

803-428-2122

1745 - Cline

Qty: 13

Truss: T1

Customer: David & Shelly Johnson

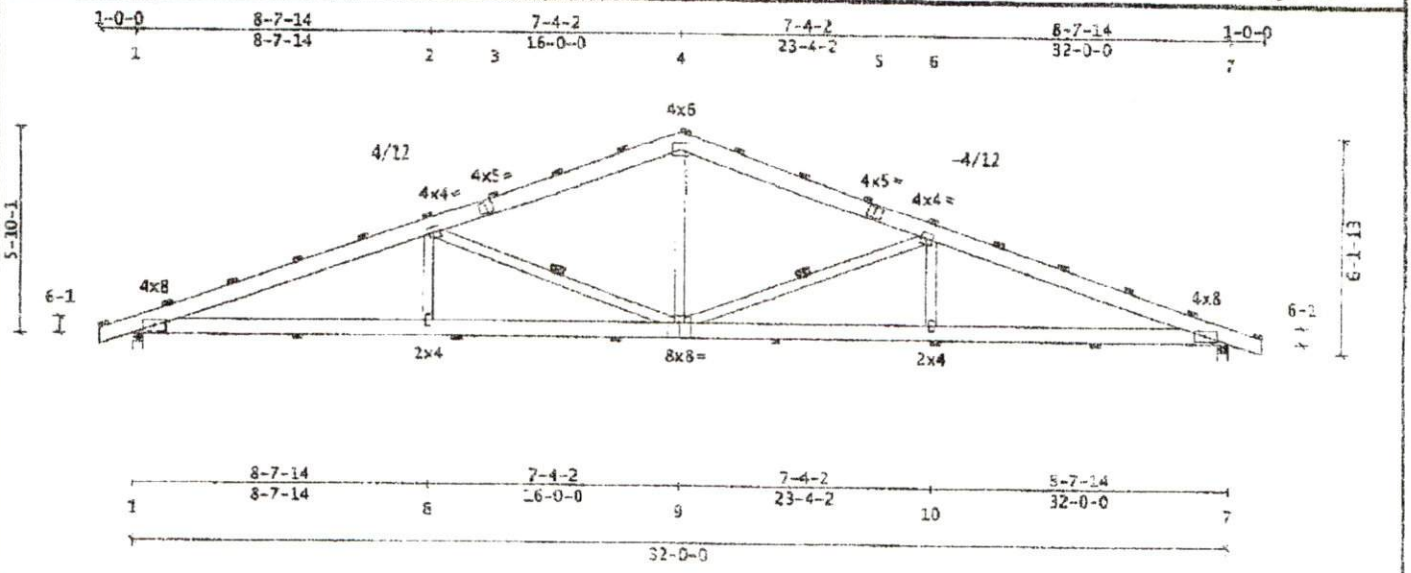
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TID: 137203

Date: 10/13/21

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Truss Mfr. Contact: McKayla Cole



Truss Weight = 195.1 lb

<p>Code/Design: 180-2515/TPI-2014</p> <p>PSF Live Dead Sur Factors</p> <p>TC 20.0 5.0 Live Wind Snow</p> <p>BC 0.0 5.0 LJM 1.25 1.60 N/A</p> <p>Total 50.0 Plr 1.25 1.60 N/A</p> <p>Spacing: 4'-00"-00" o.c. Piles: 1</p> <p>Repetitive Member Increase: No</p> <p>Greener Imports: No Wet Service: No</p> <p>Flap Tolerance: 20% Creep (Kcr) = 2.0</p> <p>CR Soffit Load: 2.0 psf</p>	<p>Snow Load Specs</p> <p>ASCE7-10 Ground Snow (Pg) = N/A</p> <p>Risk Cat: 1 Terrain Cat: C</p> <p>Roof Exposure: Partially Exposed</p> <p>Thermal Condition: Unheated (1.2)</p> <p>Unobstructed Slippery Roof: Yes</p> <p>Low-Slope Minimums (P/Pin): No</p> <p>Unbalanced Snow Loads: No</p> <p>Rain Surcharge: No Ice Dam Chk: Yes</p>	<p>Wind Load Specs</p> <p>ASCE7-10 Wind Speed (V) = 110 mph</p> <p>Risk Cat: 1 Exposure Cat: C</p> <p>Bldg Dims: L = 0.0 ft B = 0.0 ft</p> <p>M.R.H(h) = 15.0 ft Rec = 1.0</p> <p>Bldg Enclosure: Enclosed</p> <p>Wind Dir (psf): TC = 4.0 BC = 1.0</p> <p>Wind Vertical Exposure: L = Yes E = Yes</p> <p>Wind Uplift Reporting: ASCE7 MWFRS</p> <p>CAC End Date: 3-02-06</p>	<p>Additional Design Checks</p> <p>10 psf Non-Concurrent ECLL: No</p> <p>20 psf BC Limited Storage: No</p> <p>200 lb BC Accessible Ceiling: No</p> <p>300 lb TC Maintenance Loads: No</p> <p>2000 lb TC Safe Load: No</p> <p>Unbalanced TCLL: Yes</p>
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<p><b>Material Summary</b></p> <p>TC 2x4 SP (ALSC6-2013) #1</p> <p>BC 2x6 SP (ALSC6-2013) #1</p> <p>Robs 2x4 SP (ALSC6-2013) #3/Stud</p>	<p><b>Reaction Summary</b></p> <p>Reaction Summary (lbs)</p> <p>Unt --K-Loc-React-Up--W-Act--B-End--Mat PSI</p> <p>1 01-12 2027 520 03-08 C2-14 SPF 470</p> <p>7 31-10-04 2328 520 03-05 C2-14 SPF 470</p> <p>Max Horiz = -121 / +121 at Joint 1</p>	<p><b>Deflection Summary</b></p> <p>Truss Spc: Limit Actual (in) Location</p> <p>Vert LL L/240 L/999 (-0.25) 9-10</p> <p>Vert DL L/120 L/499 (-0.14) 8-9</p> <p>Vert CR L/180 L/95 (-0.39) 8-9</p> <p>Horz DL 0.75in 0.09) Jct 7</p> <p>Horz CR 1.25in 0.13) Jct 7</p> <p>Chng CR 2L/180 2L/999 (0.00) 1-1</p> <p>Chng CR 2L/180 2L/999 (0.00) 7-7</p>
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**Member Forces Summary**

Mem	Trs	Comp	CSR
TC	CR-1	38	0.05
	1-2	2241	0.67
	2-3	1697	0.52
	3-4	1726	0.47
	4-5	1725	0.47
	5-6	1697	0.52
	6-7	2241	0.67
	7-0H	38	0.05
BC	1-8	4225	0.68
	7-10	4225	0.68
	8-9	4225	0.56
	9-10	4225	0.56
Web	2-6	156	0.08
	2-9	839	0.52
	4-9	1259	0.48
	6-9	618	0.32
	8-10	156	0.08

**Loads Summary**

This truss has been designed for the effects of an unbalanced top chord live load occurring at [16'-00-00] using a 1.00 Full and 0.60 Reduced load factor.

See Loadcase Report for loading combinations and additional details.

Dead Loads may be slope adjusted: > 12.0/12

**Notes**

Plates designed for C<sub>y</sub> at 0.90 and Rotational Tolerance of 10.0 degrees

Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unbracketed diaphragm loads across those joints.

Continuous Laterals: Restraint (CLR) rows require diagonal bracing per I-NEBC-BRACE. Alternatively, see D-WEBREINFORCE.

Vert CR and Horz CR are the vertical and horizontal deflections due to live load plus the creep component of deflection due to dead load, computed as Def<sub>LL</sub> + (CR = 1) \* Def<sub>DL</sub> in accordance with ANSI/TPI 1.

**Bracing Data Summary**

-----Bracing Data-----

Chords: Sheathing required or bracing indicated:

-----Purlins-----	-----From-----	-----To-----	#Bays
TC	2-00-00	11-00-00	33-00-00 11
BC	4-00-00	0	32-00-00 9

-----Web Bracing----- CLR -----

Single: 1-9 9-6

Continuous Restraint Bracing Req'd See KSI-B3 7.2

**Plate offsets (X, Y):**

(None unless indicated below)

Jct1(-05-06,-90-02), Jct7(00-16,-00-02), Jct9(0,-31-35)

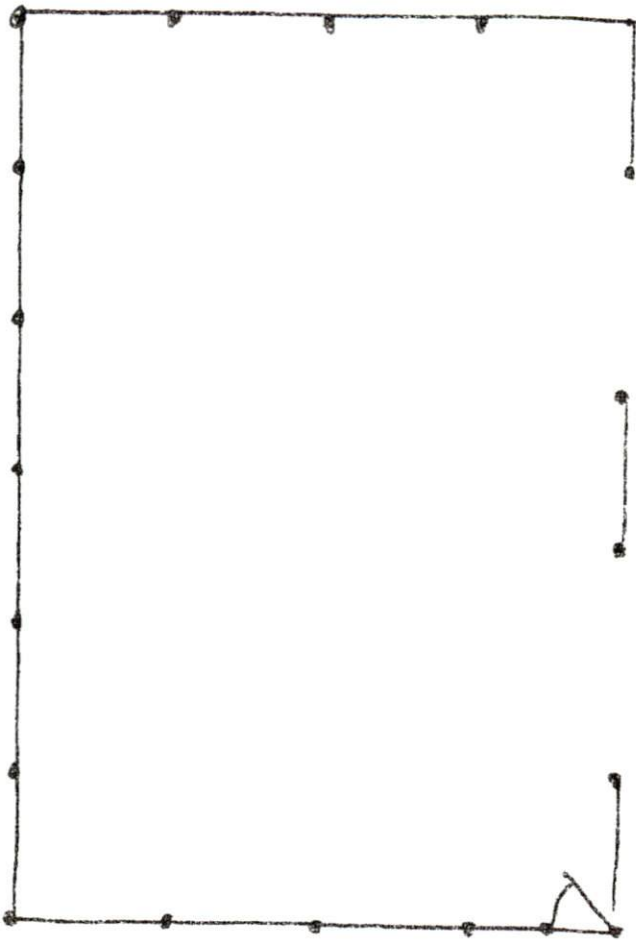


Job# PER212897 P.E. Robbins, P.E. 1777 State Route 167 Victoria IL 61485 10/12/2021

NOTICE: A copy of this design shall be furnished to the erection contractor. The design of this individual truss is based on design criteria and requirements supplied by the Truss Manufacturer and relies upon the accuracy and completeness of the information set forth by the Building Designer. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. See the cover page and the "Important Information & General Notes" page for additional information. All connector plates shall be manufactured by Simpson Strong-Tie Company, Inc in accordance with ESR-2782. All connector plates are 20 gauge, unless the specified plate size is followed by an "-18" which indicates an 18 gauge plate, or "S18" which indicates a high tensile 18 gauge plate.

**SIMPSON** Connector Solutions  
Truss Solutions  
2021.3.0.190

2-9 1/4 LVL OVER GARAGE DOORS  
TRUSS 4' O.C  
6x6 POST 8' O.C



32x48 POLE BUILDING

6" CONC. MIX FOOTINGS + BACKFILL

12" x 36" DEEP HOLE

NOT TO SCALE

6x6 POST  
8' O.C.

2x12 PLATES  
3/4" THROUGH BOLTS

25  
HANGNAIL  
TIES

PRE-ENGINEERED  
TRUSS 4'-0" O.C.

2x4 RAFTERS  
2'-0" O.C.

29 GA. METAL  
ROOFING

