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STRUCTURAL CALCULATIONS

PREPARED FOR

TARHEEL BASEMENT SYSTEMS

FOR

ADAMS RESIDENCE **FOUNDATION REPAIR**

386 CAMELIA RD

ANGIER, NC

PROJECT NUMBER: 21.083.TBS

DATE: March 19, 2021

PROJECT MANAGER: DANIEL STARK, P.E.

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Project No.: 21.083.TBS

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March 19, 2021

Tonya Gunn Tarheel Basement Systems 2910 Griffith Road Winston-Salem, North Carolina 27103

RE: Foundation repair - 386 Camelia Rd, Angier, NC

PROJECT BACKGROUND

We understand that the structure is a single-family residence and has experienced settlement at the front elevation of the structure. A recent floor level survey (attached) indicates as much as ~1" of differential settlement may have occurred. It is our understanding that (2) 2 7/8 inch diameter push piers have been proposed to provide additional foundation support.



Image 1: Front Elevation

THE CAME

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GEOLOGIC SETTING

The existing structure is located in Angier, NC. The geologic structure in the area is comprised of silty loam and the site is relatively flat. It is our opinion that the localized settlement is a result of improper foundation drainage and/or undersized footings. We believe that suitable support can be achieved by installing helical and/or push piers.

SUMMARY

The ultimate load requirement for the push piers is 20000 lbs, and based on the geologic setting, we expect the piers to achieve adequate capacity at approximately 8 - 25 feet. We recommend that the piers with a 2 7/8 inch shaft and be installed to a minimum depth of 8 feet and a minimum installation pressure of 2100 psi, or refusal, using a 9.62 square inch hydraulic ram.

Regards,

Daniel Stark, P.E. Stark Foundations From: Tonya Gunn

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PROJECT Foundation Underpinning 386 Carnelia Rd Angier, NC

Date: 19-Mar-21

Designed by: NDS

Project No.: 21.083.TBS

Design Criteria

Code(s):

International Building Code (IBC) 2015/2018 **ASCE 7-10**

Design Loads:

Dead:

Roof = 15

psf Chimney = 45 psf

Third Floor = 15 psf

Second Floor = 15 psf First Floor = 15 psf

Walls =

8" Foundation Wall = 100 psf

Soil = 110 psf

Live:

Roof (snow) = 25

First Floor = 40

Wind: (not applicable)

Exposure = C Wind Speed, V = 120

mph

Gust Effect Factor, G = 0.85 Internal Pressure Coefficient, GCpt = -0.18

External Pressure Coefficient, Cp = 0.8

Soil:

Allow Lateral Bearing Pressure = 200

Active Pressure =

psf/ft

psf/ft

psf

Third Floor = 40 psf

Second Floor = 40

Risk Category = 11

Design Load Combo = D + 0.6W

 $K_{z1} =$ 1.0

 $K_d = 0.85$ $K_z =$ 0.98

Height, hz = 30

Design Wind Pressure:

 $p_w = q_z (GCp - GC_{pi})$ where:

 $\omega = 0.6$

 $q_z = 0.00256 K_z K_{z1} K_d V^2$

Therefore:

 $q_z = 30.7$ psf

 $p_w = 26.4$ psf

Factored Wind Pressure, p'w = 15.8 psf (say 16 psf)

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PROJECT Foundation Underpinning 386 Camelia Rd Angier, NC

Date: 19-Mar-21

Project No.: 21.083.TBS

Designed by: NDS

Push Pier Design - Worst Case

Vertical Design Loads:

Tributary	Widths:

Roof =	11	ft	>	165	plf
Third Floor =	0	ft	>	0	plf
Second Floor =	4	ft	>	60	plf
First Floor =	4	ft	>	60	plf
Walls =	12	ft	>	600	plf
Foundation Wall (height) =	2	ft	>	200	plf
Soil (height) =	0.5	ft	>	55	plf
			ΣDL =	1140	plf

Live:

Roof (snow) =	11	ft	>	275	plf
Third Floor =	0	ft	>	0	plf
Second Floor =	4	ft	>	160	plf
First Floor =	4	ft	>	160	plf
			ΣLL =	595	plf

Max Pier Spacing or Trib = 6 ft

Pier Working Loads:

 $P_{DL} = 6840$ lbs $0.75^{\circ}P_{LL} = 2678$ lbs Working Load, $P_{TL} = 10000$ lbs Ultimate Load, Pult = 20000 lbs

Pier Design:

Pier Type: Push Pier

Bracket: PP21617-34	Bracket Cap =	29340	lbs	Therefore ok	Reference ICC report (attached)

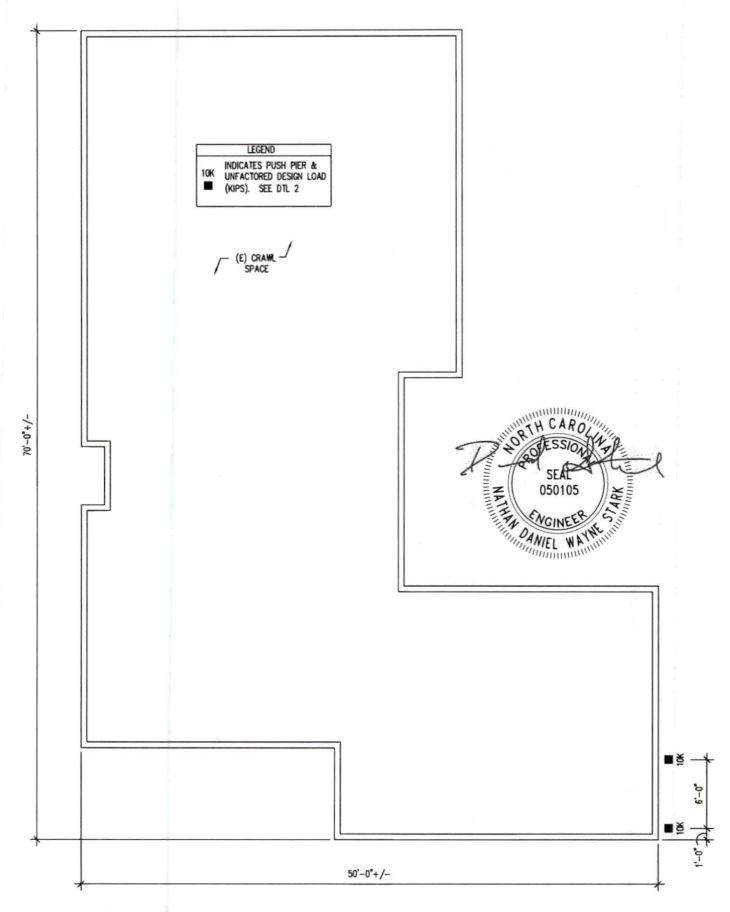
Shaft Diameter: 2.875"

Installation Pressure, P:

 $Q_{ult} = 2 (P_{TL})$ 20000 lbs $Q_{ult} = A_{cyl}$ (P) where $A_{cyl} =$ working area of the dual bore installation cylinder

 $A_{cyl} = 9.62 \text{ in}^2$

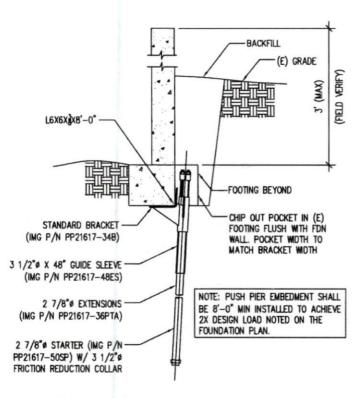
Therefore, PREQ = Quit / Acyl 2100 psi Fax: 13364649739



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2 7/8" PUSH PIER DETAIL