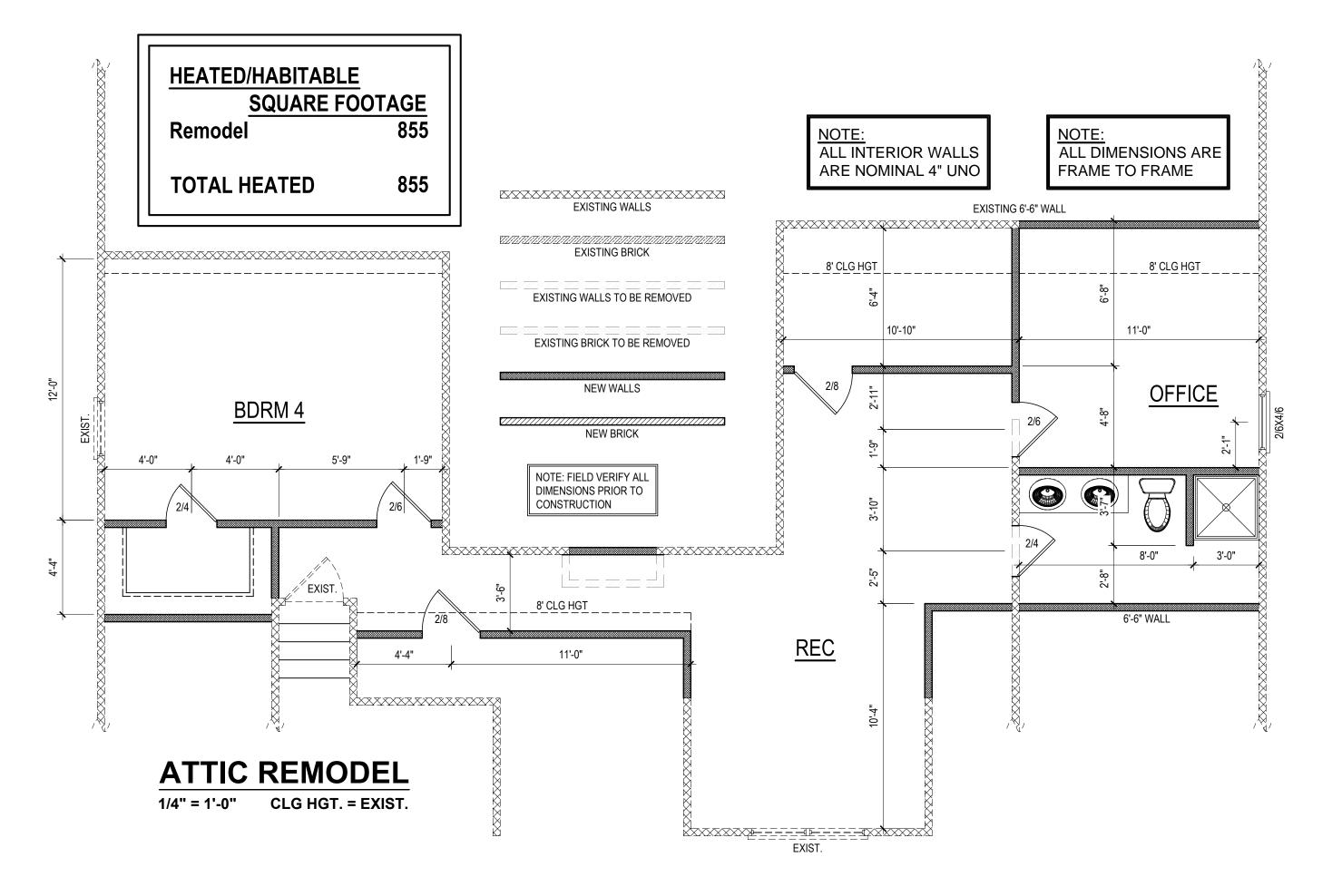
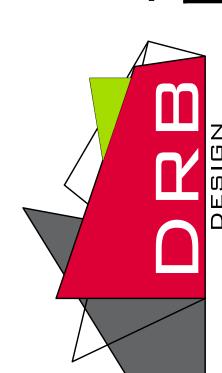
BLEGENREMODEL





1. DRB DESIGN assumes no liability for any home constructed from this plan.

All construction shall conform to the latest requirements of "North Carolina State 2018 residential building code" in addition to all local codes and regulations.

3. Should these plans require structural calculations for permitting the contractor shall be required to obtain the

services of a structural engineer after notifying DRB DESIGN that such services are required.

Release of these plans requires further cooperation among the owner, his/her contractor, and DRB DESIGN.

Design and construction are complex and, although the designer performed his services with due care and diligence, perfection is not a guarantee.

6. Communication is imperfect and every contingency cannot be anticipated.

 Any ambiguity or discrepancy discovered by the use of these plans shall be reported immediately to DRB DESIGN. Failure to notify the DRB DESIGN compounds misunderstandings and increases construction costs.

 A failure to cooperate by a simple notice to DRB DESIGN shall relieve the designer from any and all responsibilities for all consequences.

Changes made to these plans without the consent of the designer are unauthorized and shall relieve DRB DESIGN of responsibility for any and all consequences arriving out of such changes.

Written dimensions on these plans always have precedence over scaled dimensions.

11. It is the contractors responsibility to verify and be responsible for all dimensions and square footage prior to construction, as well as conditions on the job site. DRB DESIGN is not responsible for dimension and square footage errors once construction has begun.

DRB DESIGN must be notified of any variations from the dimensions and conditions shown on these drawings.

DESIGN idbyrd@drbhomedesign.com 250 Shipwash Dr Suite 105 Garner

DRB2001-0046

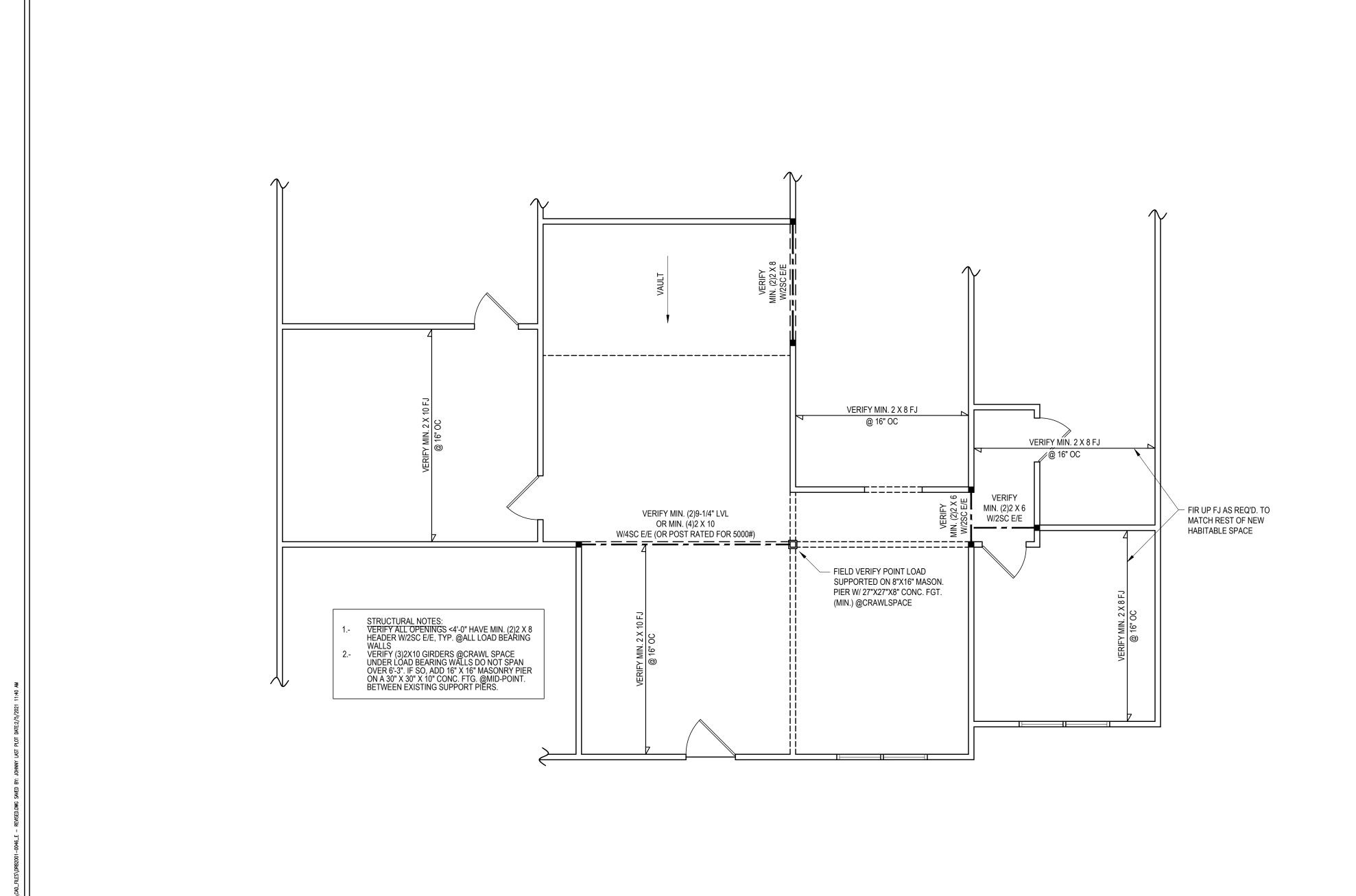
11/30/20 **DESIGNED BY**

CHECKED BY

Dana Blegen 188 Saddle Ln Lillington, NC 27546 910-322-4925 dsblegen@gmail.com

SHEET NAME
FLOOR PLAN
SHEET #

A1



* Engineers seal does not include construction means, methods, techniques, sequences, procedures or safety precaution.

* Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering & Design, P.A. liability.

* Please review these documents carefully. Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommendations, etc. presented in these documents were deemed acceptable once construction begins.



ENGINEERING & DESIGN, P.A.



1ST FLR. HEADER
2ND FLR. FRMG.

Project #:	
DRB2001-0046	
DKD2001-0040	
Date:	
12/30/20	
Drawn/Design By:	
JTT	
J 1 1	
DWG, Checked By:	
PAT	

	SEE PL	AN
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No.	Date:	Re

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Sheet Number

S1

FIRST FLOOR PLAN

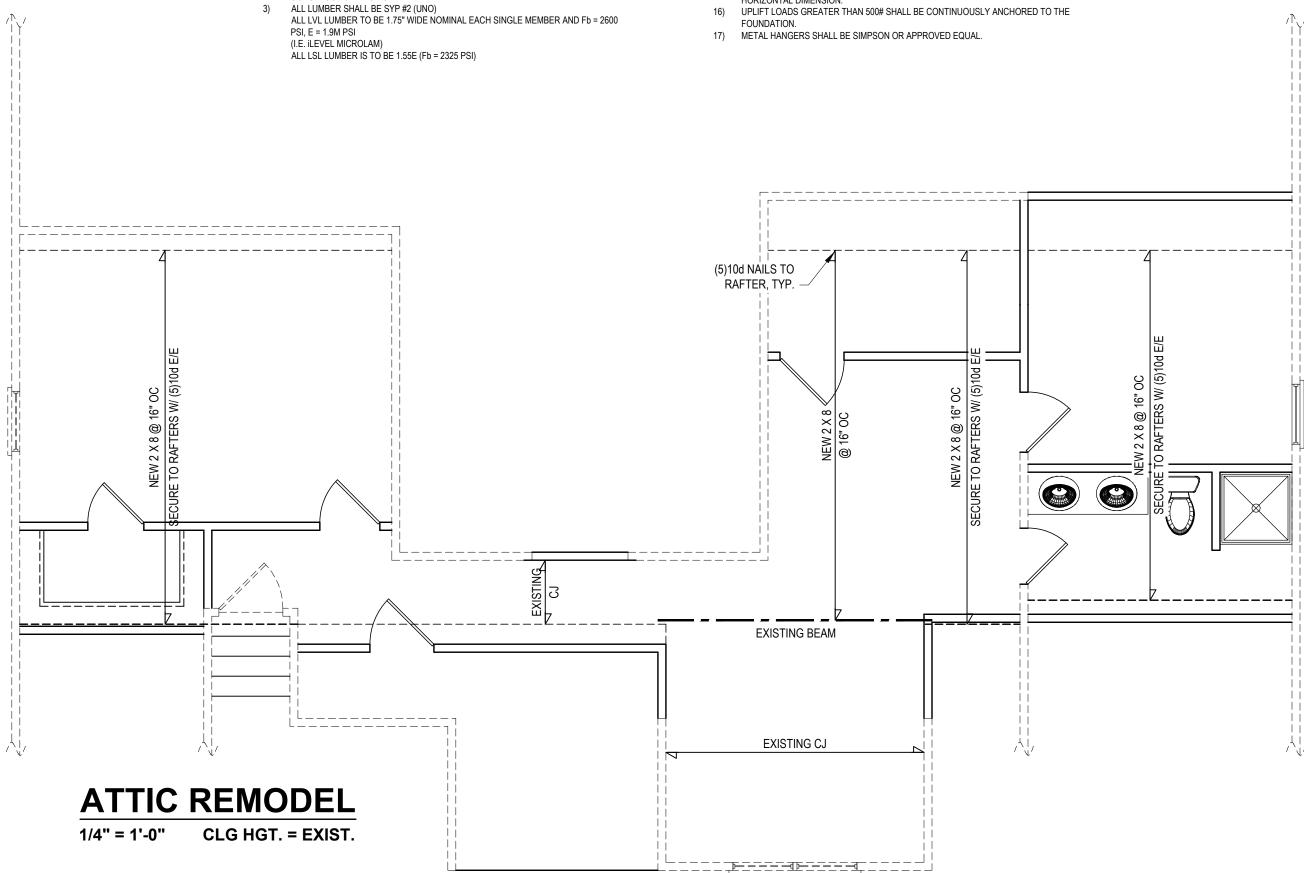
1/4" = 1'-0"

DESIGN LOADS

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION		
	, ,	, ,	LL	TL	
FLOOR (primary)	40	10	L/360	L/240	
FLOOR (secondary)	40	10	L/360	L/240	
ATTIC (w/ storage)	20	10	L/240	L/180	
ATTIC (no access)	10	5	L/240	L/180	
EXTERNAL BALCONY	40	10	L/360	L/240	
ROOF	20	10	L/240	L/180	
ROOF TRUSS	20	20	L/240	L/180	
WIND LOAD	BAS	SED ON 120 MPH (E	XPOSURE B)		
SEISMIC	BAS	SED ON SEISMIC ZO	NES A, B & C		

- ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN ADDITION TO ALL LOCAL CODES AND REGULATIONS.
- IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION. TYNDALL ENGINEERING & DESIGN, PA IS NOT RESPONSIBLE FOR DIMENSIONS AND SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.

- ALL LOAD BEARING EXTERIOR WINDOW HEADERS WITH MAXIMUM SPAN OF 5'-6" SHOULD BE A (2) 2x10 w/ (1) 2x4 KING STUD AND (1) 2x4 JACK STUD NAILED TOGETHER w/ (2) 10d @ 8" O.C. PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6'-8", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1'-6", OTHERWISE REFER TO TABLE R502.5(1).
- ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFER TO TABLE R502.5(1) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND
- EXTERIOR LOAD CONDITIONS (UNO)
 REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION OF ALL
- WALLS OVER 10'-0" IN HEIGHT.
- ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50
- Fy = 50 KSI MIN. (UNO) ALL EXTERIOR LUMBER TO BE #2 SYP PT
- ALL CONCRETE, fc = 3000 PSI MIN. PRESUMPTIVE BEARING CAPACITY = 2000 PSF
- 1/2"Ø ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT
- SHALL EXTEND 7" INTO CONCRETE OR MASONRY. PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO)
- PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP AND BOTTOM OF PORCH COLUMNS. (U.N.O.)
- PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.4 OF THE 2018 IRC.
 MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST
- HORIZONTAL DIMENSION.



* Engineers seal does not include construction means, methods, techniques, sequences, procedures or safety precaution.

* Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering & Design, P.A. liability.

* Please review these documents carefully.

Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommendations, etc. presented in these documents were deemed acceptable once construction





Project #: DRB2001-0046 12/30/20 JTT

PAT SEE PLAN

DWG. Checked By:

REVISIONS

Sheet Number

STRUCTURAL NOTES

ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN ADDITION TO ALL LOCAL CODES AND REGULATIONS.

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLE	CTION
	()	(/	LL	TL
ALL FLOORS	40	10	L/360	L/240
ATTIC (w/ walk up stairs)	30	10	L/360	L/240
ATTIC (pull down access)	20	10	L/240	L/180
ATTIC (no access)	10	5	L/240	L/180
EXTERNAL BALCONY	40	10	L/360	L/240
ROOF	20	10	L/240	L/180
ROOF TRUSS	20	20	L/240	L/180
WIND LOAD		BASED ON 120 MF	PH (EXPOSURE B)	
SEISMIC		SEISMIC ZOI	NES A, B & C	

- 3) MINIMUM ALLOWABLE SOIL BEARING PRESSURE = 2000 PSF
- 4) CONCRETE SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI AND A MAXIMUM SLUMP OF FIVE INCHES
- MAXIMUM DEPTH OF UNBALANCED FILL AGAINST FOUNDATION WALLS TO BE LESS THAN 4'-0" WITHOUT USING SUFFICIENT WALL BRACING. REFER TO SECTION R404 OF 2018 NC BUILDING CODE FOR BACKFILL LIMITATIONS BASED ON WALL HEIGHT, WALL THICKNESS, SOIL TYPE, AND UNBALANCED BACKFILL HEIGHT.
- 6) ALL FRAMING LUMBER SHALL BE SYP #2 (Fb = 800 PSI, BASED ON 2x10) UNO.
- ALL FRAMING LUMBER EXPOSED TO THE ELEMENTS SHALL BE TREATED MATERIAL.
 ALL LUV. LUMBER TO BE 1.75° WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2600 PSI, E = 1.9M PSI (U.N.O.)
 ALL LSL LUMBER TO BE 3.5° WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2325 PSI, E = 1.6M PSI (U.N.O.)
 ALL PSL LUMBER TO BE 3.5° WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2400 PSI, E = 1.8M PSI (U.N.O.)
- ALL LOAD BEARING EXTERIOR HEADERS SHALL BE AT (2) 2x10. (U.N.O.) REFER TO TABLE R602.7(1) & (2) FOR JACK STUD
- REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LÓAD CONDITIONS UNLESS SPECÍFICALLY NOTED ON PLANS.
- 8) ALL STRUCTURAL STEEL W-SHAPES (I-BEAMS) SHALL BE ASTM A992 GRADE 50. ALL STEEL ANGLES, PLATES, AND C-CHANNELS SHALL BE ASTM A36. ALL STEEL PIPE SHALL BE ASTM A53 GRADE B.
- 9) STEEL BEAMS SHALL BE SUPPORTED AT EACH END WITH A MINIMUM BEARING LENGTH OF 3-1/2" AND FULL FLANGE WIDTH. PROVIDE SOLID BEARING FROM BEAM SUPPORT TO FOUNDATION. BEAMS SHALL BE ATTACHED TO EACH SUPPORT WITH TWO (2) LAG SCREWS (1/2") A" 1 CNDIS, LATERAL SUPPORT IS CONSIDERED ADEQUATE PROVIDED THE JOISTS ARE TOE NAILED TO THE SOLE PLATES, AND THE SOLE PLATES ARE NAILED OR BOLTED TO THE BEAM FLANGES @ 48" O.C.
- 10) PROVIDE ANCHOR BOLT PLACEMENT PER SECTION 403.1.6: 1/2"Ø ANCHOR BOLTS SPACED AT 6"-0" O.C. AND PLACED 12" FROM THE END OF EACH PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3"-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY. THE BOLTS SHALL BE LOCATED IN THE MIDDLE THIRD OF THE WIDTH OF THE PLATE. THERE SHALL BE A MINIMUM TWO ANCHOR BOLTS PER PLATE SECTION.
- 11) FOUNDATION DRAINAGE-DAMP PROOFING OR WATERPROOFING PER SECTION 405 AND 406 OF NC BUILDING CODE.
- 12) WALL AND ROOF CLADDING VALUES:
 WALL CLADDING SHALL BE DESIGNED FOR 28.0 POUNDS PER SQUARE FOOT (LBS/SQFT) OR GREATER POSITIVE AND NEGATIVE PRESSURE.
 ROOF VALUES BOTH POSITIVE AND NEGATIVE SHALL BE AS FOLLOWS:
 39.0 LBS/SQFT FOR ROOF PITCHES 01/2 TO 1.5/12
 36.0 LBS/SQFT FOR ROOF PITCHES 1.5/12 TO 6/12
 18.0 LBS/SQFT FOR ROOF PITCHES 6/12 TO 12/12
 "MEAN ROOF HEIGHT 30'-0" OR LESS
- 13) FOR ROOF SLOPES FROM 2/12 THROUGH 4/12, BUILDER TO INSTALL 2 LAYERS OF 15# FELT PAPER.
- 14) REFER TO SECTION R602.3 FOR FRAMING OF ALL WALLS OVER 10'-0" IN HEIGHT.
- 15) PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.3 OF THE 2018 NCRC.
- 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY ANCHORED TO THE FOUNDATION.
- 17) REFER TO TABLE N1102.1 FOR PRESCRIPTIVE BUILDING ENVELOPE THERMAL COMPONENT CRITERIA.
- 18) PSL COLUMNS DESIGNED WITH MAXIMUM HEIGHT OF 9'-0" (U.N.O.)
- 19) PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP AND BOTTOM OF PORCH COLUMNS. (U.N.O.)
- 20) MAXIMUM MASONRY PEIR HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.
- 21) IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION. TYNDALL ENGINEERING & DESIGN, PA IS NOT RESPONSIBLE FOR DIMENSION OR SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.

CLIMATE ZONES	FENESTRATION U-FACTOR b,j	SKYLIGHT ^b U-FACTOR	GLAZED FENESTRATION SHGC ^{b,k}	CEILING ^m R-VALUE	WOOD FRAMED WALL R-VALUE	MASS WALL R-VALUE i	FLOOR R-VALUE	BASEMENT ^{C,Q} WALL R-VALUE	SLAB ^d R-VALUE AND DEPTH	CRAWL SPACE CWALL R-VALUE
3	0.35	0.55	0.30	38 or 30 cont	15 or 13 + 2.5	5/13 or 5/10 cont	19	<u>5/13</u> f	0	5/13
4	0.35	0.55	0.30	38 or 30 cont j	15 or 13 + <u>2.5</u> h	5/13 or 5/10 cont	19	10/15	10	10/15
5	0.35	0.55	NR	38 or 30 cont j	n 19, or 13 + 5 or 15 + 3	13/17 <u>or</u> 13/12.5 cont	30 ⁹	10/15	10	10/19
NO SCALE	a. R-VALUES A	RE MINIMUMS. U-FACTO		WHEN INSULATION IS	INSTALLED IN A CAVITY WHICH SS THAN THE R-VALUE SPECIFIE		DESIGN THICKNESS			

- b. THE FENESTRATION U-FACTOR COLUMN EXCLUDED SKYLIGHTS. THE SOLAR HEAT GAIN COEFFICIENT (SHGC) COLUMN APPLIES TO ALL GLAZED FENESTRATION.
- (SHICL) DOLUMN APPLIES TO ALL GLAZED FENESTHATION.

 C 1919 MEMBER & DOLUMBLOUS BULLED SHEATHING ON THE INTERIOR OR EXTERIOR OF THE HOME

 OR RESCAUTT INSULATION AT THE INTERIOR OF THE BESELBERT WALL OR CRAWL SPACE WALL

 FOR MONICHING SLABS, INSULATION SHALL BE APPLIED FROM THE INSPECTION GAP DOWNWARD TO THE BOTTOM
 OF THE FOOTING ON A BUANDAM OF 2H BELOW GRADE WHICHCHE'S ILESS FOR FLOATING SUASS, INSULATION
 SHALL EXTERN TO THE BOTTOM OF THE FOUNDATION WALL OR 2H, WHICHCHE'R IS LESS. RS SHALL BE
 ADDED TO THE REQUIRED SHA BODE RAVALLES FOR HACTED SLABS.
- e. <u>DELETED</u>

 f. BASEMENT WALL INSULATION IS NOT REQUIRED IN WARM-HUMID LOCATIONS AS DEFINED BY <u>FIGURE N1101.7</u>. AND <u>TABLE N1101.7</u>. g. OR INSULATION SUFFICIENT TO FILL THE FRAMING CAVITY. R-19 MINIMUM.
- h. THE FIRST VALUE IS CAVITY INSULATION, THE SECOND VALUE IS CONTINUOUS INSULATION, SO "13+5" MEANS R-13 CAVITY INSULATION F ENGLISHMENT SHEATH RESIDENCE THE RESIDENCE OF THE CONTROLLED SHEATHING ESTRUCTURAL SHEATHING COVERS SAY OF THE EXTENDING INSULATING SHEATHING IS NOT REQUIRED WHERE THE STRUCTURAL SHEATHING COVERS SAY OF THE EXTENDING INSULATING SHEATHING COVERS MORE THAN 25 PERCENT OF THE EXTENDED SHALL BE SUPPLEMENTED WITH INSULATED SHEATHING OF AT LEAST R2. "13 - 25" MEANS R-13 CAVITY INSULATION SHEATHING OF AT LEAST R2. "15 - 25" MEANS R-13 CAVITY INSULATION SHEATHING OF AT LEAST R2. "15 - 25" MEANS R-13 CAVITY INSULATION SHEATHING OF AT LEAST R2. "15 - 25" MEANS R-13 CAVITY INSULATION SHEATHING OF AT LEAST R2. "15 - 25" MEANS R-13 CAVITY INSULATION SHEATHING OF AT LEAST R2. "15 - 25" MEANS R-13 CAVITY INSULATION SHEATHING OF AT LEAST R2. "15 - 25" MEANS R-13 CAVITY INSULATION SHEATHING OF AT LEAST R2. "15 - 25" MEANS R-13 CAVITY INSULATION SHEATHING OF AT LEAST R2. "15 - 25" MEANS R-13 CAVITY INSULATION SHEATHING OF AT LEAST R2. "15 - 25" MEANS R-13 CAVITY INSULATION SHEATHING OF AT LEAST R2. "15 - 25" MEANS R-13 CAVITY INSULATION SHEATHING OF AT LEAST R2. "15 - 25" MEANS R-13 CAVITY INSULATION SHEATHING OF AT LEAST R2. "15 - 25" MEANS R-13 CAVITY INSULATION SHEATHING OF AT LEAST R2. "15 - 25" MEANS R-13 CAVITY INSULATION SHEATHING OF AT LEAST R2. "15 - 25" MEANS R-13 CAVITY INSULATION SHEATHING OF AT LEAST R2. "15 - 25" MEANS R-13 CAVITY INSULATION SHEATHING OF AT LEAST R2. "15 - 25" MEANS R-13 CAVITY INSULATION SHEATHING OF AT LEAST R2. "15 - 25" MEANS R-13 CAVITY INSULATION SHEATHING OF AT LEAST R2. "15 - 25" MEANS R-13 CAVITY INSULATION SHEATHING OF AT LEAST R2. "15 - 25" MEANS R-13 CAVITY RATHER SHEATHING OF AT LEAST R2. "15 - 25" MEANS R-13 CAVITY RATHER SHEATHING OF AT LEAST R2. "15 - 25" MEANS R-13 CAVITY RATHER SHEATHING RATHER RATHE
- OR MASS WALLS, THE SECOND R-VALUE APPLIES WHEN MORE THAN HALF THE INSULATION IS ON THE INTERIOR MASS WAL

- I. FOR MASS WALLS, THE SECOND R-VALLE APPLE SHEW NOTE HE HAND, AT THE RISALATION SO THE INTERIOR MASS WALL.

 II. NADOTION TO THE EXEMPTION IN SECTION IN 102.3.3 ANAMANAN OF TWO GAZED PRESISTRATION PRODUCT ASSEMBLES HAWING A LPACTOR NO GREATER THAN 0.55 SHALL BE PRESITED TO BE SUBSTITUTED FOR IMMUNIMACION OF THE PRESISTRATION PRODUCT ASSEMBLES WITHOUT FREMATY.

 II. NADOTION TO THE EXEMPTION IN 102.3 ANAMANAN OF TWO GAZED PRESISTRATION PRODUCT ASSEMBLES WITHOUT FREMATY.

 IF ADMINISTRATION OF SUBSTITUTED FOR IMMUNIMACION OF COMPANY THE PRESISTRATION PRODUCT ASSEMBLES WITHOUT PREMATY.

 IF AS SHALL BE REPORT TO STATES THE CELL HAND REAL ATTOMIC PROPRIESTRATION PRODUCT ASSEMBLES WITHOUT PREMATY.

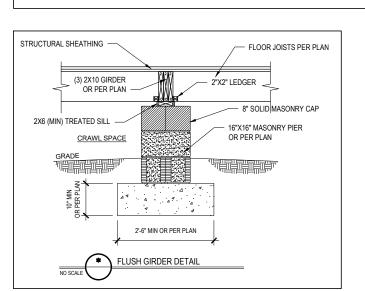
 IF AS SHALL BE REPORT TO STATES THE CELL HAND REAL ATTOMIC PROPRIESTRATION PRODUCT ASSEMBLES WITHOUT PREMATY.

 IF AS SHALL BE REPORT TO STATES THE CELL HAND REAL ATTOMIC PROPRIESTRATION PRODUCT ASSEMBLES WITHOUT PREMATY.

 IF AS SHALL BE REPORT TO STATES THE CELL HAND REAL ATTOMIC PROPRIESTRATION PRODUCT ASSEMBLES WITHOUT PREMATY.

 IF AS SHALL BE REPORT TO STATES THE CELL HAND REAL ATTOMIC PROPRIESTRATION PROPRIESTR
- OF THE ATTIC ROOF DECK.

 TABLE VALUE REQUIRED EXCEPT FOR ROOF EDGE WHERE THE SPACE IS LIMITED BY THE PITCH OF THE ROOF. THERE THE INSULATION MUST FILL THE SPACE UP TO THE AIR BAFFLE.
 R-19 FIRERQUASS BATTS COMPRESSED AND ROTATED BY A MOMINING 2 × 6 FRANING CAVITY IS DEEMED TO COMPLY. FIBERDLASS BATTS RATED R-19 OR HIGHER COMPRESSED.
 AND RISTALLED IN AZ VIVIAL IS NOT DEEMED TO COMPLY. S WALL SPECIFIC HEAT CONTENT REQUIREMENT MAY USE THE MASS WALL R-VALUE AS THE MINIMUM REQUIREMENT



DEFINITIONS FOR COMMON ABBREVIATIONS CANT CJ CMU COL CONC CONT CT CEILING JOIS' CONCRETE MASONRY UNIT ON CENTER POINT LOAD PRESSURE TREATED REINFORCED COLUMN REQUIRED ROOF JOIST ROOF SUPPORT STUD COLUMN REQD COLLAR TIE DOUBLE STUD COLUMN SCHEDULE SPECIFIED THICK TRIPLE JOIST TREATED TYPICAL UNLESS NOTED OTHERWISE WIDE FLANGE BEAM WELDED WIRE FABRIC EXTRA JOIST DOUBLE JOIST DOUBLE RAFTER EACH EACH END FLOOR JOIST FOUNDATION FOOTING GALVANIZED HORIZONTAL HEIGHT MANUFACTURER

POST SIZE	MAX. POST HEIGHT**
4 x 4	8'-0"
6 x 6	20'-0"
***	OVER 20'-0"

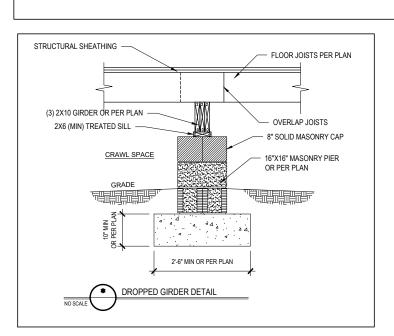
- THIS TABLE IS BASED ON NO. 2 TREATED SOUTHERN PINE POSTS.

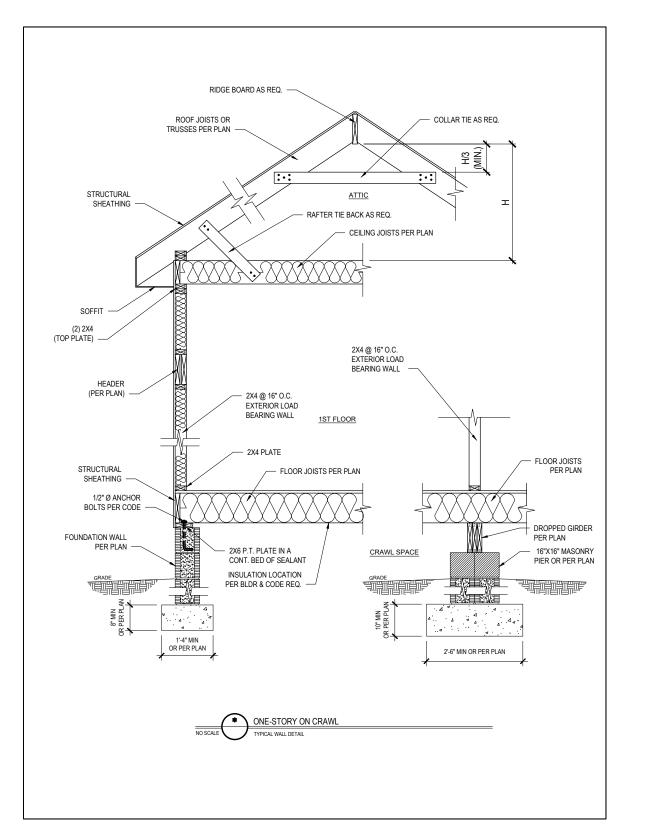
 MAXIMUM TRIBUTARY AREA IS BASED ON 128 TOTAL SQUARE FEET
 WHICH MAY BE LOCATED AT DIFFERENT LEVELS.
 FROM TOP OF FOOTING TO BOTTOM OF GIRDER
 DECKS WITH POST HEIGHTS OVER 20'-0" SHALL BE DESIGNED AND
 SEALED BY A PROFESSIONAL ENGINEER OR REGISTERED ARCHITECT.

- DECKS SHALL BE BRACED TO PROVIDE LATERAL STABILITY BY ONE OF THESE METHODS:
- THE DECK FLOOR HEIGHT IS LESS THAN 4'-0" AND THE DECK IS
 ATTACHED TO THE STRUCTURE IN ACCORDANCE WITH SECTION (4)
 ABOVE. LATERAL BRACING IS NOT REQUIRED.
 4 x 4 WOOD KNEE BRACES MAY BE PROVIDED ON EACH COLUMN IN
 BOTH DIRECTIONS. THE KNEE BRACES SHALL ATTACH TO EACH POST
 AT A POINT NOT LESS THAN 1/3 OF THE POST LENGTH FROM THE
 TOP OF THE POST, AND THE BRACES SHALL BE ANGLED BETWEEN
 45" AND 60" FROM THE HORIZONTAL KNEE BRACES SHALL BE
 DOLTED
 TO THE POST AND GIRDER WITH ONE 58"0" HOT DIPPED GALVANIZED
 BOLT AT EACH END OF THE BRACE.
- BOLT AT EACH END OF THE BRACE:
 C. FOR TREESTANDING DECKS WITHOUT KINEE BRACES OR DIAGONAL
 BRACING, LATERAL STABILITY MAY BE PROVIDED BY EMBEDDING THE
 POSTS IN ACCORDANCE WITH THE FOLLOWING:

POST SIZE	MAX. TRIBUTARY AREA	MAX. POST HEIGHT	EMBEDMENT DEPTH	CONCRETE DIAMETER
4 x 4	48 SQ. FT.	4'-0"	2'-6"	1'-0"
6 x 6	120 SQ. FT.	6'-0"	3'-6"	1'-8"

- D. 2 x 6 DIAGONAL VERTICAL CROSS BRACING MAY BE PROVIDED IN TWO (2) PERPENDICULAR DIRECTIONS FOR FREESTANDING DECKS OR PARALLEL TO THE STRUCTURE AT THE EXTERIOR COLUMN LINE FOR ATTACHED DECKS. THE 2 x 6s SHALL BE ATTACHED TO THE POSTS WITH ONE 5/8"Ø HOT ED GALVANIZED BOLT AT EACH END OF EACH BRACING MEMBER.
- E. FOR EMBEDMENT OF PILES IN COASTAL REGIONS, SEE CHAPTER 46.





HARDWARE C	ROSS-REFERENCE CHART
SIMPSON STRONG-TIE	USP STRUCTURAL CONNECTORS
PRODUCT NUMBER	PRODUCT NUMBER
A35	MPA1
ABE	PAE
CBSQ	CBSQ
CCQ	KCCQ
CMSTC16	CMSTC16
CS	RS
H1	RT15
H2.5A	RT7A
H10	RT16
HDQ8-SDS3	UPHD8
HDU2-SDS2.5	PHD2
HDU5-SDS2.5	PHD5
HETA	НТА
HGAM10KTA	HGAM
HHDQ14-SDS2.5	UPHD14
HTS	HTW
HTT	нтт
HUS	HUS
LTA1	LPTA
LTHJA26	HJC26
LTP4	MP4F
LUS	JUS
MAS	FA3
MSTAM	MSTAM
PC	PCM
PHD-SDS3	PHD
SSP	RSPT6
STC	TR1
STHD	STAD

neans, methods, techniques, sequences, procedures or safety precaution. Any deviations or discrepancies on * Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering & Design, P.A. liability. * Please review these documents carefully. Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommendations, etc. presented in these documents were deemed secretable once construction. emed acceptable once construction







REMODE

BLEGEN

DANA

R S \triangleleft

DRB2001-0046 12/30/20 Drawn/Design By: JTT DWG. Checked By: PAT

NOT TO SCALE

REVISIONS Date: Remarks

Sheet Number

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