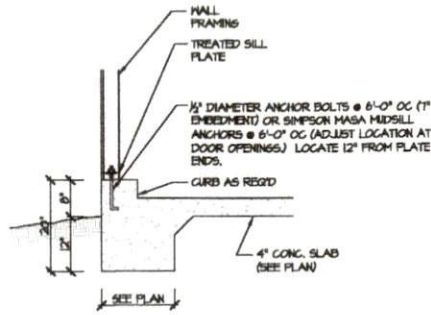
GARAGE

FILE: 081021.3

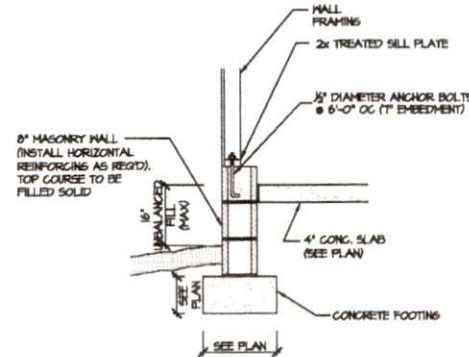
STRUCTURAL NOTES

NC (2018 NCRG), Wind: 115-120 mph

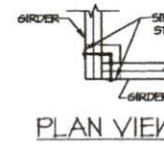
- ENGINEER'S SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS INCLUDING ROOF RAFTERS, HPFS, VALLEYS, RIDGES, FLOORS, WALLS, BEAMS AND HEADERS, COLUMNS, CANTILEVERS, OFFSET LOAD BEARING WALLS, PIER & GIRDER SYSTEM, FOOTING, AND PILING SYSTEM. ENGINEER'S SEAL DOES NOT CERTIFY DIMENSIONAL ACCURACY OR ARCHITECTURAL LAYOUT INCLUDING ROOF SYSTEM. ALL REQUIREMENTS FOR PROFESSIONAL CERTIFICATION SHALL BE PROVIDED BY THE APPROPRIATE PROFESSIONAL. SOUTHERN ENGINEERS, P.A. CERTIFIES ONLY THE STRUCTURAL COMPONENTS AS SPECIFICALLY STATED.
- ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF THE 2018 NC RESIDENTIAL CODE, PLUS ALL LOCAL CODES AND REGULATIONS. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR, AND WILL NOT HAVE CONTROL OF, CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES, OR FOR SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE CONSTRUCTION WORK, NOR WILL THE ENGINEER BE RESPONSIBLE FOR THE CONTRACTOR'S FAILURE TO CARRY OUT THE CONSTRUCTION WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. "CONSTRUCTION REVIEW" SERVICES ARE NOT PART OF OUR CONTRACT. ALL MEMBERS SHALL BE FRAMED ANCHORED, TIED AND BRACED IN ACCORDANCE WITH GOOD CONSTRUCTION PRACTICE AND THE BUILDING CODE.
- DESIGN LOADS (LISTED AS: LIVE LOAD, DEAD LOAD, DEFLECTION)
 - ROOMS OTHER THAN SLEEPING ROOMS: (40 PSF, 10 PSF, L/360)
 - SLEEPING ROOMS: (30 PSF, 10 PSF, L/360)
 - ATTIC WITH PERMANENT STAIR: (40 PSF, 10 PSF, L/360)
 - ATTIC WITHOUT PERMANENT STAIR: (20 PSF, 10 PSF, L/360)
 - ATTIC WITHOUT STORAGE: (10 PSF, 10 PSF, L/240)
 - STAIRS: (40 PSF, 10 PSF, L/360)
 - EXTERIOR BALCONIES: (60 PSF, 10 PSF, L/360)
 - DECKS: (40 PSF, 10 PSF, L/360)
 - GUARDRAILS AND HANDRAILS: (200 LBS)
 - PASSENGER VEHICLE GARAGES: (50 PSF, 10 PSF, L/360)
 - FIRE ESCAPES: (40 PSF, 10 PSF, L/360)
 - SNOW: (20 PSF)
- WALLS SHALL BE BRACED BY SHEATHING WALLS ON ALL STORIES WITH WOOD STRUCTURAL PANELS. SEE FRAMING NOTES FOR THICKNESS AND NAILING REQUIREMENTS.
- SEE APPENDIX M (DCA6) FOR EXTERIOR DECK REQUIREMENTS INCLUDING ATTACHMENTS FOR LATERAL LOADS.
- CONCRETE SHALL HAVE A MINIMUM 28 DAY STRENGTH OF 3000 PSI AND A MAXIMUM SLUMP OF 3 INCHES UNLESS NOTED OTHERWISE (M.O). AIR ENTRAINMENT PER TABLE 402.2. ALL CONCRETE SHALL BE PROPORTIONED, MIXED, HANDLED, SAMPLED, TESTED, AND PLACED IN ACCORDANCE WITH ACI STANDARDS. ALL SAMPLES FOR PUMPING SHALL BE TAKEN FROM THE EXIT END OF THE PUMP. CONTROL JOINTS IN SLABS SHALL BE SPACED ON A GRID OF ≈ 30 TIMES THE DEPTH (D). CONTROL JOINTS SHALL BE SAWCUT TO A DEPTH OF 1/3 (I.E. 4" CONCRETE SLABS SHALL HAVE 1 1/2" DEEP CONTROL JOINTS SAWCUT IN SLAB ON A $\pm 10'-0"$ x $\pm 10'-0"$ GRID).
- ALLOWABLE SOIL BEARING PRESSURE ASSUMED TO BE 2000 PSF. THE CONTRACTOR MUST CONTACT A GEOTECHNICAL ENGINEER AND THE STRUCTURAL ENGINEER IF UNSATISFACTORY SUBSURFACE CONDITIONS ARE ENCOUNTERED. THE SURFACE AREA ADJACENT TO THE FOUNDATION WALL SHALL BE PROVIDED WITH ADEQUATE DRAINAGE, AND SHALL BE GRADED 50 AS TO DRAIN SURFACE WATER AWAY FROM FOUNDATION WALLS.
- ALL FRAMING LUMBER SHALL BE SPF #2 (Fb = 875 PSI) UNLESS NOTED OTHERWISE (M.O). ALL TREATED LUMBER SHALL BE SYP # 2. PLATE MATERIAL MAY BE SYP # 3 OR SYP #3 (F_c(app) = 425 PSI - MIN).
 - L.V.L. SHALL BE LAMINATED VENEER LUMBER: Fb=2600 PSI, Fv=285 PSI, E=1.9x10⁶ PSI.
 - P.S.L. SHALL BE PARALLEL STRAND LUMBER: Fb=2400 PSI, Fv=240 PSI, E=2.0x10⁶ PSI.
 - L.S.L. SHALL BE LAMINATED STRAND LUMBER: Fb=2250 PSI, Fv=400 PSI, E=1.55x10⁶ PSI. INSTALL ALL CONNECTIONS PER MANUFACTURERS INSTRUCTIONS.
- ALL ROOF TRUSS AND I-JOIST LAYOUTS SHALL BE PREPARED IN ACCORDANCE WITH THE SEALED STRUCTURAL DRAWINGS. TRUSSES AND I-JOISTS SHALL BE INSTALLED ACCORDING TO THE MANUFACTURERS SPECIFICATIONS. ANY CHANGE IN TRUSS OR I-JOIST LAYOUT SHALL BE COORDINATED WITH SOUTHERN ENGINEERS.
- ALL STRUCTURAL STEEL SHALL BE ASTM A-36. STEEL BEAMS SHALL BE SUPPORTED AT EACH END WITH A MINIMUM BEARING LENGTH OF 3 1/2" INCHES AND FULL FLANGE WIDTH. PROVIDE SOLID BEARING FROM BEAM SUPPORT TO FOUNDATION. BEAMS SHALL BE ATTACHED TO EACH SUPPORT WITH TWO LAG SCREWS (1/2" DIAMETER x 4" LONG). LATERAL SUPPORT IS CONSIDERED ADEQUATE PROVIDING THE JOIST ARE TOE NAILED TO THE SOLE PLATE, AND SOLE PLATE IS NAILED OR BOLTED TO THE BEAM FLANGE @ 48" O.C. ALL STEEL TUBING SHALL BE ASTM A500.
- REBAR SHALL BE DEFORMED STEEL, ASTM#615, GRADE 60. LAP ALL REBAR SPLICES 30 BAR DIAMETERS.
- FLITCH BEAMS SHALL BE BOLTED TOGETHER USING (2) ROWS OF 1/2" DIAMETER BOLTS (ASTM A325) WITH WASHERS PLACED UNDER THE THREADED END OF BOLT. BOLTS SHALL BE SPACED AT 24" O.C. (MAX), AND STAGGERED AT THE TOP AND BOTTOM OF BEAM (2" EDGE DISTANCE), WITH 2 BOLTS LOCATED AT 6" FROM EACH END.
- BRICK LINTELS (WHEN REQUIRED) SHALL BE 3 1/2"x3 1/2"x1/4" STEEL ANGLE FOR UP TO 6'-0" SPAN AND 6"x4"x5/16" STEEL ANGLE WITH 6" LEG VERTICAL FOR SPANS UP TO 4'-0". SEE PLANS FOR SPANS OVER 4'-0". SEE ALSO SECTION R103.0.3 LINTELS.



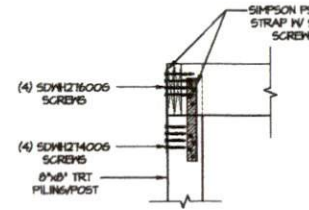
115-120 MPH (101A SD) MONOLITHIC SLAB @ GARAGE (SIDING OR EQUAL)



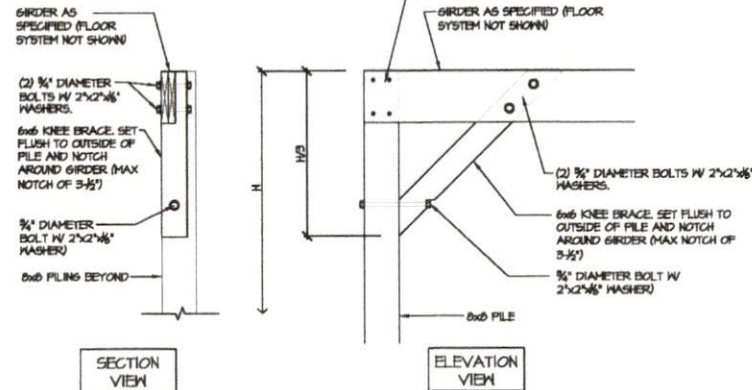
115-120 MPH (107A SD) STEM WALL SLAB @ GARAGE (SIDING OR EQUAL)



PLAN VIEW



202C SD PILE/PILE CONNECTION AT CORNER



205A SD TYP. 6x6 "Y" BRACE (APPROVED ALTERNATE METHODS ACCEPTABLE)



PROJECT # 22-1210

Engineers seal applies only to structural components on this document. Seal does not include construction means, methods, techniques, sequences, procedures or safety on plans, as to be brought to the immediate attention of Southern Engineers. Failure to do so will void Southern Engineer's liability. Seal is valid for projects permitted one year from date of seal. Use of these plans constitutes approval of terms & conditions as defined in the customer agreement.

Southern Engineers, P.A.
 3716 Benson Drive, Raleigh, NC 27609
 Phone: (919) 878-1617
 License: C-4772
 www.southernengineers.com

H Squared Home Design, Inc.
 165 Heatherstone Ct.
 Benson, NC 27504
 Phone: (919) 207-1403

CURT & ASHLEY HONEYCUTT GARAGE

SD



Trenco
818 Soundside Rd
Edenton, NC 27932

Re: J1021-5941
Honeycutt Garage

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: I53107762 thru I53107766

My license renewal date for the state of North Carolina is December 31, 2022.

North Carolina COA: C-0844



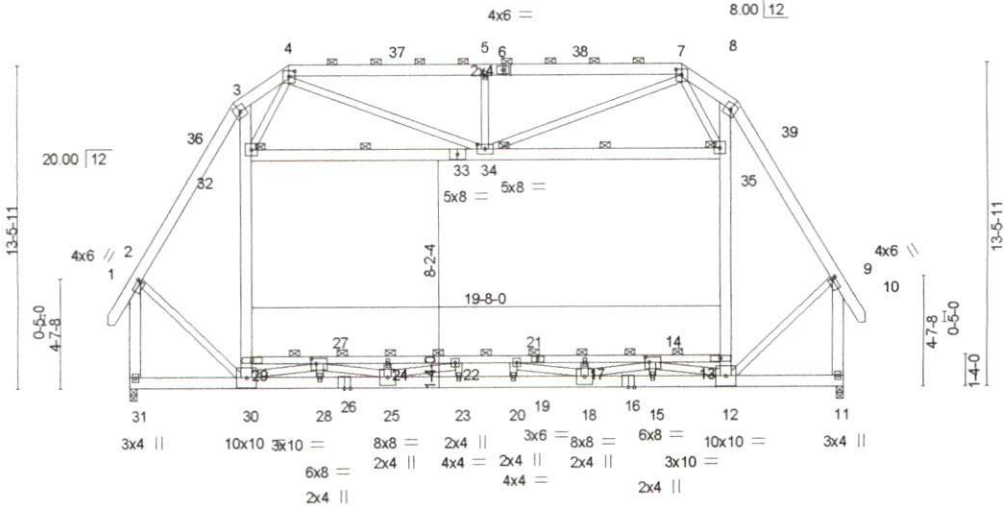
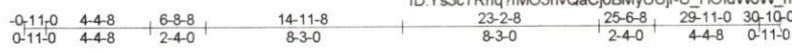
July 15, 2022

Gilbert, Eric

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Job J1021-5941	Truss A1	Truss Type ATTIC	Qty 12	Ply 1	Honeycutt Garage I53107762
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Comtech, Inc., Fayetteville, NC - 28314, 8 430 s Aug 16 2021 MITEK Industries, Inc. Thu Jul 14 14:44:13 2022 Page 1
 ID:Ys3cTRhq?IM05rnvQaCj0BMjUjU-H0fdW6W_nOxYNuSfho6PUcvDq3gySTZyl7Tyy6zm



Scale = 1:90

Plate Offsets (X, Y) -	[2-0-0-8, 0-1-12], [4-0-3-0, 0-2-12], [7-0-3-0, 0-2-12], [9-0-0-8, 0-1-12], [14-0-2-0, 0-2-8], [27-0-2-8, 0-2-8], [34-0-4-0, 0-2-4]
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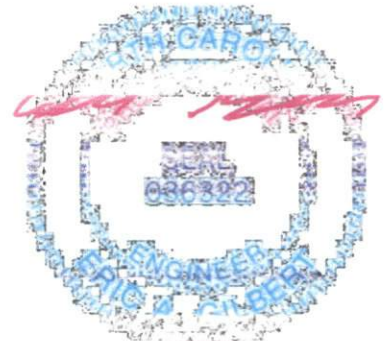
LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.35	Vert(LL)	-0.38	20-23	>943	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.90	Vert(CT)	-0.67	20-23	>528		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.82	Horz(CT)	0.06	11	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL)	0.08	28-30	>999		
							Weight: 406 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x6 SP No. 1	TOP CHORD Structural wood sheathing directly applied or 5-8-6 oc purlins, except end verticals, and 2-0-0 oc purlins (5-7-2 max.); 4-7.
BOT CHORD 2x6 SP 2400F 2.0E *Except* 19-29, 13-19: 2x4 SP No. 1	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 30-31, 11-12. 2-4-0 oc bracing: 13-29
WEBS 2x4 SP No. 2 *Except* 3-30, 8-12, 32-33, 2-31, 9-11, 33-35: 2x6 SP No. 1	WEBS 1 Row at midpt 32-34, 34-35
	JOINTS 1 Brace at Jt(s): 32, 34, 35

REACTIONS. (size) 31=0-3-8, 11=0-3-8
 Max Horz 31=-429(LC 10)
 Max Grav 31=2374(LC 2), 11=2374(LC 2)


FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-1848/20, 4-5=-1766/377, 5-7=-1766/377, 8-9=-1848/18, 2-31=-2507/0,
 9-11=-2507/0, 7-8=-1247/221, 3-4=-1246/221
 BOT CHORD 30-31=-434/394, 28-30=0/2973, 25-28=0/2973, 23-25=0/6252, 20-23=0/6252,
 18-20=0/6252, 15-18=0/2854, 12-15=0/2854, 27-29=-159/908, 24-27=-4114/0,
 22-24=-4114/0, 21-22=-5306/0, 17-21=-4114/0, 14-17=-4114/0, 13-14=-177/923
 WEBS 29-30=0/714, 29-32=0/955, 3-32=0/812, 12-13=0/714, 13-35=0/955, 8-35=0/812,
 34-35=-253/139, 2-30=0/1287, 9-12=0/1291, 5-34=-548/279, 27-30=-2769/0,
 27-28=-338/0, 25-27=0/2328, 24-25=-313/0, 22-25=-1251/0, 18-21=-1251/0,
 17-18=-313/0, 14-18=0/2328, 14-15=-351/15, 12-14=-2769/0, 7-34=-177/1050,
 7-35=-298/364, 4-34=-177/1050, 4-32=-298/363

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-5 to 3-7-8, Interior(1) 3-7-8 to 4-6-5, Exterior(2) 4-6-5 to 11-1-5, Interior(1) 11-1-5 to 23-2-8, Exterior(2) 23-2-8 to 29-8-4, Interior(1) 29-8-4 to 30-8-5 zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - All plates are 6x6 MT20 unless otherwise indicated.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Ceiling dead load (10.0 psf) on member(s). 32-34, 34-35; Wall dead load (5.0psf) on member(s) 29-32, 13-35
 - Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 27-29, 24-27, 22-24, 21-22, 17-21, 14-17, 13-14
 - This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 - Attic room checked for L/360 deflection.



July 15, 2022

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev 5/19/2020 BEFORE USE.
 Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2870 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Road
 Edenton, NC 27932

Job J1021-5941	Truss A1-GR	Truss Type ATTIC	Qty 1	Ply 2	Honeycutt Garage	I53107763
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Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Thu Jul 14 14:44:19 2022 Page 1

ID: Ys3cTRhQ?lMO5rvQaCjOBMyUJf-I8efwgbt5qXXfTq1ovoDLgkgDdtX4iGMxtAgC6yy6zg



Scale = 1:90.9

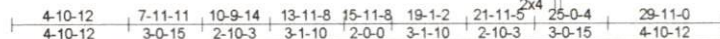
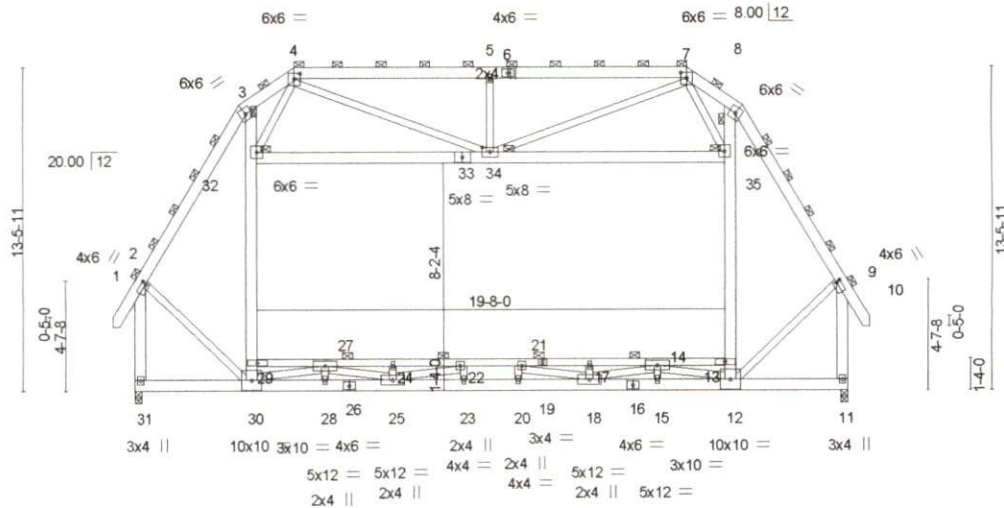


Plate Offsets (X, Y) - [2-0-0-8,0-2-0], [4-0-3-0,0-2-12], [7-0-3-0,0-2-12], [9-0-0-8,0-2-0], [34-0-4-0,0-2-4]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	3-0-0	TC 0.23	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.90	Vert(LL) -0.31 20-23 >999 360		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.67	Vert(CT) -0.55 20-23 >641 240		
BCDL 10.0	Rep Stress Incr NO	Matrix-S	Horz(CT) 0.06 11 n/a n/a		
	Code IRC2015/TP12014		Wind(LL) 0.05 28-30 >999 240		
				Weight: 811 lb	FT = 20%

LUMBER-

TOP CHORD 2x6 SP No.1
 BOT CHORD 2x6 SP No.1 "Except"
 19-29,13-19: 2x4 SP No.1
 WEBS 2x4 SP No.2 "Except"
 3-30,8-12,32-33,2-31,9-11,33-35: 2x6 SP No.1

REACTIONS.

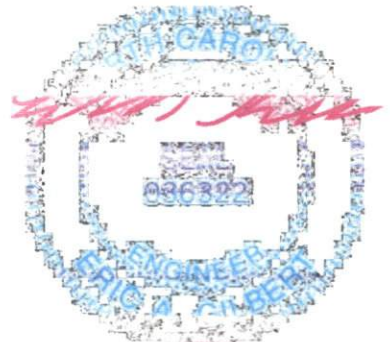
(size) 31=0-3-8, 11=0-3-8
 Max Horz 31=417(LC 7)
 Max Grav 31=3562(LC 2), 11=3562(LC 2)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-2755/0, 4-5=-2641/54, 5-7=-2641/54, 8-9=-2755/0, 2-31=-3737/0, 9-11=-3737/0, 7-8=-1807/75, 3-4=-1805/74
 BOT CHORD 30-31=-446/453, 28-30=0/4377, 25-28=0/4377, 23-25=0/9417, 20-23=0/9417, 18-20=0/9417, 15-18=0/4306, 12-15=0/4306, 27-29=-171/1439, 24-27=-6314/0, 22-24=-6314/0, 21-22=-7993/0, 17-21=-6314/0, 14-17=-6314/0, 13-14=-192/1457
 WEBS 29-30=0/1050, 29-32=0/1414, 3-32=0/1219, 12-13=0/1050, 13-35=0/1414, 8-35=0/1219, 32-34=-370/200, 34-35=-373/200, 2-30=0/1934, 9-12=0/1936, 5-34=-823/295, 27-30=-4299/0, 27-28=-496/0, 25-27=0/3621, 24-25=-480/0, 22-25=-1764/0, 18-21=-1764/0, 17-18=-480/0, 14-18=0/3621, 14-15=-496/0, 12-14=-4299/0, 7-34=-250/1577, 7-35=-461/523, 4-34=-250/1577, 4-32=-461/522

NOTES-

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
 Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.
 Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc, 2x4 - 1 row at 0-9-0 oc.
 Webs connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc, 2x4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft, Cat. II; Exp C; Enclosed; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Ceiling dead load (10.0 psf) on member(s). 32-34, 34-35; Wall dead load (5.0psf) on member(s) 29-32, 13-35
- Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 27-29, 24-27, 22-24, 21-22, 17-21, 14-17, 13-14
- This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



July 15, 2022

Continued on page 2.

Design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Road
 Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Honeycutt Garage	I53107763
J1021-5941	A1-GR	ATTIC	1	2	Job Reference (optional)	

Comtech, Inc., Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Thu Jul 14 14:44:19 2022 Page 2
 ID: Ys3cTRhq?IM05rvQaCj0BMyUUjf-I8efwgbt5qXXftq1ovoDLgkgDdtX4iGMxtAgC6yy6zg

NOTES-

- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 12) Attic room checked for L/360 deflection.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Road
 Edenton, NC 27932

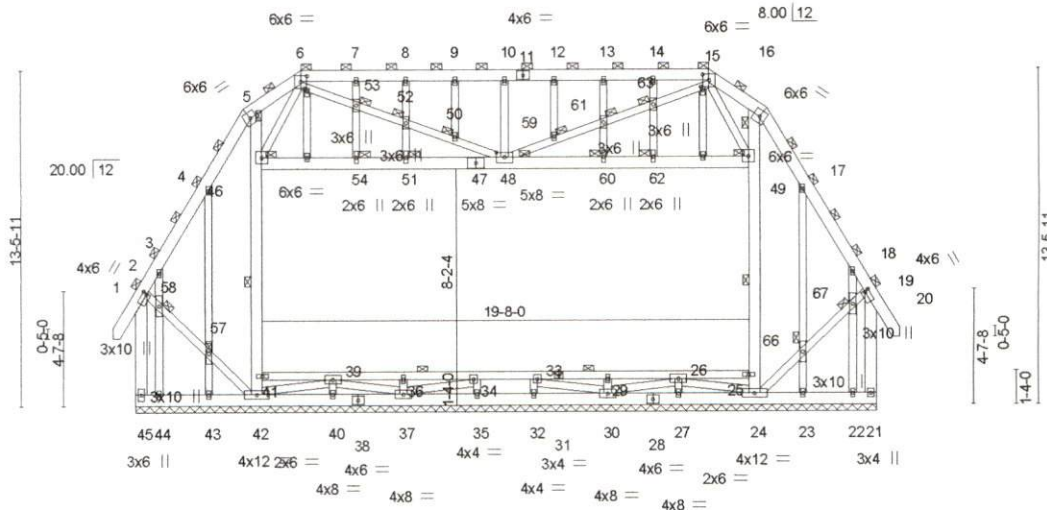
Job	Truss	Truss Type	Qty	Ply	Honeycutt Garage	I53107764
J1021-5941	A1GE	GABLE	2	1		

Comtech, Inc, Fayetteville, NC - 28314,

8 430 s Aug 16 2021 MiTek Industries, Inc. Thu Jul 14 14:44:16 2022 Page 1

ID:Ys3cTRhq?IMOSnvGaCjOBMyUJf-uZzWleY_pv9zo?5S7nEVk2671Q0tOhvFvyObnyy6zj

0-11-0 4-4-8 6-8-8 14-11-8 23-2-8 25-6-8 29-11-0 30-10-0
 0-11-0 4-4-8 2-4-0 8-3-0 8-3-0 2-4-0 4-4-8 0-11-0



Scale = 1:87.5

Plate Offsets (X, Y)-- [2:0-0-8,0-2-0], [6:0-3-0,0-2-12], [15:0-3-0,0-2-12], [19:0-0-8,0-2-0], [48:0-4-0,0-2-4], [56:0-1-9,0-1-0], [65:0-1-9,0-1-0]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	3-0-0	TC 0.35	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.19	Vert(LL) 0.00 19 n/r 120		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.47	Vert(CT) 0.00 19-20 n/r 120		
BCDL 10.0	Rep Stress Incr NO	Matrix-S	Horz(CT) -0.01 21 n/a n/a		
	Code IRC2015/TPI2014			Weight: 477 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x6 SP No. 1	TOP CHORD 2-0-0 oc purlins (5-1-8 max.), except end verticals
BOT CHORD 2x6 SP No. 1 "Except"	(Switched from sheeted. Spacing > 2-8-0).
31-41, 25-31: 2x4 SP No. 1	Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
WEBS 2x4 SP No. 2 "Except"	10-0-0 oc bracing: 25-41
5-42, 16-24, 46-47, 2-45, 19-21, 47-49: 2x6 SP No. 1	1 Row at midpt 41-46, 25-49
OTHERS 2x4 SP No. 2	1 Brace at Jt(s): 5, 15, 16, 46, 2, 19, 48, 49, 6, 50, 51, 52, 53, 54, 57, 58, 59, 60, 61, 62, 63, 66, 67

REACTIONS. All bearings 29-11-0.
 (lb) - Max Horz 45=-808(LC 10)
 Max Uplift All uplift 100 lb or less at joint(s) except 45=-537(LC 8), 42=-469(LC 9), 24=-471(LC 8), 21=-444(LC 9), 43=-386(LC 12), 44=-122(LC 13), 23=-385(LC 13), 22=-123(LC 12)
 Max Grav All reactions 250 lb or less at joint(s) 44, 22 except 45=723(LC 21), 42=1384(LC 20), 24=1385(LC 21), 21=648(LC 20), 35=624(LC 18), 32=624(LC 18), 40=767(LC 18), 37=703(LC 18), 30=703(LC 18), 27=767(LC 18), 43=332(LC 20), 23=327(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-502/378, 3-4=-499/370, 4-5=-538/430, 6-7=-2029/834, 7-8=-2029/834, 8-9=-2029/834, 9-10=-2029/834, 10-12=-2029/834, 12-13=-2029/834, 13-14=-2029/834, 14-15=-2029/834, 16-17=-535/431, 17-18=-442/301, 18-19=-498/308, 2-45=-684/474, 19-21=-639/392, 15-16=-457/443, 5-6=-453/440
 BOT CHORD 44-45=-764/695, 43-44=-764/695, 42-43=-764/695, 40-42=-261/275, 37-40=-261/275, 27-30=-242/274, 24-27=-242/274
 WEBS 41-42=-1230/197, 41-46=-1193/348, 24-25=-1230/143, 25-49=-1193/294, 46-54=-136/552, 51-54=-136/552, 48-51=-135/551, 48-60=-128/552, 60-62=-128/552, 49-62=-128/552, 2-58=-423/451, 57-58=-481/514, 42-57=-483/515, 24-66=-436/471, 66-67=-433/470, 19-67=-381/413, 34-35=-312/0, 32-33=-312/0, 10-48=-750/322, 39-40=-399/0, 36-37=-343/0, 29-30=-343/0, 26-27=-399/0, 48-59=-406/1459, 59-61=-398/1418, 61-63=-397/1421, 15-63=-424/1519, 15-49=-1210/285, 6-53=-424/1520, 52-53=-398/1421, 50-52=-398/1419, 48-50=-406/1460, 6-46=-1210/318, 4-57=-408/413, 43-57=-416/416, 3-58=-393/261, 44-58=-378/159, 17-66=-412/412, 23-66=-421/416, 18-67=-392/261, 22-67=-371/160

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf, BCCL=6.0psf, h=15ft, Cat. II; Exp C, Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed, C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1




July 15, 2022

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI 1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20687



818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Honeycutt Garage	I53107764
J1021-5941	A1GE	GABLE	2	1	Job Reference (optional)	

Comtech, Inc. Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Thu Jul 14 14:44:17 2022 Page 2
 ID:Ys3cTRhq?IM05nvQaCj0BMjUJf-MiWuV_ZcZDHqP9gfhUikGffinqM6crx3UZhZ8Eyy6zi

NOTES-

- 4) Provide adequate drainage to prevent water ponding.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 10) Ceiling dead load (10.0 psf) on member(s). 46-54, 51-54, 48-51, 48-60, 60-62, 49-62; Wall dead load (5.0psf) on member(s). 41-46, 25-49
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 537 lb uplift at joint 45, 469 lb uplift at joint 42, 471 lb uplift at joint 24, 444 lb uplift at joint 21, 386 lb uplift at joint 43, 122 lb uplift at joint 44, 385 lb uplift at joint 23 and 123 lb uplift at joint 22.
- 12) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 14) Attic room checked for L/360 deflection.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE!

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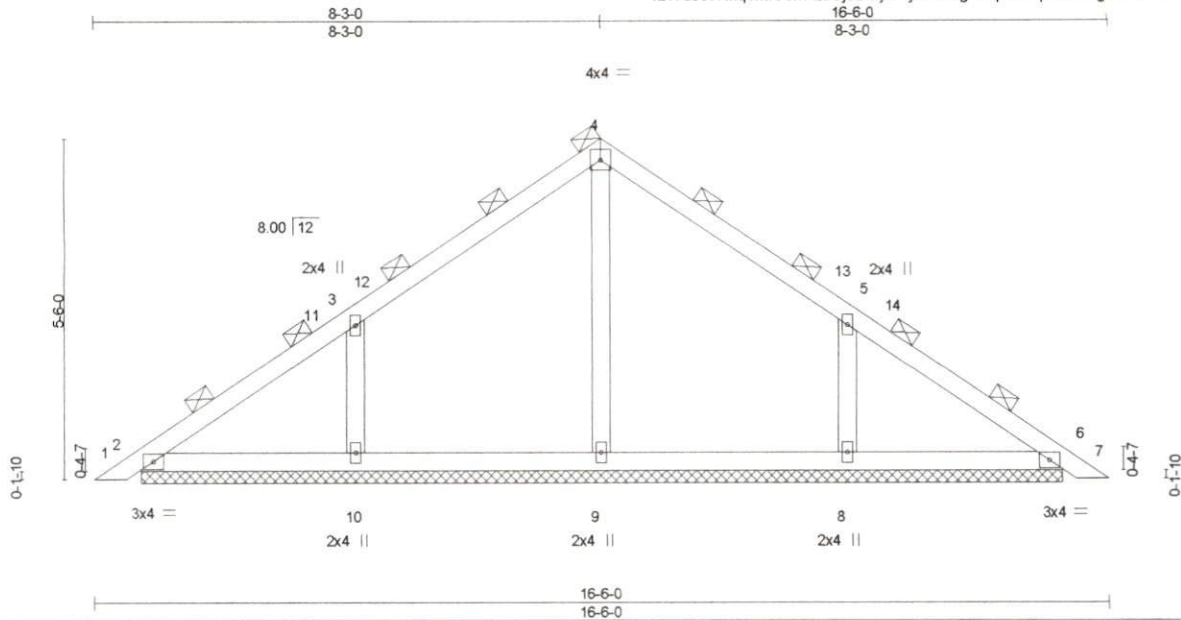


818 Soundside Road
 Edenton, NC 27932

Job J1021-5941	Truss PB	Truss Type GABLE	Qty 13	Ply 1	Honeycutt Garage Job Reference (optional)	153107765
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Comtech, Inc., Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MITek Industries, Inc. Thu Jul 14 14:44:19 2022 Page 1
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Scale = 1:35.2

LOADING (psf)	SPACING-	3-0-0	CSI	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC 0.26	Vert(LL)	0.00	7	n/r	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.14	Vert(CT)	0.00	7	n/r		
BCLL 0.0 *	Rep Stress Incr	NO	WB 0.11	Horz(CT)	0.00	6	n/a		
BCDL 10.0	Code IRC2015/TPI2014		Matrix-S					Weight: 65 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.1
BOT CHORD 2x4 SP No.1
OTHERS 2x4 SP No.2

BRACING-

TOP CHORD 2-0-0 oc purlins (6-0-0 max.)
(Switched from sheeted. Spacing > 2-8-0).
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

All bearings 14-11-12.
(lb) - Max Horz 2=191(LC 11)
Max Uplift All uplift 100 lb or less at joint(s) 2 except 10=-161(LC 12), 8=-160(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 2, 6 except 9=371(LC 1), 10=565(LC 19), 8=564(LC 20)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 4-9=-257/0, 3-10=-478/316, 5-8=-478/316

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf, BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-3-2 to 4-7-15, Interior(1) 4-7-15 to 8-3-0, Exterior(2) 8-3-0 to 12-7-13, Interior(1) 12-7-13 to 16-2-14 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 10=161, 8=160.
- This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



July 15, 2022

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/18/2020 BEFORE USE.

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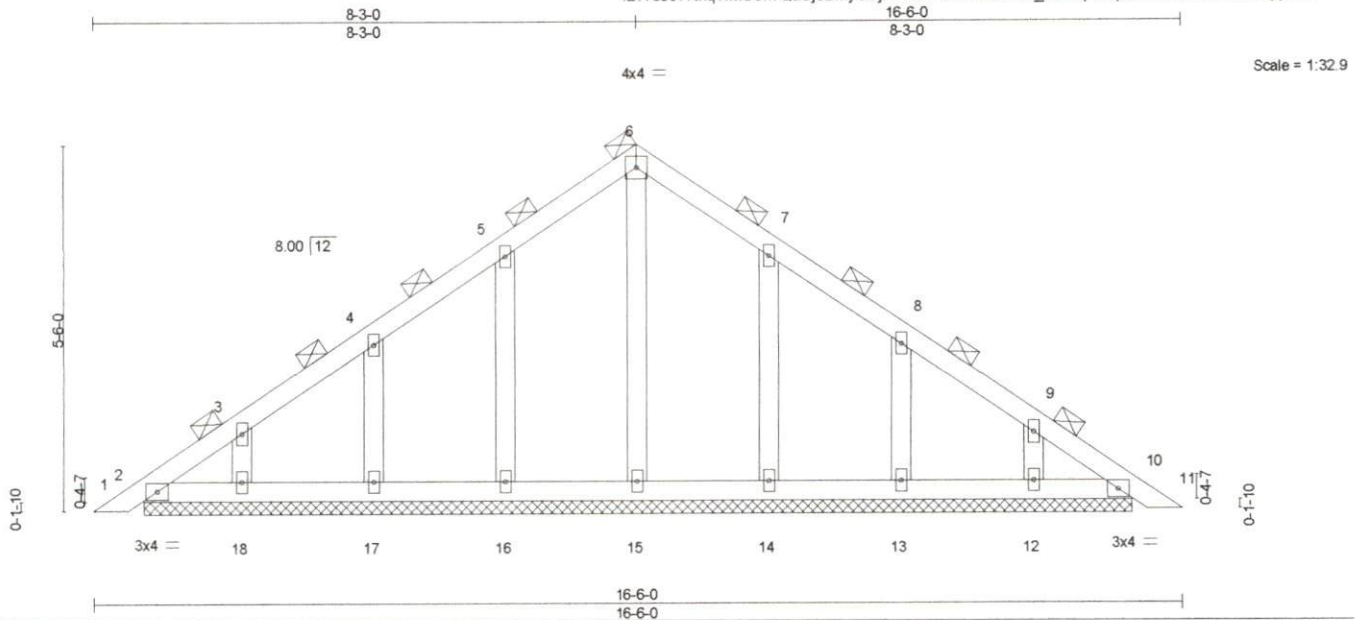
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Honeycutt Garage	I53107766
J1021-5941	PBGE	GABLE	2	1	Job Reference (optional)	

Comtech, Inc, Fayetteville, NC - 28314,

8 430 s Aug 16 2021 MiTek Industries, Inc. Thu Jul 14 14:44:21 2022 Page 1

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LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	3-0-0	TC 0.06	in (lc)	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.03	Vert(LL) 0.00 10 n/r 120		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.08	Vert(CT) 0.00 10 n/r 120		
BCDL 10.0	Rep Stress Incr NO	Matrix-S	Horz(CT) 0.00 10 n/a n/a		
	Code IRC2015/TPI2014			Weight: 78 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.1
 BOT CHORD 2x4 SP No.1
 OTHERS 2x4 SP No.2

BRACING-

TOP CHORD 2-0-0 oc purlins (6-0-0 max.)
 (Switched from sheeted: Spacing > 2-8-0).
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

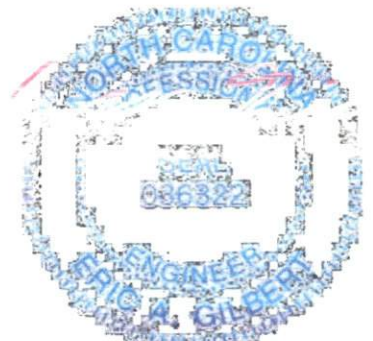
REACTIONS.

All bearings 14-11-12.
 (lb) - Max Horz 2=-239(LC 10)
 Max Uplift All uplift 100 lb or less at joint(s) 2, 10 except 16=-136(LC 12), 17=-133(LC 12), 18=-126(LC 12), 14=-134(LC 13), 13=-134(LC 13), 12=-125(LC 13)
 Max Grav All reactions 250 lb or less at joint(s) 2, 10, 15, 18, 12 except 16=278(LC 19), 17=266(LC 19), 14=276(LC 20), 13=267(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft, Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10 except (t=lb) 16=136, 17=133, 18=126, 14=134, 13=134, 12=125.
- This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



July 15, 2022

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

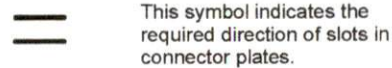
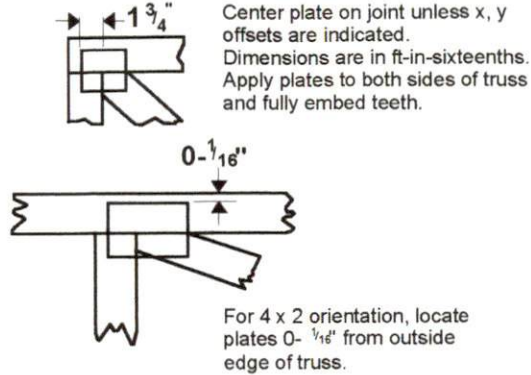
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web, and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20681



818 Soundside Road
 Edenton, NC 27932

Symbol

PLATE LOCATION AND ORIENTATION



* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING

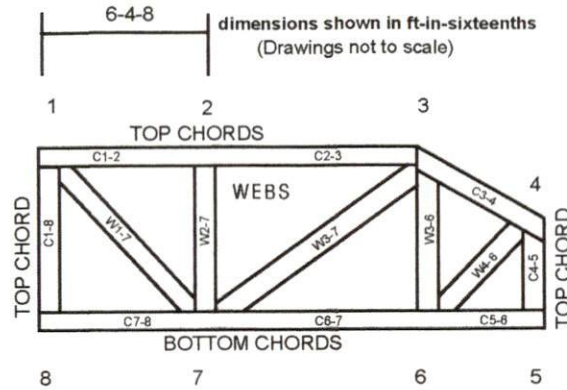


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988
ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 5/19/2020

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

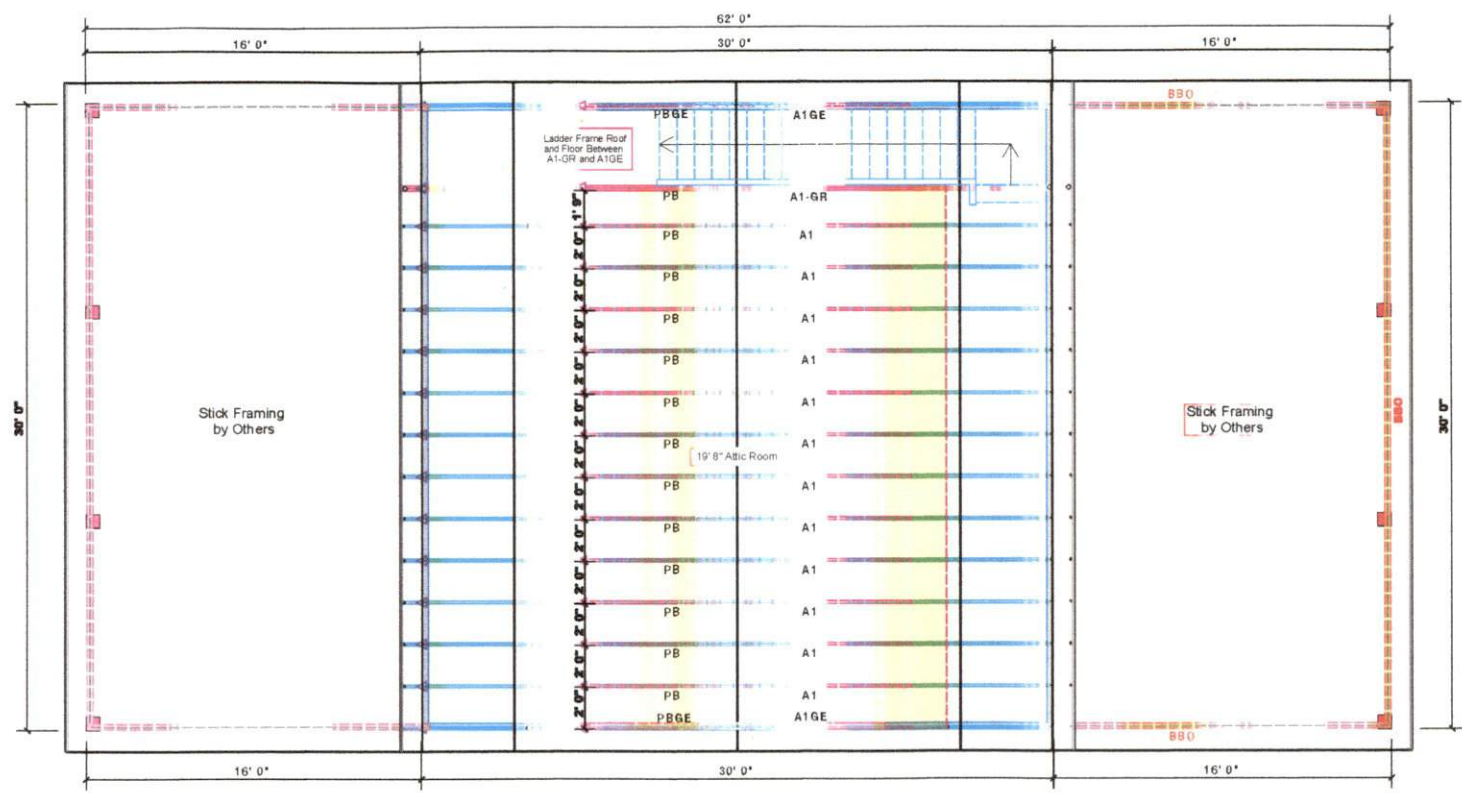
- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- The design does not take into account any dynamic or other loads other than those expressly stated.

Complete Truss and Beam Design	1.0
Complete Truss and Beam Detailing	2.0
Complete Truss and Beam Fabrication	3.0
Complete Truss and Beam Installation	4.0
Complete Truss and Beam Inspection	5.0
Complete Truss and Beam Maintenance	6.0
Complete Truss and Beam Replacement	7.0
Complete Truss and Beam Repair	8.0
Complete Truss and Beam Renovation	9.0
Complete Truss and Beam Restoration	10.0
Complete Truss and Beam Reconstruction	11.0
Complete Truss and Beam Rebuilding	12.0
Complete Truss and Beam Refurbishing	13.0
Complete Truss and Beam Refinishing	14.0
Complete Truss and Beam Refitting	15.0
Complete Truss and Beam Refinishing	16.0
Complete Truss and Beam Refinishing	17.0
Complete Truss and Beam Refinishing	18.0
Complete Truss and Beam Refinishing	19.0
Complete Truss and Beam Refinishing	20.0

Signature: **David Landry**
 David Landry

LOAD CHART FOR JACK STUDS
(BASED ON TABLE 2.102 FROM THE 2005 INTERNATIONAL BUILDING CODE)

TRUSS SPACING	TRUSS TYPE	MAXIMUM LOAD (PSF)	MAXIMUM LOAD (KIP/FT)
1700	1	2560	3400
2400	2	3500	4800
3000	3	4500	6000
3600	4	5500	7500
4200	5	6500	8800
4800	6	7500	10200
5400	7	8500	11500
6000	8	9500	12800
6600	9	10500	14100
7200	10	11500	15400



PlotID	Length	Product	Piles	Net Qty
GDH	14' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2

1 Truss Placement Plan
 Scale: 1/4" = 1'

Hatch Legend	
	Drop Beam

Dimension Notes
 1. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise.
 2. All interior wall dimensions are to face of frame wall unless noted otherwise.
 3. All exterior wall to truss dimensions are to face of frame wall unless noted otherwise.

All Walls Shown Are Considered Load Bearing

Roof Area	= 2612.09 sq.ft.
Ridge Line	= 96 ft.
Hip Line	= 0 ft.
Horiz. OH	= 252 ft.
Raked OH	= 163.26 ft.
Decking	= 90 sheets

Indicates Left End of Truss
 (Reference Engineered Truss Drawing)
 Do NOT Erect Truss Backwards

BUILDER	CITY / CO.	ADDRESS	MODEL	DATE REV.	DRAWN BY	SALES REP.
Cash - Lenny	Johnston Co. / Johnston		Roof	07/15/22	Jonathan Landry	Lenny Norris
JOB NAME						
PLAN						
SEAL DATE						
QUOTE #						
JOB #						J1021-5941

THIS IS A TRUSS PLACEMENT DRAWING ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. The individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general questions regarding truss design, call (910) 864-8787 and (910) 864-4444 with the email: sales@comtech.com or info@comtech.com

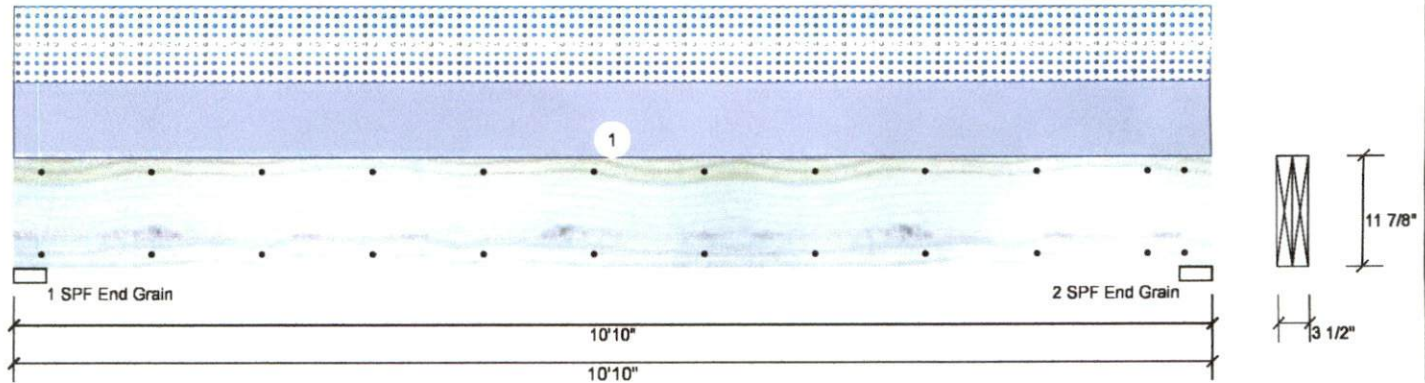


Client: Cash/Lenny
 Project: Custom
 Address:

Date: 7/15/2022
 Input by: Jonathan Landry
 Job Name: Honeycutt Garage
 Project #: J1021-5941

GDH Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	240
Importance:	Normal - II
Temperature:	Temp <= 100°F

Application:	Floor
Design Method:	ASD
Building Code:	IBC/IRC 2015
Load Sharing:	No
Deck:	Not Checked

Reactions UNPATTERNED lb (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	4302	4252	0	0
2	Vertical	0	4302	4252	0	0

Bearings

Bearing	Length	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	Vert	83%	4302 / 4252	8554	L	D+S
2 - SPF End Grain	3.500"	Vert	83%	4302 / 4252	8554	L	D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	21249 ft-lb	5'5"	22897 ft-lb	0.928 (93%)	D+S	L
Unbraced	21249 ft-lb	5'5"	21299 ft-lb	0.998 (100%)	D+S	L
Shear	6540 lb	1'3 3/8"	10197 lb	0.641 (64%)	D+S	L
LL Defl inch	0.239 (L/521)	5'5"	0.259 (L/480)	0.921 (92%)	S	L
TL Defl inch	0.480 (L/259)	5'5"	0.519 (L/240)	0.926 (93%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at a maximum of 2'6 5/8" o.c.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	785 PLF	0 PLF	785 PLF	0 PLF	0 PLF	A1GE
	Self Weight				9 PLF					

Notes
 Calculated Structural Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Handling & Installation**
1. LVL beams must not be cut or drilled
 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 3. Damaged Beams must not be used
 4. Design assumes top edge is laterally restrained
 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Metsa Wood
 301 Merritt 7 Building, 2nd Floor
 Norwalk, CT 06851
 (800) 622-5850
 www.metsawood.com/us

Comtech, Inc.
 1001 S. Rally Road, Suite #639
 Fayetteville, NC
 USA
 28314
 910-864-TRUS



This design is valid until 11/3/2024



Client: Cash/Lenny

Date: 7/15/2022

Project: Custom

Input by: Jonathan Landry

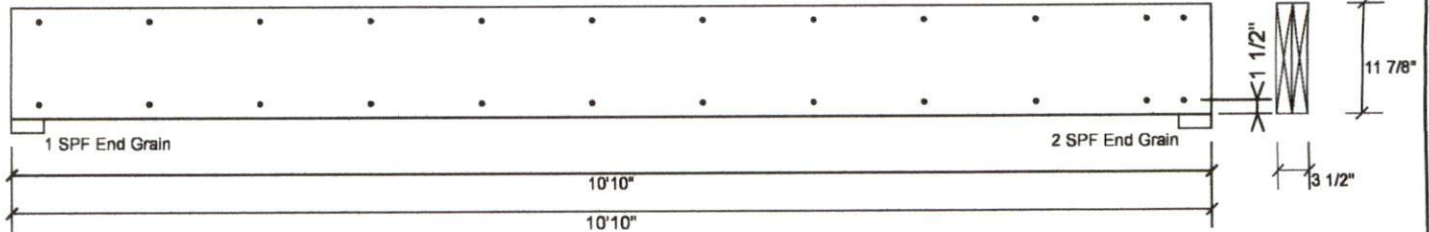
Address:

Job Name: Honeycutt Garage

Project #: J1021-5941

GDH Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	183.7 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

Calculated Structural Designs is responsible only of the structural adequacy of the component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

Manufacturer Info

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