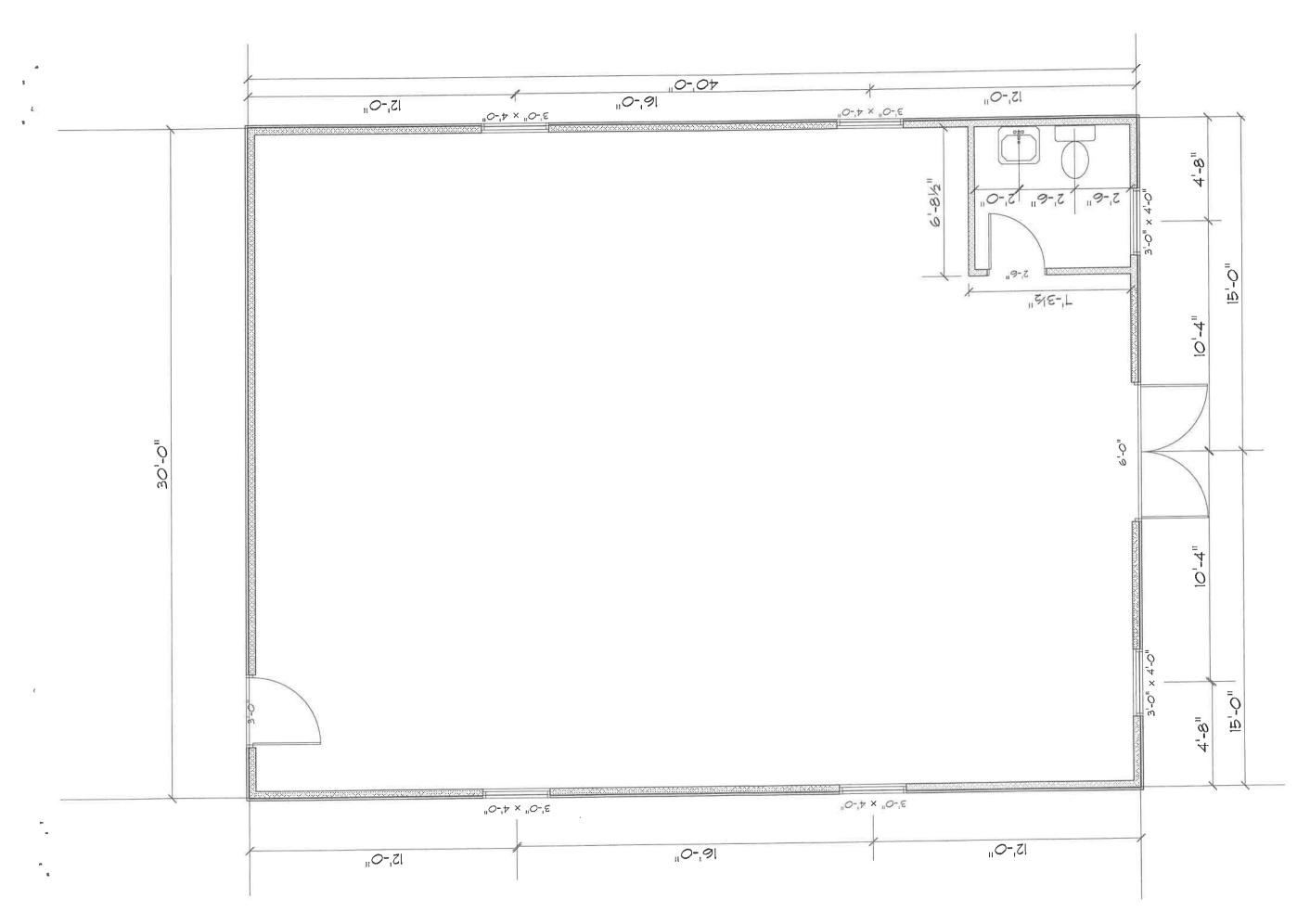


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Weiss Accessory Bldg 4894 NC 27W. L:111ngton NC 27546 Drawn by Custom Contracting Corp. NCLBGC #8664 1-19-22 12006F



Weiss Accessory Bldg Floor Plan 4894 NC 2TW, Lillington, NC 27546 Drawn by Custom Contracting Corp NCLBGC #8664 1-19-22 1200 6F 1/4"=1 .

B <sup>1</sup> K e <sup>1</sup> footing 3000psi Concrete B <sup>1</sup> Foundation will Bish 4 <sup>1</sup> Concretes 3000Psi unitient Bish 4 <sup>1</sup> Concretes 3000Psi unitient Dish 4 <sup>1</sup> Concretes 3000Psi unitient			
8" footing 3000psi Concrete outdition wall avel fill w/ 6 mil poly vepor barrier	r		
30-0" a" facting 3000pal Concrete outdation uall 4" Concrete 3000PSI w/riber gravel fill w/ 6 mil poly vapor barrier			
		8" footing 3000psi Concrete oundation wall 4" Concrete 3000PSI w/fibe gravel fill w/ 6 mil poly vapor	

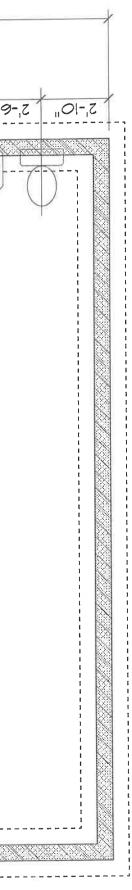
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Weiss Accessory Bldg, Foundation Plan 4894 NC 27W. Lillington, NC 27546 Drawn By Custom Contracting Corp. NCLBGC #8664 I-19-22 1200 SF 1/4"=1'



Trenco 818 Soundside Rd Edenton, NC 27932

Re: 21070050-A 4894 HWY 27-Garage

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Carter Components (Sanford, NC)).

Pages or sheets covered by this seal: 149884698 thru 149884699

My license renewal date for the state of North Carolina is December 31, 2022.

North Carolina COA: C-0844



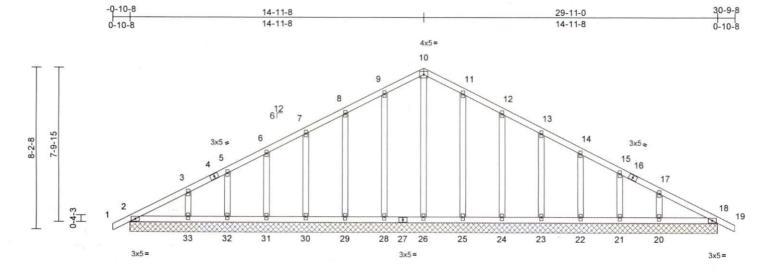
January 26,2022

### Sevier, Scott

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Job	Truss	Truss Type	Qty	Ply	4894 HWY 27-Garage	
21070050-A	AE	Common Supported Gable	2	1	Job Reference (optional)	149884698
Carter Components (Sanf	ord), Sanford, NC - 27332,	Run: 8.53 S [	Dec 6 2021 Print:	8.530 S Dec	6 2021 MiTek Industries, Inc. Tue Jan 25 14:00:51	Page: 1

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29-1	1-0

Scale = 1:56.5					11							1
Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 13.9/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2015/TPI2	CSI TC BC WB 014 Matrix-MSF	0.08 0.06 0.15 H	DEFL Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 37	l/defi n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 176 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER TOP CHORD BOT CHORD OTHERS BRACING TOP CHORD BOT CHORD	2x4 SP No.2 2x4 SP No.3 Structural wood she 6-0-0 oc purlins.	athing directly applied	TOP CH	5-6=-67/59, 6 8-9=-76/183, 11-12=-76/18 13-14=-44/86 17-18=-83/40 ORD 2-33=-34/127 31-32=-34/12	3, 12-13=-59/ 5, 14-15=-43/3 9, 18-19=0/29 7, 32-33=-34/1 27, 30-31=-34/	3=-59/133, , 10-11=-93/2 133, 8, 15-17=-53 27, 127,	228, ( /17,	6) This load ove 7) All p 8) Gat	ign. s truss h d of 12.0 rhangs r plates ar ple requi	as bee ) psf or non-co re 2x4 ires cor	en designed for g 2.00 times flat n ncurrent with oth	erwise indicated.
REACTIONS	20=202/2 22=146/2 24=140/2 26=120/2 29=140/2 31=146/2 33=202/2 37=141/2		0, 0, 0, WEBS 0, 0, 0,	26-28=-34/12 24-25=-34/12	7, 21-22=-34/ 7, 18-20=-34/ 9, 9-28=-161/7 9, 6-31=-128/8 7, 11-25=-161/ 3, 13-23=-124 1, 15-21=-112	127, 127, 127, 127 5, 8-29=-124, 1, 5-32=-112, 75, W79,	/83,	on t 3-06	he botto 5-00 tall	by 2-0	rd in all areas wh	a live load of 20.0ps here a rectangle between the bottom
	23=-16 (L 25=-14 (L 29=-18 (L 31=-17 (L 33=-25 (L Max Grav 2=169 (L 20=237 (l	11), 20=-24 (LC 16), C 16), 22=-17 (LC 16), C 16), 24=-18 (LC 16), C 16), 28=-15 (LC 15), C 15), 30=-16 (LC 15), C 15), 32=-14 (LC 15), C 15), 34=-1 (LC 11)	this 2) Win Vas Cat. Exte vert forc DOI	alanced roof live load design. d: ASCE 7-10; Vult=1: =103mph; TCDL=6.0; ll; Exp B; Enclosed; I rior (2) zone; cantilev cal left and right expo s & MWFRS for reac =1.60 plate grip DOL.	s have been of 30mph (3-sec Dpsf; BCDL=6 MWFRS (enve er left and rig lsed;C-C for m tions shown; =1.33	ond gust) .0psf; h=25ft; elope) and C- nt exposed ; e nembers and _umber	C end		No.	A.	ORTH CA	ROLL
FORCES	26=156 (l 29=164 (l 31=171 (l		), only see or c 4) TCL DOL snow	ss designed for wind I For studs exposed t Standard Industry Ga Insult qualified buildin L: ASCE 7-10; Pr=20. =1.15 Plate DOL=1.1 /); Pf=13.9 psf (flat ro DOL=1.15); Categor .10	to wind (normable End Detail og designer as 0 psf (roof live 5); Pg=20.0 p of snow: Lum	al to the face) ls as applicat per ANSI/TF e load: Lumbe sf (ground ber DOL=1.1	), ble, PI 1. er		11111111111	Sum	SEA 0449 0449 0017 M.	

M. SP January 26,2022

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent culculate event buckling of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

RE 818 Soundside Road Edenton, NC 27932

· ·									
lob	Truss	Truss Type		Qty	Ply	4894 HW	Y 27-Garage		
1070050-A	А	Common		19	1	Job Refer	ence (optional)		149884699
rter Components (Sanford)	Sanford, NC - 27332,			Dec 6 2021 Print: UHgOmQmj46HH6		6 2021 MiTek	Industries, Inc. Ti		Page: 1
-(	-10-8 7-9-	0	14-11-8	1	22	-2-0	1	29-11-0	30-9-8
C	-10-8 7-9-	0	7-2-8	1		2-8		7-9-0	0-10-8
				4x5=					
T 8-2-8 F 7-9-15 H 0-4-3 1	2	6 <sup>12</sup> 2x4, 3	3x5 = 19 4	5		20 3x5 a	2x4 #		8 9
	3x5=		12 21 3x5=	11 4x5=	22	10 3x5=			⊠ \ 3x5=
		10-1-13	1	19-9-3				29-11-0	a last
icale = 1:57.2		10-1-13		9-7-5			1.1	10-1-13	
bading CLL (roof) now (Pf/Pg) 13 CDL CLL CDL	(psf) Spacing   20.0 Plate Grip DC   .9/20.0 Lumber DOL   10.0 Rep Stress In   0.0* Code   10.0	1.15	CSI TC BC WB Matrix-MSH	0.74 Ver	-(LL) -( (CT) -(	in (loc) 0.35 10-12 0.48 10-12 0.06 8	l/defl L/d >999 240 >753 180 n/a n/a	PLATES MT20 Weight: 136 lb	<b>GRIP</b> 244/190 FT = 20%
DT CHORD Rigid ceili bracing. EACTIONS (Ib/size) Max Horiz Max Grav (Ib) - Maxi Tension DP CHORD 1-2=0/29, 5-7=-1938 DT CHORD 2-12=-3335 5-10=-153 5-12=-153 DTES Unbalanced roof live for this design. Wind: ASCE 7-10; Vul Vasd=103mph; TCDL= Cat. II; Exp B; Enclose	1.1 1.3 wood sheathing directly a ng directly applied or 10-0 2=1090/0-3-8, 8=1090/0- 2=-85 (LC 13) 2=1284 (LC 2), 8=1284 (I mum Compression/Maxim 2-3=-2169/505, 3-5=-1939 /519, 7-8=-2169/505, 8-9= /1888, 10-12=-99/1235,	load of 1 overhan 6) * This tru on the b pplied. 3-06-00 -0 oc 7) One H2. 7) One H2. recomm UPLIFT and doe 8) This trus num Internati R802.10 9/519, LOAD CASE d for	s has been designe 2.0 psf or 2.00 time gs non-concurrent v iss has been design ottom chord in all at tall by 2-00-00 wide d any other membe 5A Simpson Strong ended to connect tr at jt(s) 2 and 8. This s not consider laters is is designed in acc onal Residential Co .2 and referenced s 5(S) Standard	es flat roof load o with other live loa ned for a live loa reas where a recc will fit between i ers, with BCDL = I-Tie connectors uss to bearing w s connection is for al forces. cordance with the de sections R500	f 13.9 psf o ds. d of 20.0ps tangle he bottom 10.0psf. alls due to r uplift only 2015 2.11.1 and	'n		TH CA	ROLIN
vertical left and right e: forces & MWFRS for m DOL=1.60 plate grip D TCLL: ASCE 7-10; Pr= DOL=1.15 Plate DOL= snow); Pf=13.9 psf (fla Plate DOL=1.15); Cate Ct=1.10	posed;C-C for members eactions shown; Lumber	and umber d =1.15					Survey State	SEAL 04492	

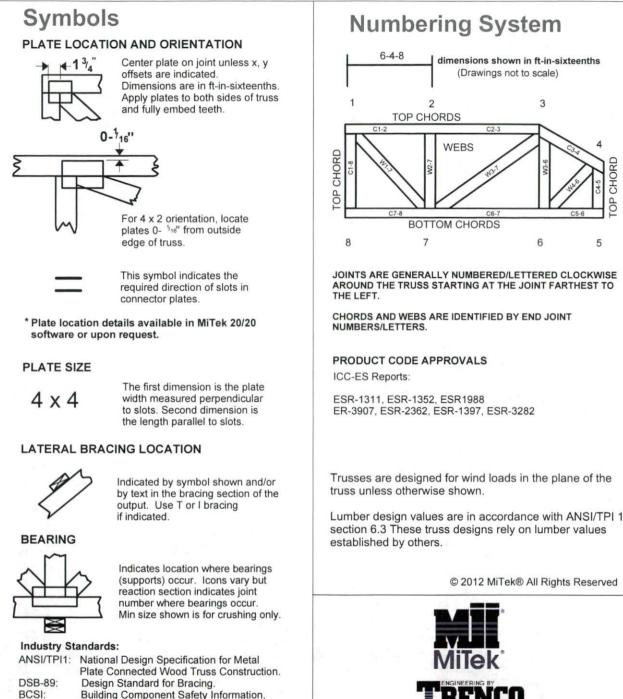
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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSITPI 1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Road Edenton, NC 27932



Connected Wood Trusses.

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# Guide to Good Practice for Handling, Installing & Bracing of Metal Plate

MiTek Engineering Reference Sheet: MII-7473 rev. 5/19/2020

# General Safety Notes

#### Failure to Follow Could Cause Property Damage or Personal Injury

- 1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- 2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- 3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- 4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- 5. Cut members to bear tightly against each other.
- 6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- 7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- 8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- 9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- 10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- 11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- 12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- 13. Top chords must be sheathed or purlins provided at spacing indicated on design.
- 14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- 15. Connections not shown are the responsibility of others.
- 16. Do not cut or alter truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- 18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- 19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- 20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- 21. The design does not take into account any dynamic or other loads other than those expressly stated.

ICC-ES Reports: ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

6-4-8

C1-2

C7-8

2

WEBS

BOTTOM CHORDS

TOP CHORDS

7

dimensions shown in ft-in-sixteenths

3

6

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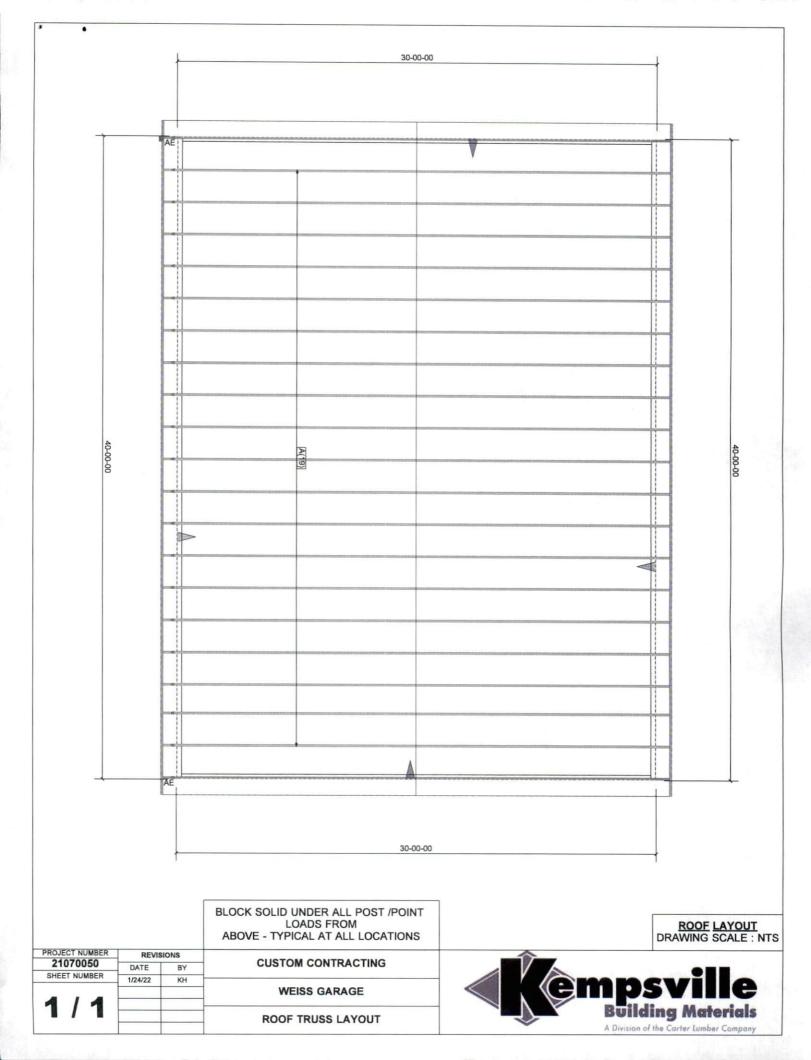
CHORD

dO.

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(Drawings not to scale)

C2-3



# RJB, P.E., P.A.

C-0269

### ROBERT J. BRACKEN ENGINEER + SURVEYOR 3768 Carbonton Road • Sanford, North Carolina 27330

February 17, 2022

Harnett County Building Inspections P.O. Box 65 Lillington, NC 27546

> REF: Footing Inspection William Weiss, Res 4894 NC Hwy 27W Lillington, NC 27546 Custom Contracting Co, Contractor

Dear Sir/Madam:

I made an onsite visit to the above referenced Residential site on February 15, 2022. The purpose of the visit was to examine the recently placed Footings for a detached Building. The footings and soil bearing were adequate.

The footings will support the live and dead loads as specified in the North Carolina Residential Building Code, 2018,ed.

If you have any questions, please feel free to contact me @ 919-774-6074.

Sincerely, Robert J. Bracken, RE RJB:jeb

## DESIGN PROFESSIONAL INSPECTION FORM

RECORD OF THE INSPECTION OF A COMPONENT OR ELEMENT BY A NC LICENSED ARCHITECT OR ENGINEER

Project Information:		
Residential Single-Family Project: 0	N	Commercial Project: Y 0
Code Enforcement Project No:		Permit No:
Project Name: Weiss Residence		Owner: William Weiss
Project Address: 4894 NC Hwy 27W, Broadway, NC	2754	Suite No: 6
Date Inspected: 2/17/2022		Contractor Name: Custom Contracting Co
Component Inspected: Footing & Se	oil Con	npaction

#### Responsible Licensed NC Architect or NC Engineer

Name:	Roberet J Bracken	
Firm Name:	RJB,PE,PA	
Phone Numbers:	Office: 919-774-6074	Mobile: 919-770-0809
Email Address:	bjbracken@windstream.net	
Mailing Address:	3768 Carbonton Rd, Sanfo	d, NC 27330

#### APPLICABLE CODE: 2018 NCRC

2018 NCBC = 2018 NC Building Code: 2018 NCRC = 2018 NC Residential Code

#### Describe Element/Component/Type of Inspection: "

(See Attached Letter)

"subgrade form/letter may also be required;

#### Attestation/Signature:

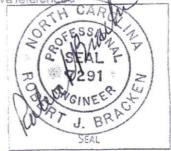
By signing below, I certify that the component and/or element of the building as identified on this form has been inspected by me or someone under my direct supervision per subsection (b2) of NC G.S. 153A-352 and is in compliance with the Cade or other proposal of the architect or engineer for the project. This inspection is in compliance with all of the requirements of the above referenced.

Code. Attach any additional documents if needed.

#### Robert J Bracken, PE

Ucensed Architect or Engineer

2/17/2022



#### Inspection Department disclaimer:

Upon the receipt of a signed written document as required under subsection (a) of Article 160A-413.5., Code Enforcement shall be discharged and released from any liabilities, duties and responsibilities imposed by this article or in common law from any claim arising out of or attributed to the component or element in the construction of the building for which the signed written document was submitted. Be aware that this inspection will be noted in all inspection records including the Certificate of Occupancy or Certificate of Compliance. This inspection does not address any local ordinances or zoning requirements.

12/2019