Mike Fino 220 Bishop Mill Ln. Lillington, NC 27546 24'x31x'9'

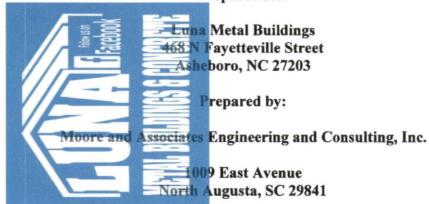


## STRUCTURAL DESIGN RISK CATEGORY I/II ENCLOSED BUILDING

## 30'- 0" MAXIMUM WIDE X 20'- 0" HEIGHT-BOX EAVE FRAME

10 November 2020 Revision 0 M&A Project No. 20214S

## Prepared for:



401 S. Main Street, Suite 200 Mount Airy, NC 27030



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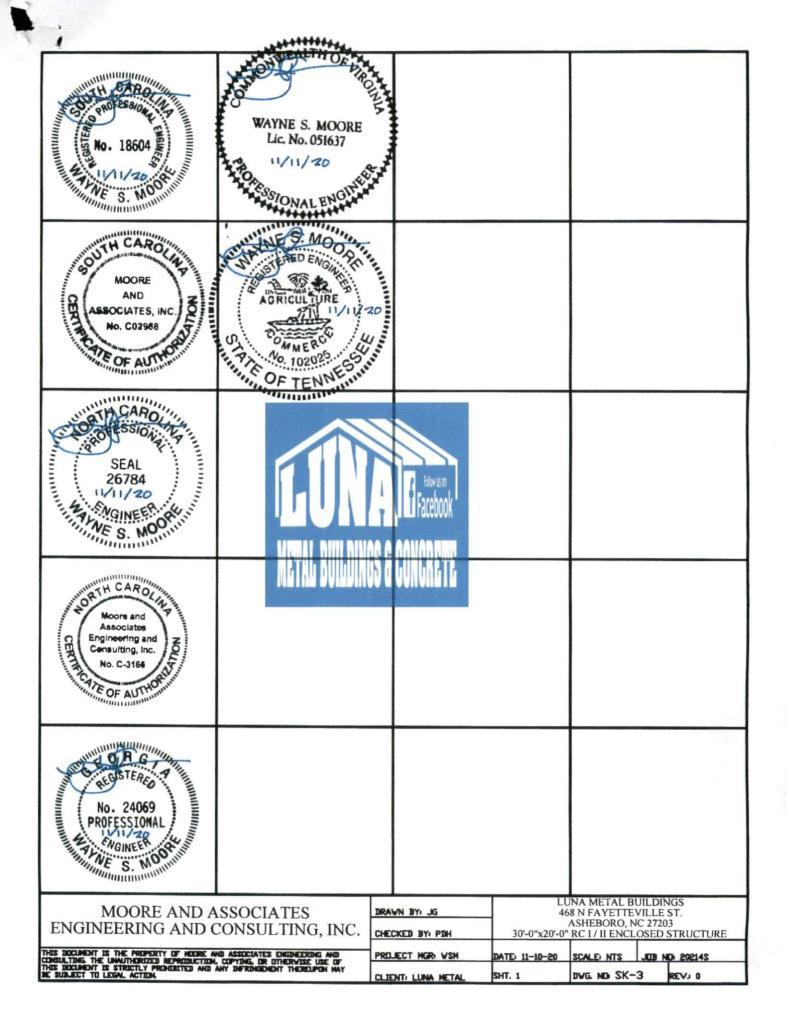
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# DRAWING INDEX

ı			DICH WING INDEX
ı	SHEET	1	PE SEAL COVER SHEET
ı	SHEET	2	DRAWING INDEX
ı	SHEET	3	INSTALLATION NOTES AND SPECIFICATIONS
l	SHEET	4	TYPICAL SIDE AND END ELEVATIONS
ı	SHEET	5	TYPICAL RAFTER/COLUMN END FRAMING AND SIDE FRAMING SECTIONS (EXPOSURE B)
	SHEET	5A	TYPICAL RAFTER/COLUMN END AND SIDE FRAMING SECTIONS (EXPOSURE C)
	SHEET	5B	TYPICAL RAFTER/COLUMN END AND SIDE FRAMING SECTIONS (EXPOSURE C)
l	SHEET	6	COLUMN CONNECTION DETAILS (EXPOSURE B)
l	SHEET	6A	COLUMN CONNECTION DETAILS (EXPOSURE B)
l	SHEET	6B	COLUMN CONNECTION DETAILS (EXPOSURE C)
	SHEET	6C	COLUMN CONNECTION DETAILS (EXPOSURE C)
ı	SHEET	7	BASE RAIL ANCHORAGE OPTIONS (EXPOSURE B)
	SHEET		BASE RAIL ANCHORAGE OPTIONS (EXPOSURE C)
	SHEET		BOX EAVE RAFTER END WALL OPENINGS
	SHEET		CONNECTION DETAILS
ı	SHEET		CONNECTION DETAILS
	SHEET		CONNECTION DETAILS
	SHEET		BOX EAVE RAFTER LEAN-TO OPTIONS
	SHEET		BOX EAVE RAFTER LEAN-IN DRIVING
l	SHEET	13	BOX EAVE RAFTER VERTICAL ROOTS DING OPTION
	SHEET	72 (0.0)	BOX EAVE RAFTER VERTICAL ROOF STOING OPTION
ı	SHEET	F 198	HEADER OPTIONS (EXPOSURE B)
l	SHEET	14A	HEADER OPTIONS (EXPOSURE C) (1000)
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MOORE AND ASSOCIATES	DRAWN BY: JG	LUNA METAL BUILDINGS 468 N FAYETTEVILLE ST.			

#### INSTALLATION NOTES AND SPECIFICATIONS

- 1 DESIGN IS FOR MAXIMUM 30'-0" WIDE x 20'-0" EAVE HEIGHT ENCLOSED STRUCTURES
- 2. DESIGN WAS DONE IN ACCORDANCE WITH THE 2018 NORTH CAROLINA BUILDING CODE, 2012 INTERNATIONAL BUILDING CODE (IBC), 2015 IBC, AND 2018 IBC
- 3 DESIGN LOADS ARE AS FOLLOWS:

= 115 PSF FOR RISK CATEGORY II A) DEAD LOAD

= 15 PSF FOR RISK CATEGORY I = 20 PSF FOR RISK CATEGORY II B) LIVE LOAD

= 12 PSF FOR RISK CATEGORY I

C) GROUND SNOW LOAD = 30 PSF WITH U-CHANNEL PEAK BRACE (W < 24-'0)

= 35 PSF

(UNBALANCED SNOW LOADS DUE TO DRIFTING HAVE NOT BEEN EVALUATED)

- 4. 3-SECOND ULTIMATE WIND SPEED (V<sub>ULT</sub>) = ≤ 155 MPH (NOMINAL WIND SPEED = 120 MPH) FOR RISK CATEGORY II.
- 5. 3-SECOND ULTIMATE WIND SPEED ( $V_{ULT}$ ) =  $\le$  145 MPH (NOMINAL WIND SPEED = 112 MPH) FOR RISK CATEGORY I.
- 6 MAXIMUM RAFTER/POST AND END POST SPACING = 4.0 FEET FOR RISK CATEGORY II (UNLESS NOTED OTHERWISE)
- 7 MAXIMUM RAFTER/POST AND END POST SPACING = 5.0 FEET FOR RISK CATEGORY I (UNLESS NOTED OTHERWISE).
- 8. END WALL COLUMNS/POSTS ARE EQUIVALENT TO SIDE WALL COLUMNS/POSTS IN SIZE AND SPACING UNLESS NOTED OTHERWISE
- 9 RISK CATEGORY I/II
- 10. WIND EXPOSURE CATEGORY B (RISK CATEGORY I)/C (RISK CATEGORY II)
- 11 SPECIFICATIONS APPLICABLE TO 29 GAUGE METAL PANELS FASTENED DIRECTLY TO 2 1/4\*x2 1/4\*-14 GAUGE TUBE STEEL (TS) FRAMING MEMBERS (UNLESS NOTED OTHERWISE). WHERE TS 2 1/4\* x 2 1/4\* 14 GAUGE IS SPECIFIED, TS 2 1/4\* x 2 1/4\* 12 GAUGE MAY BE USED AS AN OPTION
- 12 AVERAGE FASTENER SPACING ON-CENTERS ALONG RAFTERS OR HAT CHANNELS, AND COLUMNS (INTERIOR OR END) = 10" DC (MAX.) FOR RISK CATEGORY I AND 8" D.C. (MAX.) FOR RISK CATEGORY II. FOR WIND SPEEDS > 145 MPH = 6" D.C. (MAX.).
- 13. FASTENERS CONSIST OF #12-14×3/4" SELF-DRILLING FASTENER (SDF), USE CONTROL SEAL WASHER WITH EXTERIOR FASTENERS.

  SPECIFICATIONS APPLICABLE ONLY FOR MEAN ROOF HEIGHT OF 20 FFET OR LESS, AND ROOF SLOPES OF 14" (3:12 PITCH) OR LESS SPACING REQUIREMENTS FOR OTHER ROOF HEIGHTS AND/OR SLOPES MAY VARY ROOF SLOPES LESS THAN 3:12 REQUIRE USE OF LAP JOINT SEALANT.
- 14 GROUND ANCHORS SHALL BE INSTALLED THE

15 STANDARD SOIL NAIL FOUNDATION SYSTEM STIFF (SUITABLE) SOILS. SOIL NAILS MAY AND MUST BE USED FOR WIND SPEEDS > 14 REGARDING MINIMUM LENGTH FOR FROST DE

16 WIND FORCES GOVERN OVER SEISMIC FORCE

SOIL SITE CLASS = D RISK CATEGORY I/II R= 325 IE= 10 S 6502 = 503 V= CsW

S<sub>D1</sub>= 1.258 g

17. FOR RISK CATEGORY II STRUCTURES, MAXIM

ACH RAFTER COLUMN ALONG SIDES

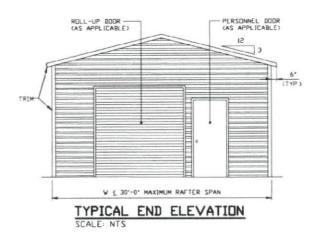
30' LONG AND ARE APPLICABLE ONLY IN MEDIUM TO DNAL ANCHORAGE MAY BE USED IN SUITABLE SOILS TED COORDINATE WITH LOCAL CODES/ORDINANCES

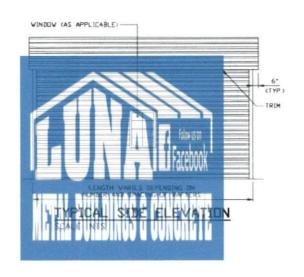
INNEL DOORS UTILIZED AS MEANS OF EGRESS

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ENGINEERING AND CONSULTING, INC	

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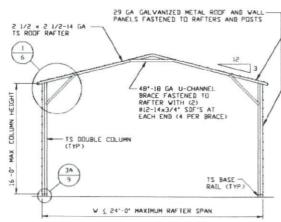
	CHIPPITA LIBIA METAL	SHT. 3	DVG ND SK-		BEAT U	
_	PROJECT MGR: VSM	DATE: 11-10-20	SCALE: NTS	JEEB	ND 20214S	
	CHECKED BY: PDH	30'-0"x20'-0'	ASHEBORO, 1 " RC I / II ENCI		03 STRUCTURE	
	DRAWN BY: JG		68 N FAYETTE	VILLE	EST.	

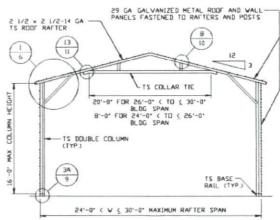




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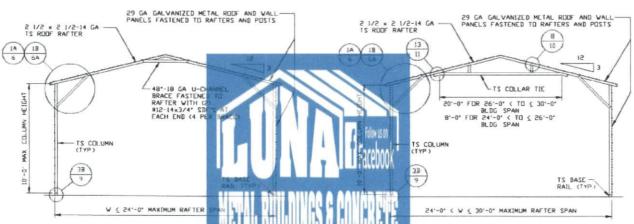




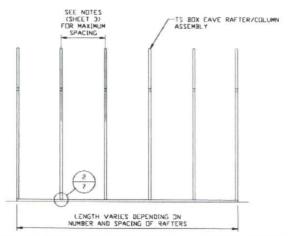


#### TYPICAL RAFTER/COLUMN END FRAME SECTION SCALE: NTS

TYPICAL RAFTER/COLUMN END FRAME SECTION SCALE: NTS



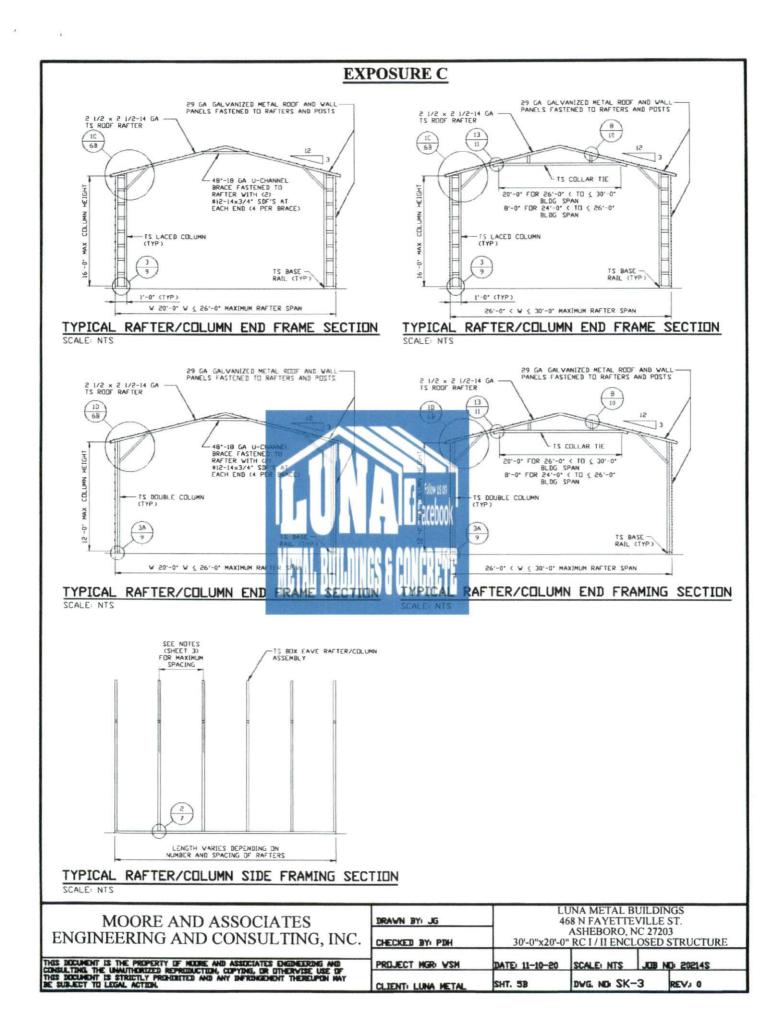
RAFTER/COLUMN END AFTER/COLUMN END FRAMING SECTION



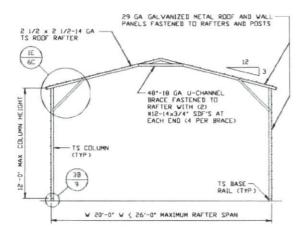
## TYPICAL RAFTER/COLUMN SIDE FRAMING SECTION

SCALE: NTS

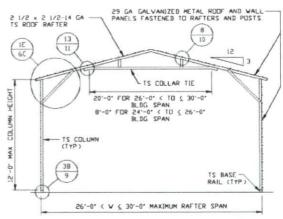
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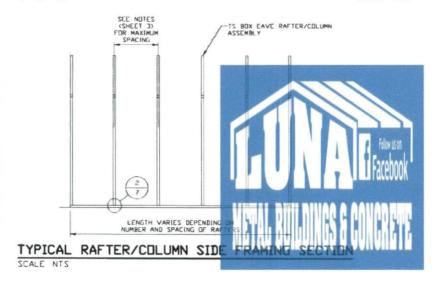
## **EXPOSURE C**





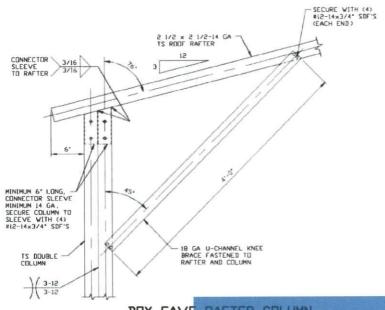


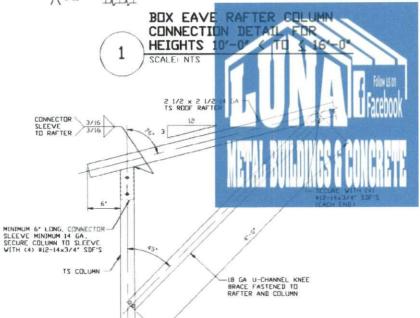
TYPICAL RAFTER/COLUMN END FRAME SECTION



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# EXPOSURE B COLUMN CONNECTION DETAILS





BOX EAVE RAFTER COLUMN CONNECTION DETAIL FOR HEIGHTS 8'-0' < TO < 10'-0"

1A

SCALE: NTS NDTE: SINGLE COLUMN HEIGHT UP TO 12'-0' FOR MAXIMUM 115 MPH WIND SPEED

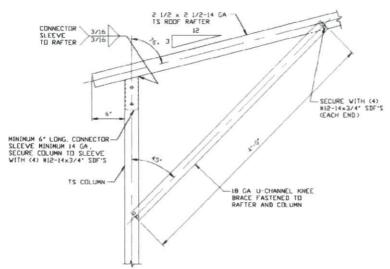
	λı	
5 1/5.	IB GA U-CHANNEL BRACE FASTENE THE COLUMN AND ROOF BEAM, WIT (4) #12-14x3/4' SDF'S AT EACH E (8 PER BRACE)	H

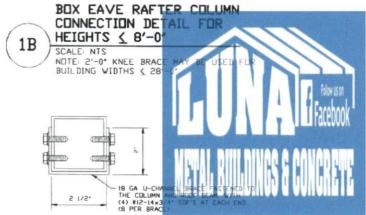
## BRACE SECTION

SCALE: NTS

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# EXPOSURE B COLUMN CONNECTION DETAILS



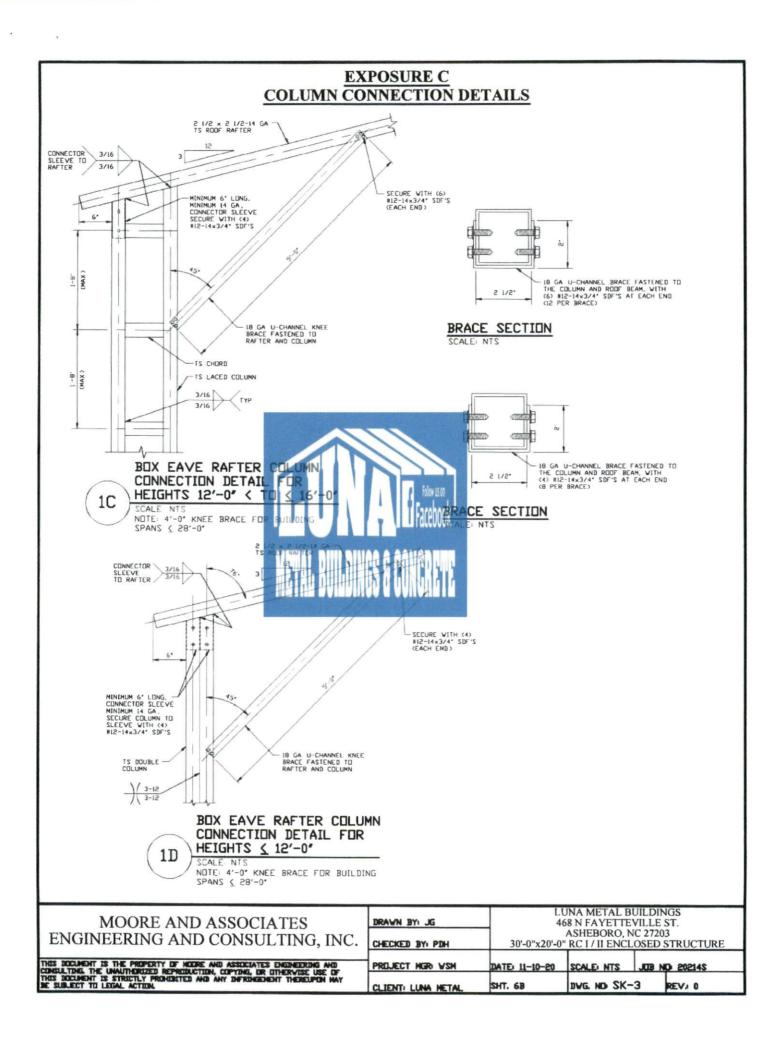


## BRACE SECTION

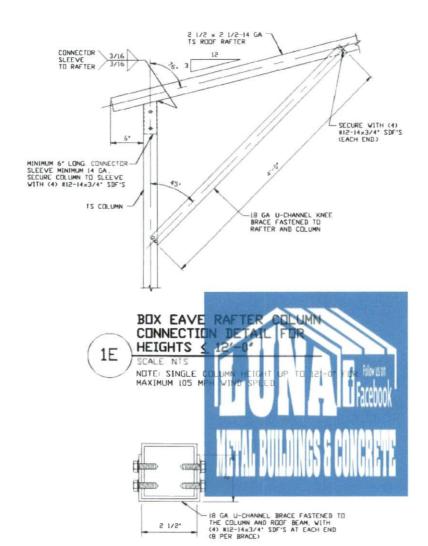
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PROJECT MGR: VSM CLIENT: LUNA METAL	DATE: 11-10-20 SHT. 6A	SCALE: NTS		EV. 0		



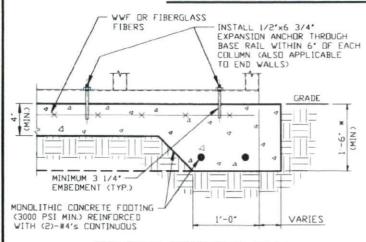
# EXPOSURE C COLUMN CONNECTION DETAILS



BRACE SECTION
SCALE: NTS

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MOORE AND ASSOCIATES	DRAWN BY: JG	46	JNA METAL B 8 N FAYETTE	VILLE	ST.

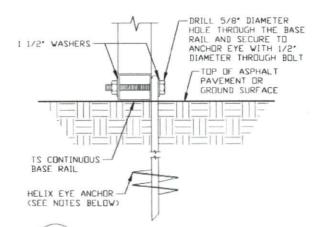
### BASE RAIL ANCHORAGE OPTIONS (EXPOSURE B)





#### CONCRETE MONOLITHIC SLAB BASE RAIL ANCHORAGE

NOTE: MIN ANCHOR EDGE DISTANCE IS 4" \* COORDINATE WITH LOCAL BUILDING CODE ORD REGARDING REQUIRED FROST DEPTH (LENGTH)



2B

#### GROUND BASE HELIX ANCHORAGE

SCALE: NTS (CAN BE USED FOR ASPHALT)
\* COORDINATE WITH LOCAL CODES/ORD.
REGARDING MIN FROST DEPTH (LENGTH).

#### **GENERAL NOTES**

NOTE: CONCRETE MONDLITHIC SLAB DESIGN BEARING CAPACITY OF 1,500 PSF

#### CONCRETE

CONCRETE SHALL HAVE A MINIMUM SPECIFIE STRENGTH OF 3,000 PSI AT 28 DAYS

## COVER OVER REINFORCING ST

FOR FOUNDATIONS, MINIMUM CONCRETE COVE BARS SHALL BE PER ACT-318:

3' IN FOUNDATIONS WHERE THE CONCRETE
PERMANENTLY IN CONTACT WITH THE EARTH
EARTH OR WEATHER, AND 1 1/2' ELSEWHERI

#### REINFORCING STEEL

THE TURNDOWN REINFORCING STEEL SHALL 60 THE SLAB REINFORCEMENT SHALL BE W MEETING ASTM A185 OR FIBERGLASS FIBER

#### REINFORCEMENT MAY BE BENT IN SHOP OR THE FIELD PROVIDED

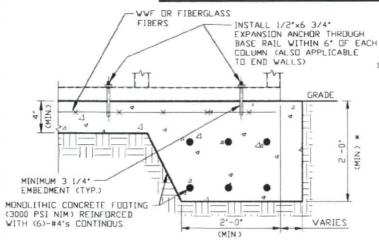
- REINFORCEMENT IS BENT COLD
  THE DIAMETER OF THE BEND, MEASURED ON THE INSIDE OF THE BAR, IS NOT LESS THAN SIX-BAR DIAMETERS
- 3 REINFORCEMENT PARTIALLY EMBEDDED IN CONCRETE SHALL NOT BE FIELD BENT

#### HELIX ANCHOR NOTES

- 1 FOR VERY DENSE AND/OR CEMENTED SANDS, COARSE GRAVEL AND COBBLES, CALICHE, PRELOADED SILTS AND CLAYS, USE MINIMUM (2) 4" HELICES WITH MINIMUM 30" EMBEDMENT OR SINGLE 6" HELIX WITH MINIMUM 50" EMBEDMENT
- 2 FOR CORAL USE MINIMUM (2) 4' HELICES WITH MINIMUM 30' EMBEDMENT OR SINGLE 6' HELIX WITH MINIMUM 50° EMBEDMENT.
- 3. FOR MEDIUM DENSE COARSE SANDS, SANDY GRAVELS, VERY STIFF SILTS, AND CLAYS USE MINIMUM (2) 4' HELICES WITH MINIMUM 30 INCH EMBEDMENT OR SINGLE 6' HELIX WITH MINIMUM 50" EMBEDMENT.
- 4 FOR LODSE TO MEDIUM DENSE SANDS, FIRM TO STIFF CLAYS AND SILTS ALLUVIAL FILL, USE MINIMUM (2) 6' HELICES WITH MINIMUM 50" EMBEDMENT
- 5 FOR VERY LOSE TO MEDIUM DENSE SANDS, FIRM TO STIFFER CLAYS AND SILTS, ALLUVIAL FILL, USE MINIMUM (2) 8° HELICES WITH MINIMUM 60' EMBEDMENT

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## BASE RAIL ANCHORAGE OPTIONS (EXPOSURE C)



HOLE THROUGH THE BASE RAIL AND SECURE TO 1 1/2" WASHERS -ANCHOR EYE WITH 1/2" DIAMETER THROUGH BOLT TOP OF ASPHALT PAVEMENT OR of GROUND SURFACE TS CONTINUOUS BASE RAIL HELIX EYE ANCHOR (SEE NOTES BELOW)

CONCRETE MONOLITHIC SLAB BASE RAIL ANCHURAGE 2

SCALE: NTS NOTE: MIN ANCHOR EDGE DISTANCE IS 4" \* COORDINATE WITH LOCAL BUILDING CODE URD REGARDING REQUIRED FROST DEPTH (LENGTH)

## GROUND BASE HELIX ANCHORAGE

SCALE: NTS (CAN BE USED FOR A \* COORDINATE WITH LOCAL CODES/ORD REGARDING MIN. FROST DEPTH (LENGTH)

DRILL 5/8' DIAMETER

#### **GENERAL NOTES**

NOTE: CONCRETE MONOLITHIC SLAB DESIGN BEARING CAPACITY OF 1,500 PSF.

#### CONCRETE

CONCRETE SHALL HAVE A MINIMUM SPECIFIS STRENGTH OF 3,000 PSI AT 28 DAYS.

COVER OVER REINFORCING S FOR FOUNDATIONS, MINIMUM CONCRETE COVE BARS SHALL BE PER ACT-318: 3' IN FOUNDATIONS WHERE THE CONCRETE PERMANENTLY IN CONTACT WITH THE EARTH EARTH OR WEATHER. AND 1 1/2' ELSEWHER!

#### REINFORCING STEEL

THE TURNDOWN REINFORCING STEEL SHALL 60 THE SLAB REINFORCEMENT SHALL BE W MEETING ASTM A185 DR FIBERGLASS FIBER

#### REINFORCEMENT MAY BE BENT IN SHOP OR THE FIELD PROVIDED:

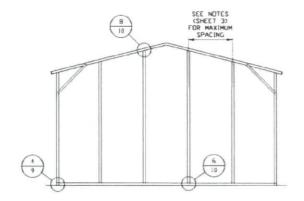
- REINFORCEMENT IS BENT COLD
  THE DIAMETER OF THE BEND, MEASURED ON THE INSIDE OF THE BAR, IS NOT LESS THAN SIX-BAR DIAMETERS.
- 3 REINFORCEMENT PARTIALLY EMBEDDED IN CONCRETE SHALL NUT BE FIELD BENT

#### HELIX ANCHOR NOTES:

- 1 FOR VERY DENSE AND/OR CEMENTED SANDS, COARSE GRAVEL AND COBBLES, CALICHE, PRELOADED SILTS AND CLAYS, USE MINIMUM (2) 4" HELICES WITH MINIMUM 30" EMBEDMENT OR SINGLE 6" HELIX WITH MINIMUM 50" EMBEDMENT
- 2 FOR CORAL USE MINIMUM (2) 4' HELICES WITH MINIMUM 30' EMBEDMENT OR SINGLE 6' HELIX WITH MINIMUM 50" EMBEDMENT
- 3 FOR MEDIUM DENSE COARSE SANDS, SANDY GRAVELS, VERY STIFF SILTS, AND CLAYS USE MINIMUM (2) 4° HELICES WITH MINIMUM 30 INCH EMBEDMENT OR SINGLE 6° HELIX WITH MINIMUM 50' EMBEDMENT
- 4 FOR LODSE TO MEDIUM DENSE SANDS, FIRM TO STIFF CLAYS AND SILTS ALLUVIAL FILL, USE MINIMUM (2) 6" HELICES WITH MINIMUM 50' EMBEDMENT
- 5 FOR VERY LOSE TO MEDIUM DENSE SANDS, FIRM TO STIFFER CLAYS AND SILTS, ALLUVIAL FILL, USE MINIMUM (2) 8° HELICES WITH MINIMUM 60' EMBEDMENT

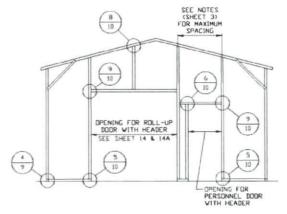
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## BOX EAVE RAFTER END WALL AND END WALL OPENINGS



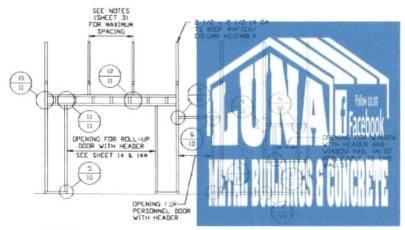
TYPICAL BOX EAVE RAFTER END WALL FRAMING SECTION

SCALE: NTS



TYPICAL BOX EAVE RAFTER END WALL OPENINGS FRAMING SECTION

SCALE: NTS

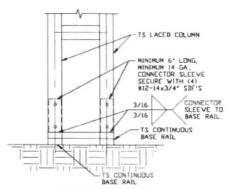


TYPICAL BOX EAVE RAFTER SIDE WALL OPENINGS FRAMING SECTION

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## CONNECTION DETAILS



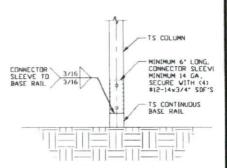
RAFTER COLUMN/ BASE RAIL CONNECTION DETAIL

3

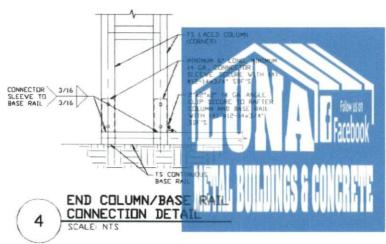
CONNECTOR SLEEVE HINIMUM 6' LONG, CONNECTOR SLEEVE HINIMUM 14 GA, SECURE VITH (4) BI2-14×3/4' SDF'S

TS CONTINUOUS BASE RAIL

RAFTER COLUMN/
BASE RAIL
CONNECTION DETAIL
SCALE: NTS



RAFTER COLUMN/
BASE RAIL
CONNECTION DETAIL
SCALE: NTS



TS DOUBLE COLUMN
(CURNER)

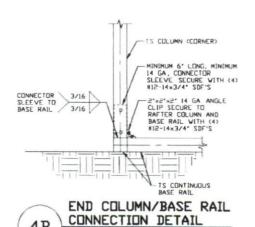
HINIMUM 6' LONG, MINIMUM
14 GA, CONNECTOR
SLEEVE SECURE WITH
(4) MI2-14×3/4' SDF'S

P RAFTER COLUMN AND
BASE RAIL

15 CONTINUOUS
BASE RAIL

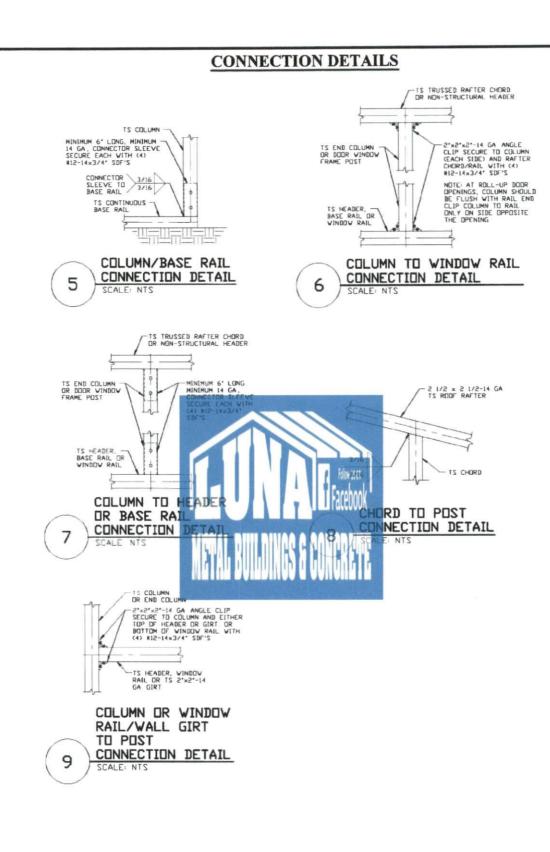
4A CONNECTION DETAIL

SCALE NTS

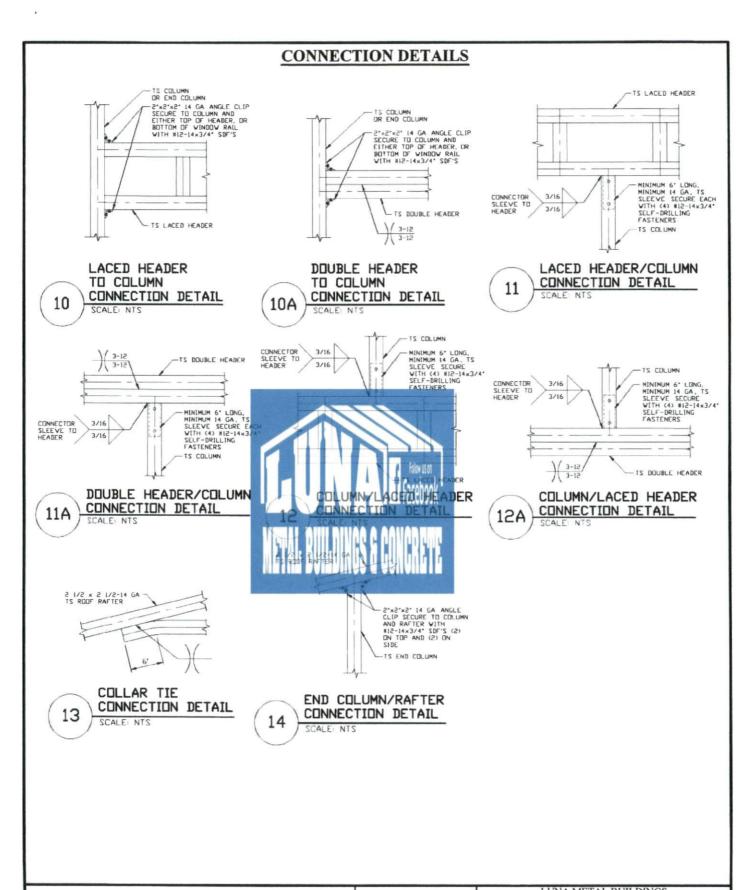


4B SCALE: NTS

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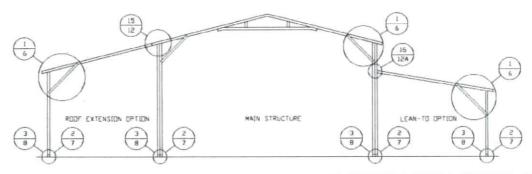


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MOORE AND ASSOCIATES	DRAWN BY: JG	46	NA METAL BU 8 N FAYETTE	VILLE	ST.

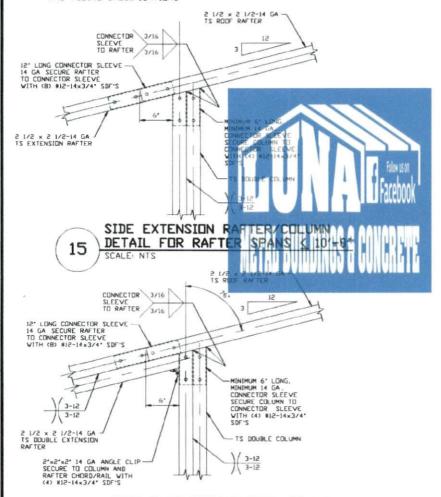
### **BOX EAVE RAFTER LEAN-TO OPTIONS**



### TYPICAL BOX EAVE RAFTER LEAN-TO OPTIONS FRAMING SECTION (BOTH OPTIONS SHOWN)

SCALE: NTS

FOR SHARED COLUMNS REFERENCE RAFTER COLUMN CONNECTION DETAILS FOR APPROPRIATE COLUMN HEIGHT AND TUBING SPECIFICATIONS



SIDE EXTENSION RAFTER/COLUMN
DETAIL FOR RAFTER SPANS
10'-0' < TO \( \sum \) 12'-0'

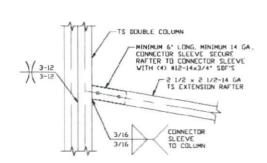
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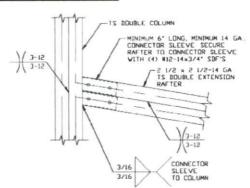
15A

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-	PROJECT MGR: VSM				D 20214S		
	CLIENT: LUNA HETAL	SHT. 12	DVG. ND SK-	-3	REV. 0		

### **BOX EAVE RAFTER LEAN-TO OPTIONS**



LEAN-TO RAFTER TO RAFTER COLUMN CONNECTION DETAIL FOR RAFTER SPANS & 10'-0'



LEAN-TO RAFTER TO RAFTER COLUMN CONNECTION DETAIL FOR RAFTER SPANS

> -2"x2"x2" 14 GA ANGLE CLIP SECURE TO COLUMN AND RAFTER CHORD/RAIL VITH (4) #12-14x3/4" SDF'S

-2 1/2 x 2 1/2-14 GA TS DOUBLE EXTENSION RAFTER

16A) 10'-0' < T□ ≤ 12'-0"



LEAN-TO RAFTER TO RAFTER COLUMN CONNECTION DETAIL FOR RAFTER SPANS 10'-0" < TO ≤ 12'-0"

TS DOUBLE

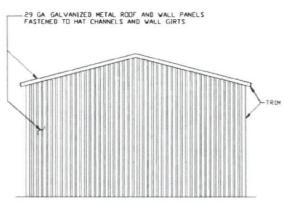
SCALE: NTS

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PROJECT HGR: VSH CLIENT: LUNA METAL	DATE: 11-10-20 SHT. 12A	SCALE: NTS DVG. ND: SK-		ID 20214\$		

## BOX EAVE RAFTER VERTICAL ROOF/SIDING OPTION



## $\frac{\text{TYPICAL END ELEVATION VERTICAL ROOF/SIDING}}{\text{SCALE: NTS}}$

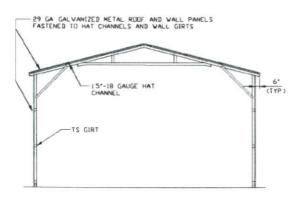
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PASTEMED TO HAT CHANNELS AND VALL GIRTS

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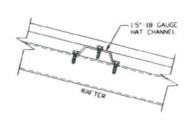
ELEVATION VERTICAL ROOF/SIDING

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## **BOX EAVE RAFTER VERTICAL ROOF/SIDING**

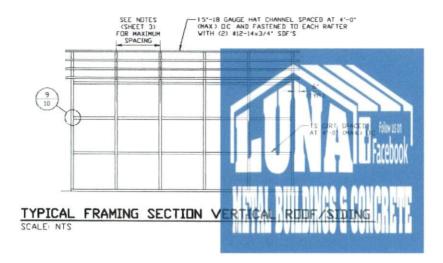


TYPICAL SECTION VERTICAL ROOF/SIDING



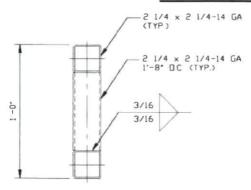
PANEL ATTACHMENT

(ALTERNATE FOR VERTICAL ROOF PANELS)

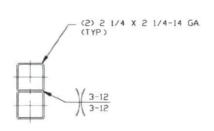


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# EXPOSURE B SIDE WALL HEADER OPTIONS

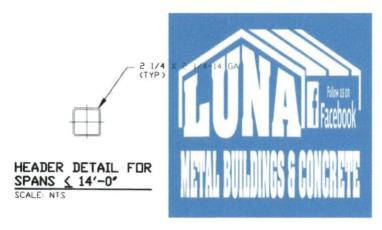


HEADER DETAIL FOR SPANS 10'-0" < TO \( \sqrt{10} \) 14'-0"



HEADER DETAIL FOR SPANS ≤ 10'-0' SCALE: NTS

## END WALL HEADER OPTIONS

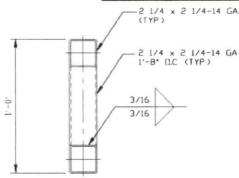


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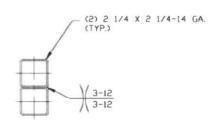
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CHECKED BY: PDH	30'-0"x20'-0	ASHEBORO, NC 27203 30'-0"x20'-0" RC 1 / II ENCLOSED STRUCTURE			
PROJECT MGR: VSM	DATE 11-10-20	SCALE: NTS	JOB	ND 20214S	
CLIENTI LUNA METAL	SHT. 14	DWG. NO SK-	-3	REV. 0	

## **EXPOSURE C** SIDE WALL HEADER OPTIONS

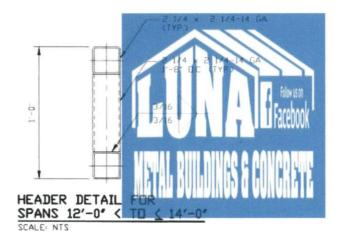


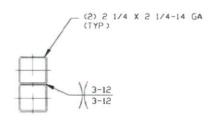
HEADER DETAIL FOR SPANS 8'-0' < T□ < 14'-0'
SCALE: NTS



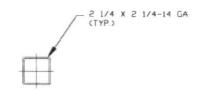
HEADER DETAIL FOR SPANS <u>≤</u> 8'-0"

## END WALL HEADER OPTIONS





HEADER DETAIL FOR SPANS 7'-0" < T□ < 12'-0"



HEADER DETAIL FOR SPANS & 7'-0'

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