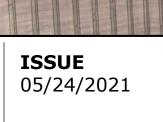


PROJECT NO. 21.16.01.0519



DRAWN BY TM, MM

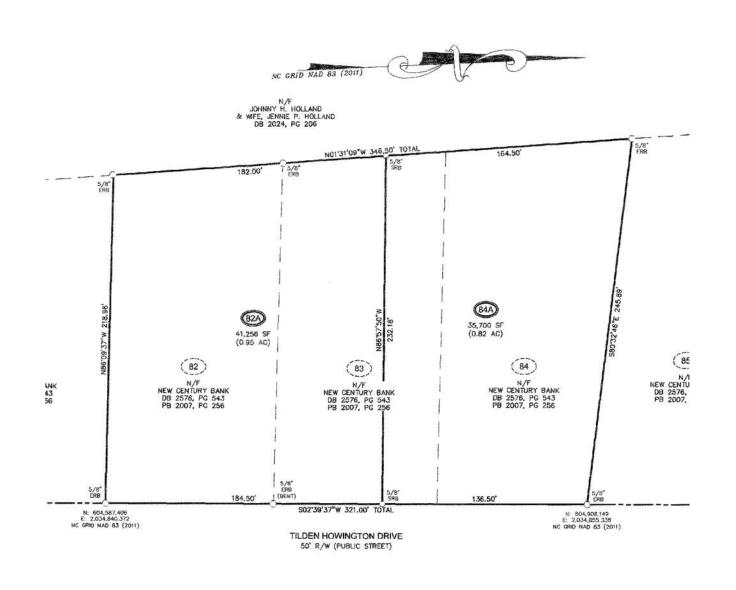
DESCRIPTION COVER



CLIENTTAYLOR Family

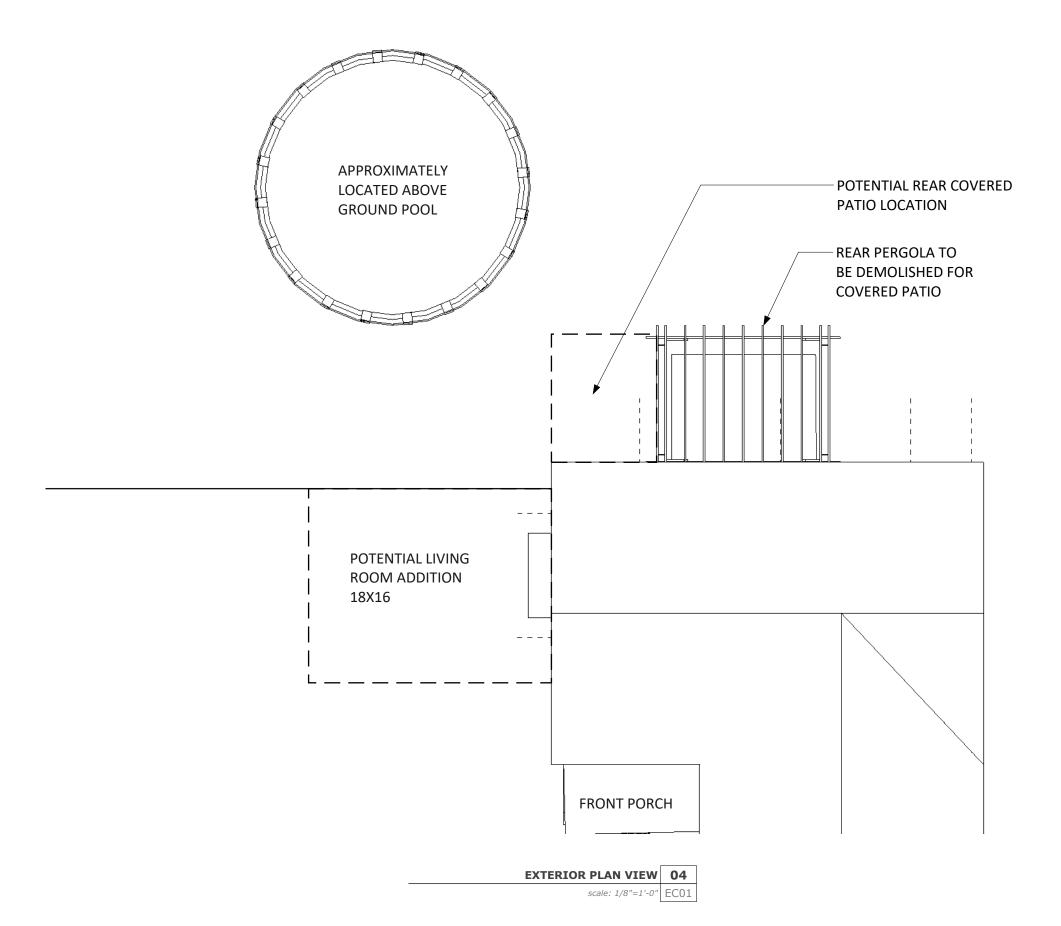
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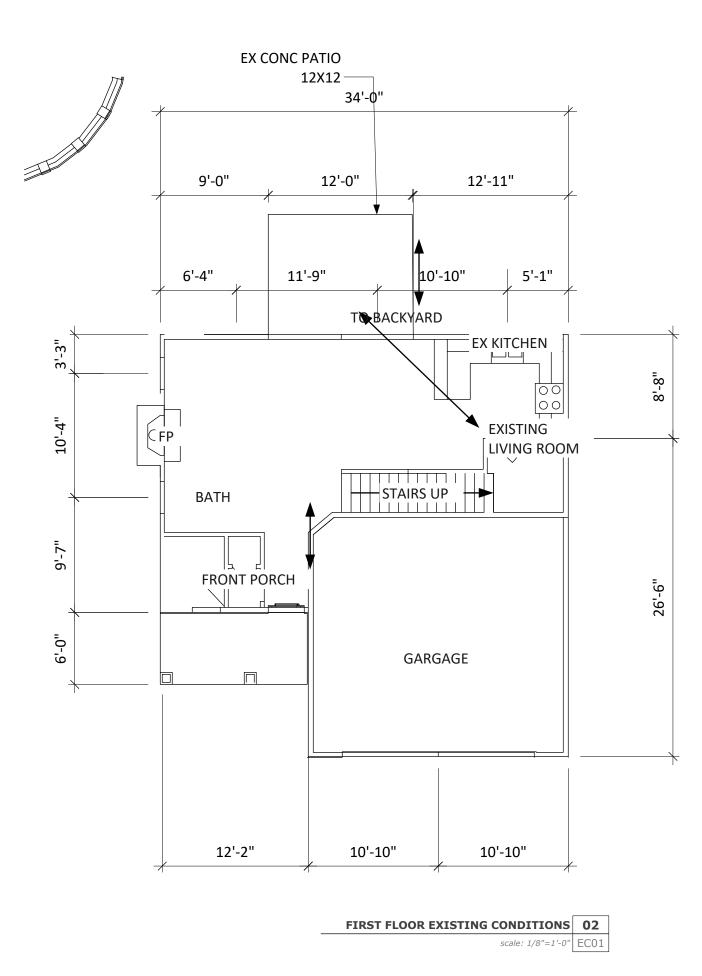
INITIAL MEASUREMENTS TO DEFINE PROJECT SCOPE. MR. AND MRS. TAYLOR HAVE CONTACTED THE END VISION TEAM TO HELP ASSIST, PRIMARILY WITH THE EXPANSION AND OR RELOCATION OF LIVING ROOM AND REAR ENTERTAINMENT SPACES. THE FAMILY'S FIRST FLOOR IS CURRENTLY 444.25 ft² AND DOES NOT OFFER ENOUGH COMFORTABLE MOVEMENT BETWEEN EXISTING LIVING, DINING AND KITCHEN. THE FAMILY WOULD LIKE TO OPEN UP THE EXISTING SOUTH WALL AT THE LOCATION OF THE FIREPLACE TO CREATE A CASED OPENING LEADING TO A NEW 18X16 (288 SQ FT) LIVING ROOM. THE FAMILY WOULD LIKE TO GENTLY REMOVE AND REUSE EXISTING WINDOWS AND DOORS FOR THE EXPANDED SPACE WHILE RELOCATING THE EXISTING SLIDING GLASS DOOR. THE REAR ENTERTAINING PATIO CURRENTLY 10X12 WOULD BE EXPANDED TO AN ADDITIONAL 10X12 FOR A TOTAL 10X24 WITH SHED ROOF (240 SQ FT.) AS A POTENTIAL THIRD PROJECT THE FAMILY WOULD LIKE TO EXPAND THIER EXISTING KITCHEN FOOTPRINT, INCREASE CABINETRY AND REMOVE PENINSULA/BAR. ALL SPACES COMBINED EQUATE TO 972.25 SQ FT.

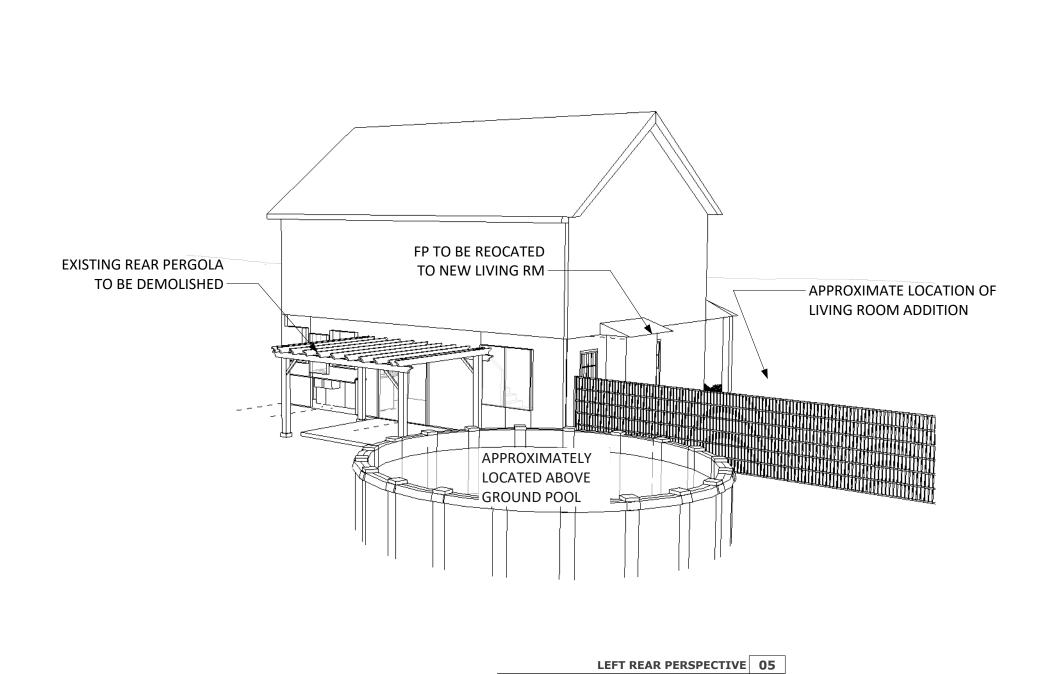


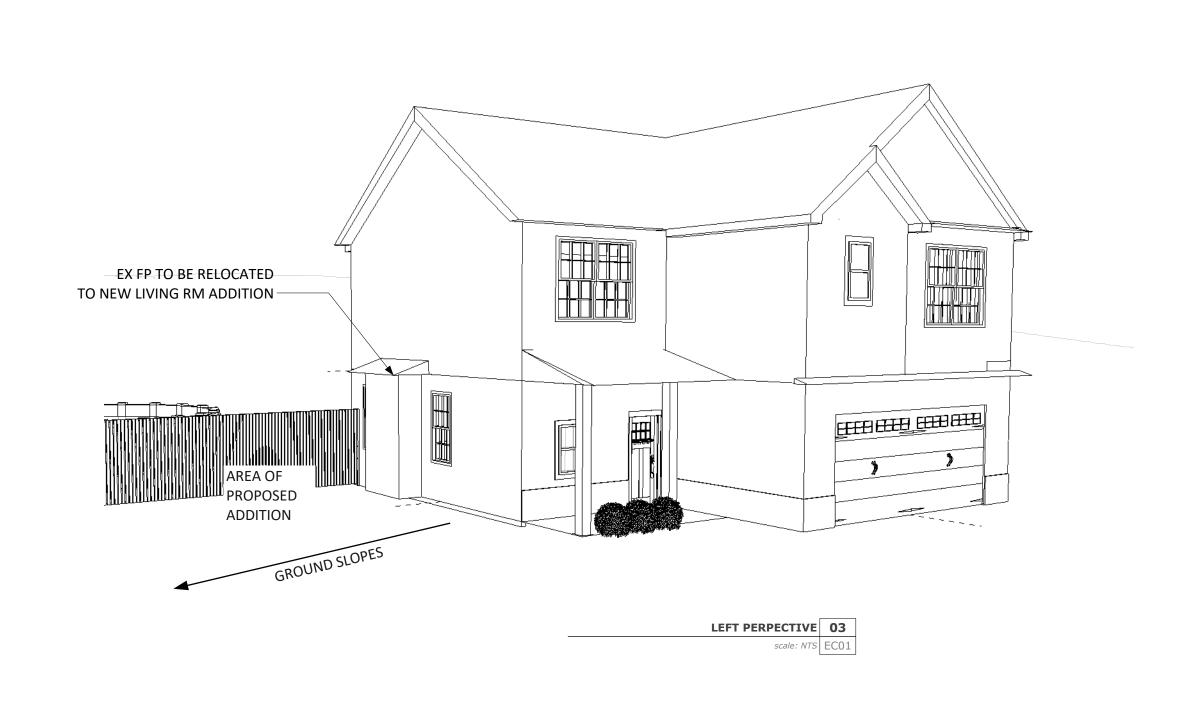
CUMBERLAND COUNTY RECORDED SURVEY 01

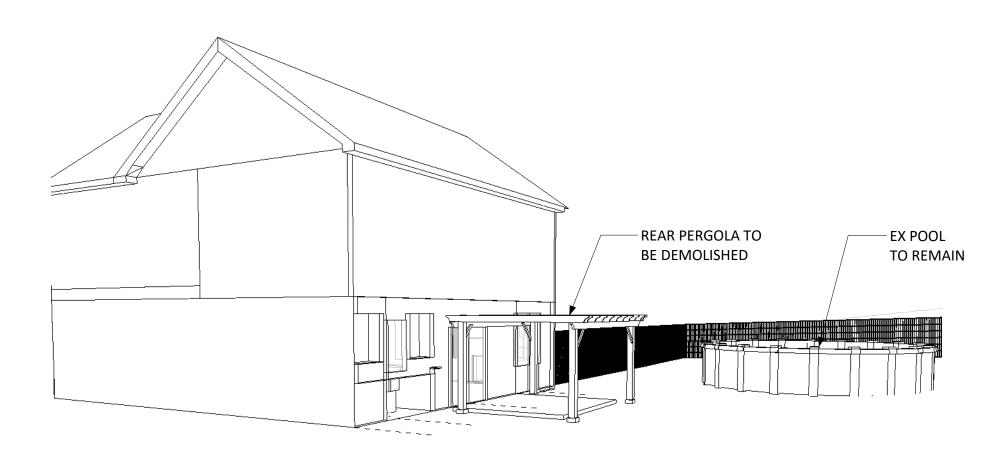
scale: NTS EC01











RIGHT REAR PERSPECTIVE 06

CLIENTTAYLOR Family

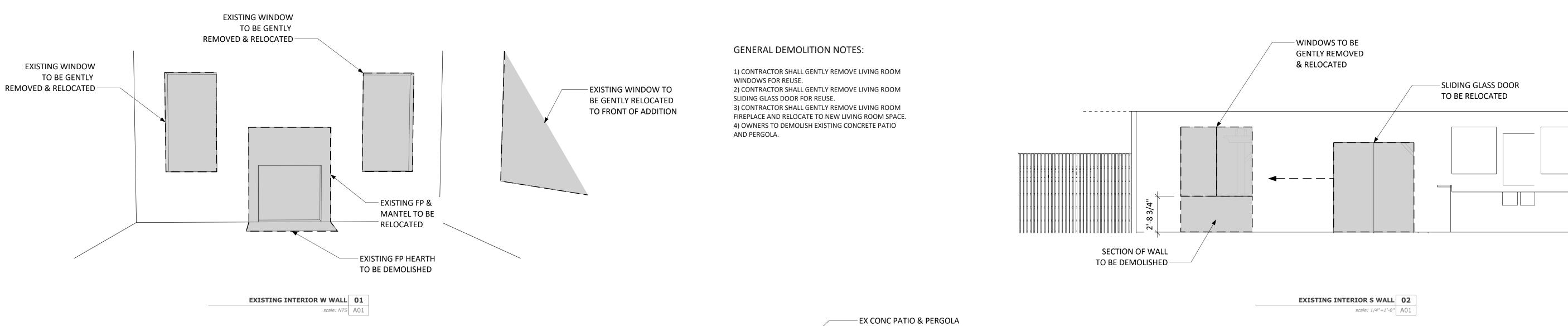
PROJECT
288 TILDEN HOWINGTON DR.,
LILLINGTON, NC

PROJECT NO. 21.16.01.0519

ISSUE 05/24/2021

DRAWN BY TM, MM **DESCRIPTION**MEASUREMENTS
EX. CONDITIONS





TO BE DEMOLISHED BY OWNERS EXISTING PICTURE WINDOW TO BE GENTLY REMOVED & RELOCATED TO FRONT OF ADDITION -EXISTING SLIDING **EXISTING WINDOW GLASS DOOR TO** TO BE REMOVED BE RELOCATED & RELOCATED -EXISTING FIRE PLACE TO BE RELOCATED -EX KITCHEN EXISTING HEARTH
TO BE DEMOLISHED **EXISTING** PANTRY EXISTING STAIR UP EXISTING WINDOW TO BE REMOVED & RELOCATED — EXISTING GARAGE

PLAN NORTH $\left(N\right)$

DEMOLITION FLOOR PLAN 03

scale: 1/4"=1'-0" A01

DRAWN BY

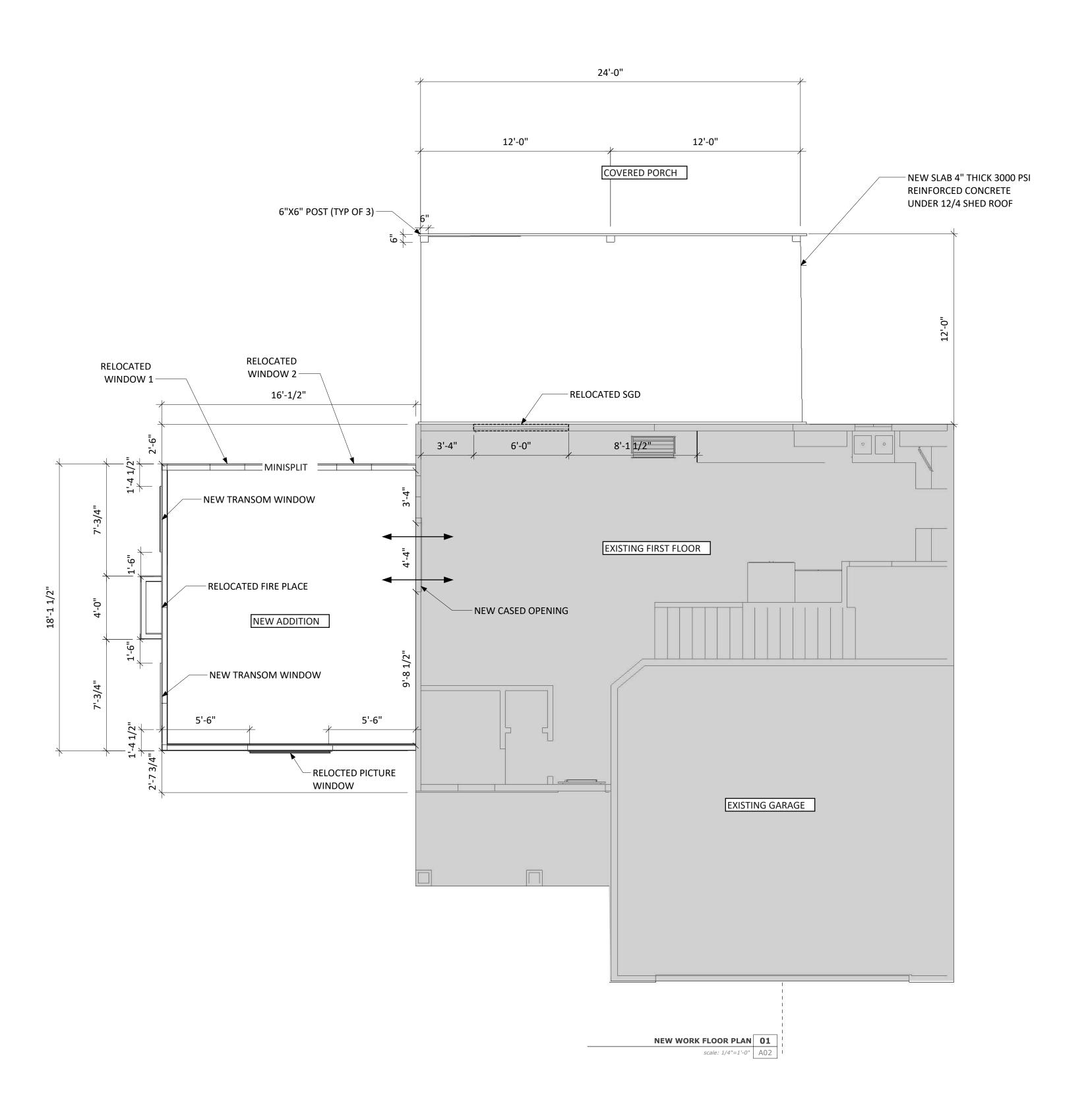
PROJECT NO. 21.16.01.0519

CLIENTMR|MRS TAYLOR
288 TILDEN HOWINGTON
DR., LILLINGTON, NC

ISSUE 06/29/2021

RE-ISSUE <ReIssueDa

PROJECT Taylor resi



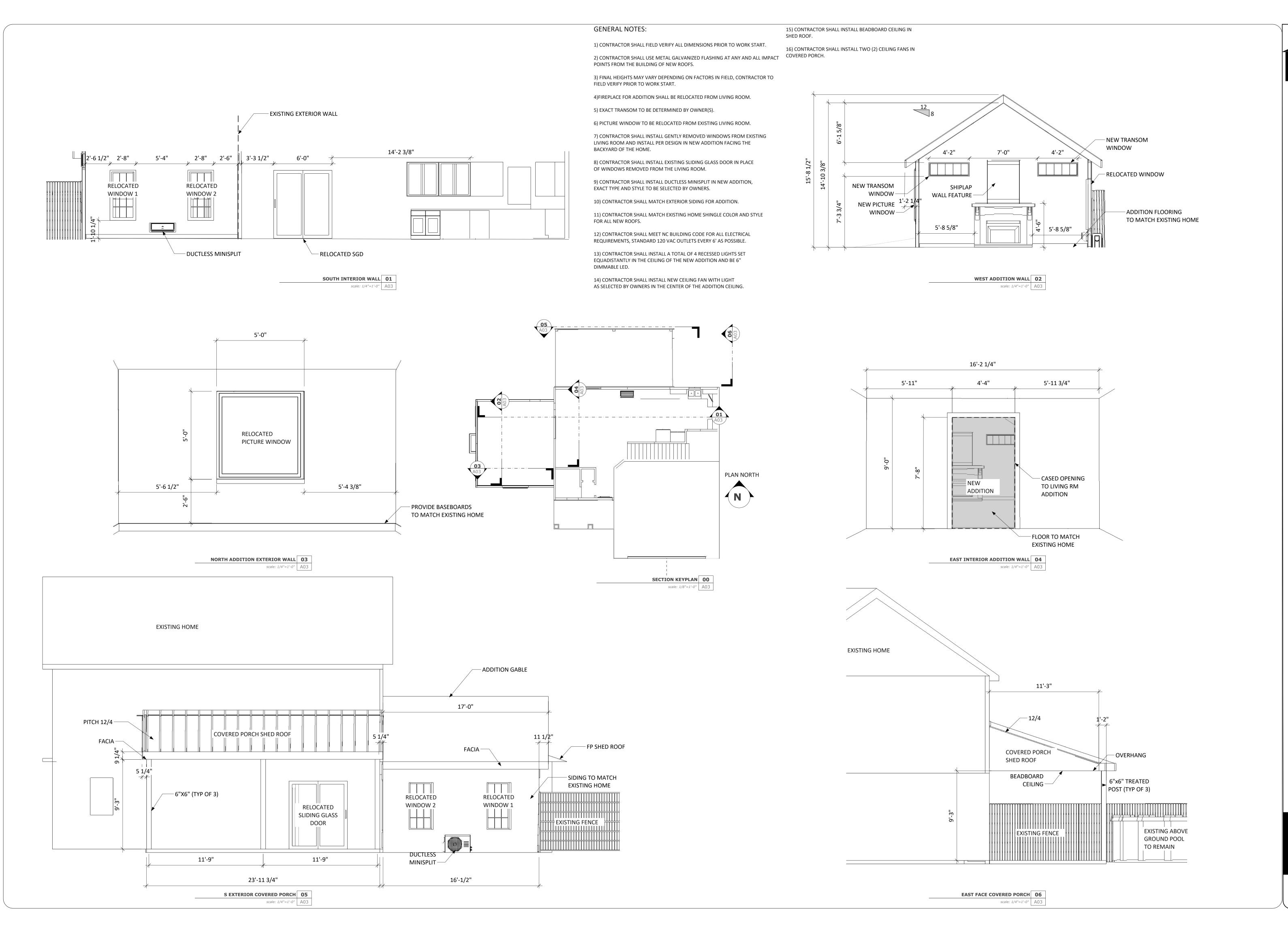
Visual Design Group
Visual Design Group
Fuquay-Varina, NC
919 762 5415
www.endvisionyda.com

CLIENT
MR|MRS TAYLOR
288 TILDEN HOWINGTON
DR., LILLINGTON, NC

ISSUE 06/29/2021 RE-ISSUE <ReIssueDate

PROJECT NO.
21.16.01.0519
PROJECT
Taylor residence

DRAWN BY
TM, MM
DESCRIPTION
NEW WORK FLOOR PLAN

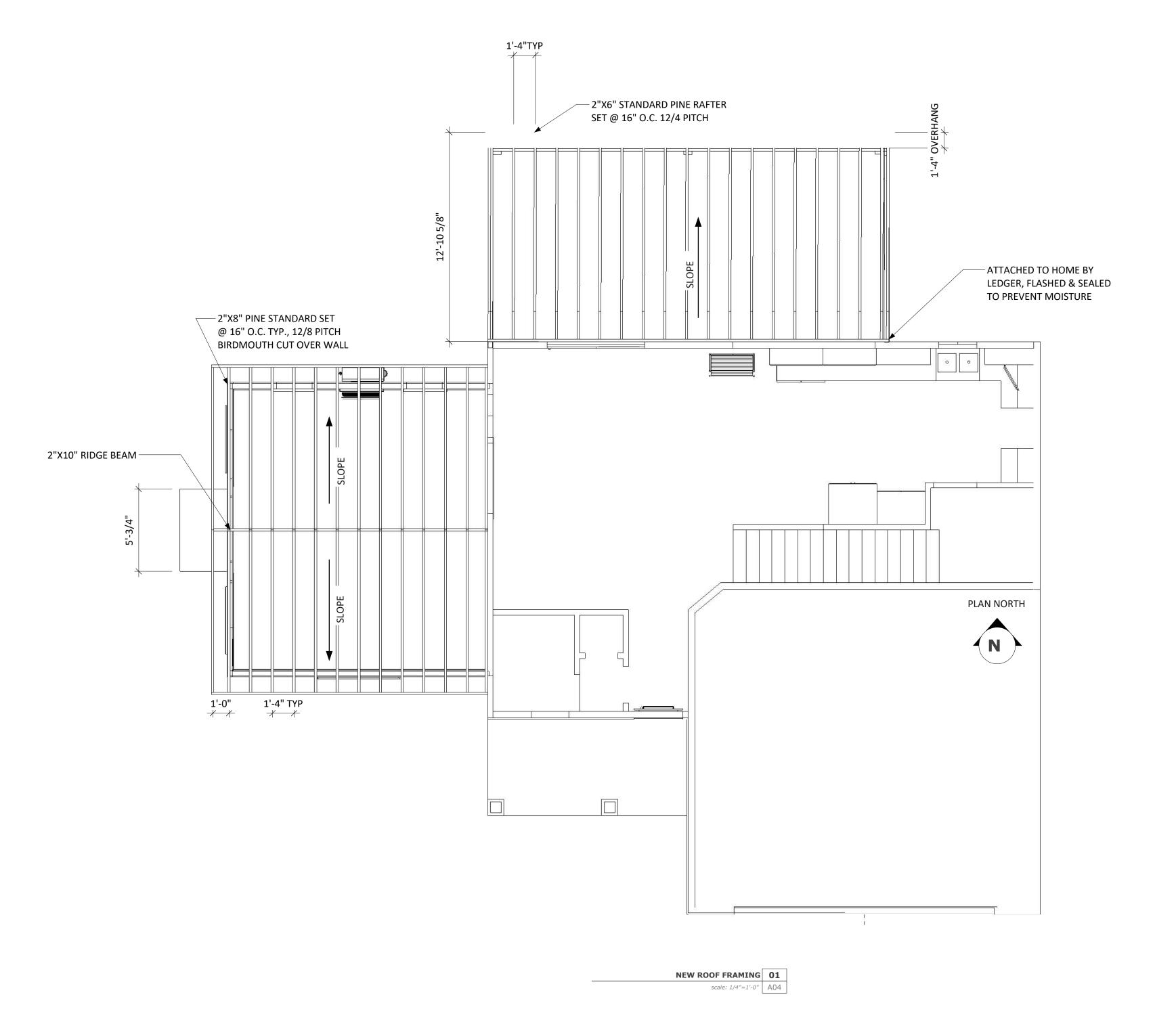


CL1MR|
288
DR.

RE-ISSUE <ReIssueDa

PROJECT NO. 21.16.01.0519 **PROJECT** Taylor resi

DESCRIPTIONNEW WORK ELEVATIONS DRAWN BY TM, MM



GENERAL NOTES:

1) CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS PRIOR TO WORK START.

2) CONTRACTOR SHALL USE METAL GALVANIZED FLASHING AT ANY AND ALL IMPACT POINTS FROM THE BUILDING OF NEW ROOFS.

3) FINAL DOCUMENTATION TO BE REVIEWED AND SEALED BY STRUCTURAL ENGINEER PRIOR TO FILING PERMITS.

END VISION

End Vision Lal Design Group Lay-Varina, NC 762 5415

CLIENTMR|MRS TAYLOR
288 TILDEN HOWINGTON
DR., LILLINGTON, NC

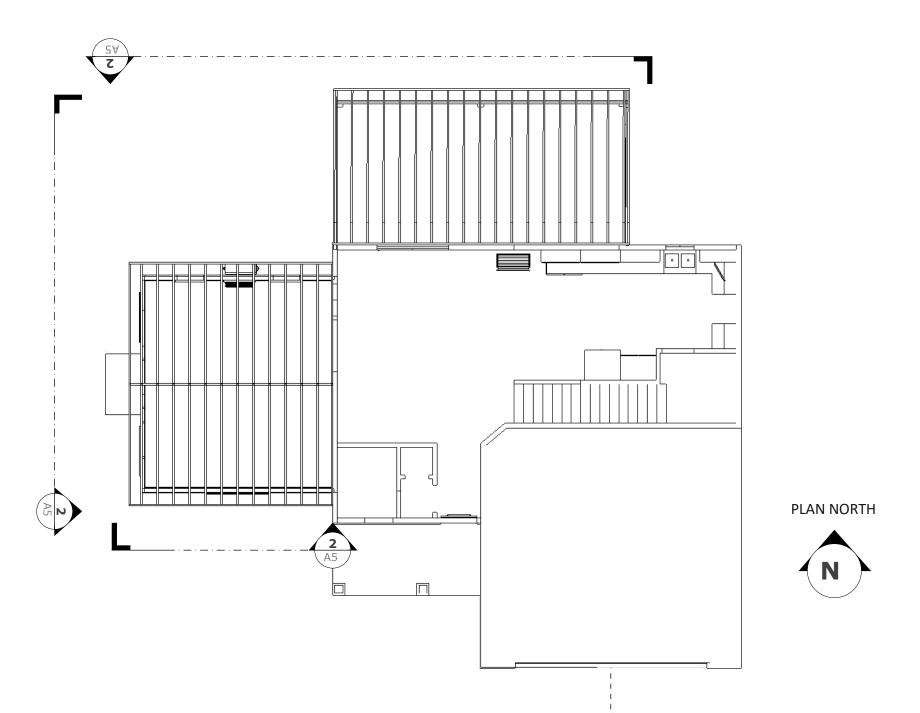
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RE-ISSUE
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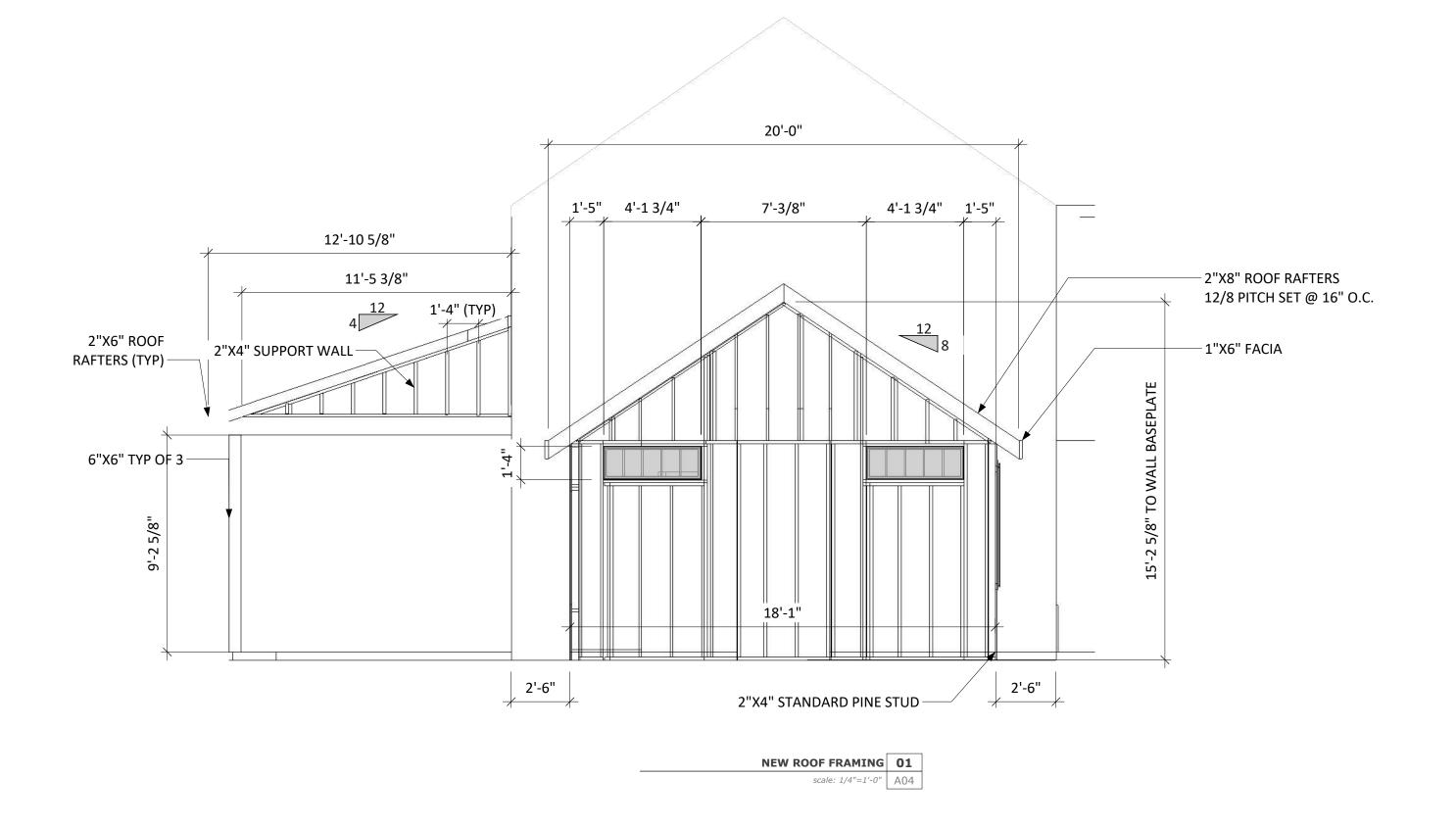
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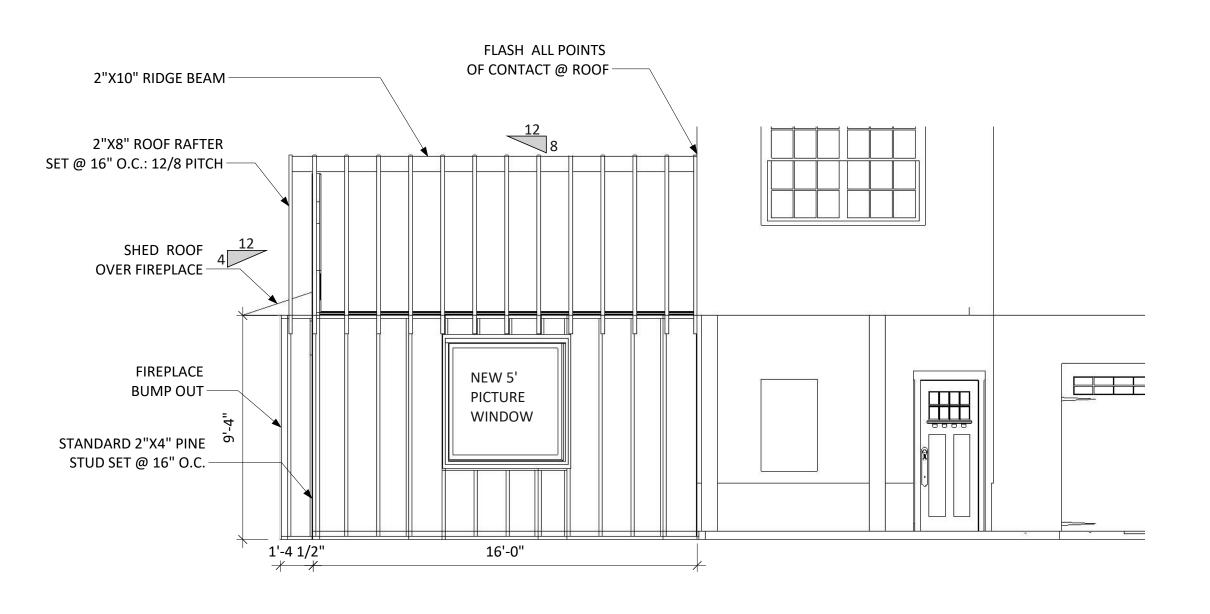
PROJECT
Taylor residences in the state of th

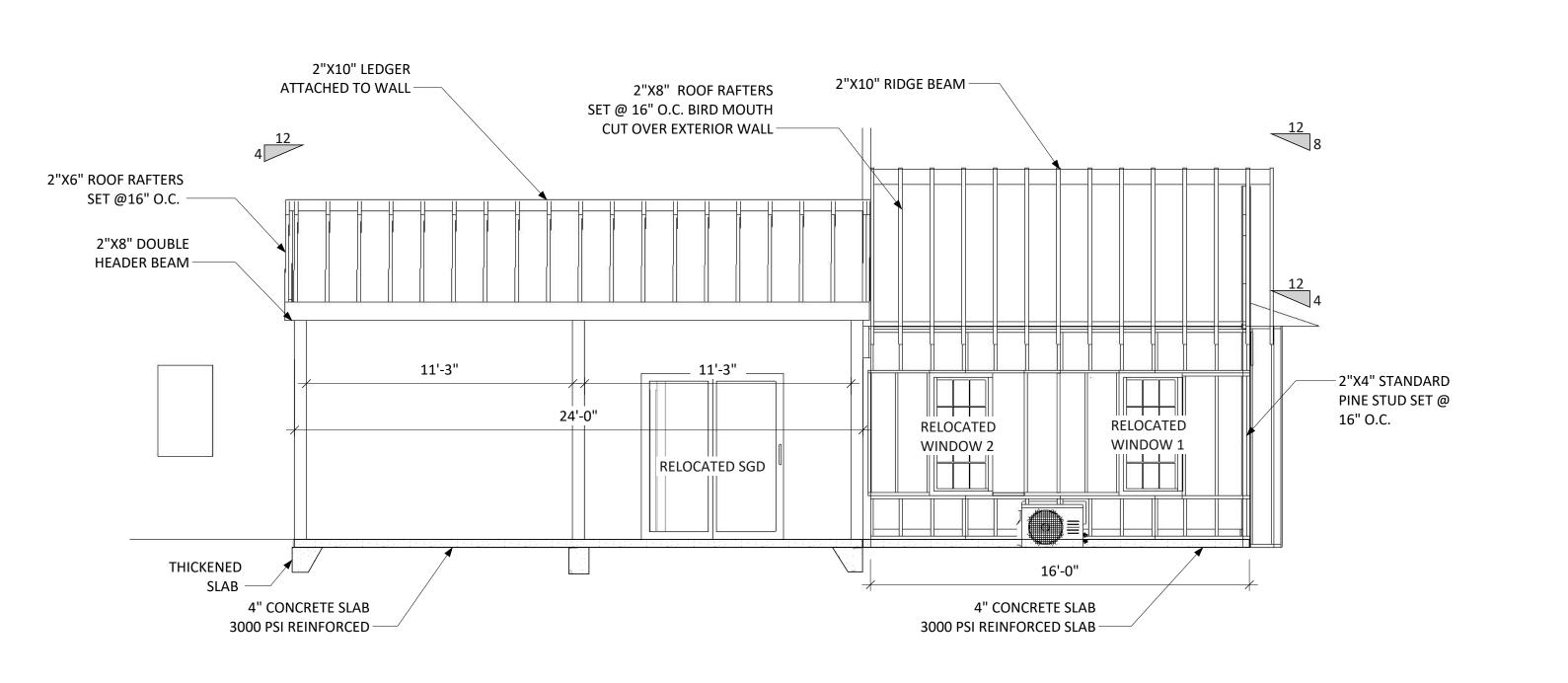
DRAWN BY
TM, MM
DESCRIPTION
ROOF FRAMING

1) FINAL DOCUMENTATION SHALL BE REVIEWED & SEALED BY STRUCTURAL ENGINEER











The End Vision
Visual Design Group
Fuquay-Varina, NC
919 762 5415

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MR|MRS TAYLOR
288 TILDEN HOWINGTOI
DR., LILLINGTON, NC

06/29/2021 **RE-ISSUE**<ReIssueDate>

PROJECT NO. 21.16.01.0519 **PROJECT**

PROJECT Taylor resi

DRAWN BY
TM, MM
DESCRIPTION

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CLI MR 288 DR.

RE-ISSUE <ReIssueDate> **ISSUE** 06/29/2021

PROJECT Taylor residence **PROJECT NO.** 21.16.01.0519

DRAWN BY TM, MM

DESCRIPTION RENDERS

a

STRUCTURAL NOTES

ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF "2018 INTERNATIONAL RESIDENTIAL BUILDING CODE", IN ADDITION TO ALL LOCAL CODES AND REGULATIONS.

1) DESIGN LOADS:

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION		
			LL	TL	
ALL FLOORS	40	10	L/360	L/240	
ATTIC (w/ walk up stairs)	30	10	L/360	L/240	
ATTIC (pull down access)	20	10	L/240	L/180	
ATTIC (no access)	10	5	L/240	L/180	
EXTERNAL BALCONY	40	10	L/360	L/240	
ROOF	20	10	L/240	L/180	
ROOF TRUSS	20	20	L/240	L/180	
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)				
GROUND SNOW LOAD	85 PSF				
SEISMIC	SEISMIC ZONES A, B & C				

- 2) MINIMUM ALLOWABLE SOIL BEARING PRESSURE = 2000 PSF
- 3) CONCRETE SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI AND A MAXIMUM SLUMP OF FIVE INCHES UNLESS NOTED OTHERWISE. (U.N.O.)
- 4) MAXIMUM DEPTH OF UNBALANCED FILL AGAINST FOUNDATION WALLS TO BE LESS THAN 4'-0" WITHOUT USING SUFFICIENT WALL BRACING. REFER TO SECTION R404 OF 2018 INTERNATIONAL RESIDENT BUILDING CODE FOR BACKFILL LIMITATIONS BASED ON WALL HEIGHT, WALL THICKNESS, SOIL TYPE, AND UNBALANCED BACKFILL HEIGHT.
- 5) ALL FRAMING LUMBER SHALL BE SYP #2 (Fb = 800 PSI, BASED ON 2x10) UNO. ALL FRAMING LUMBER EXPOSED TO THE ELEMENTS SHALL BE TREATED MATERIAL. ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2600 PSI, E = 1.9M PSI (U.N.O.) ALL LSL LUMBER TO BE 3.5" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2325 PSI, E = 1.6M PSI (U.N.O.) ALL PSL LUMBER TO BE 3.5" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2400 PSI, E = 1.8M PSI (U.N.O.)
- 6) ALL LOAD BEARING EXTERIOR HEADERS SHALL BE AT (2) 2x10. (U.N.O.) REFER TO TABLE R602.7(1) & (2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS UNLESS SPECIFICALLY NOTED ON PLANS.
- 7) ALL STRUCTURAL STEEL W-SHAPES (I-BEAMS) SHALL BE ASTM A992 GRADE 50. ALL STEEL ANGLES, PLATES, AND C-CHANNELS SHALL BE ASTM A36. ALL STEEL PIPE SHALL BE ASTM A53 GRADE B.
- 8) STEEL BEAMS SHALL BE SUPPORTED AT EACH END WITH A MINIMUM BEARING LENGTH OF 3-1/2" AND FULL FLANGE WIDTH. PROVIDE SOLID BEARING FROM BEAM SUPPORT TO FOUNDATION. BEAMS SHALL BE ATTACHED TO EACH SUPPORT WITH TWO (2) LAG SCREWS (1/2" x 4" LONG). LATERAL SUPPORT IS CONSIDERED ADEQUATE PROVIDED THE JOISTS ARE TOE NAILED TO THE SOLE PLATES, AND THE SOLE PLATES ARE NAILED OR BOLTED TO THE BEAM FLANGES @ 48" O.C.
- 9) PROVIDE ANCHOR BOLT PLACEMENT PER SECTION 403.1.6: 1/2" ANCHOR BOLTS SPACED AT 6'-0" O.C. AND PLACED 12" FROM THE END OF EACH PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY. THE BOLTS SHALL BE LOCATED IN THE MIDDLE THIRD OF THE WIDTH OF THE PLATE. THERE SHALL BE A MINIMUM TWO ANCHOR BOLTS PER PLATE SECTION.
- 10) FOUNDATION DRAINAGE-DAMP PROOFING OR WATERPROOFING PER SECTION 405 AND 406 OF 2018 IRC..
- 11) WALL AND ROOF CLADDING VALUES:

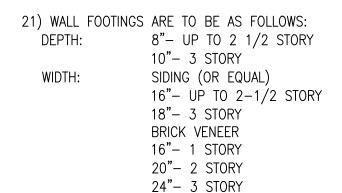
WALL CLADDING SHALL BE DESIGNED FOR 28.0 POUNDS PER SQUARE FOOT (LBS/SQFT) OR GREATER POSITIVE AND NEGATIVE PRESSURE ROOF VALUES BOTH POSITIVE AND NEGATIVE SHALL BE AS FOLLOWS:

39.0 LBS/SQFT FOR ROOF PITCHES 0/12 TO 1.5/12

36.0 LBS/SQFT FOR ROOF PITCHES 1.5/12 TO 6/12

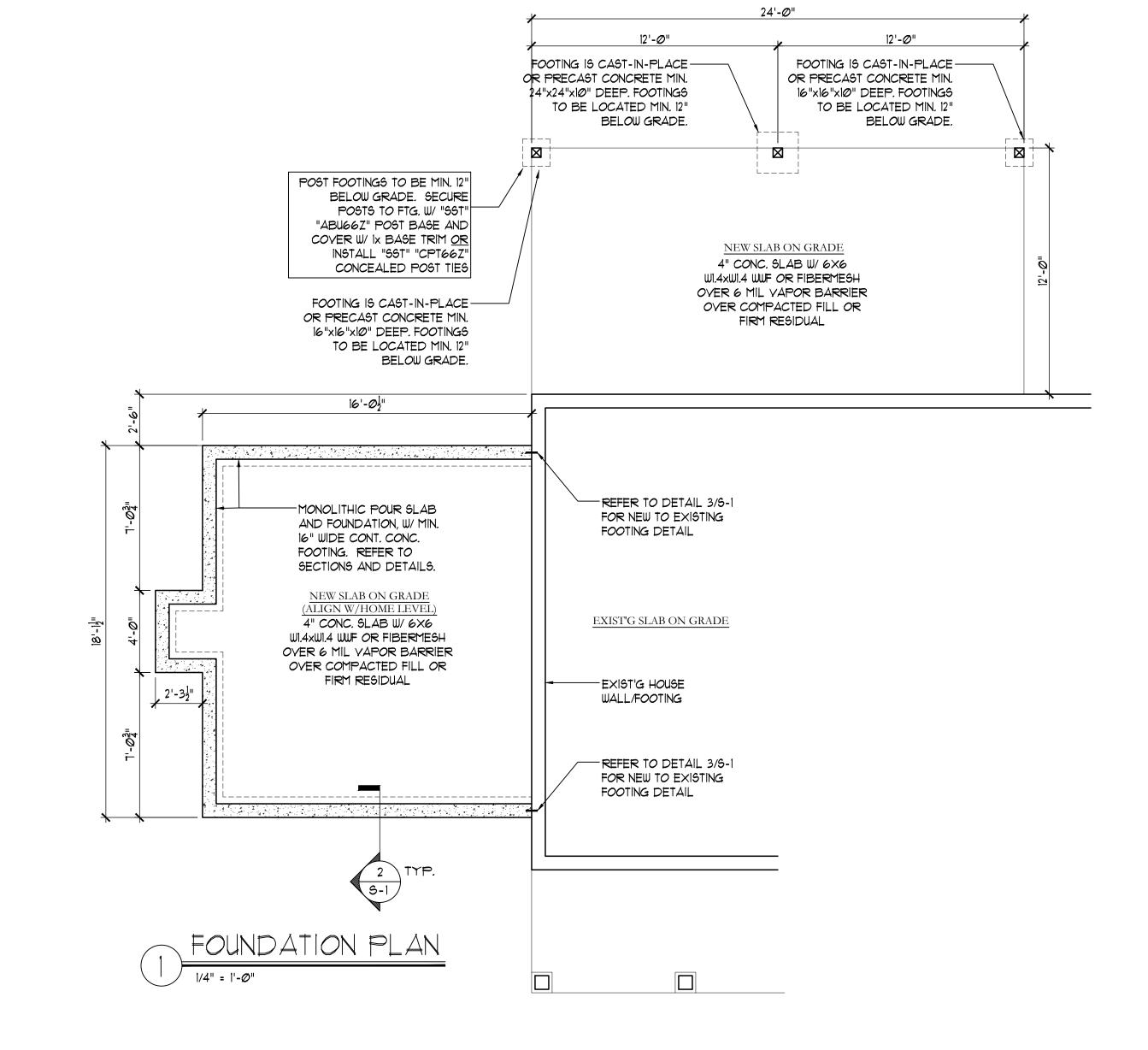
18.0 LBS/SQFT FOR ROOF PITCHES 6/12 TO 12/12 **MEAN ROOF HEIGHT 30'-0" OR LESS

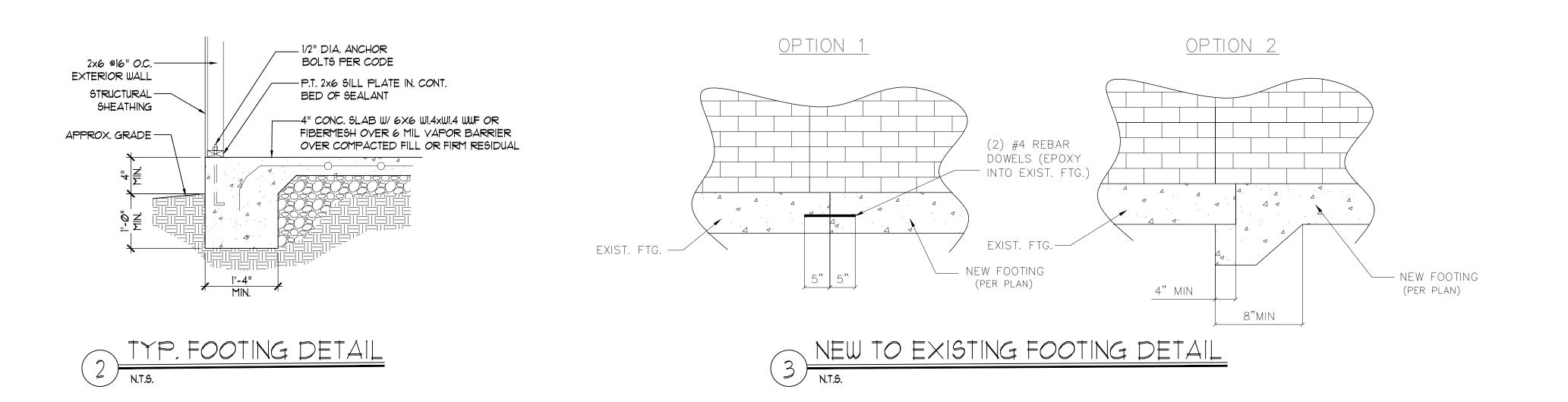
- 12) FOR ROOF SLOPES FROM 2/12 THROUGH 4/12, BUILDER TO INSTALL 2 LAYERS OF 15# FELT PAPER.
- 13) REFER TO SECTION R602.3 FOR FRAMING OF ALL WALLS OVER 10'-0" IN HEIGHT.
- 14) PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.3 OF THE 2018 IRC.
- 15) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY ANCHORED TO THE FOUNDATION.
- 16) REFER TO TABLE N1102.1 FOR PRESCRIPTIVE BUILDING ENVELOPE THERMAL COMPONENT CRITERIA.
- 17) PSL COLUMNS DESIGNED WITH MAXIMUM HEIGHT OF 9'-0" (U.N.O.)
- 18) PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP AND BOTTOM OF PORCH COLUMNS. (U.N.O.)
- 19) MAXIMUM MASONRY PEIR HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.
- 20) IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION. TYNDALL ENGINEERING & DESIGN, PA IS NOT RESPONSIBLE FOR DIMENSION OR SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.

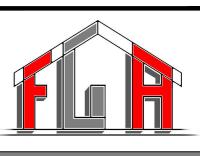


FOR FOUNDATION WALL HEIGHT AND BACKFILL REQUIREMENTS, REFER TO NORTH CAROLINA RESIDENTIAL CODE TABLE R404.1.1 (1 THRU 4) NOTE: ASSUMED SOIL BEARING CAPACITY= 2000 PSF. CONTRACTOR MUST VERIFY SITE CONDITIONS AND CONTACT SOILS ENGINEER IF MARGINAL OR UNSTABLE SOILS ARE ENCOUNTERED.

ATTACH SILL PLATE WITH 1/2" DIA. ANCHOR BOLTS AT 6'-0" CENTERS (7" EMBEDMENT) AND 12" FROM EACH PLATE END.







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ASSOCIATES, LL NK GALLUCCI, ARCHITECT

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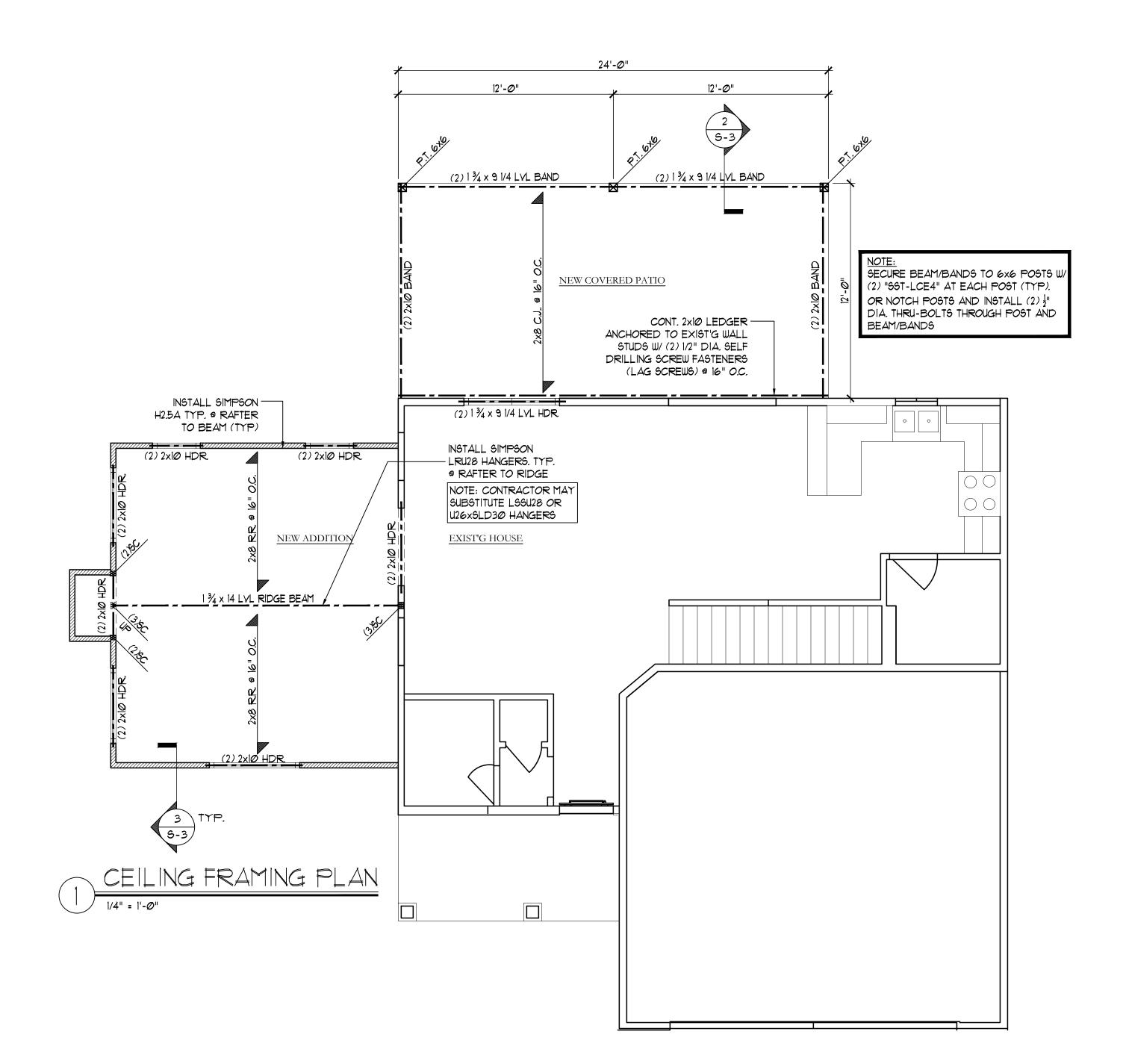
JOB NUMBER: 21-74 EXIST'G AND PROPOSED FLOOR

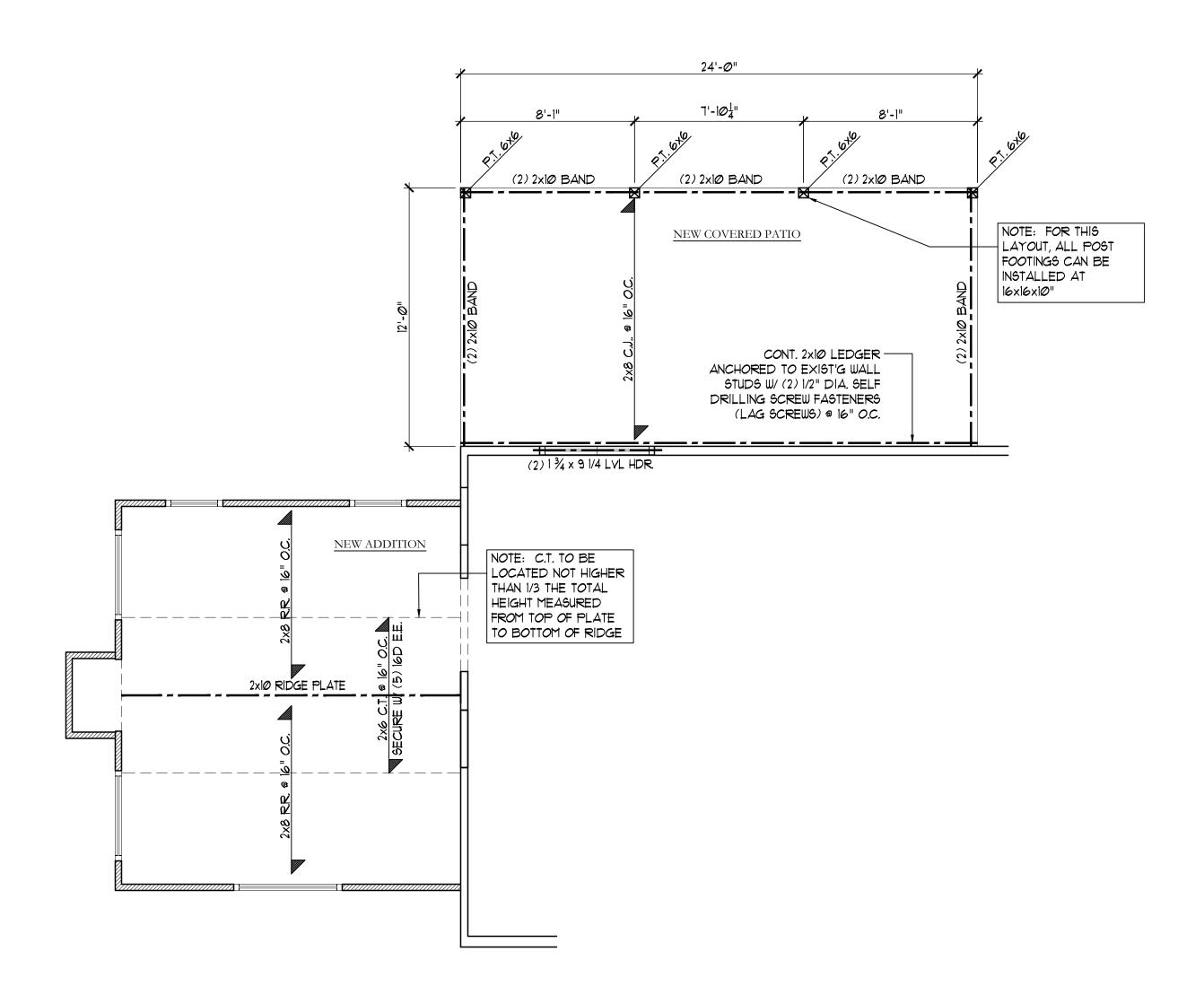
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PLANS

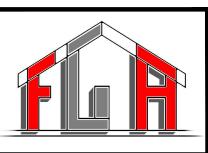
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CEILING FRAMING PLAN (ALTERNATE OPTION)

1/4" = 1'-0"



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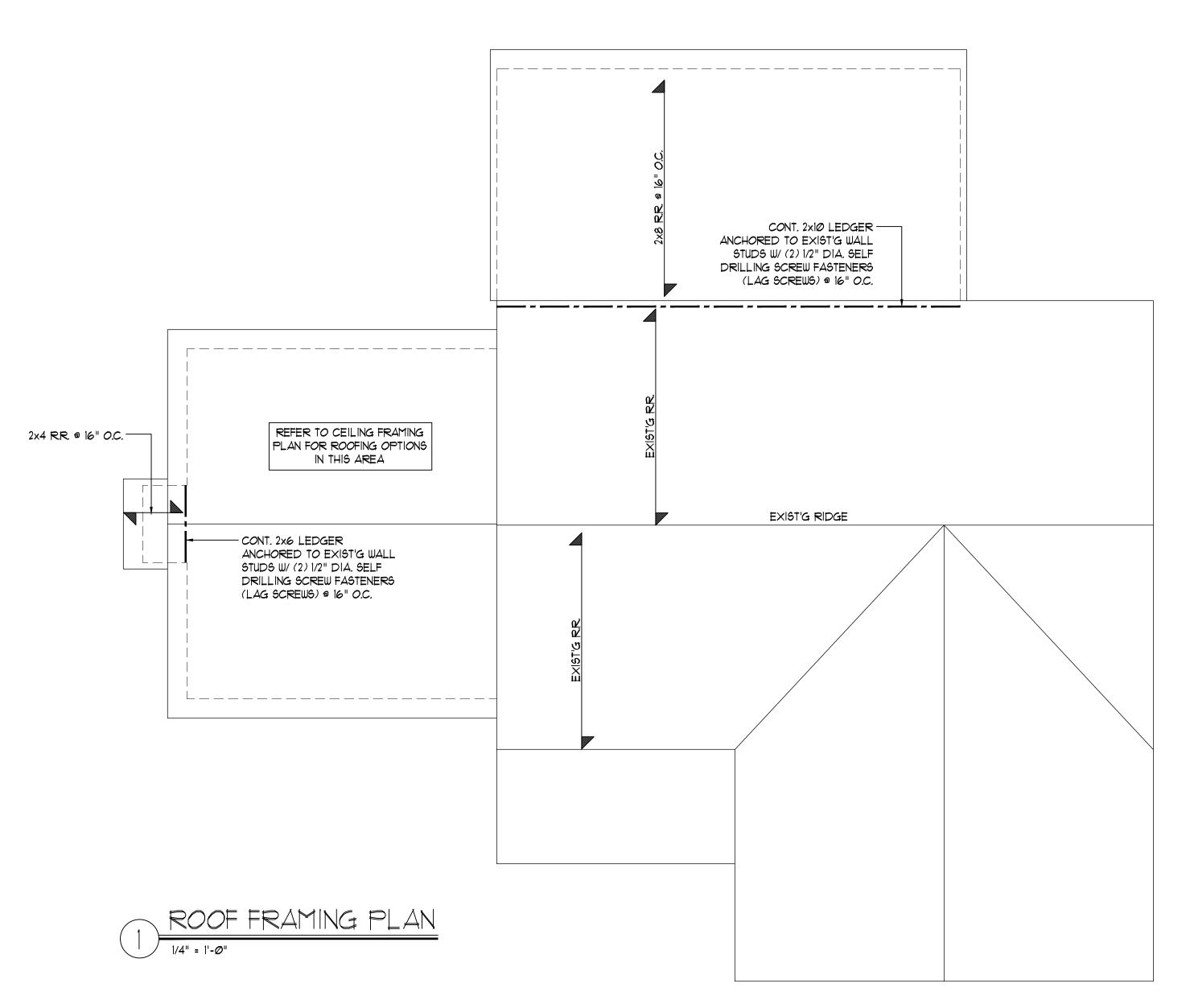
TAYLOR RESIDENCE 288 TILDEN HOWINGTON LILLINGTON, NC

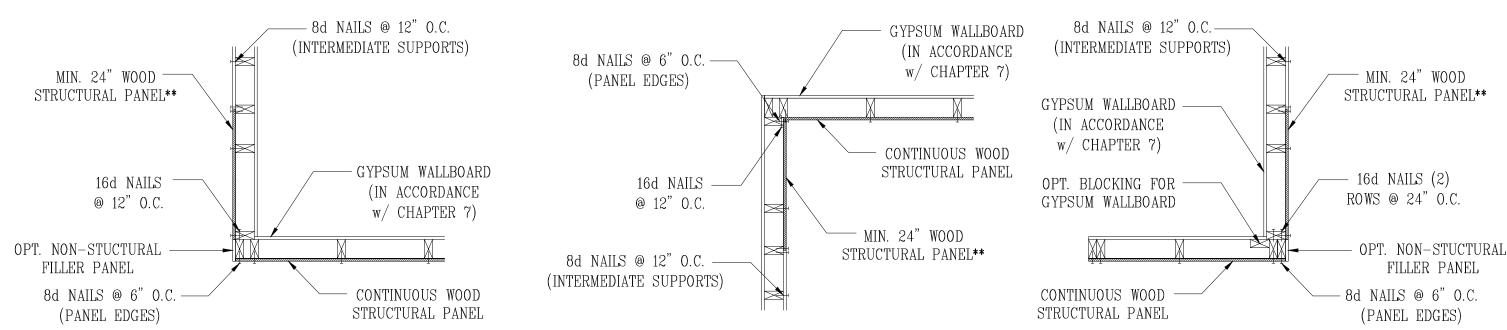
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10-7-2021 DRAWN BY: JOB NUMBER: 21-74

CEILING FRAMING PLAN





a) OUTSIDE CORNER DETAIL

b) INSIDE CORNER DETAIL

c) GARAGE DOOR CORNER

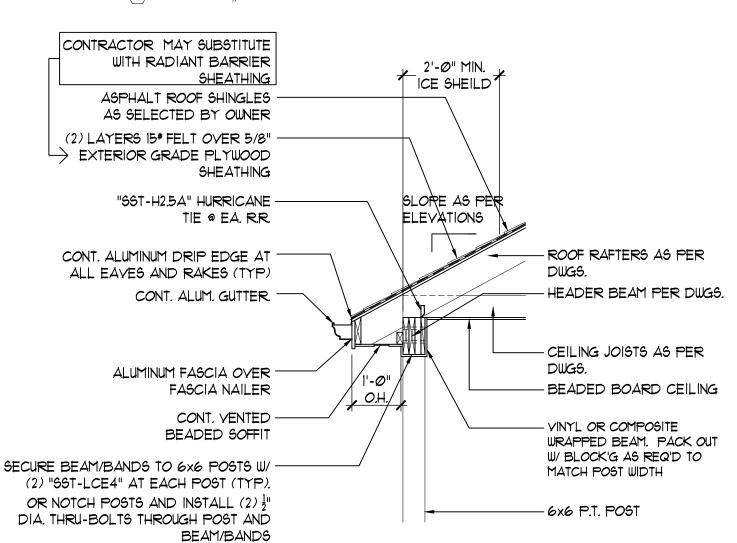
** IN LIEU OF THE 24" (MIN.) CORNER RETURN, A HOLD-DOWN DEVICE WITH A MINIMUM UPLIFT DESIGN VALUE OF 800# SHALL BE FASTENED TO THE CORNER STUD AND TO THE FOUNDATION OR FRAMING BELOW.

TYPICAL EXTERIOR CORNER FRAMING FOR CONTINUOUS SHEATHING NO SCALE

STRUCTURAL SHEATHING NOTES

- 1) DESIGNED FOR SEISMIC ZONE A-C AND WIND SPEEDS OF 120 MPH OR LESS.
- 2) WALLS SHALL BE BRACED IN ACCORDANCE WITH SECTION R602.10 OF THE 2018 NCRC. 3) BRACING REQUIREMENTS SHALL BE PER TABLE R602.10.3. REFER TO SECTION R602.10.4 FOR LOAD PATH DETAILS INCLUDING CONNECTIONS & SUPPORT OF BRACED WALL
- 1) REFERENCE FIGURE R602.10.4.3 OF THE 2018 NCRC.
- 4) INTERIOR BRACED WALL PANELS (BWP) INDICATED SHALL BE SHEATHED IN ACCORDANCE WITH THE GB METHOD OR WSP METHOD AS PRESCRIBED IN SECTION R602.10.1 (UNO)
- $\langle 2 \rangle$ 1/2" GYPSUM BOARD (GB) MINIMUM LENGTH OF 8'-0" (ISOLATED PANELS) OR 4'-0" (CONTINUOUS SHEATHING). SECURE w/ 5d COOLER NAILS (OR EQUAL PER TABLE R702.3.5) SPACED @ 7" O.C. AT PANEL EDGES, INCLUDING TOP AND BOTTOM PLATES & 7" O.C. AT INTERMEDIATE SUPPORTS
- $\langle 3 \rangle$ 3/8" wood structural panel (wsp) SECURE w/ 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS
- 5) EXTERIOR BRACED WALL PANELS (BWP) SHALL BE CONSTRUCTED IN ACCORDANCE WITH CS-WSP METHOD AS PRESCRIBED IN SECTION R602.10.3 (UNO)
- 6) ALL SHEATHABLE SURFACES OF EXTERIOR WALLS (INCLUDING AREAS ABOVE AND BELOW OPENINGS AND GABLE END WALLS) SHALL BE CONTINUOUSLY SHEATHED WITH WOOD STRUCTURAL PANEL (WSP) SHEATHING WITH A MINIMUM THICKNESS OF 3/8". SHEATHING SHALL BE SECURED WITH MINIMUM 64 COMMON NAILS SPACED AT O.C. AT PANEL EDGES AND SPACED AT 12" O.C. AT INTERMEDIATE SUPPORTS.
- 7) MINIMUM BRACED WALL PANEL LENGTHS WITH CS-WSP METHOD SHALL BE AS FOLLOWS: - 24" ADJACENT TO OPENINGS NOT MORE THAN 67% OF WALL HEIGHT - 30" ADJACENT TO OPENINGS GREATER THAN 67% AND LESS THAN 85% OF WALL HEIGHT. - 48" FOR OPENINGS GREATER THAN 85% OF WALL HEIGHT
- $\langle 4 \rangle$ sheath interior & exterior
- 8) FOR CS-WSP METHOD, A MINIMUM 24" BRACED WALL PANEL CORNER RETURN SHALL BE PROVIDED AT BOTH ENDS OF A BRACED WALL LINE IN ACCORDANCE WITH FIGURE R602.10.3(4). IN LIEU OF A CORNER RETURN, EITHER A MIN. 48" BRACED WALL PANEL SHALL BE PROVIDED AT THE CORNER OR A HOLD-DOWN DEVICE WITH A MINIMUM UPLIFT DESIGN VALUE OF 800# SHALL BE FASTENED TO THE EDGE OF THE BRACED WALL PANEL CLOSEST TO THE CORNER AND TO THE FOUNDATION OR FRAMING BELOW.

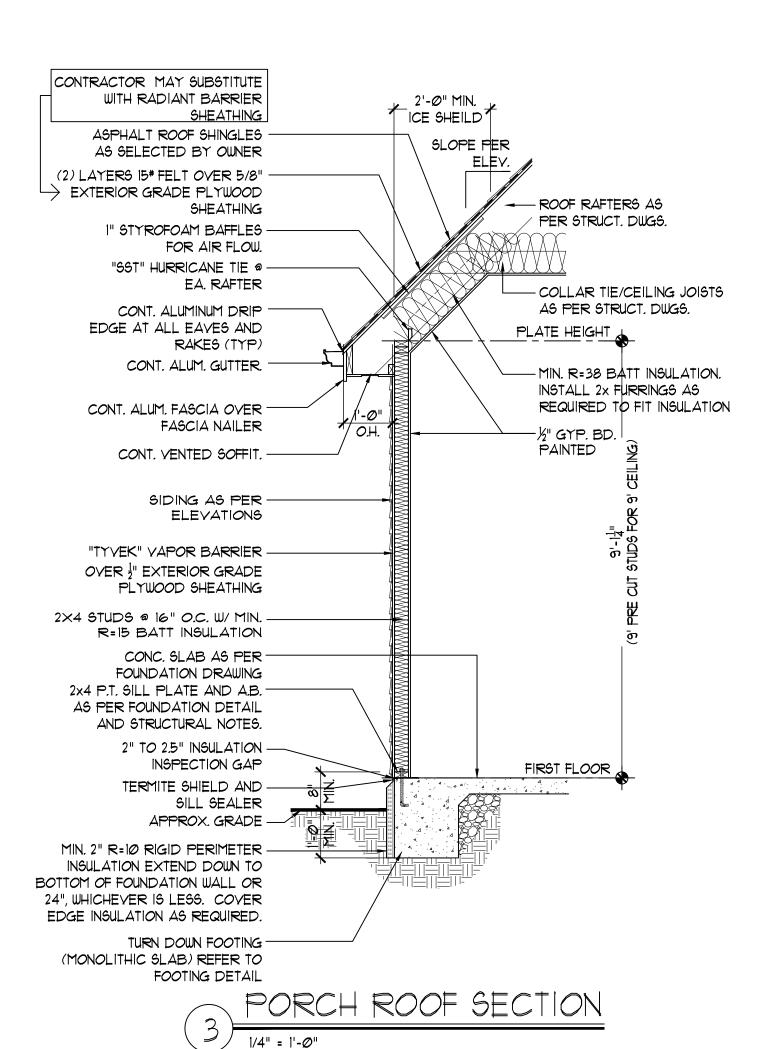
(5) MINIMUM 800# HOLD-DOWN DEVICE





		REQUIRED BRACEI) WALL PANEL CONNECTIONS		
			REQUIRED CONNECTION		
METHOD	MATERIAL	MIN. THICKNESS	@ PANEL EDGES	@ INTERMEDIATE SUPPORTS	
CS-WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS @ 6"O.C.	6d COMMON NAILS @ 12" O.C.	
GB	GYPSUM BOARD	1/2"	5d COOLER NAIL** @ 7"O.C.	5d COOLER NAIL** @ 7" O.C.	
WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS @ 6" O.C.	6d COMMON NAILS @ 12" O.C.	

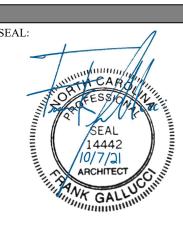
**OR EQUIVALENT PER TABLE R702.3.5 B3: BRACE WALL PANEL CONNECTIONS NO SCALE





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10-7-2021 DRAWN BY: FG JOB NUMBER: 21-74

ROOF FRAMING PLAN AND DETAILS

SHEET: