



3365 Skyway Drive  
Auburn, AL 36830  
P: 360.566.7343

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**STRUCTURAL CALCULATIONS**  
PREPARED FOR  
**TARHEEL BASEMENT SYSTEMS**  
FOR  
**LLOYD RESIDENCE**  
**FOUNDATION REPAIR**  
821 HIGHGROVE RD  
SPRING LAKE, NORTH CAROLINA

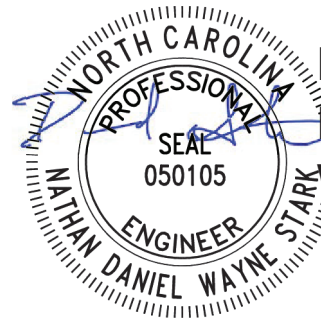
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**PROJECT NUMBER:** 22.087.TBS

**DATE:** April 8, 2022

**PROJECT MANAGER:** Daniel Stark, P.E.

**COA:** C-4876



Nathan Daniel W Stark  
Digitally signed  
by Nathan  
Daniel W Stark  
Date:  
2022.04.11  
09:58:54 -05'00'



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Auburn, AL 36830  
P: 360.566.7343

April 8, 2022

Project No.:22.087.TBS

Tonya Gunn  
Tarheel Basement Systems  
2910 Griffith Road  
Winston-Salem, North Carolina 27103

RE: Foundation Repair - 821 Highgrove Rd, Spring Lake, North Carolina

### **PROJECT BACKGROUND**

We understand the structure is a single-family residence and has experienced settlement at the left rear corner of the structure. A recent floor level survey (attached) indicates as much as ~1" of differential settlement may have occurred. It is our understanding (7) 2 7/8 inch diameter push piers have been proposed to provide additional foundation support.



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**Image 1: Front Elevation**

## **GEOLOGIC SETTING**

The existing structure is located in Spring Lake, North Carolina. The geologic structure in the area is comprised of sandy loam and the site is relatively flat. It is our opinion the localized settlement is a result of improper foundation drainage and/or undersized footings. We believe suitable support can be achieved by installing push piers.

## **SUMMARY**

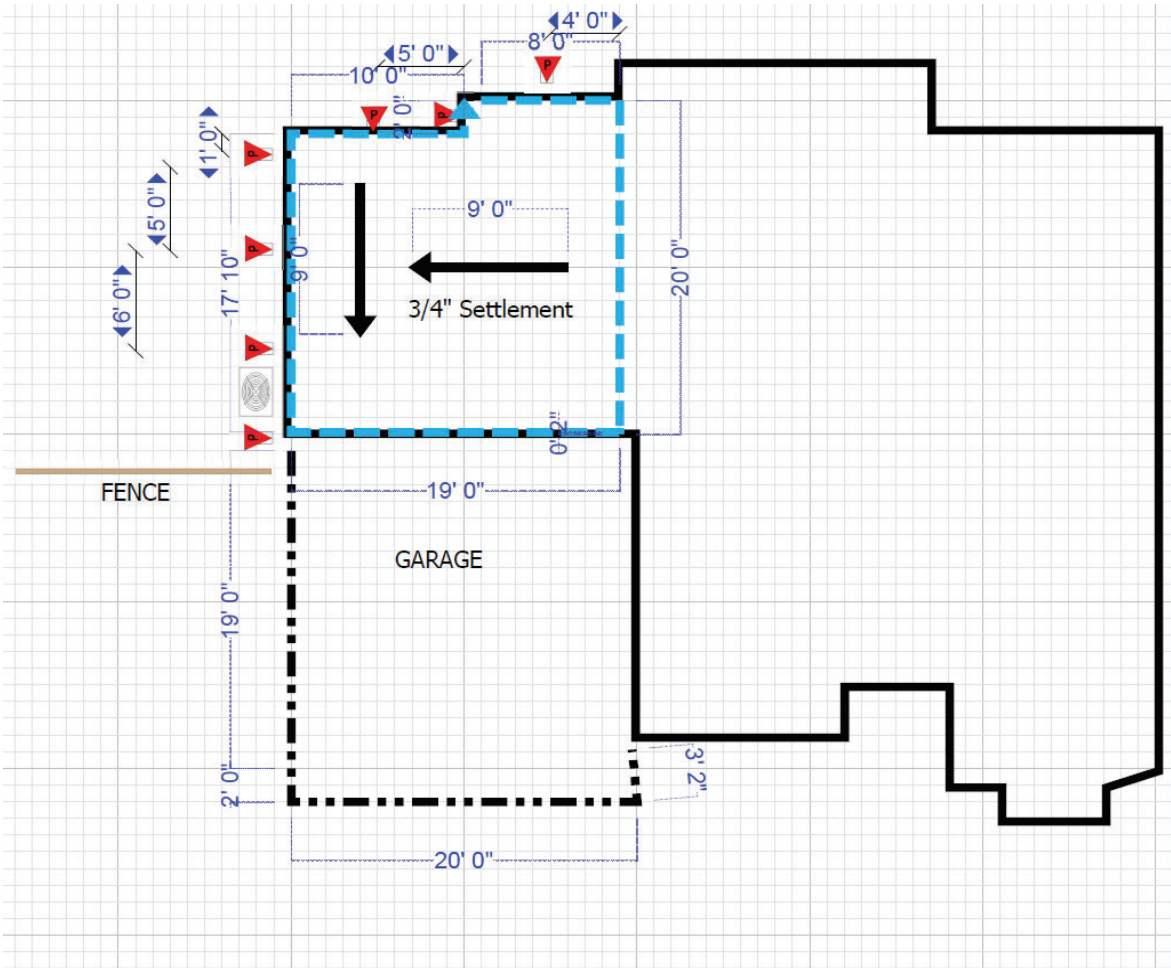
The ultimate load requirement for the push piers is 14000 lbs, and based on the geologic setting, we expect the piers to achieve adequate capacity at approximately 8 – 25 feet. We recommend the piers with a 2 7/8 inch shaft be installed to a minimum depth of 8 feet and a minimum installation pressure of 1500 psi, or refusal, using a 9.62 square inch hydraulic ram.

Regards,

A handwritten signature in blue ink, appearing to read "D. Stark".

Daniel Stark, P.E.  
Stark Foundations

# FLOOR LEVEL SURVEY





**PROJECT**

Foundation Repair  
821 Highgrove Rd  
Spring Lake, North Carolina

Date: 8-Apr-22  
Designed by: ACS

Project No.: 22.087.TBS

**Design Criteria**

Code(s):

2018 North Carolina State Building Code  
International Building Code (IBC) 2015  
ASCE 7-10

Design Loads:

Dead:

Roof = 15 psf  
Chimney = 45 psf  
Third Floor = 15 psf  
Second Floor = 15 psf  
First Floor (4" Conc. Slab) = 50 psf  
Walls = 8 psf  
8" Foundation Wall = 100 psf  
Soil = 110 psf

Soil:

Allow Lateral Bearing Pressure = 100 psf/ft  
Active Pressure = 60 psf/ft

Live:

Roof (Snow) = 10 psf  
Roof Live Load = 20 psf governs  
Third Floor = 40 psf  
Second Floor = 40 psf  
First Floor = 40 psf

Wind: (not applicable)

Exposure = C Risk Category = II  
Wind Speed, V = 118 mph  $K_{zt} = 1$   
Gust Factor, G = 0.85  $K_d = 0.85$   
Int. Pressure Coefficient,  $GC_{pi} = -0.18$   $K_z = 0.98$   
Ext. Pressure Coefficient,  $C_p = 0.8$  Height,  $h_z = 30$  ft

Design Wind Pressure:

Design Load Combo = D + 0.6W

where:  $p_w = q_z (GC_p - GC_{pi})$   $\omega = 0.6$   
 $q_z = 0.00256 K_z K_{zt} K_d V^2$

Therefore:

$q_z = 29.7$  psf  
 $p_w = 25.5$  psf  
Factored Wind Pressure,  $p'_w = 15.3$  psf (say 16 psf)



**PROJECT**

Foundation Repair  
821 Highgrove Rd  
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Date: 8-Apr-22  
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**Push Pier Design - Worst Case**

Vertical Design Loads:

Tributary Widths:

Dead:

Roof =	9	ft	----->	135	plf
Third Floor =	0	ft	----->	0	plf
Second Floor =	4	ft	----->	60	plf
First Floor =	4	ft	----->	200	plf
Walls =	16	ft	----->	128	plf
Foundation Wall (height) =	1	ft	----->	100	plf
Soil (height) =	1	ft	----->	55	plf
				<u>ΣDL =</u>	<u>678</u> plf

Live:

Roof (live) =	9	ft	----->	180	plf
Third Floor =	0	ft	----->	0	plf
Second Floor =	4	ft	----->	160	plf
First Floor =	4	ft	----->	160	plf
				<u>ΣLL =</u>	<u>500</u> plf

Max Pier Spacing or Trib = 6 ft

Pier Working Loads:

$P_{DL}$ =	4068	lbs
$0.75 * P_{LL}$ =	2250	lbs
Working Load, $P_{TL}$ =	7000	lbs
Ultimate Load, $P_{ULT}$ =	14000	lbs

Pier Design:

Pier Type: Push Pier

Bracket: PP21617-34      Bracket Cap = 29340 lbs      Therefore OK

Shaft Diameter: 2.875"

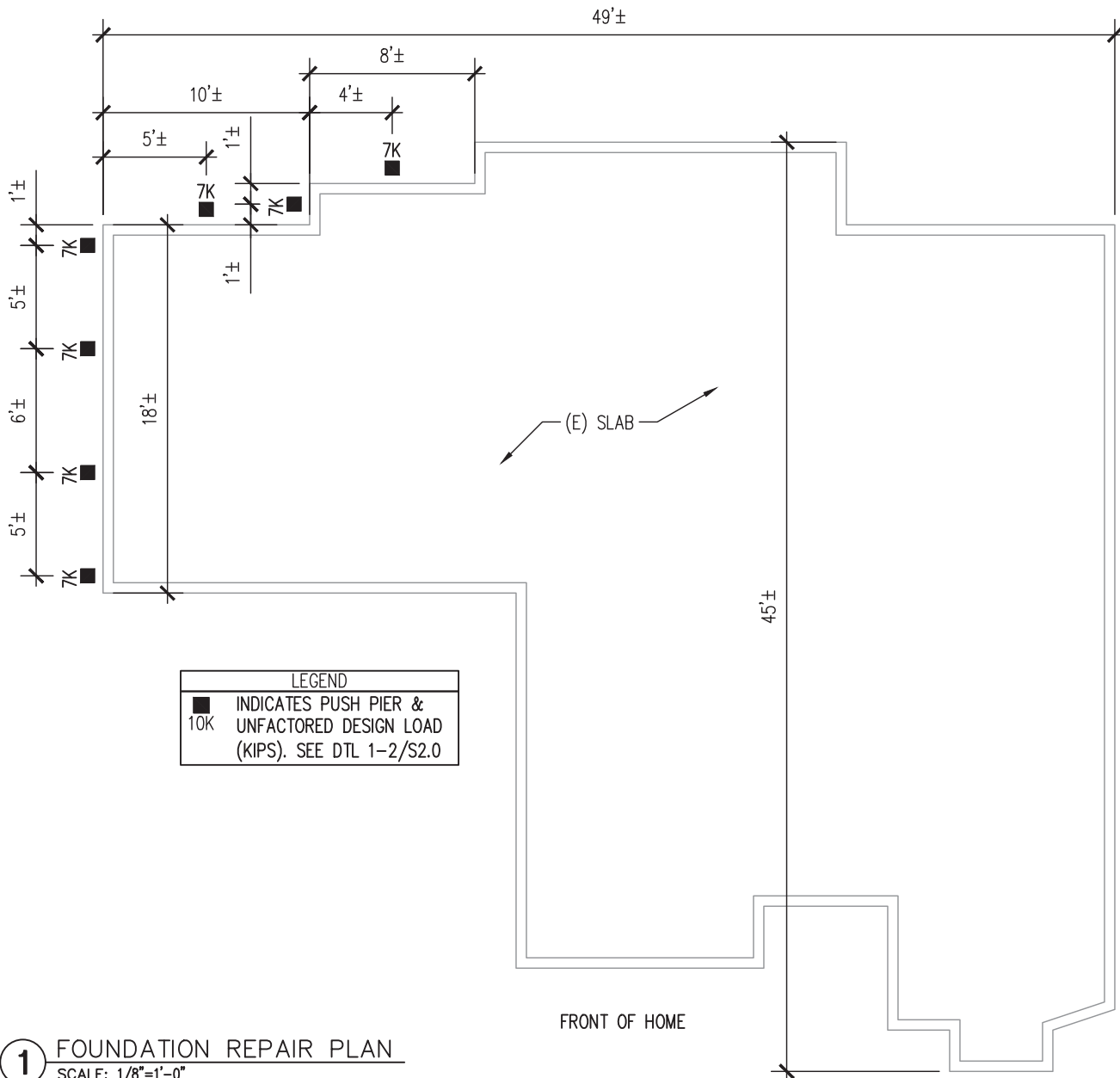
Installation Pressure, P:

$Q_{ult} = 2 (P_{TL})$   
14000 lbs

$Q_{ult} = A_{cyl} (P)$  where  $A_{cyl}$  = working area of the dual bore installation

$A_{cyl} = 9.62 \text{ in}^2$

Therefore,  $P_{REC} = Q_{ult} / A_{cyl}$   
1500 psi



**LEGEND**  
 ■ INDICATES PUSH PIER &  
 10K UNFACTORED DESIGN LOAD  
 (KIPS). SEE DTL 1-2/S2.0

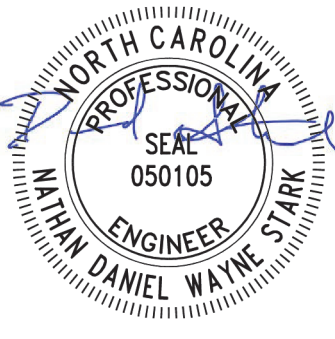
FRONT OF HOME

**1 FOUNDATION REPAIR PLAN**  
 SCALE: 1/8"=1'-0"

PLAN NOTES:

- DO NOT INSTALL PIERS UNDER WINDOWS OR OPENINGS, UNLESS NOTED OTHERWISE.
- PIERS CAN BE ADJUSTED A DISTANCE OF 1'-0"± AS LONG AS THE SPAN BETWEEN THE ADJUSTED PIER AND ADJACENT PIER DOES NOT EXCEED THE MAXIMUM RECOMMENDED SPACING. WHEN THE DISTANCE EXCEEDS THE MAXIMUM RECOMMENDED SPACING, CONSULT WITH THE ENGINEER OF RECORD FOR FURTHER DIRECTION.
- PUSH PIERS SHALL BE INSTALLED TO A MINIMUM 2.0X THE DESIGN LOAD AS NOTED ON THE FOUNDATION REPAIR PLAN TO ACHIEVE AN ULTIMATE AXIAL LOAD OF 14,000 LBS MINIMUM. PIER EMBEDMENT SHALL BE 8'-0" MIN.

STAMP

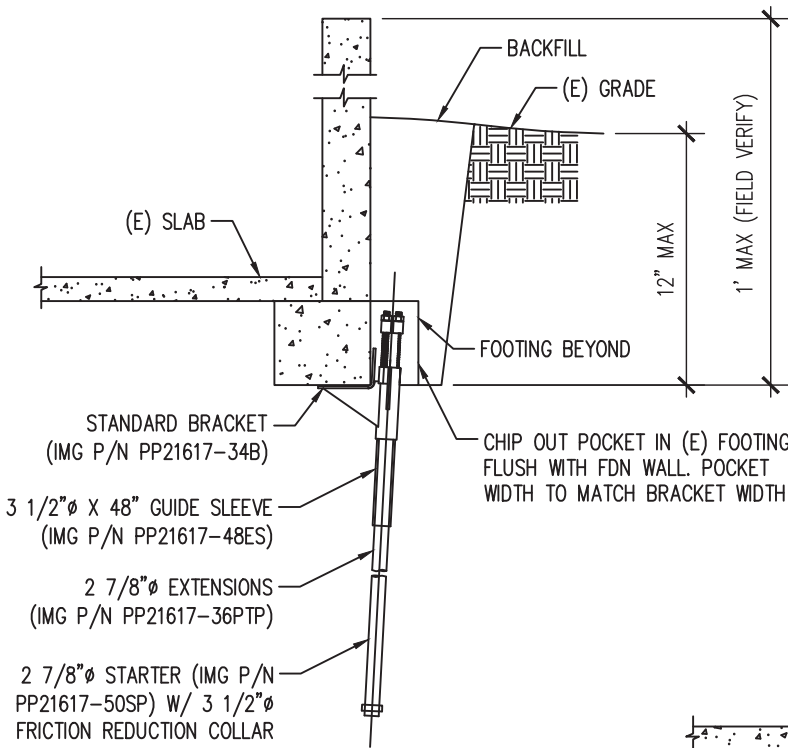


PROJECT  
 FOUNDATION REPAIR  
 LLOYD RESIDENCE  
 821 HIGHGROVE RD.  
 SPRING LAKE, NC

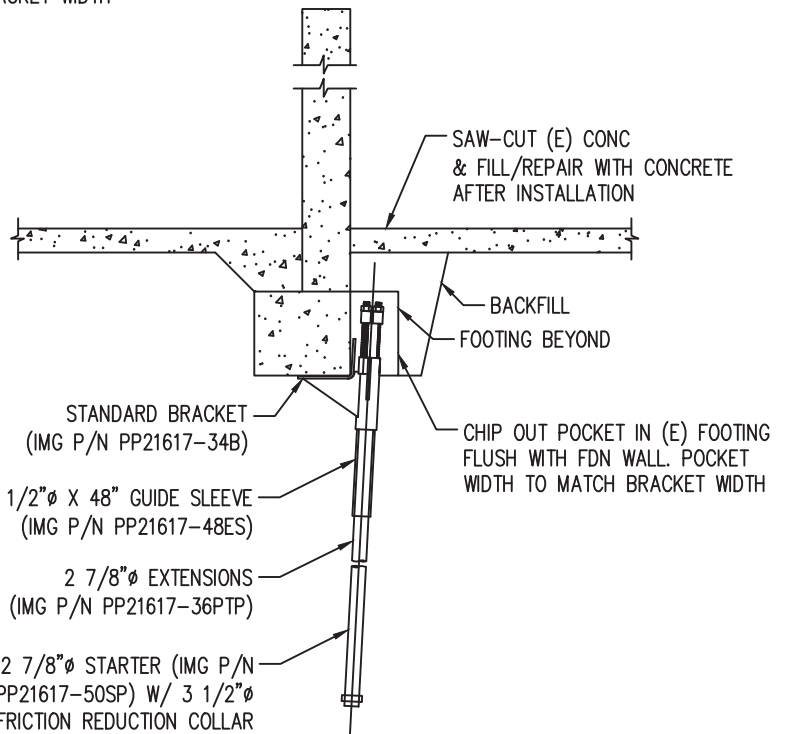
CLIENT  
 TARHEEL  
 BASEMENT SYSTEMS  
 2910 GRIFFITH ROAD  
 WINSTON-SALEM, NC



NO	REVISIONS	BY	DATE
ISSUED:	04.08.22	PROJ NO.:	22.087.TBS
DRAWN BY:	TLD	CHECKED BY:	DS
SHEET TITLE			
<b>FDN REPAIR PLAN</b>			
SHEET NUMBER			
<b>S1.0</b>			

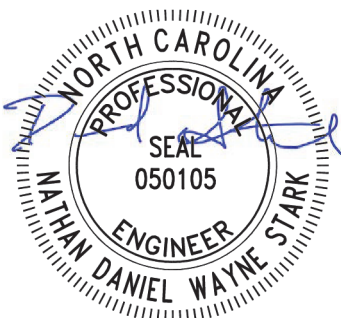


**1** 2 7/8"  $\phi$  PUSH PIER DETAIL  
 SCALE: 3/8"=1'-0"



**2** 2 7/8"  $\phi$  PUSH PIER DETAIL  
 SCALE: 3/8"=1'-0"

STAMP



PROJECT

FOUNDATION REPAIR  
 LLOYD RESIDENCE  
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 SPRING LAKE, NC

CLIENT

TARHEEL  
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FRONT ELEVATION

NO	REVISIONS	BY	DATE
ISSUED:	04.08.22	PROJ NO.:	22.087.TBS
DRAWN BY:	TLD	CHECKED BY:	DS
SHEET TITLE			
<b>DETAILS</b>			
SHEET NUMBER			
<b>S2.0</b>			