

SHOP DRAWING GENERAL INFORMATION PAGE

CODES & STANDARDS

Building Code:	2012/2015/2018/2019 IRC, 2012 IBC
ESR Report number:	ESR-1662 August 2018
3rd Party Inspection Agency:	PFS Corporation, Madison WI
Quality Assurance Manual:	Superior Walls of America 2005 Edition
Site Preparation Guide:	Superior Walls Builder Guideline Booklet Rev. June 2018
Fire Test Standards:	ASTM E119 ANSI/UL 1715

WALL MATERIALS

Concrete Compressive Strength:	Min. 5,000 PSI
water/cement Ratio:	<0.40
Reinforcing Steel:	No. 4 and larger - 60,000 PSI No. 3 and smaller - 40,000 PSI
Secondary Reinforcement:	Polypropylene Fiber
Embedded Wood Blocking:	Preservatively Pressure Treated
EPS Foam Insulation:	Flame Spread: 20 Smoke Development: 240
XPS Foam Insulation:	Flame Spread: 5 Smoke Development: 165

SITE/WALL CONDITIONS

Frost Depth:	Min. 12 inches
assumed Soil Bearing Capacity:	2,000 PSF
Seismic Category:	A, B, C
Basic Wind Speed:	155 maximum PSF
Wall Loading:	7,500 Pounds/LF (uniform) Maximum
Brickledge Loading:	2,900 Pounds/LF Maximum
Crushed Stone Footing Depth:	Min. 6 Inches thick or more (see table in Builder Guideline Booklet Table R403.4)
Crushed Stone Size:	$\frac{3}{8}$ Inch and smaller (cleaned)
Backfill Material:	100 LB/CF Equivalent Fluid Pressure Max (see Builder Guideline booklet for more information)
Beam Pocket(s) & Point Load(s):	38,000 Pound Maximum - Data supplied by Customer/Builder (see plan for location and sizes)

GENERAL NOTES

- Jobsite shall be prepared by the builder in accordance with the Superior Walls of America builder Guideline Booklet - Site Preparation and Framing Attachment Requirements (Rev. January 2016).
- Auxiliary drain pipe must be four (4) inch diameter perforated, covered with filter fabric and directed to a sump pit or daylight.
- Builder shall establish the elevation benchmark (if necessary)
- Builder shall insure proper site access for trucks and crane.

INSTALLATION NOTES

- Installation shall be supervised by a Superior Walls certified installer. Certification is obtained through Superior Walls of America, Ltd.
- Installation shall comply with Superior Walls of America's Installation Manual (Rev. July 2011).

DRAWING NOTES

- All measurements for brick, stone, or support ledges are from Top Of Wall (TOW).
- Drawings are not to scale.

DAMP PROOFING

Superior Walls are recognized by the ICC-ES as an alternative method of providing foundation wall damp proofing. No additional damp proofing is required. (See ESR-1662 & ICC-ES Legacy Report 21-72)

PLEASE NOTE

To comply with building code requirements, the framing/decking connections at the top of the Superior Walls and floor slab at the bottom of the Superior Wall **MUST** be completed **PRIOR** to backfilling.

CUSTOMER RELEASE

The attached drawing was created from information and dimensions provided by the customer/builder. Superior Walls of North Carolina, Inc. is not responsible for deviations from the Blue Print or information provided by the customer/builder.

I have reviewed the attached drawing & all of the dimensions and objects therein; I understand the the Superior Walls will be custom manufactured per this drawing specifically for my project. By signing below I am certifying that I have reviewed the attached drawing and all of its listed dimensions and I accept FULL RESPONSIBILITY of any and all measurements and information provided by me/my associates/my company.

CUSTOMER MUST SIGN & DATE BELOW

Customer/Builder Signature & Date

PROJECT:

Job Number: -
Job Name: Chandler_Barn
Job Address: 794 Silas Moore Rd
Lot #: ---

BUILDER:

Company: Chandler
Contact:
Phone/Email: ---

MUNICIPALITY:

--- County
COATS, NC



3570 S. Main Street
Salisbury, NC 28147
Phone: 704-636-6200
Toll-Free: 877-896-9255
www.superiorwallsonc.com

DRAWING DATA:

Job Number: -
Sales Rep: R. MUSSO

Drawn By: ---

Date Created: Aug. 11, 2021
Date Modified: Sep. 02, 2021
Revision: 3

Pages: 5

Superior Walls® FOUNDATION DRAWING

12' WALLS - TOTAL LENGTH: 160'

1/2" DIA. x 6" BOLTS FOR SILL PLATE

#	DESCRIPTION	
6	BRICK LEDGE TOTALING 53'-10"	
40	SLAB CONNECTOR	

ID	#	OBJECT	DESCRIPTION	WIDTH	HEIGHT	FROM TOP OF WALL	FROM BOT OF WALL	MAX HDR CAPACITY
A	1	DOOR	GARAGE DOOR, STYLE 1	120"	96"	44"	4"	1250 PLF
B	1	DOOR	STYLE 1	38"	83"	57"	4"	5500 PLF
C	1	DOOR	GARAGE DOOR, STYLE 1	96"	96"	44"	4"	1950 PLF
D	2	DOOR	GARAGE DOOR, STYLE 1	60"	96"	44"	4"	4820 PLF
E	3	WINDOW	STYLE 1	48"	24"	44"	76"	5500 PLF

PLEASE NOTE:
Adjustments made after sign-offs may incur an additional \$200 service charge

OWNER/BUILDER NOTIFICATION:
BY SIGNING THESE DRAWINGS YOU ARE ACKNOWLEDGING THAT THE WALLS WILL BE BUILT TO THE DIMENSIONS INDICATED ON THESE PLANS, AND THAT YOU ARE ASSUMING ANY AND ALL LIABILITY THAT MAY RESULT FROM THE WALLS BEING MANUFACTURED AS SHOWN

BUILDER CHECK LIST:
-RO's/DIMS/WALL HEIGHT CORRECT?
-OBJECT OPENINGS CORRECT?
-WOOD BUTTS IND./REQ'D?
-SUPPORT/BRICK LEDGES CORRECT?
-EXTRA SUPPORT IND. FOR PT. LOAD?

SIGNATURE: _____
DATE: _____

THESE DRAWINGS ARE APPROVED FOR FINAL PRODUCTION AS ILLUSTRATED AND NOT SUBJECT TO CHANGE.

CUSTOMER SIGNATURE: _____ DATE: _____

Superior Walls
of North Carolina
3570 S. Main Street
Salisbury, NC 28147
Phone: 704-636-6200
Toll-Free: 877-896-9255

REV	DATE	BY
1	08-11-21	JG
2	08-26-21	JG
3	09-02-21	MJR

PROJECT:
Chandler Barn
BUILDER:
Chandler

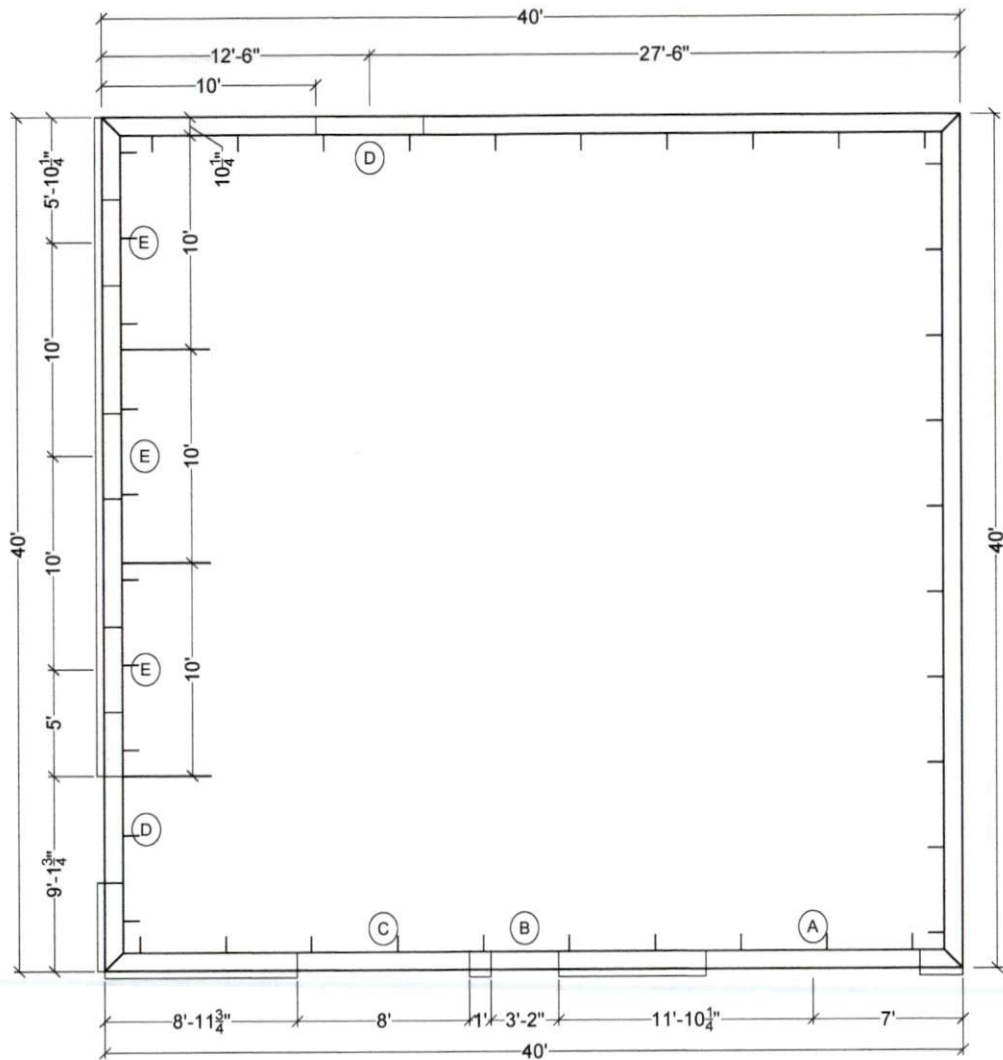
STATUS: ISSUED FOR APPROVAL

SALESMAN: R. Musso

FILENAME: Chandler_Barn

SHEET TITLE:
SUMMARY

SHEET NO 2 of 5



R. MUSSO
 CHANDLER
 794 SILAS MOORE RD

12' Walls

MIN STONE DEPTH
 FOR 2,000 PSF SOIL BEARING
 1-STORY - 6"
 2-STORY - 8"
 3-STORY - 14"

Shear walls may be required,
 refer to builder guideline
 booklet for more information

Top of wall slab detail
 per builder guideline
 book must be adhered to

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REV.	DATE	BY
1	08-11-21	JG
2	08-26-21	JG
3	09-02-21	MJR

PROJECT:
 BUILDER:

STATUS: ISSUED FOR APPROVAL

SALESMAN: R. MUSSO

FILENAME: Chandler_Barn

SHEET TITLE:
 PLAN W/O DIMS

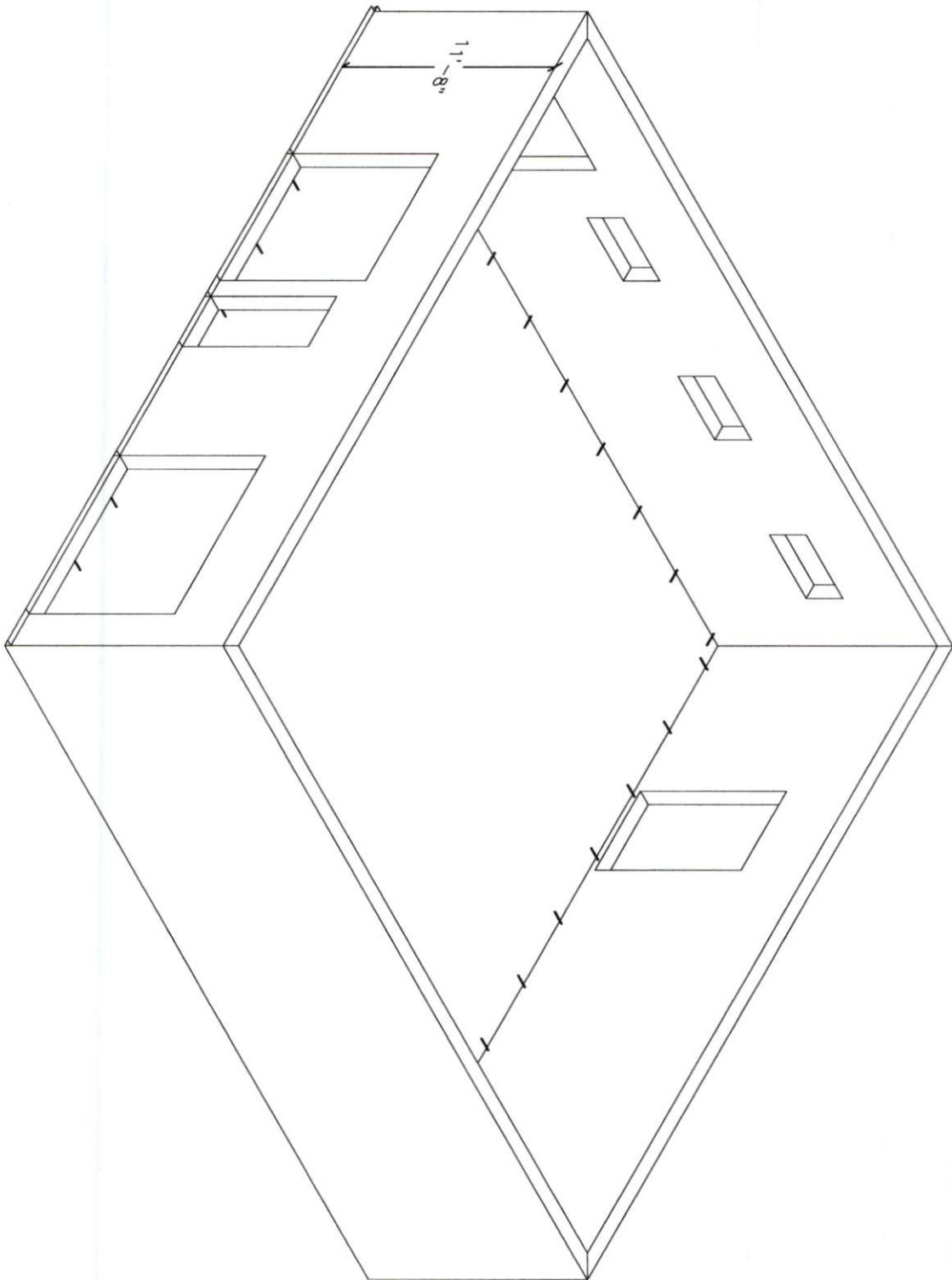
SHEET NO 3 of 5

CUSTOMER SIGNATURE: _____ DATE: _____

Chandler Barn
 Chandler

CUSTOMER SIGNATURE: _____

DATE: _____



SHEET NO 4 of 5

SHEET TITLE:
ISOMETRIC 1

FILENAME: Chandler Barn

SALESMAN: R. MURPHY

STATUS: ISSUED FOR APPROVAL

PROJECT:
Chandler Barn
BUILDER:

Chandler Barn Chandler

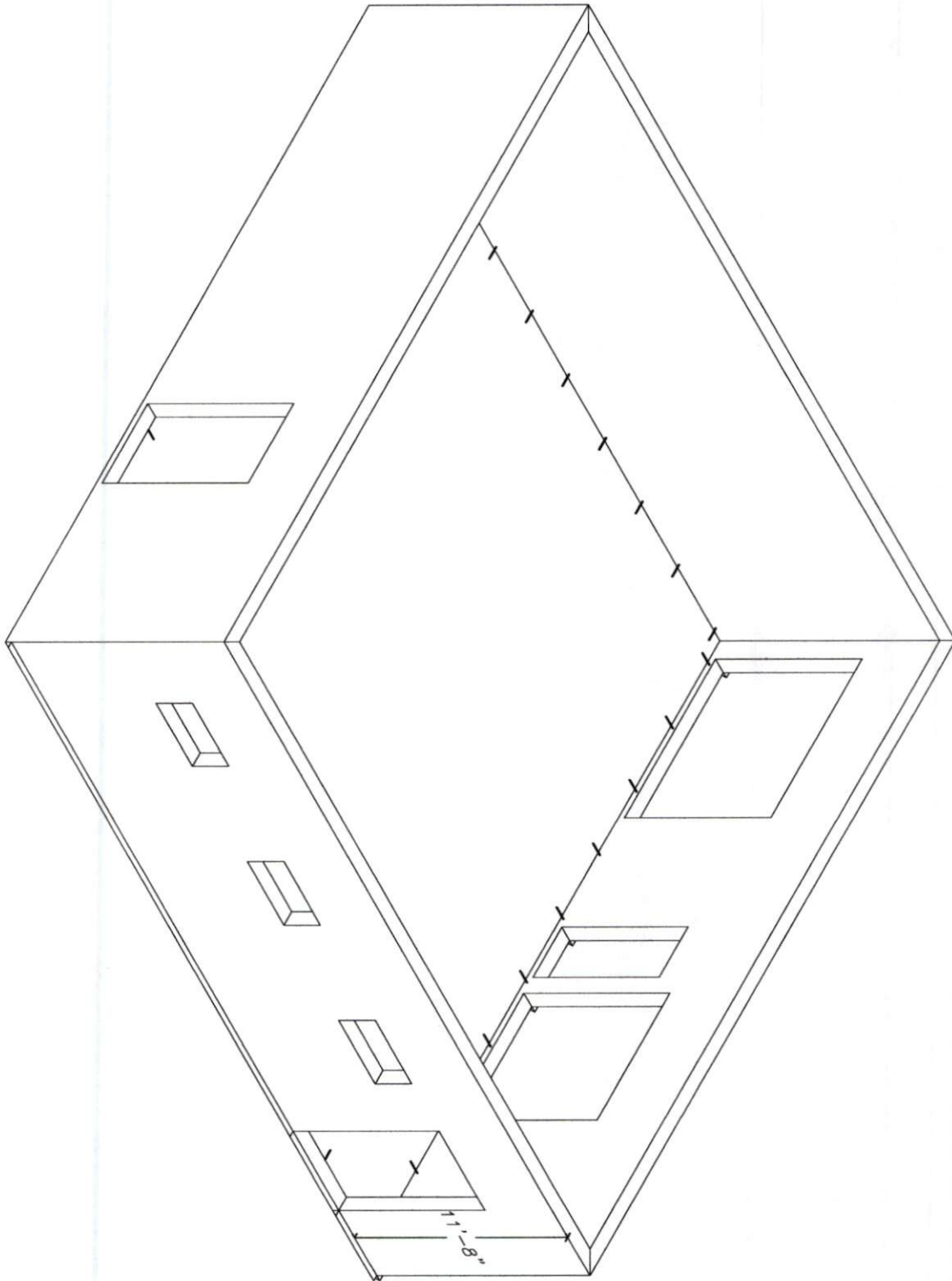
REV.	DATE	BY
1	08-11-21	JG
2	08-26-21	JG
3	09-02-21	MJR

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of North Carolina

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Salisbury, NC 28147
Phone: 704-636-6200
Toll-Free: 877-896-9255

CUSTOMER SIGNATURE: _____

DATE: _____



SHEET NO. 5 of 5

SHEET TITLE:
ISOMETRIC 2

FILENAME: Chandler_Barn

SALESMAN: R. MUSIBO

STATUS: ISSUED FOR APPROVAL

PROJECT:
Chandler Barn
Chandler

BUILDER:

REV.	DATE	BY
1	08-11-21	JG
2	08-26-21	JG
3	09-02-21	MJR



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Salisbury, NC 28147
Phone: 704-636-6200
Toll-Free: 877-896-9255

Trenco
818 Soundside Rd
Edenton, NC 27932

Re: T21-09034
DAVID CHANDLER

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Longleaf Truss Company.

Pages or sheets covered by this seal: I48506201 thru I48506201

My license renewal date for the state of North Carolina is December 31, 2021.

North Carolina COA: C-0844



October 26, 2021

Sevier, Scott

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Job	Truss	Truss Type	Qty	Ply	DAVID CHANDLER	I48506201
T21-09034	T01	DBL. FINK	11	1		

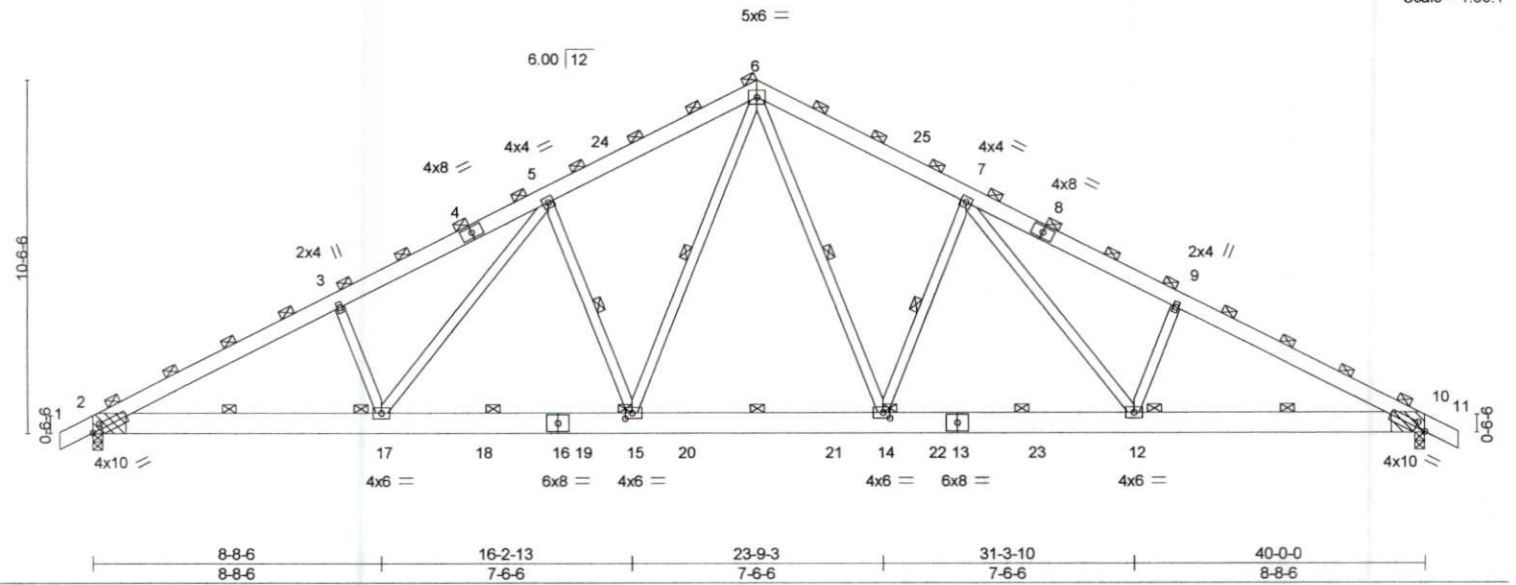
Longleaf Truss Company, West End, NC - 27376,

8.430 s Aug 16 2021 MiTek Industries, Inc. Mon Oct 25 10:24:19 2021 Page 1

ID:nDPMJPchQB4UpnRDem3ZZGybmEV-pq?jGvc_Ue02WCcHIAxRWL_ov77eCxWtGudl_lyPv1Q

1-0-0	7-5-5	13-8-11	20-0-0	26-3-5	32-6-11	40-0-0	41-0-0
1-0-0	7-5-5	6-3-5	6-3-5	6-3-5	6-3-5	7-5-5	1-0-0

Scale = 1:69.1



LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	4-0-0	TC 0.56	in (loc) l/defl L/d	MT20	244/190
Snow (Pf/Pg) 12.6/15.0	Plate Grip DOL 1.15	BC 0.39	Vert(LL) -0.24 14-15 >999 240		
TCDL 5.0	Lumber DOL 1.15	WB 0.64	Vert(CT) -0.33 14-15 >999 180		
BCLL 0.0 *	Rep Stress Incr NO	Matrix-S	Horz(CT) 0.09 10 n/a n/a		
BCDL 5.0	Code IBC2018/TPI2014			Weight: 324 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x6 SP No.1	TOP CHORD 2-0-0 oc purlins (2-10-11 max.)
BOT CHORD 2x8 SP DSS	(Switched from sheeted: Spacing > 2-8-0).
WEBS 2x4 SP No.3	BOT CHORD 4-0-0 oc bracing.
	WEBS 1 Row at midpt 5-15, 6-15, 6-14, 7-14

REACTIONS. (size) 2=(0-3-8 + bearing block) (req. 0-5-2), 10=(0-3-8 + bearing block) (req. 0-5-2)
 Max Horz 2=398(LC 11)
 Max Uplift 2=918(LC 12), 10=918(LC 12)
 Max Grav 2=3258(LC 24), 10=3258(LC 25)

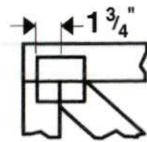
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-5869/1591, 3-5=-5689/1675, 5-6=-4392/1437, 6-7=-4392/1437, 7-9=-5690/1675, 9-10=-5870/1591
 BOT CHORD 2-17=-1243/5365, 15-17=-869/4313, 14-15=-486/3194, 12-14=-869/4074, 10-12=-1243/5067
 WEBS 3-17=-704/436, 5-17=-379/1366, 5-15=-1316/590, 6-15=-508/1884, 6-14=-508/1884, 7-14=-1316/590, 7-12=-379/1368, 9-12=-704/436

- NOTES-**
- 2x8 SP DSS bearing block 12" long at jt. 2 attached to front face with 4 rows of 10d (0.131"x3") nails spaced 3" o.c. 16 Total fasteners. User Defined Bearing crushing capacity= 425psi.
 - 2x8 SP DSS bearing block 12" long at jt. 10 attached to front face with 4 rows of 10d (0.131"x3") nails spaced 3" o.c. 16 Total fasteners. User Defined Bearing crushing capacity= 425psi.
 - Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=3.0psf; BCDL=3.0psf; h=12ft; B=45ft; L=40ft; eave=5ft; Cat. II; Exp B; Pr. Enclosed; MWFRS (directional); cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=12.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.20
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 12.6 psf on overhangs non-concurrent with other live loads.
 - Dead loads shown include weight of truss. Top chord dead load of 5.0 psf (or less) is not adequate for a shingle roof. Architect to verify adequacy of top chord dead load.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=918, 10=918.
 - This truss is designed in accordance with the 2018 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

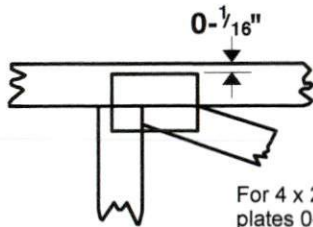


Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

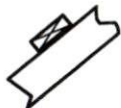
* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING

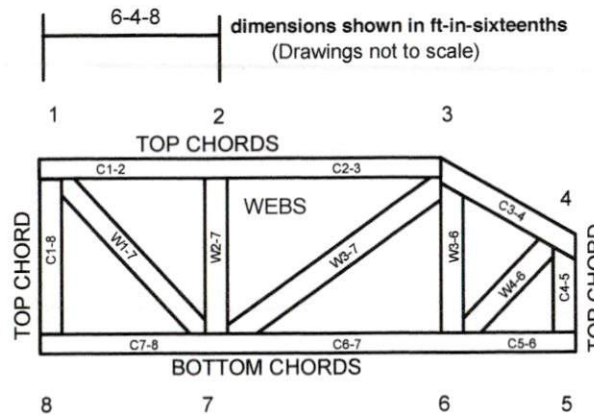


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

- ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
- DSB-89: Design Standard for Bracing.
- BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988
ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 5/19/2020



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
21. The design does not take into account any dynamic or other loads other than those expressly stated.