

# ENGINEERS PLANNERS CONSULTANTS

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March 28, 2019

Mr. Mike Hamm, PE State of North Carolina Department of Insurance Manufactured Building Division 322 Chapanoke Road Suite 200 Raleigh, NC 27603

RE: CMH Manufacturing, Inc. #958

Model: 3440 - NC

Dear Mr. Hamm,

Enclosed, you will find one (1) copy of the above mentioned project for your files.

Should you have any questions or comments, please contact me at your earliest convenience.

Sincerely,

# David Richter

David Richter Account Manager

**Enclosures** 







Date: 3/27/2019

**TYPE: MODULAR** 

# **MODEL PLAN INDEX**

Model #	3440	State
Manufacturer	CMH Manufacturing, Inc.	
Brand Name	CLAYTON	NC
Unit Size	29'-8" x76'-0"	INC
Description	4 BEDROOM / 2 BATH	

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SEE APPROVED M	ODULAR MANUAL FOR ;	
1. SECTIONS		2. TYPICAL DETAILS
3. REQUIRED CON	STRUCTION METHODS	4. MATERIALS

# **CMH**

Manufacturing, Inc. engineering department - modular

	REVISIONS					
DATE :	REVISION BY :	GCK				
March 27, 2019	REVISION DATE :					

# **TECHNICAL SHEET FOR LIGHT / VENT DATA**

MODEL NUMBER	3	440	
SIZE OF UNIT	29'-8" x76'-0"		
WINDOW SQ. FTG. STD.			
WINDOW SQ. FTG. W/ OPT.			
FIGURED FOR :	<b>CLAYTON WINDOWS</b>		
PERCENTAGE OF LIGHT REQ'D.	•	8%	
PERCENTAGE OF VENT REQ'D.		4%	

		Square	Footage			Percen	tage of		
		Inst	talled	Required		Insta	alled	Artifical	Artifical
Room	Area	Light	Vent	Light	Vent	Light	Vent	Light	Vent
LIVING ROOM	336.8	29.7	15.6	26.9	13.5	8.8%	4.6%		
MASTER BEDROOM	274.3	30.2	14.0	21.9	11.0	11.0%	5.1%		
BEDROOM 2	131.8	13.7	7.0	10.5	5.3	10.4%	5.3%		
BEDROOM 3	131.5	13.7	7.0	10.5	5.3	10.4%	5.3%		
BEDROOM 4	122.1	13.7	7.0	9.8	4.9	11.2%	5.7%		
DINING ROOM	83.1	39.6	20.8	6.6	3.3	47.7%	25.0%		
KITCHEN	228.8	19.2	9.8	18.3	9.2	8.4%	4.3%	YES	YES



# APPLICATION ENGINEERING FOR HEATING AND COOLING

CMH Mfg., Inc. 2225 South Holden Road Richfield, NC 27417-0386

Manufacturer's Model #: 3440

HVAC System Type: INFLOOR STRAIGHT ALUM. WITH PER REG - CMH DESIGN -

Prepared By LaSalle Air Systems 3/20/2019 {Method & Output © 2019}

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Calculations on this page are based on design standards set forth in ASHRAE and ACCA Manuals J Rev 8.2 and D Rev 1.1. System registers are located for best distribution based on Manual T. Design calculations are based on worst case orientation.

# ENTIRE HOUSE VALUES - DESIGN ZONE: NC, Region 4 NCECC (2018)/IECC (2015NC) 36N Latitude

 $\textbf{COOLING LOAD:} \qquad \qquad \textbf{34,180} \quad \text{Btuh for Outside Temp/Humidity of} \qquad \qquad 92 \quad ^{\circ} \text{ F ( 33 C)/ 48\% and Inside reduced to} \qquad 75 \quad ^{\circ} \quad \text{F ( 23 C)/ 50\%}$ 

HEATING LOAD: 37,742 Btuh based on outside temp of 16 ° F (-9 C) with inside temp raised to 72 ° F (22 C)

Crawlspace is not heated by the primary air handler.

Actual UA = 355.5 Max UA (Table R402.1.2) = 379.4

Use net wall area, not gross wall

# CONSTRUCTION DETAILS & U / SHGC VALUES: (22+Non-ins Rim - 15 - 38)

Total Cond. Floor Area:	2108.82 s.f.	TRUE Outside Perimeter:	211.33 ft	
Level 1 Ceiling: 108	to 108 in. Level	I 2 Ceiling: 0 to 0 in.	Level 3 Ceiling: 0 to 0 in.	Net Roof Area (less ducts): 2045.3 s.f.
Primary Wall Area:	1598.89 s.f. (Net)	Dark Roof(U): 0.027	FLOOR DUCTS (U):	0.0444 Duct TEL
Secondary Wall Area:	0.00 s.f. (Net)	Prim Wall (U): 0.070	ATTIC DUCTS (U):	0.125 468.2 ft
TOTAL Low-E window	195.33 s.f.	Sec Wall (U): 0.035	EXT. DUCTS (U):	0.125
TOTAL Patio Door	0.00 s.f.	Exp Floor(U): 0.044	INFLOOR DUCT AREA:	389.5 S.F. @ 51.2 TD/ 26.6 TD
TOTAL Shaded Window	50.00 s.f.	Low-E wi 0.350 / 0.28	ATTIC DUCT AREA:	86.434 S.F.(return) @ 96 TD/ 88.2 TD
TOTAL Skylite	0.00 s.f.	Patio Doc 0.330 / 0.27	EXT. DUCT AREA:	100.53 S.F. @ 96 TD/ 45 TD
TOTAL Door1 Area:	57.78 s.f.	Shaded V 0.350 / 0.05	PEOPLE: 5	4118.1 Btuh Total Appliances
TOTAL Door2 Area:	0.00 s.f.	Skylite 0.790 / 0.64	FIREPLACES:	0
All Glass % of Floor:	11.63 %	Door 1: 0.140	DUCT GAIN: @ Semi-Tight	2897 Btuh
All Glass % of Wall:	12.90 %	Door 2: 0.670	DUCT LOSS:	6719 Btuh
LATENT GAIN:	7757 Btuh		Summer Infiltr (7.5 mph):	36.9 cfm
Mech. Ventilation:	119.11 cl (56.2 L/s)	Altitude: 1000 ft	Winter Infiltration (15 mph):	69.6 cfm @ Semi-Tight

# **ROOM BY ROOM VALUES:**

98 deg

A/C Exiting:

49 deg

Heat Exiting Furnace:

825.7 FPM, max velocity in trunk #: 2 0.34 Max pressure at A/H

Actu	al heatin	g and cooling re	equired in each r	oom and	Cooling Air		Heating Air			
	flow set	to maximum o	either heating o	r cooling	Values for		Values for	50	12.5 KW	Maximum A/C capacity
		HEATING	COOLING	CFM	<b>3</b> to	on unit	90	% Gas/Oil	Elec	Calibrated Blower Test
ROOM NAME		LOSS (Btu)	GAIN (Btu)	DIST	CFM	Btuh	CFM	Btuh E	Btuh	Btuh (alt adj)
Kitchen	h	5,511	4,983	160	178	5,037	168	6,129	5,809	5,246
Utility	С	2,591	2,286	77	91	2,590	86	3,152	2,987	2,698
Bath #3	h	1,219	752	35	57	1,600	53	1,947	1,846	1,665
Hall	h	1,282	890	37	57	1,621	54	1,972	1,869	1,686
Bedroom #4	С	2,880	2,650	89	97	2,754	92	3,351	3,176	2,869
Bath #2	h	1,480	1,078	43	60	1,696	57	2,064	1,956	1,764
M. Bath	С	4,302	4,129	138	95	2,698	90	3,283	3,112	2,811
M. Bedroom	С	4,916	4,752	153	192	5,437	182	6,616	6,270	5,664
Bedroom #2	С	2,909	2,664	90	93	2,634	88	3,205	3,038	2,744
Bedroom #3	С	2,405	2,293	77	91	2,589	86	3,150	2,985	2,696
Living Room	С	5,007	4,847	156	154	4,362	146	5,308	5,031	4,542
Dining Room	С	3,240	2,856	96	93	2,633	88	3,204	3,036	2,743
TOTALS		37,742	34,180	1,150	1,259	35,650	1,191	43,380	41,115	37,127



# APPLICATION ENGINEERING DUCT AIR FLOW AND SIZING WORKSHEET (MANUAL D)

Manufacturer: CMH Mfg., Inc. Model #: 3440

2225 South Holden Road HVAC System Type: INFLOOR STRAIGHT ALUM. WITH PER REG - CMH DESIGN -

Richfield, NC 27417-0386 Design Zone: NC, Region 4 NCECC (2018)/IECC (2015NC)

Prepared by LaSalle Air Systems 3/20/2019 All rights reserved. This information proprietary to LaSalle Bristol Co. and CMH Mfg., Inc. Calculations include factors for duct air temperature change and pressure drops through ducts. All joints are tightly fitted or sealed.

Blower CFM	1296	@	0.8	E.S.F	<b>)</b> .	TEL=	519.2905		FR=	0.0944	(A/C	Coil inclu	ded)				
					Al	titude =	1,000 f	ft						User	Inpu	t	
BRANCH DUCT LISTING	ANALYSIS								Elec	(Altitud	e Adj.)					Final	Final
BR	Trunk	Metal	F. G.	Flex	Bends/	Total Eq.	Heat	Cool	Heat	Cool	Design	Round	Rect	angle S	Size	Round	Velocity
#	#	(ft)	(ft)	(ft)	Fittings(ft)	Length	Btuh	Btuh	cfm	cfm	cfm	Size	(i.d.)	х (	i.d.)	Size	fpm
1 Bath #3	4	33	0	17	406.7	456.7	1,219	752	42	26	42	4.74				5.0	307.1
2 Utility	4	33	0	18	397	448.0	2,591	2,286	89	79	89	6.31				6.0	453.3
3 Kitchen	4	33	0	18	417.2	468.2	2,731	2,469	94	85	94	6.55				6.0	477.6
4 Kitchen	4	33	0	18	407.2	458.2	2,781	2,514	96	87	96	6.54				6.0	486.4
5 Hall	5	24	0	18	407.7	449.7	1,282	890	44	31	44	4.82				5.0	322.8
6 Bedroom #4	5	24	0	18	377.1	419.1	2,880	2,650	99	91	99	6.42				6.0	503.7
7 Bath #2	5	24	0	17	387.4	428.4	1,480	1,078	51	37	51	5.04				5.0	372.9
8 M. Bath	5	24	0	25	379.8	428.8	4,302	4,129	148	142	148	7.57				6.0	752.6
9 Bedroom #3	6	32	0	27	389.3	448.3	2,405	2,293	83	79	83	6.14				6.0	420.7
10 Living Room	6	32	0	27	379.3	438.3	1,906	1,846	65	64	65	5.54				5.0	480.2
11 Living Room	6	32	0	27	369.3	428.3	3,100	3,002	106	103	106	6.67				6.0	542.3
12 Dining Room	6	32	0	29	379.4	440.4	3,240	2,856	111	98	111	6.88				6.0	566.7
13 Bedroom #2	7	34	0	27	379.2	440.2	2,909	2,664	100	92	100	6.57				6.0	508.8
14 M. Bedroom	7	34	0	27	369.2	430.2	2,433	2,351	84	81	84	6.07				6.0	425.5
15 M. Bedroom	7	34	0	27	359.2	420.2	2,484	2,401	85	83	85	6.06				6.0	434.5
N/A Other Rooms							-	-									
							37,742	34,180	1,296	 1,177	1,296						



TRUNK DUCT LISTING	ANALYSIS												
TRUNK#	1	32		90	122.0	37,742	34,180	1296	12.64	12	14	14.2	1111.1
TRUNK#	2		11	229.75	240.8	19,266	16,768	662	11.06			12.0	842.5
TRUNK#	3		21	232.75	253.8	18,476	17,412	635	11.03			12.0	808.0
TRUNK #	4	33		240.75	273.8	9,322	8,020	320	8.74	5	14	8.9	658.6
TRUNK#	5	24		240.75	264.8	9,944	8,747	342	8.88	5	14	8.9	702.6
TRUNK#	6	32		253.75	285.8	10,651	9,996	366	9.31	5	14	8.9	752.5
TRUNK#	7	34		253.75	287.8	7,825	7,416	269	8.28	5	14	8.9	552.9
TRUNK#	8					-	-	0		0	0		
TRUNK#	9					-	-	0		0	0		
TRUNK#	10					-	-	0		0	0		
TRUNK#	11					-	-	0		0	0		
TRUNK#	12					-	-	0		0	0		
TRUNK#	13					-	-	0		0	0		
TRUNK#	14		12			-	-	0					
TRUNK#	15		31			-	-	0					
LONGEST													
RETURN DUCT	Γ		31	20	51			1296	11.78	18	24	22.7	432.1

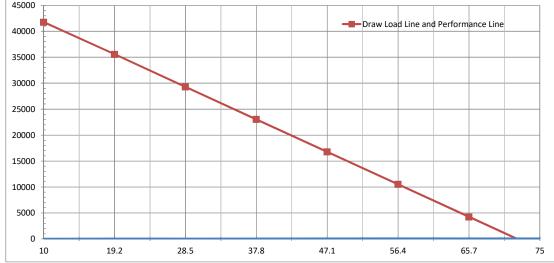
# APPLICATION ENGINEERING EQUIPMENT SELECTION AND SIZING WORKSHEET (MANUAL S)

Manufacturer: CMH Mfg., Inc. Mode

2225 South Holden Road HVAC System Type: INFLOOR STRAIGHT ALUM. WITH PER REG - CMH DESIGN -

Richfield, NC 27417-0386 Design Zone: NC, Region 4 NCECC (2018)/IECC (2015NC)

### Prepared by LaSalle Air Systems 3/20/2019 All rights reserved. This information proprietary to LaSalle Bristol Co. and CMH Mfg., Inc. RESULTS FROM MANUAL-J CALCULATIONS: Worst Case Orientation 16 ° HEATING LOAD: 37,742 Btuh at REQ'D BLOWER CFM: 1,259 cfm at altitude of 1,000 ft 92 ° Entering Air DRY Bulb: 76.6 $^{\circ}$ Mech. Ventilation : 119 SENSIBLE CLG LOAD: 26,423 Btuh at Entering Air WET Bulb: 61.4 $^{\circ}$ **7,757** Btuh at 92 $^{\circ}$ LATENT CLG LOAD: Entering Air RH: 53 % GRAINS DIFFERENCE: Outside wet bulb: 72.0 $^{\circ}$ outside RH: 48.2 % FILL IN BLANKS IN EACH SECTION FROM THE H.V.A.C. EQUIPMENT DATA CHARTS: (Do not use ARI Ratings!) Air handler model #:\_\_\_\_\_ Condenser model #:\_\_\_\_\_ Blower Data Select blower speed in COOLING mode: \_\_\_ Blower CFM is between 1102 > < 1491 for Total (External) Static Pressure between 0.7> <0.9 Electric, Gas or Oil Furnace Select blower speed in HEATING mode: \_\_\_\_\_ Output Btuh is between 39629>\_\_\_\_\_<52838 Blower CFM is between 684 >\_\_\_\_< 808 for Temp. rise of 55-65 Blower CFM is between 808 >\_\_\_\_\_< 988 for Temp. rise of 45-55 APPROVED BY Blower CFM is between 988 >\_\_\_\_\_< 1271 for Temp. rise of 35-45 S/T Ratio = 0.77 Leaving Temp = 50.4 $^{\circ}$ Cooling Equipment TD = 24.6 $^{\circ}$ At 92F outside, Total A/C output from 34863 btuh \_\_\_\_\_\_ to 39306 btuh is GOOD. At 92F outside, Total A/C output from 39306 btuh \_\_\_\_\_\_ to 41015 btuh is MARGINAL. Approval of this document does not authorize approve any deviation or deviations from the requirements of applicable State Laws Sensible Capacity is from 22544 btuh \_\_\_\_\_ to 30301 btuh David Richter Latent Capacity is from 7601 btuh \_\_\_\_\_ to 11635 btuh Dry bulb increases by: 1.5 $\,^\circ$ Wet bulb increases by: 0.9 $\,^\circ$ Mechanical Ventilation is 9.1 % of blower cfm. Heat Pump with Supplemental Heating Coils Data from performace charts Data from load calculation \_\_\_\_ btuh at \_\_\_\_\_ F outside 0 btub at 72 Foutside \_\_\_\_\_ btuh at \_\_\_\_\_ F outside 37.742 btuh at 16 F outside 45000 Draw Load Line and Performance Line 40000 35000 30000



At winter design temperature of 16 F outside, the distance between the lines is \_\_\_\_\_\_ btuh which is the Supplemental Heat divided by 3400 = \_\_\_\_\_ KW.

# APPLICATION ENGINEERING INTERNATIONAL MECHANICAL CODE - Chapter 4 Ventilation Worsheet

Manufacturer: CMH Mfg., Inc. Model #: 3440

2225 South Holden Road HVAC System Type: INFLOOR STRAIGHT ALUM. WITH PER REG - CMH DESIGN

Richfield, NC 27417-0386 Design Zone: NC, Region 4 NCECC (2018)/IECC (2015NC)

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# RESULTS FROM MANUAL-J CALCULATIONS: Worst Case Orientation

**HEATING LOAD:** 37,742 Btuh at 16  $^{\circ}$  **REQ'D BLOWER CFM:** 1,259 cfm at altitude of 1000 ft

92 ° 76.6 ° Mech. Ventilation : SENSIBLE CLG LOAD: 26,423 Btuh at Entering Air DRY Bulb: 119 92 ° 61.4 ° LATENT CLG LOAD: 7,757 Btuh at Entering Air WET Bulb: Entering Air RH: **GRAINS DIFFERENCE:** Outside wet bulb: 72.0 ° outside RH:

Natural or Mechanical: Test the infiltration at 50 Pa should result in 607.0 CFM infiltration being 1.919 ACH (to be confirmed by testing)

(5 ACH = 1581 CFM) (3 ACH = 948 CFM) **Mechanical ventilation is required** 

# To Meet Natural Ventilation: Increase Openable Area by 160 %

		Openal	ole Area			Opena	ible Area
ROOM NAME	Room Area	Required	Built	ROOM NAME	Room Area	Require	Built
Kitchen	234.9	9.3	12.00	Bedroom #3	154.5	6.1	8.33
Utility	186.7	7.4	6.25	Living Room	283.1	11.3	18.75
Bath #3	84.1	3.3	0.00	Dining Room	90.3	3.6	0.00
Hall	65.5	2.6	0.00		0.0	0.0	0.00
Bedroom #4	210.1	8.4	8.33		0.0	0.0	0.00
Bath #2	101.4	4.0	1.00		0.0	0.0	0.00
M. Bath	244.8	9.7	15.00		0.0	0.0	0.00
M. Bedroom	258.3	10.3	19.67		0.0	0.0	0.00
Bedroom #2	195.3	7.8	8.33		0.0	0.0	0.00
				TOTAL	2108.8	83.8	97.67

# Mechanical Ventilation Is Required In These Areas To Meet IMC 2012/2015 Per Table 403.3.1.1:

			Outdoor	Exhaust		Air
SPACE CLASSIFICATIONS	Occupancy	Area	Air	Air	ZONE AIR DISTRIBUTION	Flow
Private Living Area	5.1	1443.8	119.1	0.0	Floor Supply of Warm Air/Floor Return	869.4
Private Kitchen	0.0	234.9	0.0	25.0	Floor Supply of Warm Air/Floor Return	177.9
Private Baths	0.0	430.2	0.0	105.0	Floor Supply of Warm Air/Floor Return	211.7
	0.0	0.0	0.0	0.0		0
	0.0	0.0	0.0	0.0		0
	0.0	0.0	0.0	0.0		0
Total	5.1	2,108.8	119.1	130.0		1,259
					System Ventitlation Efficiency	y: 1



# ELECTRICAL FEEDER CALCULATION

# CMH Manufacturing, Inc. engineering department - modular PAGE: 1 of 1 DATE: 27-Mar-19 BY: GCK

MODEL NO	2440	
MODEL NO.	3440	Per NEC 220-30

1. LIGHTING LOAD:											
1st floor			2nd floor								
length =	76.00	FT.	length =	0.00	FT.						
width =	29.67	FT.	width =	0.00	FT.						
		-	•	-	-						
Total area =	2254	SQ. FT.	Minimum number	4							
X	3	VA	of 15 Amp circuits =	4							
TOTAL	6762	VA									

2. SMALL AP	PLIANCE	LOAD:	3. LAUNDRY LOAD:					
Number of	3		Number of	1				
circuits			circuits					
X	1500	VA	X	1500	VA			
TOTAL	4500	VA	TOTAL	1500	VA			

4. APPLIANCE LOAD:		
Electric Range =	12100	VA
Electric Water Heater =	5000	VA
Electric Clothes Dryer =	5600	VA
Cooktop =	0	VA
Wall Oven =	0	VA
Freezer =	1200	VA
Dishwasher & Disposal =	2376	VA
Gas furnace motor =	0	VA
Micro-wave oven	1200	VA

5. TOTAL OF OTHER I	5. TOTAL OF OTHER LOADS (1, 2 & 3)									
	LEG A									
Lighting load =	6762									
Small appliance load =	4500									
Laundry =	1500									
Appliance load =	27476									
Sub-Total =	40238									
10000 VA @ 100% =	10000									
Remainder @ 40% =	12095									
Total =	22095	VA								
	92.06	AMPS								



6. HVAC LOA	6. HVAC LOAD:											
Lineal feet of bas	0											
Number of baset	0											
Total baseboard	heater load	=		0.0	Amps							
Use 65% w/ less than 4 or 40% w/ 4 or more circuits (*)												
Electric furnace (	@ 65% (*)											
Circuit 1 =	40	Amps		26.00	Amps							
Circuit 2 =	30	Amps		19.50	Amps							
Air conditioner (*	Air conditioner (*)											
Total HVAC load	Total HVAC load (*- Use largest of these only) = 45.50 Amps											
		-										

7. TOTAL OF ALL LOADS =

FURN SIZE 12KW

137.56 Amps

### DOOR AND WINDOW SCHEDULE NOTE: FLOOR PLAN WINDOW SIZES WITH AN "SG" DESIGNATION REPRESENTS SAFETY GLAZING REQUIRED PER IRC SECTION R308.4 **SIZES** ROUGH OPENING LIGHT (@8%) VENT (@4%) 14 X 40 WDW. 14 1/4" X 40 1/4" 2.50 1.30 24 X35 WDW. 24 1/4" X 35 1/4" 4.10 2.10 24 X54 WDW. 24 1/4" X 54 1/4" 6.80 3.50 30 X 60 WDW. 30 1/4" X 60 1/4" 9.90 5.20 36 1/4" X 35 1/4" 6.60 3.40 36 X 35 WDW. 36 X 54 WDW. 36 1/4" X 54 1/4" 10.80 5.60 36 X 60 WDW. 36 1/4" X 60 1/4" 12.20 6.20 7.70 36 X 72 WDW. 36 1/4" X 72 1/4" 14.90 36 X 08 WDW. 36 1/4" X 08 1/4" 0.50 0.00 36 x 12 WDW. 36 1/4" X 12 1/4" 1.10 0.00 64 1/4" X 35 1/4" 11.50 2.60 64 x 35 WDW. 58 x 35 WDW. 58 1/4" X 35 1/4" 2.20 10.10 **DOORS** 2-8 X 6-8 DOOR 35 1/2" X 80" 3-0 X 6-8 DOOR 38" X 80" PATIO DOOR 72" X 80" 16.8 33.6 ATRIUM DOOR 75 3/8" X 82 1/2" 21.15 17.3 FASTENING REQUIREMENTS: FOR DOORS AND WINDOWS, USE EITHER # 8 X 1" SCREWS, 7/16" X 1 1/2" X 16 GA. STAPLES, OR .092 X 2 1/4" PD NAILS, AT 12" ON CENTER MAXIMUM. DESIGN CRITERIA **CLASSIFICATION:** FLOOR LIVE LOAD = 40 PSF USE GROUP = R - GROUND SNOW LOAD = 30 PSF **R3 RESIDENTIAL (NON-TRANSIENT)**

# - SEISMIC DESIGN CATEGORY "C" - SOIL PROFILE CATEGORY "C" - WIND EXPOSURE - 'C' -ROOF MEAN HT 22'-0" DESIGN WIND SPEED = 90 MPH 100 MPH 120 MPH ULITMATE WIND SPEED = 117 MPH 130 MPH 152 MPH

**CONSTRUCTION TYPE IS V-B** 

(UNPROTECTED)

### ATTENTION LOCAL INSPECTION DEPARTMENT

SET-UP INSTRUCTIONS FOR THIS MODULAR UNIT ARE INCLUDED BY ATTACHMENT TO THESE PLANS. ANY PLAN SET WHICH DOES NOT INCLUDE AN ATTACHMENT ENTITLED "SET UP MANUAL" IS INCOMPLETE SET- UP INSTRUCTIONS

SEE SETUP MANUAL SENT WITH HOME

- ATTIC LIVE LOAD = 10 PSF

# REQUIREMENTS FOR FIRESTOPPING

INSTALLATION OF NON- COMBUSTIBLE MATERIALS AROUND ALL OPENINGS THAT ARE VERTICAL PENETRATIONS IN THE FLR. AND CLG. ATTENTION LOCAL INSPECTION DEPARTMENT

THE FOLLOWING ITEMS LISTED HAVE NOT BEEN COMPLETED BY CMH MFG, Inc., HAVE NOT BEEN INSPECTED BY NTA, INC AND ARE NOT CERTIFIED BY THE STATE OF NORTH CAROLINA MODULAR LABEL. CODE COMPLIANCES MUST BE DETERMINED BY THE LOCAL JURISDICTION FOR THE FOLLOWING:

- HVAC SYSTEM (SITE INSTALLATION AND CONNECTIONS)
-THIS UNIT MUST BE CONNECTED TO A PUBLIC WATER SUPPLY AND SEWER SYSTEM, IF THESE ARE AVAILABLE.

# CODE COMPLIANCE

ALL PLANS MEET OR EXCEED THE FOLLOWING:

# North Carolina State Building Code Compliance:

- NC Residential Code 2018 Edition
- NC Electrical Code 2017



# RIDGE BEAMS-SIZES AND MAX. SPAN CHART RIDGE BM. CHART-SEE MATING WALL PG. RC-60.0 FOR MAX. CALCULATIONS-SEE MATING WALL PGS. CRC SECTION

Soffitt materials for this unit assume that the building face will be 10 feet or greater from the property line when installed on site. Where the building face is less than 10 feet from the property line, underlayment materials and ventilation in accordance with Section R302.1.1,NC Residential Code, must be provided and installed at the site and inspected by the local jurisdiction

### THERMAL ZONE REQUIREMENT

-THIS BUILDING DESIGN COMPLIES WITH OR EXCEEDS MINIMUM REQUIREMENTS FOR NORTH CAROLINA THERMAL ZONE 5
-MODEL IS DESIGNED TO MEET THERMAL ZONE 5 AND BELOW PER TABLE N1101.2 REFERENCED IN THE NORTH CAROLINA RESIDENTIAL CODE, 2018 EDITION FOR ONE & TWO FAMILY DWELLINGS. RESCHECK ANALYSIS AND COMPLIANCE REPORT FOR THERMAL ZONE CALCULATION IS PROVIDED FOR EACH SPECIFIC MODEL AND IS ATTACHED IN THE SUBMITTED MODEL APPROVAL PACKAGE.

BTUS PER HVAC CALCS

**FURNANCE SIZE PER HVAC CALCS** 

# **INSULATION PACKAGES**

PRESCRIPTIVE

# MODULAR MANUAL REFERENCES

ITEMS BELOW ARE REFERENCED FOR NON PRESCRIPTIVE USE

# **FLOOR: ON FRAME CONSTRUCTION**

<u>DETAILS</u> - SECTIONS ON FLOORS FOR ON FRAME: FL-500

**CALCULATIONS** - SEE CFL SECTION

# FLOOR: OFF FRAME CONSTRUCTION

**DETAILS - SECTIONS ON FLOORS FOR OFF FRAME: FL - 100** 

# **MARRIAGE WALLS - 2x CONSTRUCTION**

<u>DETAILS</u> - MW-20.0, MW-30.0, MW-40.0 CALCULATIONS - SEE CMW SECTION

# **ELECTRICAL APPLIANCES AND LOADS**

ELECTRICAL - SEE PAGES PLN-1.0 for WH & PLN-1.5 for FURN CALCULATION - SEE TECHNICAL SHEET ATTACHED FOR MODEL SPECIFIC ELECTRICAL PANEL LOAD CALC FOR

200 AMP SERVICE

# ANCHORAGE REQUIREMENTS

**FOUNDATION SECTIONS FOR PERIMETER ON FRAME:** 

PER SETUP MANUAL

**FOUNDATION SECTIONS FOR PIER SET ON-FRAME:** 

PER SETUP MANUAL

**FOUNDATION SECTIONS FOR PERIMETER OFF FRAME:** 

PER SETUP MANUAL

TRUSSES - DETAILS / CALCULATIONS

PER TRUSS PRINTS

# **PLUMBING FIXTURES**

SEE PAGE PLN - 1.8

ALL MODELS ARE AVAILABLE WITH FLOOR PLAN REVERSED FROM LEFT TO RIGHT AND / OR FRONT TO BACK.

### MARRIAGE WALL COLUMNS SPAN CHART

DETAIL - SEE MATING WALL COLUMNS (PAGE MW-20.0)

**CALCULATIONS - SEE CMW SECTION** 

# INSTRUCTIONS ON FILLING OUT PLAN SET BEFORE CONSTRUCTION

YOU MUST CHECK THE APPROPREATE BOX OF WHAT THE STRUCTURE IS TO BE BUILT TO BEFORE PRODUCTION BEGINS. THE MARK SET MUST ACCOMPANY THE UNIT THROUGH THE PRODUCTION PROCESS.

# EXTERIOR SIDEWALL HEADERS - SIZES AND MAXIMUM SPAN CHART

HEADER CHART - SEE EXTERIOR WALL PAGE EW - 20.0
CALCULATIONS - CEW SECTION

# ATTENTION LOCAL INSPECTION DEPARTMENT:

IF THIS STRUCTURE IS IN A THERMAL ZONE MORE STRINGENT THAN THAT LISTED ON THESE PLANS, IS SET ON PILINGS, OR IS INSTALLED AT A MOUNTAIN REGION OR COASTAL HIGH HAZARD SITE SUCH THAT WIND OR OTHER DESIGN PARAMETERS ARE INCREASED, THE DESIGN MUST BE DETERMINED TO BE ADEQUATE FOR ACTUAL SITE CONDITIONS. ALTERATIONS MAY BE REQUIRED TO BRING THE HOME INTO COMPLIANCE WITH THE MORE STRINGENT CONDITIONS.

"Service entrance conductors routed from their point of entrance into the structure, to their point of attachment to the service enclosure a distance horiontally not more than twice the nominal width of the service enclosure and vertically not more than the greater of 5 feet or twice the nominal height of the sevice enclosure shall be considered to be in compliance with the requirements of 230-70(a) of the current National Electrical Code. Service entrance conductors may be routed in the most direct route or at right angles. Service entrance conductors in excess of these specified limits will not be allowed unless specifically authorized by special permission from the electrical inspector having jurisdiction to accommodate adverse site conditions which would not reasonably allow installation within this criteria."

This home is NOT designed for placement in Coastal High Hazard Areas or Ocean Hazard Areas.

**CMH** Manufacturing, Inc. REVISIONS BY DATE ALL MODULAR MODELS

COVER SHEET 1-0

# NC(New)

### TYPICAL FASTENING SCHEDULE

REFERENCE 'CFL' - FLOOR CONSTRUCTION CALCULATIONS OF THE MANUAL FLOOR FASTENING

RIM JOIST TO JOIST PER FL-110 OR FL-510.0 IN APPROVED MANUAL

PER FL-100.0 IN APPROVED MANUAL FLOOR BLOCKING TO JOIST

MULTIPLE JOIST .131 x 3" NAILS @ 10" O.C., W/ GLUE 80%

DECKING TO FLOOR FRAMING PER FL-10 IN APPROVED MANUAL

### EXTERIOR WALL FASTENING

REFERENCE 'CEW' - EXTERIOR WALL CONSTRUCTION CALCULATIONS OF THE MANUAL

LOWER TOP PLATE & BOTTOM PLATE TO STUD PER EW-25 IN APPROVED MANUAL DOUBLE STUDS 7/16" x 2-1/2" x 15 GA. STAPLES @ 6" O.C.

DOUBLE TOP PLATES PER EW-1 IN APPROVED MANUAL

HEADER TO STUDS PER EW-20 CHARTS IN APPROVED MANUAL PER EW-20 IN APPROVED MANUAL HEADER COMPONENTS PER EW-20 IN APPROVED MANUAL STUDS TO SILLS

PER THE MANUFACTURER'S SPECIFICATIONS EXTERIOR SIDING

BOTTOM PLATE TO FLOOR PER EW-31 IN APPROVED MANUAL

SIDEWALL TO ENDWALL PER EW-30 FOR NON-SHEARWALL OR PER SW-40 FOR SHEARWALL OR PER EW-0.0 IN APPROVED MANUAL WALL WALL TO WALL TOP PLATES 3" x 6" x .036" (20 GA.) GALVANIZED STEEL PLATE W/ (6) .131 x 3" NAILS AT EACH SIDE AT EACH

FOR APA RATED SHEATHING; 7/16" X 1-3/4" x 15 GA. STAPLES AT 6" O.C. AT ALL EDGES & 12" O.C. FIELD. FOR EXTERIOR WALL SHEATHING

COMPOSITE WALLS, FASTEN PER EW-40. FOR SHEARWALL FASTEN PER SW-40 OR ATTACHED PAGES (IF ATTACHED). ALL

OTHER SHEATHING EASTENED PER

MANUFACTURER'S INSTALLATION INSTRUCTIONS.

### MATING WALL FASTENING

REFERENCE 'CMW' - MARRIAGE WALL CALCULATIONS OF THE MANUAL

LOWER TOP PLATE TO STUD PER MW-40 IN APPROVED MANUAL BOTTOM PLATE TO STUD PER MW-40 IN APPROVED MANUAL

 $7/16" \times 2-1/2" \times 15$  GA. STAPLES OR .131  $\times$  3" NAILS @ 16" O.C. TO EACH MEMBER MULTIPLE STUDS

PER MW-20 IN APPROVED MANUAL STANDARD COLUMN DOUBLE TOP PLATES PER MW-40 IN APPROVED MANUAL BOTTOM PLATE TO FLOOR PER MW-31 IN APPROVED MANUAL MATING WALL TO ENDWALL PER EW-30 IN APPROVED MANUAL

 $3" \times 6" \times .036"$  (20 GA.) GALVANIZED STEEL PLATE W/ (6) .131  $\times 3"$  NAILS AT EACH SIDE AT EACH WALL OR OVERLAPPED WALL TO WALL TOP PLATES

PLATE PER EW-0.

### INTERIOR WALL FASTENING

BOTTOM PLATE TO STUDS PER PT-40 IN APPROVED MANUAL TOP PLATE TO STUD PER PT-40 IN APPROVED MANUAL

DOUBLE STUDS 7/16" x 2-1/2" x 16 GA. STAPLES @ 16" O.C.

FLAT HEADER TO STUDS PER PT-20 IN APPROVED MANUAL WALL TO FLOOR PER PT-40 IN APPROVED MANUAL PER PT-30 IN APPROVED MANUAL WALL TO WALL TOP PLATE TO ROOF SYSTEM PER PT-40 IN APPROVED MANUAL

GYPSUM TO WALL FRAMING PER THE RESIDENTIAL BUILDING CODE TABLES

**ROOF FASTENING** REFERENCE 'CRC' - ROOF CONSTRUCTION CALCULATIONS OF THE MANUAL

CEILING BOARD TO TRUSS FOAM-SEAL 2100 SPRAY ADHESIVE PER THE MANUFACTURER'S SPECIFICATIONS

BLOCKING TO TRUSS (2) 7/16" x 2-1/2" x 15 GA. STAPLES DIRECT

TRUSS TO SIDEWALL TOP PLATE PER RC-30 IN APPROVED MANUAL TRUSS TO RIDGE BEAM PER RC-65 IN APPROVED MANUAL

TRUSS TO EDGE RAIL PER MW-31 CHARTS IN APPROVED MANUAL EDGE RAIL TO MATING WALL PER MW-31 CHARTS IN APPROVED MANUAL

PER SW-40 IN APPROVED MANUAL FOR SHEARWALLS AND RC-33.0 FOR NON-SHEARWALLS TRUSS TO ENDWALL TOP PLATE

ROOF DECKING TO TRUSS PER SW20.0 THRU SW-389E.2 (IF NOT ATTACHED) IN APPROVED MANUAL

SHINGLE TO ROOF DECKING PER THE MANUFACTURER'S OR ARMA SPECIFICATIONS

OUTLOOKER TO TRUSS PER RC-70 IN APPROVED MANUAL

REFERENCE INSTALLATION PAGES PROVIDED IN EACH APPROVAL. INSTALLATION FASTENING



- CS1) 7/16" APA RATED ROOF DECKING 24/16 SPAN RATING.
- CS2 15# MIN. ROOF UNDERLAYMENT; SINGLE LAYER w/ GREATER THAN 4:12 ROOF PITCH; DOUBLE LAYER w/ 4:12 OR LESS
- CS3 MIN. 20 YEAR SHINGLES.
- (CS4) 1 1/2" WIDE ENGINEERED WOOD BEAM, EACH HALF IN OPEN SPÁN AREAS GREATER THAN 48".
- CS5 ENGINEERED WOOD TRUSSES: COMPONENTS & SPACING PER TRUSS PRINT
  - \* FOR CONNECTION AND SET-UP OF ROOF: SEE MODULAR SET-UP PAGES ATTACHED TO APPROVAL
- (CS6) CEILING INSULATION, BLOWN OR BATT.
- (CS7) CONTINUOUS VENTED SOFFIT.
- CS8 DOUBLE 2x4 TOP PLATE (MIN.)
- CS9 2x4 STUDS @ 16" O.C. STUD GRADE SPF (MIN.).
- (CS10) WALL INSULATION (BATT)
- CS11) 3/8" OSB SHEATHING WITH WATER RESISTIVE BARRIER BÉLOW ALL EXT. FINISH MATERIAL CORROSION-RESISTANT FLASHING REQUIRED AT ALL LOCATIONS AS SHOWN ON APPROVED MANUAL DETAILS
- (CS12) SINGLE 2x4 BOTTOM PLATE SPF #3 (MIN.).

2 - A VAPOR RETARDER HAVING A MAXIMUM 0.05 PERM

- CS13) 3/8" (MIN.) GYPSUM WALL BOARD
- (CS14) FLOOR INSULATION (BATT.)
- CS15 MIN. 19/32" RATED DECKING 16" O.C. OR 32/16 SPAN RATING.

# (CS16a)

### <u>Duct Insulation:</u> 1 - Min R-8

IN ACCPRDANCE WITH ASTM E96, OR ALUMINUM FOILI WITH A MINIMUM THICHNESS OF 2 MILLS, SHALL BE INSTRALLED ON THE EXTERIOR OF THE INSULATION ON THE COOLING SUPPLY DUCT THAT PASS THROUGH UNCONDITIONED SPACE CONDUCIVE TO CONDENSATION EXCEPT WHERE THE INSULATION IS SPRAY POLYURETHANCE FOAM WITH A MAXIMUM WATER VAPOR PERMEANCE OF 3 PERM PER INCH AT THE INSTALLED THICKNESS. CS4 (CS25) FLAT (CS22 ALTERNATE ₩ W/ FLAT IØ CÉILING CS19 (CS8) (CS11) (CS13) (CS9) (CS29) (CS12) CS23 (CS14)

CS16 MAIN HEAT DUCT. (MAY BE SITE INSTALLED BY OTHERS)

(CS25) WEDGE SUPPORT AT CATHEDRAL CEILING, EACH END OF TRUSS.

CS29 LAP BOARD, WOOD OR VINYL SIDING, HARDI SIDING, OR EXPOSED

SHEATHING FOR ON SITE EXTERIOR FINISH INSTALLATION.

CS19 2x4 (MIN.) MARRIAGE WALL STUDS @ 16" O.C.

(S20) LISTED BOTTOM BOARD, WHERE OCCURS.

(CS21) 1/2" SHIM FOR COMPRESSION STRIP.

(CS24) 1/2" (MIN.) GYPSUM BOARD CEILING.

CS27 CONTINUOUS 2x3 SPF #3 MINIMUM FOR

(CS28) 2x FULL DEPTH BLOCKING 24" O.C. (2)

TRUSS TOP RAIL FOR RIDGE CONNECTION

JOIST BAY MIN. ENDWALL LOCATION ONLY

CS22 DOUBLE 2x4 (MIN.) TOP PLATE.

CS23 2x4 (MIN.) BOTTOM PLATE.

(CS17) OFF FRAME PER FL-110.0

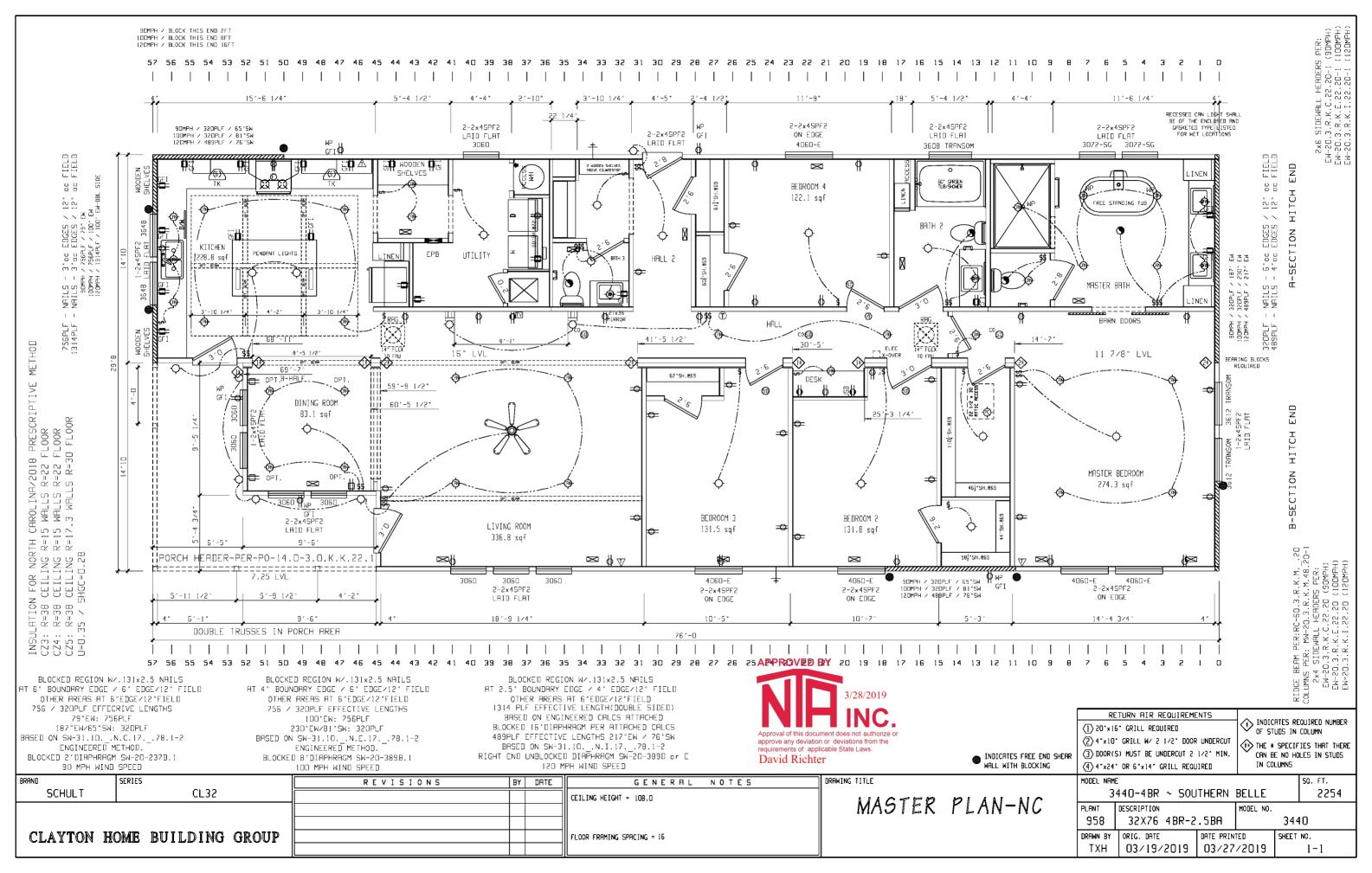
(CS18) OFF FRAME PER FL-110.0

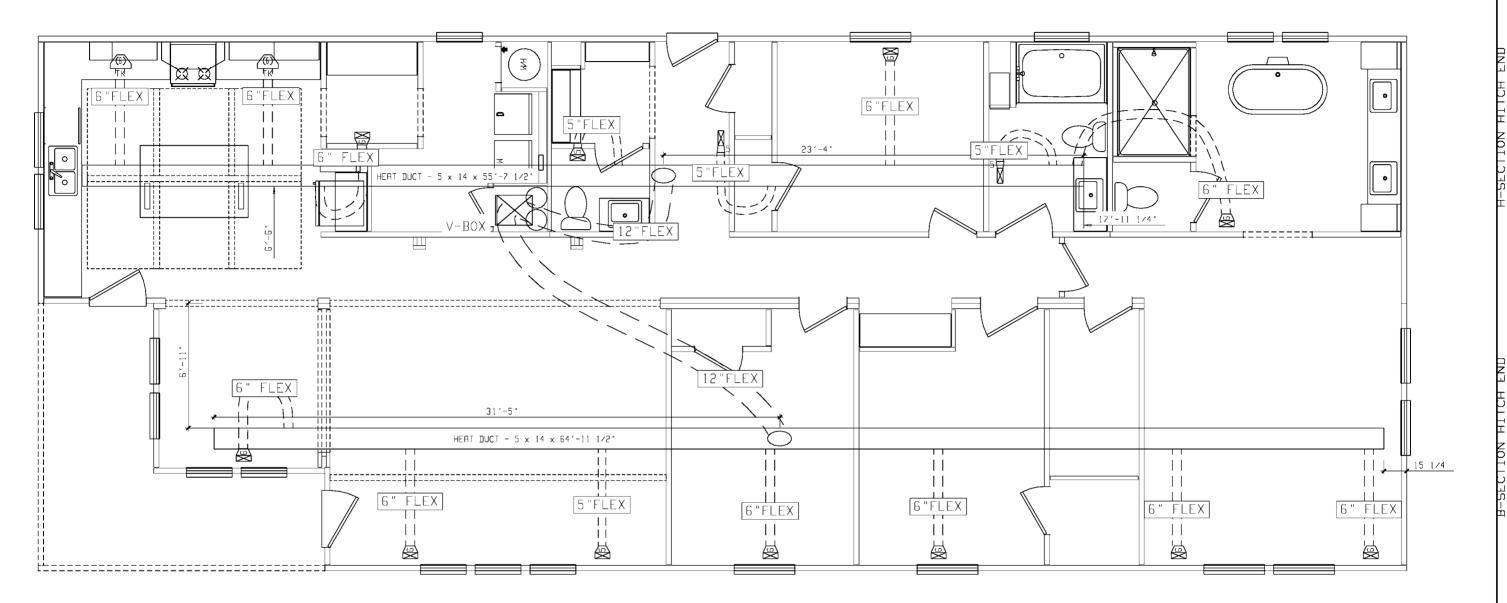
MANUFACTURING INC

(CS18)

TYPICAL CROSS SECTION & **FASTENING SCHEDULE** 

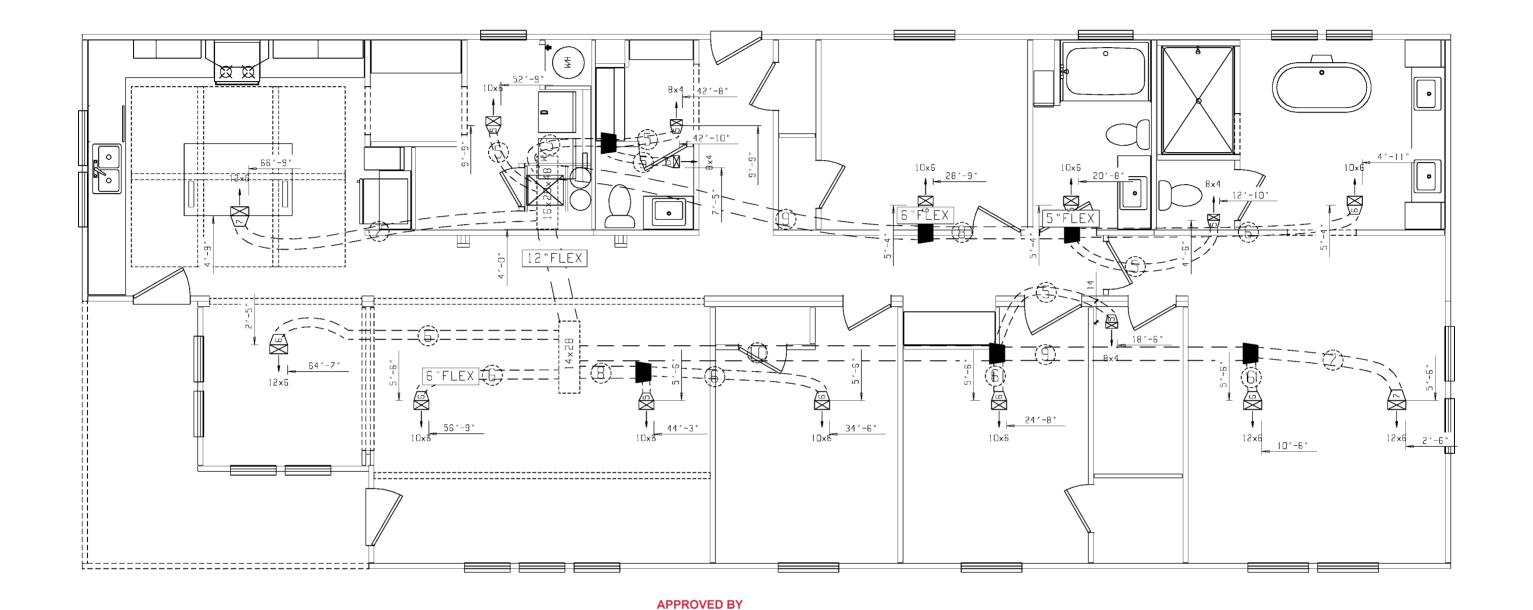
(CS18) BRAND: MODEL NO .: NC/SC/DE MODULAR DESCRIPTION: PLANT: #958 OFF FRAME HINGED ROOF DRAWN BY DATE DRAWN: DATE PRINTED: SHEET: 10-21-15 4 - 25 - 17







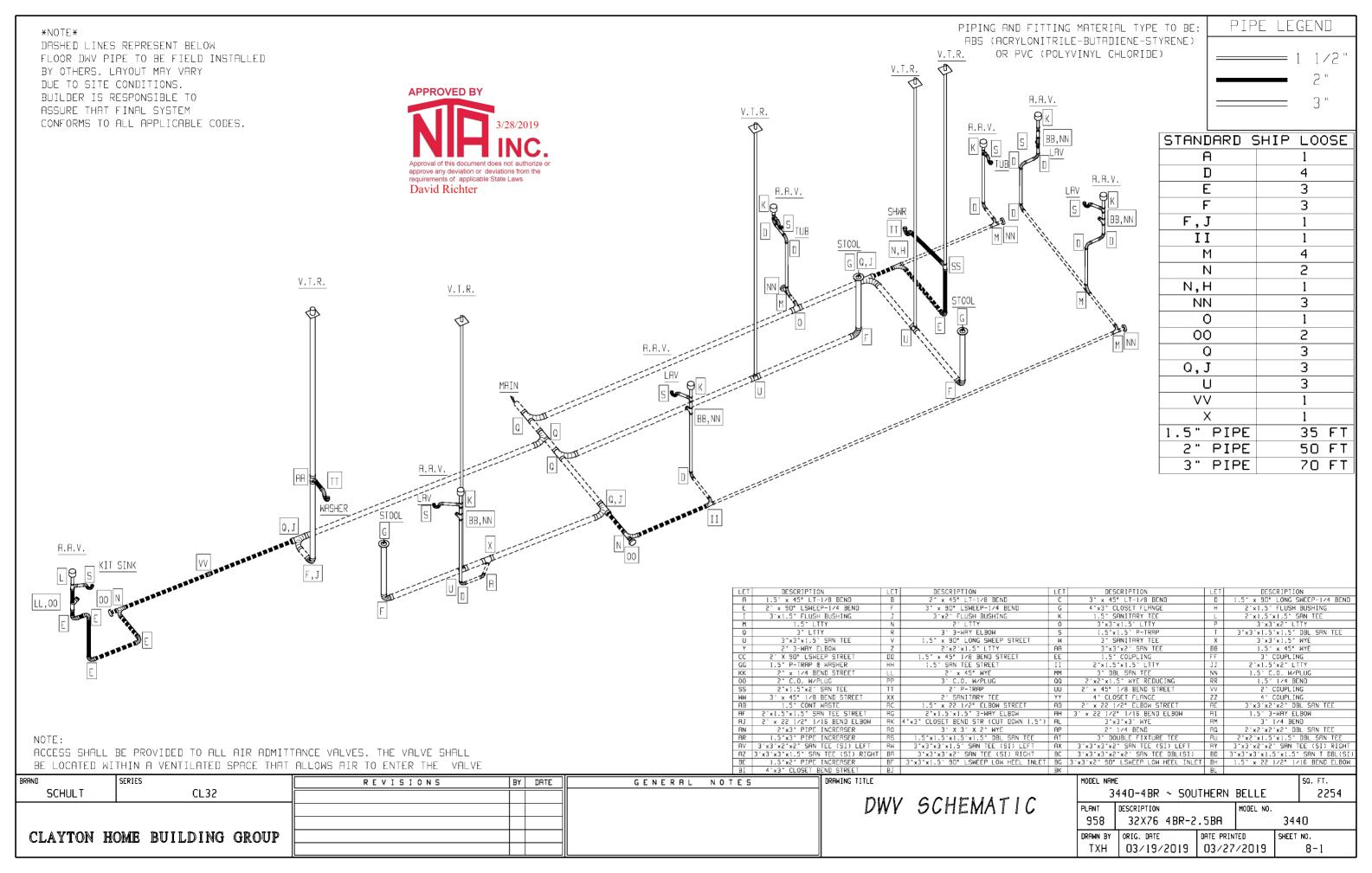
BRAND	SERIES	REVISIONS	BY DATE	GENERAL NOTES	DRAWING TITLE	MODEL NAME		SQ. FT.
SCHULT	CL32				DEBILIETED I AAD III/A/	3440-4BR ~ SOU	THERN BELLE	2254
					PERIMETER LOOP HVAC		MODEL NO.	
						958 32X76 4BR-2.	.5BA	3440
CLAYTON H	NOME BUILDING GROUP							SHEET NO.
					]	TXH 03/19/2019	03/27/2019	4-4

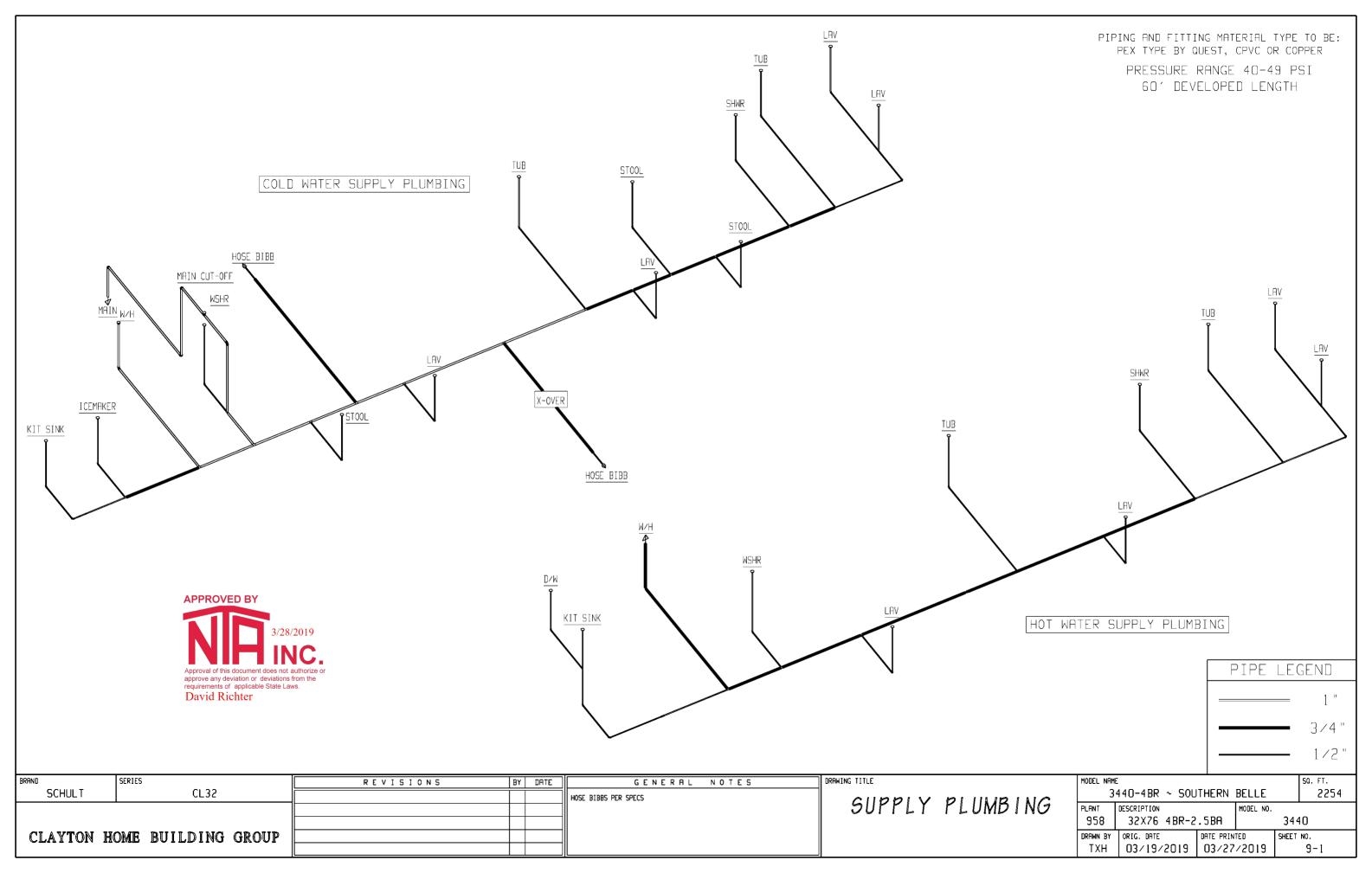


SERIES MODEL NAME REVISIONS BY DATE GENERAL NOTES CL32 3440-4BR ~ SOUTHERN BELLE 2254 SCHULT MIXER ABOVE FURNACE: 16"x20"x48" OVERHEAD HVAC DESCRIPTION MODEL NO. MIXER ON NON-FURNACE HALF WITH 14" CROSSOVER: 16"x20"x24" 958 32X76 4BR-2.5BA 3440 MIXER ON NON-FURNACE HALF WITH 12" CROSSOVER: 14"x16"x28" CLAYTON HOME BUILDING GROUP ORIG. DATE ORIG. DATE DATE PRINTED 03/19/2019 03/27/2019 SHEET NO. 4-5

approve any deviation or deviations from the requirements of applicable State Laws.

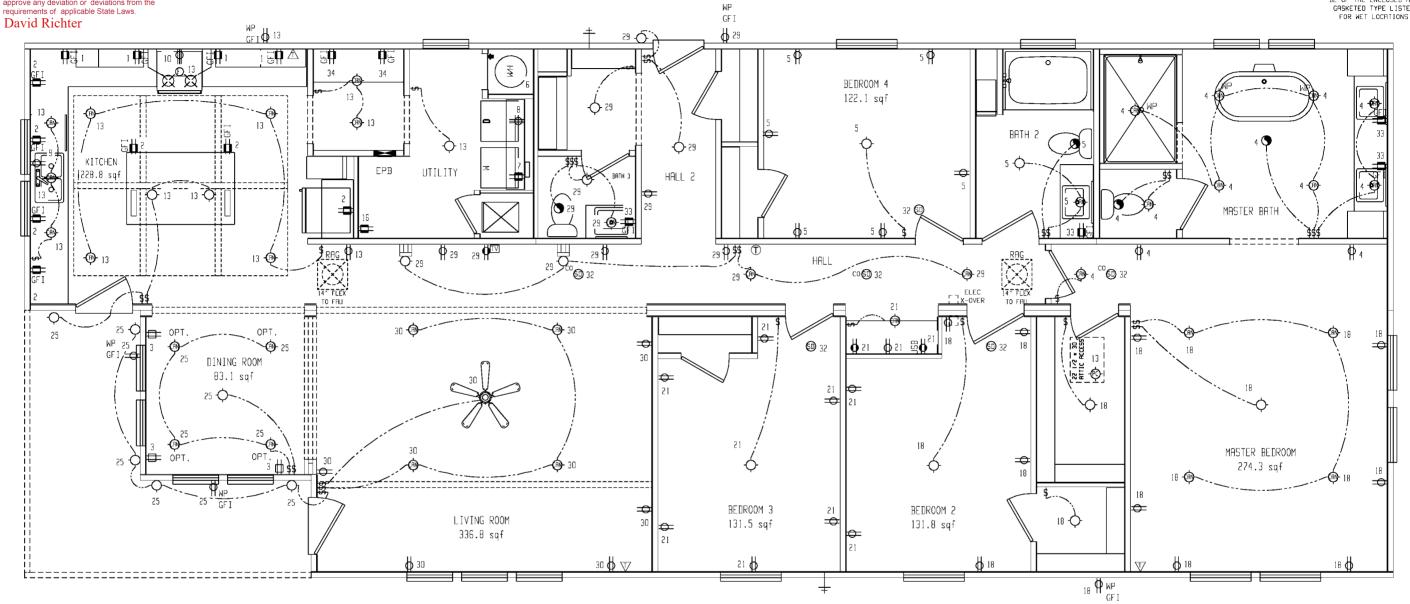
David Richter





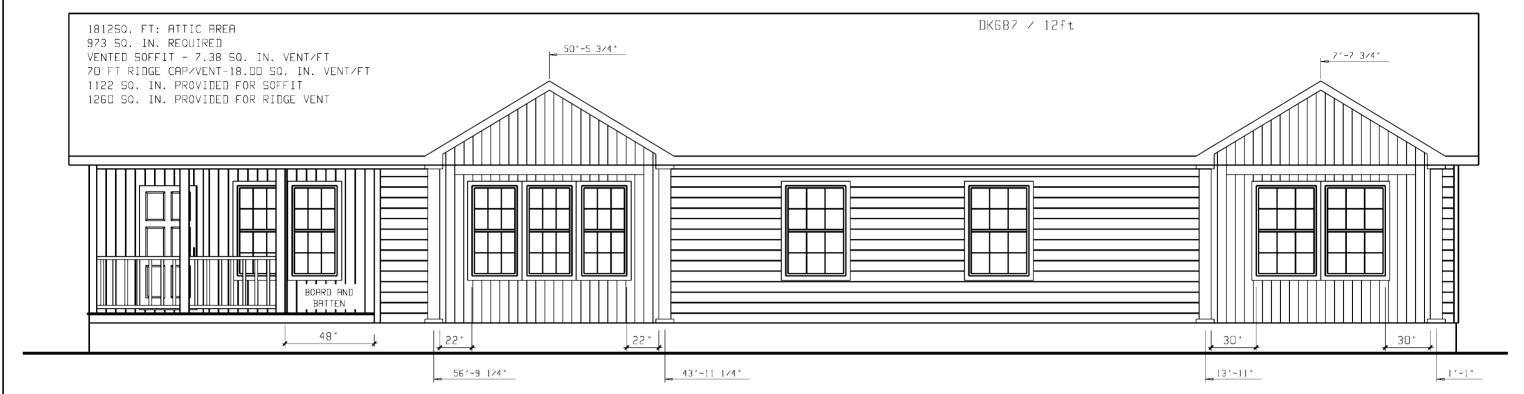


RECESSED CAN LIGHT SHALL BE OF THE ENCLOSED AND GASKETED TYPE LISTED FOR WET LOCATIONS



NOTE: ALL FAMILY, DINING, LIVING, KITCHEN, LAUNDRY, LIBRARIES, DENS, BEDROOMS, RECREATION ROOMS, CLOSETS, HALLWAYS
OR SIMILAR ROOMS OR SPACES SHALL BE PROTECTED BY A LISTED ARC-FAULT CIRCUIT INTERRUPTER IN ACCORDANCE WITH SECTION 210.12 OF THE NEC.

											ELE	CTRICAL	CHEDI	ULE													
CIR. DESCRIPTION BRK, V	OLTS COPR.	CIR. DESCRIPTION	BRK. AMP: VOLTS	COPR. CIF	R. DESCRIPTION	BRK. AMP: VOLTS	COPR. HIRE	CIR. DESCRIPTION	BRK. AMP. V	OLTS COPR.	CIR. NO.	DESCRIPTION	BRK: VOLTS	COPR. HIRE	CIR. NO.	DESCRIPTI	ON BRK. VOL	TS COPR.	CIR. NO.	DESCRIPTION	BRK: VOLTS C	OPR. CI	IR. DESCRIP	TION BRK: VOLTS	COPR. C	IR. DESCRIPTION	N BRK VOLTS COPE
1 PORTABLE APPLIANCES 20	120 12	5 GEN. LIGHTING/RECEPT.	15 120	14 B	DRYER RECEPT.	30 240	10	12 ELECTRIC FURNAC	CIRCUI	its vary, see	15 0	OPT. GARBAGE DISPOSAL	15 120	14	19 (	ŒN. LIGHTING∕1	ECEPT. 15 12	14	24 E	LECT. BUILT-IN OV	N 20 240 1	12/3 2	28 GEN. LIGHTIN	IC∕RECEPT. 15 120	14 3	32 SHOKE PLAR	is 15 120 14
2 PORTABLE APPLIANCES 20	120 12	6 ELEC. WATER HEATER	CIRCUITS VARY	Y, SEE 9	OPT. DISHWASHER	15 120	14	DAPIA PAGE PLN-3.5 FO	R HUD, PLN	I-1.5 FOR MOD	16	FREEZER	20 120	12	20 (	ŒN, LIGHTING∕1	ECEPT. 15 12	14	25	GEN, LIGHTING∕RECEPT	. 15 120	14 2	29 GEN. LIGHTIN	G∕RECEPT. 15 120	14	33 BATH GFI (MOD	ONLY) 20 120 12
3 PORTABLE APPLIANCES 20	120 12	DAPIA PAGE PLN-3.1 FOR H	UD, PLN-1.1 FO	OR MOD 10	D ELECT. RANGE/CKTOR	40 240	8	13 GEN. LIGHTING/RECEP	т. 15	120 14	17	OPT. WHIRLPOOL	20 120	12	21 (	ODN. LIGHTING∕1	ECEPT. 15 12	] [4	26	GEN. LIGHTING∕RECEPT	. 15 120	14 3	30 GEN. LIGHTIN	IG∕RECEPT. 15 120	14 3	34 GEN. LIGHTING∕R	ECEPT. 20 120 12
4 GEN. LIGHTING/RECEPT. 15	120 14	7 WASHER RECEPT.	20 120	12 11	1 GRS FURNACE	15 120	14	14 OPT. COOLER BOX	15	120 14	18 0	GEN. LIGHTING∕RECEPT.	15 120	14	55	OPT. MICRO	IANE 50 15	15/5	27	GEN. LIGHTING∕RECEPT	. 15 120	14 3	31 SITE INSTALLE	D HERT PUMP 40 240	8/3		
BRAND	SERIES				R E V :	ISION	5	BY I	ATE		(	GENERAL	N 0 T	ΕS			DRAWING TITLE						MODEL NAME				SQ. FT.
SCHULT		CL32								LOCK-OUT BR	EAKER O	ON CIRCUIT #6					<b>=</b> 1	P ^ .	T (5)	1 0 4 1	PIAN	,	3	440-4BR ~ 9	SOUTHER	RN BELLE	2254
																	EL	とし	IK	ICAL	PLAN	V	PLANT	DESCRIPTION		MODEL NO.	
																							958	32X76 4BF	R-2.5BF	٦	3440
CLAYTON HO	DMOE BU	UILDING GR	ROUP																				DRAWN BY	ORIG. DATE	DATE	PRINTED	SHEET NO.
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# FRONT ELEVATION

APPROVED BY

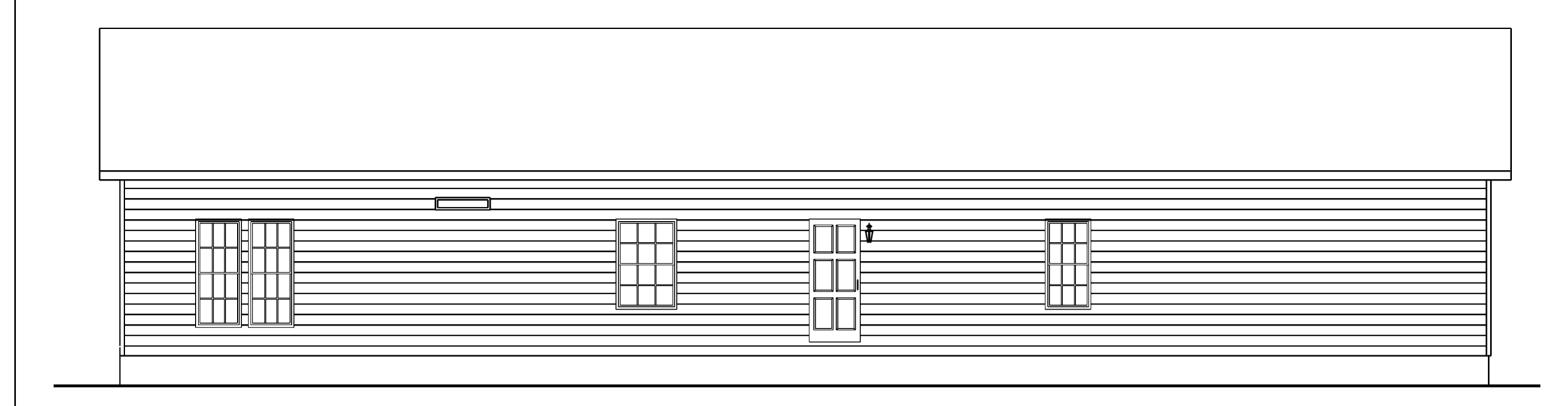
David Richter

approve any deviation or deviations from the requirements of applicable State Laws.

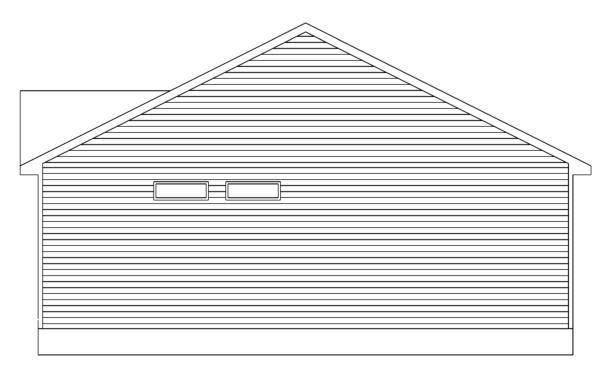


LEFT SIDE ELEVATION

BRAND	SERIES	REVISIONS	BY DATE	GENERAL NOTES	DRAWING TITLE	MODEL NAME	SQ. FT.
SCHULT	CL32				EXTERIOR FIEVATION	3440-4BR ~ SOUTHERN BELLE	2254
					EXTERIOR ELEVATION	PLANT DESCRIPTION MODEL NO.	
					FRONT & LEFT SIDE	958 32X76 4BR-2.5BA 3	440
CLAYTON	HOME BUILDING GROUP				TRUNI & LETT SIVE		ET NO.
						TXH   03/19/2019   03/27/2019	20-1



# BACK ELEVATION





# RIGHT SIDE ELEVATION

BRAND	SERIES	REVISIONS	BY DATE	GENERAL NOTES	DRAWING TITLE	MODEL NAME	SQ. FT.
SCHULT	CL32				PVTPBIAB PIPUATIAN	3440-4BR ~ SOUTHERN BELLE	2254
	•				EXTERIOR ELEVATION	PLANT DESCRIPTION MODEL NO.	·
					DICK O DICHT CIRT	958   32X76 4BR-2.5BA	3440
CLAYTON	HOME BUILDING GROUP				BACK & RIGHT SIDE	DRAWN BY ORIG. DATE DATE PRINTED	SHEET NO.
						TXH 03/19/2019 03/27/2019	20-2

DUE TO PIER LOCATION, CHASSIS MEMBERS & EXTERIOR DECK INSTALLATION

ORIG. DATE

 $\mathsf{TXH}$ 

03/19/2019 | 03/27/2019

SHEET NO.

21-30PSF

CLAYTON HOME BUILDING GROUP



# OFF FRAME BASEMENT & CRAWL FOUNDATION DESIGN FOR:

29' - 8 " 2-SECTION MODULAR 1 STORY- W.O ATTIC

Attic without storage where the maximum clear height between joist and rafter is less than 42 inches or req'd insulation depth exceeds the depth of the bottom chord.

# PERIMETER ANCHORED SYSTEM- BUILDING IS SECURED TO FOUNDATION WALLS TO SUPPORT WIND AND SEISMIC FORCES.

# SIDEWALLS ARE SUPPORTED (PERIMETER BLOCKED)

# **BUILDING CODE INFORMATION:**

IRC (2015) ASCE 7-10 2018 NORTH CAROLINA RESIDENTIAL CODE

# **BUILDING SITE INFORMATION:**

\*MAXIMUM ULTIMATE/DESIGN WIND SPEED & EXPOSURE: 130/ 100 MPH EXPOSURE C-enclosed

MINIMUM SOIL BEARING CAPACITY: 2000 PSF

MAXIMUM GROUND SNOW(S): 20 PSF, 30 PSF

Flat roof snow load (Pg)=20.0 PSF ,23.1 PSF

SEISMIC DESIGN CATEGORY: C

DESIGN SPECTRAL RESPONSE (Sps.): 0.49

SEISMIC SOIL SITE CLASS: D

# Approval of this document does not authorize or approva any deviation or deviations from the requirements of applicable State Laws. David Richter

# **HOME INFORMATION:**

UNIT WIDTH: 29' - 8 "

MAX. UNIT LENGTH: 76 ft.

ROOF PITCH: 3/12 to 6/12

DESIGN LOADS: 40 PSF FL. LL., 7PSF T.C.D.L., 8PSF B.C.

D.L., 13PSF FL. DL. &, 10PSF B.C.L.L

MAX. SIDEWALL HEIGHT: 108 INCHES

TOTAL MATING WALL RIM JOIST BEAMS: (4) 2X10 #2 SPF

RIM JOIST SPLICES: 6" X 8" MiTeK MT20 metal plates each side

OFF FRAME FLOOR PLANT NUMBER: 958



\*Ultimate wind speed Vult. Per ASCE 7-10/ allowable stress design wind speed Vasd
This design is the property of CMH Manufacturing and cannot
be used without authorization. This design is exclusively for
use with new homes built by CMH Manufacturing. Use with

FILENAME-9581-14 R. LE 2

use with new homes built by CMH Manufacturing. Use with homes built by other companies is strictly prohibited.

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# **Preface**

This foundation design manual is dedicated to the ever-growing trend to place homes over basements and permanent foundations. CMH Manufacturing, Inc. has attempted to address the more common installation configurations. These may or may not be the only acceptable designs for basements or permanent foundations. If deviations are made from these details, it is the homeowner's and/or installation contractor's responsibility to obtain proper documentation and engineer's details of construction acceptable to the local authority having jurisdictions. CMH Manufacturing, Inc. will not supply any details other than what is contained in the following design manual. If an alternate design is requested it must be provided by an independent engineer subject to local approval. The owner/contractor is responsible for any additional construction details, permits, inspections and fees associated with these items.

Setting a home over a basement or permanent foundation requires special knowledge, experience and equipment to accomplish a safe and proper set. Contractors performing this type of installation must be licensed, bonded and insured to protect all aspects of this type of work.



# Instructions

- 1. Determine site soil classification, (see table R405.1).
- 2. The provided foundation and anchorage designs are not applicable for the following conditions. In all these cases a complete geotechnical evaluation must be performed and foundation must be designed by a professional engineer in accordance with section 1805.8 (IBC) for site specific conditions.
- Site contains OL, OH or Pt class soils.
- Site contains compressible or shifting soils.
- Site contains expansive soils per IRC (R403.1.8.1) or per local authority and adopted code.
- Site contains soils which do not provide the minimum allowable soil bearing strength as specified per the provided designs.
- Foundation walls support unbalanced loads on opposite sides of building, such as a daylight basement or walk out basement where the building aspect ratio, L/W, exceeds the values specified in Table L.
- Site with soils subject to liquifaction or soil containing high concerntration of sulfate.
- 3. Determine foundation wall height for each wall of foundation. Reference **Detail D1 or D2** for wall height.
- 4. Determine height of backfill for each wall of foundation. Reference *Table L* when backfill heights along the foundation wall are unbalanced. Reference *Detail D1 or D2* for perimeter foundation wall construction.
- 5. Determine what type of mateline supports will be used. Reference **Detail D3, D4, D5 or D7** for mateline columns and **Detail D14** for cross beams.
- 6. Determine if type H connector plates will be used around the perimeter of the building. Fastening and anchoring tables have been provided with and without the use of the H connectors.
- 7. Find the Floor to Sill Plate & Sill Plate to Foundation table for site soil classification.
- 8. Find site wall height and backfill height line and follow this line across. Heights are listed as maximums, therefore any line beneath (greater height) may be utilized for items 10,11 & 12 below.
- 9. If type H connectors will be installed the table labeled *With Type H Plate Connectors* can be utilized. Note (6) will specify spacing for H plates along sidewalls and Note (7) will specify spacing for H plates along each endwall.
- 10. Select desired rim to sill connection from line in table (E, F or G for sidewalls and E or G for endwalls).
- 11. Select desired anchor type (4 or 5) for sill to foundation wall connection and determine anchor spacing for sidewall and endwall under corresponding column.
- 12. Determine if shearwall foundation holddowns are required by checking far right column within selected row. See **Shearwall Foundation Holddown Detail (Detail D18)** for connection requirements.

The above process may be repeated as desired for different foundation wall and backfill combinations.



# **General Notes**

- 1. Foundation plans and details developed by CMH Manufacturing, Inc. are provided to our company owned sales centers and wholesale distribution partners. Alternate foundation systems may be used in lieu of these plans provided they are designed by a local professional Engineer or Architect familiar with the local soil and climate conditions, and are approved by the local authority having jurisdiction.
- 2. All notes stating "in field" or "by owner" are obligations pertaining to owner/contractor.
- 3. Owner /Contractor shall provide complete foundation, including footing drains, vapor barrier, sill plate, anchor bolts, stair area, slab and footing reinforcement along with damp proofing, waterproofing, backfill, and all finish work per Chapter 4 of IRC or per adopted local building code.
- 4. Owner/Contractor shall be responsible for performing all work in accordance with previously approved construction details and obtaining all necessary inspections as required by local or state authorities.
- 5. Not designed for areas likely to have collapsible, expansive, compressible, shifting, liquifaction, soil containing high concentration of sulfate or other unknown soil characteristics. In these conditions a local engineer must provide foundation design and the building official shall determine whether to require a soil test to determine the soil characteristics. This soil test shall be made by an approved testing agency using an approved method.
- 6. Pier spacing is dimensioned to centerline unless otherwise noted.
- 7. The foundation dimensions shown are nominal. An increase in module width should be expected due to module expansion, setting tolerances, etc. The foundation contractor should consult with the manufacturer of the modules prior to construction of the foundation to determine the actual width of the home and placement of anchors.
- 8. All steel support columns shall have protective coating and a load capacity equal to or greater than specified on foundation plan (k=1000 pounds).
- 9. All foundation construction materials and installation shall be in accordance with all state and local codes.
- 10. Backfill shall not be placed against the wall until the wall has sufficient strength and has been anchored to the floor above or has been sufficiently braced to prevent damage by the backfill. Heavy-equipment must be restricted to a minimum distance to the foundation at least equal to the depth of the foundation.
- 11. Solid cap block or cement fill required at top courses of all masonry piers or pilasters.
- 12. The foundation design has been designed to be placed in the seismic zone indicated on the cover of this document. Please note that all CMH structures have been designed for seismic (zone/category) A, B, or C only, unless otherwise noted on floor plan and cover page of these instructions.
- 13. All piers shall be constructed of 8"x8"x16" concrete masonry units conforming to ASTM C90 with a minimum compressive strength of 700 psi. Masonry foundation walls must be laid in type m or s mortar. When required per tables or details, piers of masonry units shall be laid in type m or s mortar. All dry stack masonry should be surfaced bonded with an approved adhesive product.



- 14. All reinforcing steel shall be Grade 60 minimum. All splices shall be lapped 24" minimum and splices shall be offset 30" minimum within same footer.
- 15. All concrete grout shall be 3000 psi at 28 days.
- 16. Reference the model plan drawing for specific foundation layout.
- 17. Concrete footings shall have a minimum compressive strength of 3000 psi at 28 days. Concrete foundation walls and other concrete exposed to weather shall have a minimum compressive strength of 3000 psi at 28 days and in moderate andsevere weather areas the concrete shall be air entrained no less than 5 percent and not more than 7 percent. See table R301.2(1) and R402.2 of IRC
- 18. All exterior footings shall be placed at least 12" below the undisturbed ground surface. All exterior footings shall extend below the frost line or otherwise frost protected in accordance with Sections R403.1.4.1 through R403.1.4.2 of IRC or per adopted local building code.
- 19. Top of foundation walls shall extend a minimum of 6" above finished adjacent grade. Wood framing members, including wood sheathing, that rest on exterior foundation walls and are less than 8" from exposed earth shall be of naturally durable or preservative-treated wood. Wood floor joist shall not be closer than 18" from exposed ground in under floor space.
- 20. Contractor shall verify all site conditions and dimensions prior to starting foundation. Notify home manufacturer of any discrepancies immediately.
- 21. The foundation must be designed and built to local codes and ordinances and must be approved and inspected by local building officials.
- 22. Access shall be to all under floor spaces. Access shall be a minimum of 18" by 24". If mechanical equipment is installed is this area, please refer to the Mechanical Code for minimum access opening. Through wall access openings shall not be located under an exterior door.
- 23. Under floor space shall be ventilated with a net area ratio not less than 1 square foot for each 150 square feet of under floor space area placed in accordance with local codes. Ratio may be reduced to 1/1,500 where ground is covered with a 6-mil polyethylene or approved vapor retarderl.
- 24. Field installed wiring in basement is subject to local inspection. Basement smoke alarms must be installed at foot of stairs and interconnected with home smoke alarms and tested on site. Smoke alarms must be located, installed, and tested in conformance with local building requirements.
- 25. Large clear spans along mating wall require a column or pier at each end. See model specific foundation plan for required capacity and additional column requirements.
- 26. Basement stairs (widths, handrails, clearances, headroom, landings, fire protection, etc.) are the responsibility of the owner/contractor and must be constructed to comply with local building codes.
- 27. Owner/contractor shall not alter basement stair opening without written approval from CMH Manufacturing, Inc.



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- 28. Lighting and receptacles in basement are the responsibility of owner/contractor.
- 29. Termite protection shall be provided per the building code and local requirements and are responsibility of owner/contractor.
- 30. Ground snow load is indicated on foundation plans. Snow load must be verified per locality. Building has not been designed to be located within a Tsunami design zone.
- 31. This structure has not been designed to be located within flood hazard locations or in Coastal A Zones. When site is located in a flood hazard area or in Coastal A Zones as determined by the local authority having jurisdiction or flood hazard maps. The unit shall have lowest floor elevated above the design floor elevation. Foundation and anchorage designs shall be provided by a local engineer in conformance with locally adopted building code and ASCE-24-14.
- 32. All connection hardware, anchor bolts, straps, hold-downs, washers and fasteners shall be minimum of ASTM A653 Type G185 zinc coated galvanized or stainless when in contact with pressure treated sill plates or other pressure treated lumber.

  APPROVED BY
- 33. Radon control, when required by a local jurisdiction, shall be provided and installed by a local jurisdiction, shall be provided and installed by a local jurisdiction, shall be provided and installed by a local jurisdiction, shall be provided and installed by a local jurisdiction, shall be provided and installed by a local jurisdiction, shall be provided and installed by a local jurisdiction, shall be provided and installed by a local jurisdiction.
- 34. Topographic wind effects have not been considered. Home has not been designed in the local ed in a cas designated as having local historical data documenting structural damage to building englicable state Laws.

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- 35. Surface drainage shall be devirted to a storm sewer or other approved collection point. Lots shall be graded to drain surface water away from foundation walls. The grade shall fall a minimum of 6 inches within the first 10 feet.
- 36 A 6-mil-thick polyethylene moisture barrier shall be applied over the porous layer with the basement floor constructed over the polyethylene.
- 37. Concrete and Masonry Foundation walls that retain earth and enclose interior spaces and floors below grade shall be damp proofed from the top of the footing to the finished grade. Masonry walls shall have not less than 3/8" Portland cement parging applied to the exterior of the wall. The parging shall be damp proofed in accordance with one of the following.
  - a. Bituminous coating, b. 3 pound per sq. yard of arcylic modified cement, c. 1/8" coat of surface-bonding cement complying with ASTM C887, d. Material permitted for waterproofing per Section R406.2, e. Other approved methods or materials.
- 38. Concrete and masonry foundation walls that retain earth and enclose interior spaces and floors below grade in areas of high water table or other severe soil-water conditions shall be waterproofed from the top of the footing to the finished grade in accordance with one of the following:
  - a. 2-ply hot-mopped felts, b. 55 pound rolled roofing, c. 6-mil polyvinyl chloride, 6-mil polyethylene,
  - d. 40-mil polymer-modified asphalt., e, 60-mil flexible polymer cement, f. 1/8" cement-based, fiber-reinforced, waterproof coating, g. 60-mil solvent-free liquid-applied synthetic rubber.
- 39. If building is located within a wind borne debris region glazed openings shall be protected from wind borne debris. Wind Borne debris protection is the responsibility of others.
- 40. When Geotechnical report is required or available, all recommendations shall be followed and geotechnical engineer shall review all foundation plans to verify applicability with recommendations and engineer shall be present on regular basis during site preparation, fill placement and foundation excavation.
- 41. Self-closing rated doors shall be installed between garage and house (on-site by other).(R302.5.1) 42.Reserved.
- 43. A 6-mil polyethylene or approved vapor retarder with joints lapped not less than 12 inches shall be placed between the concrete floor slab and the base course or the prepared subgrade.

# **SOIL CLASSIFICATION**

		TABLE R405.1 W/N	NC admendments			
LATERAL SOIL LOAD	UNIFIED SOIL CLASSIFICATION SYSTEM SYMBOL	SOIL DESCRIPTION	DRAINAGE CHARACTERISTICS <sup>a</sup>	FROST HEAVE POTENTIAL	VOL. CHANGE POTENTIAL EXPANSION <sup>b</sup>	ALLOWABLE SOIL PRESSURE
	GW	Well-graded gravels, gravel sand mixtures, little or no fines	Good	Low	Low	5000
30 psf	GP	Poorly graded gravel or gravels sand mixtures, little or no fines	Good	Low	Low	5000
LATERAL SOIL LOAD	SW	Well-graded gravels, gravelly sands, little or no fines	Good	Low	Low	3000
	SP	Poorly graded sand, or gravelly sands, little or no fines	Good	Low	Low	3000
45 mmf	GM	Silty gravels, gravel-sand-silt mixtures	Good	Medium	Low	3000
45 psf LATERAL	SM	Silty sand, sand-silt mixtures	Good	Medium	Low	3000
SOIL LOAD	GC	Clayey gravels, gravel-sand-clay mixtures	Medium	Medium	Low	3000
	SC	Clayey sands, sand-clay mixture	Medium	Medium	Low	3000
	ML	Inorganic silts and very find sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity	Medium	High	Low	2000*
60 psf LATERAL SOIL LOAD	CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays	Medium	Medium	Medium to Low	2000*
	СН	Inorganic clays of high plasticity, fat clays	Poor	Medium	High	2000*
	МН	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts	Poor	High	High	2000*
SPECIAL	OL	Organic silts and organic silty clays of low plasticity	Poor	Medium Medium		SPECIAL
INSPECTION REQUIRED	OL	Organic clays of medium to high plasticity, organic silts	Unsatisfactory	Medium High		INSPECTION REQUIRED
	Pt	Peat and other highly organic soils	Unsatisfactory	Medium	High	

a. The percolation rate for good drainage is over 4 inches per hour, medium drainage is 2 inches to 4 inches per hour, and poor is less than 2 inches per hour.

<sup>\*</sup> Where the building offical determines that in place soils with an allowable bearing capacity of less than 2000 psf are likely to be present at the site, the allowable bearing capacity shall be determined by a soils investigation.



b. Soils with low potential expansion typically have a plasticity index (PI) of 0 to 15, soils with a medium potential expansion have a PI of 10 to 35 and soils with a high potential expansion have PI greater than 20.

TABLE R404.1.1:IRC (2015) PERIMETER FOUNDATION WALL MINIMUM REQUIREMENTS [Seismic Seismic Zone: Design]

		GW, GP, SV	W, & SP Soil Class	(30 PSF)	GM, GC, SM-	SC, & ML Soil Clas	s (45 PSF)	SC, MH, ML-CL, 8	Inorganic CL Soil	Class (60 PSF)
Max.	Maximum	Plain	8" Reinforced	8" Poured	Plain	8" Reinforced	8" Poured	Plain	8" Reinforced	8" Poured
Wall	Unbalanced	Masonry 1	Masonry	Concrete	Masonry 1	Masonry	Concrete	Masonry 1	Masonry	Concrete
Height	Fill*	Walls	Walls <sup>5,9</sup>	Walls <sup>6, 7</sup>	Walls	Walls <sup>5,9</sup>	Walls <sup>6, 7</sup>	Walls	Walls <sup>5,9</sup>	Walls <sup>6, 7</sup>
0 to 5	4	6 in. solid (3) or 8 in.	-	PC	6 in. solid (3) or 8 in.	-	PC	6 in. solid (3) or 8 in.	-	PC
feet	5	6 in. solid (3) or 8 in.	-	PC	8 in.	-	PC	10 in.	-	PC
	4	6 in. solid (3) or 8 in.	#4 @ 48 in. o.c.	PC	8	#4 @ 48 in. o.c.	PC	8	#4 @ 48 in. o.c.	PC
6 feet	5	6 in. solid (3) or 8 in.	#4 @ 48 in. o.c.	PC	10 in.	#4 @ 48 in. o.c.	PC	10 in.	#4 @ 48 in. o.c.	PC
to 7 feet	6	10 in.	#4 @ 48 in. o.c.	PC	12 in.	#5 @ 48 in. o.c.	PC	10 in. solid (3)	#5 @ 48 in. o.c.	#5 @ 48 in. o.c.
	7	12 in.	#5 @ 48 in. o.c.	PC	10 in. solid (3)	#6 @ 48 in. o.c.	#5 @ 46 in. o.c.	12 in. solid (3)	'#6 @ 40 in. o.c.	#6 @ 48 in. o.c.
	4	6 in. solid (3) or 8 in.	#4 @ 48 in. o.c.	PC	6 in. solid (3) or 8 in.	#4 @ 48 in. o.c.	PC	8	#4 @ 48 in. o.c.	PC
	5	6 in. solid (3) or 8 in.	#4 @ 48 in. o.c.	PC	10 in.	#4 @ 48 in. o.c.	PC	12 in.	#4 @ 48 in. o.c.	PC
8 feet	6	10 in.	#4 @ 48 in. o.c.	PC	12 in.	#5 @ 48 in. o.c.	PC	12 in. solid (3)	#5 @ 48 in. o.c.	#6@32in o.c.
	7	12 in.	#5 @ 48 in. o.c.	PC	12 in. solid (3)	#6 @ 48 in. o.c.	#5 @ 41 in. o.c.	Footnote (4)	'#6 @ 40 in. o.c.	#6@32 in. o.c.
	8	10 in. solid (3)	#5 @ 48 in. o.c.	#6@41	12 in. solid (3)	#6 @ 48 in. o.c.	#6 @ 43 in. o.c.	Footnote (4)	'#6 @ 32 in. o.c.	#6@18 in. o.c.
	4	6 in. solid (3) or 8 in.	#4 @ 48 in. o.c.	PC	6 in. solid (3) or 8 in.	#4 @ 48 in. o.c.	PC	8 in.	#4 @ 48 in. o.c.	PC
	5	8 in.	#4 @ 48 in. o.c.	PC	10 in.	#4 @ 48 in. o.c.	PC	12 in.	#5 @ 48 in. o.c.	PC
9 feet	6	10 in.	#4 @ 48 in. o.c.	PC	12 in.	#4 @ 48 in. o.c.	PC	12 in. solid (3)	#6 @ 48 in. o.c.	#6@35 in. o.c.
9 leet	7	12 in.	#5 @ 48 in. o.c.	PC	12 in. solid (3)	#6 @ 48 in. o.c.	#6@35 in. o.c.	Footnote (4)	'#6 @ 40 in. o.c.	#6@32 in. o.c.
	8	12 in. solid (3)	#6 @ 48 in. o.c.	#6@36 in. o.c.	Footnote (4)	'#6 @ 40 in. o.c.	#6@32 in. o.c.	Footnote (4)	#6 @ 24 in. o.c.	#6@28 in. o.c.
	9	Footnote (4)	'#6 @ 40 in. o.c.	#6@35 in. o.c.	Footnote (4)	#6 @ 24 in. o.c.	#6@25 in. o.c.	Footnote (4)	#6 @ 16 in. o.c.	#6@24 in. o.c.
	8	NA	#6 @ 48 in. o.c.	#6 @ 35 in. o.c.	NA	#6 @ 32 in. o.c.	#6 @ 29 in. o.c.	NA	#6 @ 24 in. o.c.	#6 @ 21 in. o.c.
10 feet	9	NA	#6 @ 40 in. o.c.	#6@34 in. o.c.	NA	#6 @ 24in. o.c.	#6@22 in. o.c.	NA	#6 @ 16 in. o.c.	#6@16 in. o.c.
	10	NA	#6 @ 32 in. o.c.	#6 @ 27 in. o.c.	NA	#6 @ 16 in. o.c.	#6 @ 17 in. o.c.	NA	#6 @ 16 in. o.c.	#6 @ 13 in. o.c.

<sup>\*</sup>Unbalanced backfill height is the difference in height between the exterior finish grade level and the top of the basement slab or crawl space grade.

Backfill shall be placed only AFTER the home has been anchored to the foundation wall.

- (1) All block must conform to ASTM C90 (700 psi rated) and be laid in a running bond of Type M or S mortar with overlapping pattern . Ungrouted hollow masonry units are permitted except where otherwise indicated.
- (3) Solid grouted hollow units or solid masonry units.
- (4) Wall construction per reinforced units or design required.
- (5) Vertical reinforcement shall be Grade 60 minimum. The distance from the face of the soil side of the wall to the center of vertical reinforcement shall be at least 5".
- (6) PC = Plain Concrete (Concrete with less reinforement than minimum for reinforced concrete)
- (7) All reinforcement shall be Grade 60 minimum. The distance from the face of the soil side of the wall to the vertical reinforcement shall be at least 6 1/16", but not more than 6 11/16".
- 'All information above has been extracted from the 2009 IRC Tables R404.1.1(1), Tables R404.1.1(2) Tables R404.1.2(3)
- (8) Reserved



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# Maximum Aspect Ratio, L/W for Unbalanced Foundations

		SOIL CLASS								
Maximum Wall	Maximum	GW, GP, SW, & SP	GM, GC, SM-SC, &	SC, MH, ML-CL, & Inorganic CL (60						
Height	Unbalanced Fill	(30 PSF)	ML (45 PSF)	PSF)						
- i i i i i i i i i i i i i i i i i i i	1	4.0	4.0	4.0						
7 ( )	4 5									
7 feet	5	4.0	3.4	2.6						
	6	3.0	2.0	1.5						
	7	1.9	1.2	0.9						
	4	4.0	4.0	4.0						
	5	4.0	3.9	2.9						
8 feet	6	3.4	2.3	1.7						
	7	2.1	1.4	1.1						
	8	1.4	1.0	0.7						
	4	4.0	4.0	4.0						
	5	4.0	4.0	3.3						
9 feet	6	3.8	2.6	1.9						
	7	2.4	1.6	1.2						
	8	1.6	1.1	0.8						
	9	1.1	0.8	0.6						

# Instructions:

Where foundation wall support unbalnced load on opposite sides of building such as daylight basement, the building aspect ratio, L/W, shall not exceed the value specified in Table

1 - Determine foundation wall height, unbalanced fill depthrand soil class to determine aspect ratio from table above.

2 - Multiple "W" times aspect ratio.

3 - Result is equal to the maximum allowable buildin length on the exposed side.

Approval of this document does not authorize or approve any deviation or deviations from the requirements of applicable State Laws.

David Richter

me.

Unbalanced Fill

Example 1 - check sidewall for 26'-8" x 60'-0" home.

Basement Wall Height = 8'-0"

Unbalanced backfill = 7'-0"

Soil Class = SP

Aspect Ratio from Table above = 2.1

26.67 x 2.1 = 56'-0" max. allowable length - **example fails** Try again using 6'-0" max. unbalanced fill with an aspect ratio of 3.4. 26.67 x 3.4 = 90'-8" max. allowable length - **example passes Max. allowable backfill is 6'-0**"

**Example 2** - check endwall for 26'-8" x 60'-0" home.

Basement Wall Height = 8'-0"

Unbalanced backfill = 7'-0"

Soil Class = SP

Aspect Ratio from Table above = 2.1

60 x 2.1 = 126'-0" max. allowable length - example passes

"L" = total overall dimension of the building on the exposed side
"W" = the total overall dimension of the building on the side adjacent to
the exposed side

# Required Rim Joist to Sill Plate Fastening at wall "L".

Use a 20 Gauge metal angle clip at 24" o.c. with (5) 8d nails per leg or an approved connector supplying 230 pounds per linear foot capacity.

\*Page extracted from 2006 IRC section R404.1.5 & Table R404.1(3)

# **Schult**

# UNBALANCED FOUNDATIONS (TABLE L)

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## TABLE M - MINIMUM CONCRETE BLOCK PIER AND FOOTER SIZE AT MATING WALL COLUMNS (REF. DETAILS D3 OR D5) # of Uplift

		2 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1			. DE 174120 D0		# of Uplift		
GRO	OUND SNOW	20	30				Ties		
	4 '	(S) 26"x26"X9" OR	(S) 26"x26"X9" OR				0		
ပု	4	30" Dia. X 11"	30" Dia. X 11"				U		
区	6 '	` '	(D) 34"x34"X9" OR				0		
PC		40" Dia. X 16"	40" Dia. X 16"						
<u> </u>	8 '	(D) 34"x34"X9" OR	, ,				1		
S	-	40" Dia. X 16" (D) 34"x34"X9" OR	40" Dia. X 16" (D) 34"x34"X9" OR						
I≨	10 '	40" Dia. X 16"	40" Dia. X 16"				1		
3	40.1	(D) 34"x34"X9" OR							
8	12 '	40" Dia. X 16"	40" Dia. X 16"				1		
Ļ	14 '	(D) 34"x34"X9" OR					1		
l ₹		40" Dia. X 16"	40" Dia. X 16"				'		
<b>&gt;</b>	16 '	(D) 34"x34"X9" OR					1		
ž	-	40" Dia. X 16" (D) 34"x34"X9" OR	40" Dia. X 16" (D) 34"x34"X9" OR						
F	18 '	40" Dia. X 16"	40" Dia. X 16"				1		
SPAN BETWEEN MATING WALL COLUMN SUPPORTS		(D) 34"x34"X9" OR							
	20 '	40" Dia. X 16"	40" Dia. X 16"				1		
Ä	22 '	(D) 34"x34"X9" OR	(D) 34"x34"X9" OR				1		
<u> </u>		40" Dia. X 16"	40" Dia. X 16"				'		
BE	24 '	` '	(T) 42"x42"X13" OR				1		
Z		40" Dia. X 16" (D) 34"x34"X10"	48" Dia. X 20" (T) 42"x42"X13" OR						
96	26 '	OR 40" Dia. X 16"	48" Dia. X 20"				1		
Ш	28 '	(D) 34"x34"X11"	(T) 42"x42"X13" OR						
=		OR 40" Dia. X 16"	48" Dia. X 20"				1		
Θ	30 '	(D) 34"x34"X13"	(T) 42"x42"X13" OR				1		
	30	OR 40" Dia. X 16"	48" Dia. X 20"						
MAXIMUM MATING LINE	32 '		(T) 42"x42"X13" OR				1		
2 5		48" Dia. X 20"	48" Dia. X 20"				•		
Í	34 '	48" Dia. X 20"	(T) 42"x42"X13" OR 48" Dia. X 20"				1		
I ⋛			(T) 42"x42"X13" OR						
(≩	36 '	48" Dia. X 20"	48" Dia. X 20"				1		
_	46 '	(T) 42"x42"X15" OR	(T) 42"x42"X15" OR				1		
	40	48" Dia. X 20"	48" Dia. X 20"				<u>'</u>		
		SUPPORTS UN	IDER MATING OPE	NING AS CLEARS	PANS IN FEET		Ī		
PIER	R SPACING	8.4 '	8.4 '				7		
		(S) 26"x26"X9" OR	(S) 26"x26"X9" OR				7		
PIEF	R CONFIG.	25" Dia.	25" Dia.				Girder beams cons	struction to	
				ALLS- CLEARSPA	NS IN FEET		be (4) 2X10 #2 SP	,	
PIER	R SPACING	SUPPORTS UNDER MATING WALLS- CLEARSPANS IN FEET 7.1 7.1 7.1					Splices 6" X 8" MiTeK MT20 metal plates each side		
<u> </u>									
PIEF	R CONFIG.	(D) 34"x34"X9" OR	(D) 34"x34"X9" OR 30" Dia.						
		28" Dia.	ou Dia.						

Chart Key:

(Pier Configuration) Min. footer width (inches) x Min. footer length (inches) x Min. footer depth (inches)

- (S)= Single stack block configuration.
- (D)= Double stack block configuration.
- (T)= Triple stack block configuration.
- (DR)=Double stack reinforced & fully grouted configuration.
- IE. For 20 psf 178" box with 14' opening: Double stack pier on a 34"x 34" sq. footer 9" deep footing.

30' 1 STORY- W.O ATTIC OFF FRAME BASEMENT & CRAWL With Roof Pitch of 3/12 Min. to 6/12 Max.

NOTES: 1 DESIGNED FOR 100 MPH MAX. WIND SPEED.

- 2 DESIGNED FOR 2000 PSF MIN. ALLOWABLE SOIL BEARING CAPACITY.
- 3 \*Ultimate wind speed Vult. Per ASCE 7-10/ allowable stress design wind speed Vasd & ASCE 7-10 & 2018 NORTH CAROLINA RESIDENTIAL CODE 4 MAX. MATING WALL OPENINGS LISTED IN CHART ASSUME OPENING IN BOTH HALVES. IF ANCHOR IS TIED TO ONLY ONE COLUMN (ONE HALF) THEN HALF THE OPENING SIZE CAN BE USED WHEN LOOKING UP VALUE IN TABLE ABOVE. PIERS SUPPORTS REQUIRED AT EACH SIDE OF DOOR OPENINGS AND ALL EXTERIOR WALL OPENINGS GREATER THAN 4'.

5 WHEN PIER CONFIGURATION IS NOT GIVEN IN CHART THE ACTUAL LOADS EXCEED ALL PREDESIGNED PIERS AND A LOCAL ENGINEER MUST DESIGN THE SUPPORTS FOR THE GIVEN LOADS (- UPLIFT/ + GRAVITY LOADS).

FILENAME:958I-14.R.J.E.22.22.117(\_)

Approval of this document does not authorize or

approve any deviation or deviations from the

requirements of applicable State Laws.

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**APPROVED BY** 

**David Richter** 

6 ALL PIERS SHALL BE EMBEDDED IN TYPE M OR S MORTAR.

7.Round footers or Round Piles with diameter as required above may be used as alternate to square footing or square footing and block piers.

# TABLE N - STRUCTURAL STEEL POST AND FOOTER SIZE AT MATING WALL COLUMNS (REF. DETAIL D7)

	Uplift									
GROU	ND SNOW	20	30				force			
S	4 '	(9k) 26"x26"X11"	(9k) 26"x26"X11"				0 #			
ORT	6 '	(9k) 26"x26"X11"	(9k) 26"x26"X11"				0 #			
UPP	8 '	(14k) 32"x32"X13"	(14k) 32"x32"X13"				6.0466#			
N N N	10 '	(14k) 32"x32"X13"	(14k) 32"x32"X13"				129.043#			
OLU	12 '	(14k) 32"x32"X13"	(14k) 32"x32"X13"				252.04 #			
T	14 '	(14k) 32"x32"X13"	(14k) 32"x32"X13"	APPROV	ED BY		375.037 #			
/M 5	16 '	(14k) 32"x32"X13"	(14k) 32"x32"X13"		2/20/2010		498.033 #			
ATIN	18 '	(14k) 32"x32"X13"	(14k) 32"x32"X13"				621.03#			
Z	20 '	(14k) 32"x32"X13"	(20k) 38"x38"X14"	Approval of th	s document does not authoriz	e or	744.027 #			
TWE	22 '	(14k) 32"x32"X13"	(20k) 38"x38"X14"	requirements David R	of applicable State Laws. ichter		867.023 #			
BE	24 '	(14k) 32"x32"X13"	(20k) 38"x38"X14"				990.02#			
SPAN	26 '	(14k) 32"x32"X13"	(20k) 38"x38"X14"				1113.02#			
뵘	28 '	(20k) 38"x38"X14"	(20k) 38"x38"X14"				1236.01 #			
MAXIMUM MATING LINE SPAN BETWEEN MATING WALL COLUMN SUPPORTS	30 '	(20k) 38"x38"X14"	(20k) 38"x38"X14"				1359.01 #			
MAT	32 '	(20k) 38"x38"X14"	(20k) 38"x38"X14"				1482.01 #			
MUM	34 '	(20k) 38"x38"X14"	(20k) 38"x38"X14"				1605 #			
NAXII	36 '	(20k) 38"x38"X14"	(20k) 38"x38"X14"				1728 #			
_	46 '	(20k) 38"x38"X19"	(30k) 48"x48"X17"				2342.98 #			
SUPPORTS UNDER MATING OPENING AS CLEARSPANS IN FEET										
POST	SPACING	8.4 '	8.4 ' 0/C				Girder beam	าร		
FOO	TER SIZE	(9k) 26"x26"X11"	(9k) 26"x26"X11"				construction			
SUPPORTS UNDER MATING WALLS- CLEARSPANS IN FEET  2X10 #2 SPF joist Splices 6" X 8" Mi										
POST	SPACING	7.1 '	7.1 '				MT20 metal			
FOO	TER SIZE	(9k) 26"x26"X11"	(9k) 26"x26"X11"				each side			

Chart Key

(Post Load)= Minimum allowable compression rating which post must be rated in kips (1000 lbs.).

(Post Capacity and Footer Size) Min. footer width (inches) x Min. footer length (inches) x Min. footer depth (inches)

Note: Steel piers must have a minimum steel base plate size of 4 inches x 5.5 inches which bears directly on footer sized per chart.

Minimum steel column top plate size of 4"x5.5"for 9000#; 6"x6"for 14000#; 6"x8"for 20000# & 6"x12"for 30000#

# Minimum footer Reinforcement (Number of #4 bars each way):

Footer size	# of No. 4 bars	Footer size	# of No. 4 bars
26"x26"	3	38"x38"	5
32"x32"	4	48"x48"	8

### 30' 1 STORY- W.O ATTIC OFF FRAME BASEMENT & CRAWL With Roof Pitch of 3/12 Min. to 6/12 Max.

NOTES: 1 DESIGNED FOR 100 MPH MAX. WIND SPEED.

2 DESIGNED FOR 2000 PSF MIN. ALLOWABLE SOIL BEARING CAPACITY.

3 \*Ultimate wind speed Vult. Per ASCE 7-10/ allowable stress design wind speed Vasd & ASCE 7-10 & 2018 NORTH CAROLINA RESI

4 MAX. MATING WALL OPENINGS LISTED IN CHART ASSUME OPENING IN BOTH HALVES. IF ANCHOR IS TIED TO ONLY ONE

 $\hbox{COLUMN (ONE HALF) THEN HALF THE OPENING SIZE CAN BE USED WHEN LOOKING UP VALUE IN TABLE ABOVE. PIERS\\$ 

SUPPORTS REQUIRED AT EACH SIDE OF DOOR OPENINGS AND ALL EXTERIOR WALL OPENINGS GREATER THAN 4'.

5 WHEN PIER CONFIGURATION IS NOT GIVEN IN CHART THE ACTUAL LOADS EXCEED ALL PREDESIGNED FOOTERS AND A LOCAL ENGINEER MUST DESIGN THE SUPPORTS

FOR THE GIVEN LOADS (- UPLIFT/ + GRAVITY LOADS).

## Support and anchorage for 16" Max. Recess

NON CORNER- SPANS ARE NOT LOCATED WITH 6' OF END OF HOME

			PIER	CONFIGUR.	ATION AND	FION AND MINIMUM FOOTER SIZE UNDER SIDEWALL PORCH/ RECESS SUPPORT 1,4						
G	ROUND SNOW	/	20	) #	30 #		0	0 #		)#	0 #	
Max.	UPLIFT 10	#	w/ground	w/concrete	w/ground	w/concrete	w/ground	w/concrete	w/ground	w/concrete	w/ground	w/concrete
span <sup>3</sup>	LOAD	Brk <sup>2</sup>	anchors	anchors	anchors	anchors	anchors	anchors	anchors	anchors	anchors	anchors
4	-183.63854#	-1	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	S) 26"x26"X9"(	S) 26"x26"X9"	S) 26"x26"X9"
6	-275.45781 #	-1	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9" (	S) 26"x26"X9"	S) 26"x26"X9"
8	-367.27708 #	-1	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9" (	S) 26"x26"X9"	S) 26"x26"X9"
10	-459.09635#	-1	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9" (	S) 26"x26"X9"	S) 26"x26"X9"
12	-550.91562#	-1	(D) 34"x34"X9"	(D) 34"x34"X9"	(D) 34"x34"X9"	(D) 34"x34"X9"	(D) 34"x34"X9"	(D) 34"x34"X9"	D) 34"x34"X9"	D) 34"x34"X9" (	D) 34"x34"X9" (	D) 34"x34"X9"

### CORNER- SPANS ARE LOCATED WITH 6' OF END OF HOME

			PIER	PIER CONFIGURATION AND MINIMUM FOOTER SIZE UNDER SIDEWALL PORCH/ RECESS SUPPORT 1.4									
G	ROUND SNOV	/	20	) #	30 #		0	#		#	0	0 #	
Max.	UPLIFT ™	#	w/ground	w/concrete	w/ground	w/concrete	w/ground	w/concrete	w/ground	w/concrete	w/ground	w/concrete	
span <sup>3</sup>	LOAD	Brk <sup>2</sup>	anchors	anchors	anchors	anchors	anchors	anchors	anchors	anchors	anchors	anchors	
4	-129.7666#	-1	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	S) 26"x26"X9" (	S) 26"x26"X9"(	S) 26"x26"X9"	
6	-194.6499#	-1	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	S) 26"x26"X9"(	S) 26"x26"X9"(	S) 26"x26"X9"	
8	-259.5332#	-1	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	S) 26"x26"X9" (	S) 26"x26"X9"(	S) 26"x26"X9"	
10	-324.4165#	-1	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	S) 26"x26"X9" (	S) 26"x26"X9"(	S) 26"x26"X9"	
12	-389.2998#	-1	(D) 34"x34"X9"	(D) 34"x34"X9"	(D) 34"x34"X9"	(D) 34"x34"X9"	(D) 34"x34"X9"	(D) 34"x34"X9"	D) 34"x34"X9" (	D) 34"x34"X9" (	D) 34"x34"X9" (	D) 34"x34"X9"	

# Support and anchorage for 48" Max. Porch Depth

# NON CORNER- SPANS ARE NOT LOCATED WITH 6' OF END OF HOME

PIER CONFIGURATION AND MINIMUM FOOTE								E UNDER S	IDEWALL P	ORCH/ REC	ESS SUPPO	)RT <sup>1,4</sup>
G	ROUND SNOV	/	20	) #	30 #		C	0 #		)#	0 #	
Max.	UPLIFT <sup>10</sup>	#	w/ground	w/concrete	w/ground	w/concrete	w/ground	w/concrete	w/ground	w/concrete	w/ground	w/concrete
span <sup>3</sup>	LOAD	Brk <sup>2</sup>	anchors	anchors	anchors							
4	-63.923188#	-1	(S) 26"x26"X9" (	S) 26"x26"X9"	(S) 26"x26"X9"							
6	-95.884782#	-1	(S) 26"x26"X9" (	S) 26"x26"X9"	(S) 26"x26"X9"							
8	-127.84638 #	-1	(S) 26"x26"X9"	S) 26"x26"X9"	S) 26"x26"X9"(	S) 26"x26"X9"	(S) 26"x26"X9"					
10	-159.80797#	-1	(S) 26"x26"X9" (	S) 26"x26"X9"	(S) 26"x26"X9"							
12	-191.76956#	-1	(D) 34"x34"X9"	D) 34"x34"X9"	D) 34"x34"X9" (	D) 34"x34"X9" (	(D) 34"x34"X9"					

# CORNER- SPANS ARE LOCATED WITHIN 6' OF END OF HOME

			PIER	PIER CONFIGURATION AND MINIMUM FOOTER SIZE UNDER SIDEWALL PORCH/ RECESS SUPPORT 1,4									
G	ROUND SNOV	V	20	O #	30 #		C	0#		) #	(	0 #	
Max.	UPLIFT ™	#	w/ground	w/concrete	w/ground	w/concrete	w/ground	w/concrete	w/ground	w/concrete	w/ground	w/concrete	
span <sup>3</sup>	LOAD	$Brk^2$	anchors	anchors	anchors	anchors	anchors	anchors	anchors	anchors	anchors	anchors	
4	13.258128 #	1	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	S) 26"x26"X9"	(S) 26"x26"X9"	
6	19.887191 #	1	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	S) 26"x26"X9"	(S) 26"x26"X9"	
8	26.516255 #	1	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	S) 26"x26"X9"	(S) 26"x26"X9"	S) 26"x26"X9"	(S) 26"x26"X9"	
10	33.145319#	1	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	(S) 26"x26"X9"	S) 26"x26"X9"	(S) 26"x26"X9"	
12	39.774383 #	1	(D) 34"x34"X9"	(D) 34"x34"X9"	(D) 34"x34"X9"	(D) 34"x34"X9"	(D) 34"x34"X9"	(D) 34"x34"X9"	D) 34"x34"X9"	D) 34"x34"X9" (	D) 34"x34"X9"	(D) 34"x34"X9"	

### NOTES:

1. Piers supports are required under all porch/ recess post and at intersection of sidewall (see key plan).

2. # Brk- Number of uplift brackets required under the support column. Brackets per Detail D6. Brackets maybe Installed individually or in pairs and must be tied to a ground anchor or concrete anchor with a minimum design capacity of 3150#. An alternate uplift connector may be used which has the required uplift load indicated above NG- Indicates that uplift exceeds standard angle and tie down capacity and alternate design is require. 3. Max. Span- Maximum distance between adjacent porch post or supports as measure parallel to box length.

A. Piers- Indicates the minimum CMU block configuration (S)ingle, (D)ouble, (T) Triple or (DR) (D)ouble (R)eidiferset any deviation or deviations for requirements of applicable State Laws and minimum footer size. See Detail D3 of D4 for pier configuration.

5. w/ ground anchors- Minimum footer size for gravity load support at post. Uplift is taken to ground anchor anchas piate Rirchater

6. w/ concrete anchors- Minimum footer size based on gravity and uplift. Concrete anchors embedded into foot carry uplift load.

7. off frame basement & crawl foundation design for: 29' - 8 " 2-section modular

8. designed for 100 mph max. wind speed.

9. Desgin for 2000 psf min. allowable soil bearing capacity.

10. Designed to the \*Ultimate wind speed Vult. Per ASCE 7-10/ allowable stress design wind speed Vasd & ASCE 7-10

Approval of this document does not au

Schult

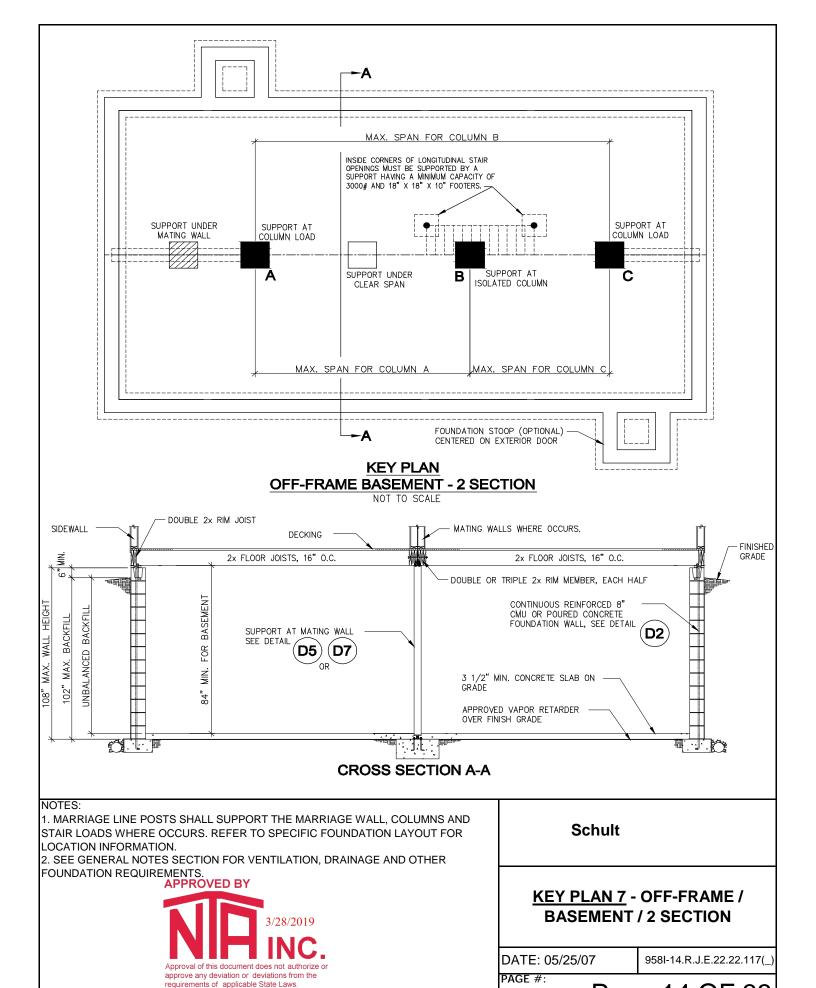
# **PORCH & RECESS** (TABLE P)

DATE: 3/27/07

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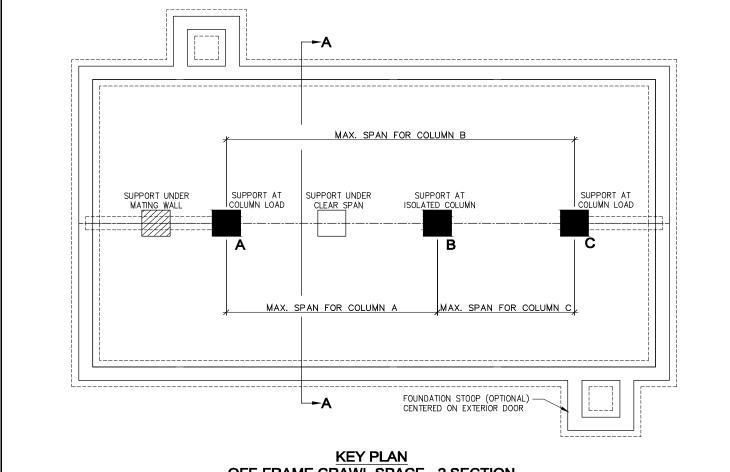
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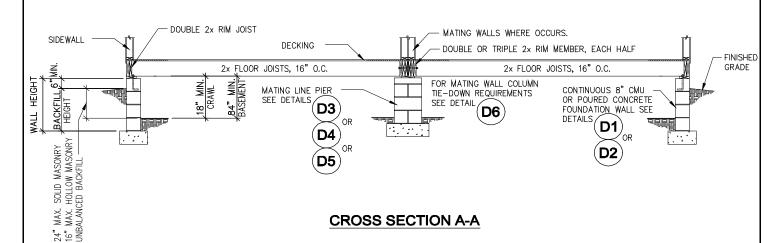


**David Richter** 

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# **OFF-FRAME CRAWL SPACE - 2 SECTION** NOT TO SCALE



# **CROSS SECTION A-A**

### NOTES:

- 1. MARRIAGE LINE POSTS SHALL SUPPORT THE MARRIAGE WALL, COLUMNS AND STAIR LOADS WHERE OCCURS. REFER TO SPECIFIC FOUNDATION LAYOUT FOR LOCATION INFORMATION.
- 2. SEE GENERAL NOTES SECTION FOR VENTILATION, DRAINAGE AND OTHER FOUNDATION REQUIREMENTS.



# **Schult**

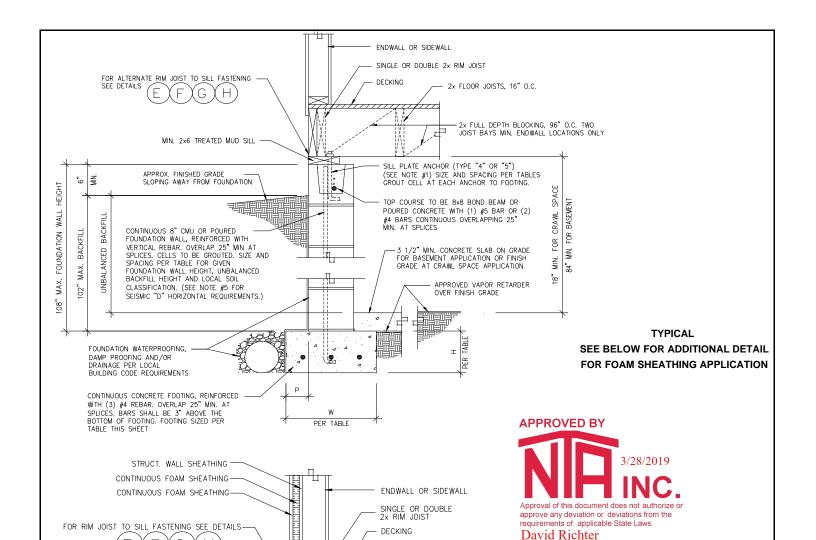
**KEY PLAN 8 - OFF-FRAME /** CRAWL SPACE / 2 SECTION

DATE: 05/25/07

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# EAATED SIZE TADI E

FUUTER SIZE TABLE											
MAX.	MIN. FOOTER SIZE										
GROUND	WIDTH	DEF	PTH H								
SNOW	W	Crawl	Basement								
20 PSF	16"	6"	6''								
30 PSF	16 "	6"	6''								

# REINFORCED PERIMETER FOUNDATION WALL

# **BASEMENT OR CRAWL SPACE**

30 ' WIDE 1 STORY- W.O ATTIC

130/ 100 \*MAXIMUM ULTIMATE/DESIGN WIND SPEED EXPOSURE C-enclosed & SEISMIC ZONE C

### NOTES

1. MUD SILL TO FOUNDATION ANCHORS:

TYPE 4:1/2" DIAMETER STEEL ANCHOR BOLTS EMBEDDED 7" MIN. INTO CONCRETE FOUNDATION WALL OR CLOSE CELL CMU WITH 2"x2"x1/8" WASHERS AND NUTS. BOLT HEADS SHALL NOT BE RECESSED INTO SINGLE SILL PLATE.

TYPE 5: SIMPSON MAB OR MAS MUD SILL ANCHOR INSTALLED PER INSTALLATION INSTRUCTIONS.

- RIM TO MUD SILL FASTENING AND SILL TO FOUNDATION ANCHOR SPACING SHALL BE THE MINIMUM OF:
  - a) SPACING GIVEN IN APPLICABLE TABLES FOR UNIT CONFIGURATION AND WIND SPEED.
  - b) SPACING GIVEN IN BACKFILL/ SIDEWALL TABLES FOR GIVEN UNIT CONFIGURATION, MAXIMUM BASEMENT WALL HEIGHT, BACKFILL DEPTH, AND LOCAL SOIL CLASSIFICATION.
- DISTANCE FROM EDGE OF FOOTER TO FACE OF FOUNDATION WALL (P) SHALL NOT BE LESS THAN 2" AND SHALL NOT EXCEED THE FOOTER THICKNESS (H)
- 4. 2000 PSF MIN. ALLOWABLE SOIL BEARING CAPACITY.

G

Z FLASHING

WOOD SHIMS

CAULK TOP OF Z AND SIDING

5. FOR SEISMIC ZONES EXCEEDING ZONE C:

VERTICALLY REINFORCED PER TABLE (R404.1.1) AT 16" O.C. ALONG ENDWALLS & 48" O.C.ELSEWHERE

# **Schult**

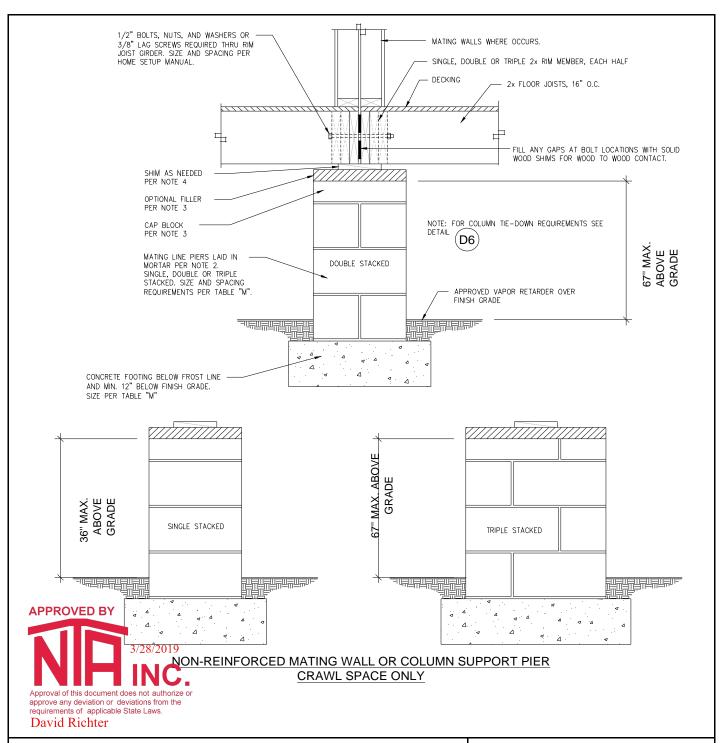
# REINFORCED PERIMETER **FOUNDATION WALL - DETAIL - D2**

DATE: 06/04/07

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### NOTES:

1. FOOTINGS MUST BE LEVEL IN ALL DIRECTIONS. PIERS ARE TO BE PLACED CENTERED ON THE FOOTING SO THAT THE FOOTING PROJECTION FROM THE PIER IS EQUAL FROM SIDE-TO-SIDE AND FRONT-TO-BACK. PIERS MUST BE LEVEL VERTICALLY ON ALL SIDES AND SQUARE WITH THE FOOTING.

2. CONCRETE BLOCKS FOR PIERS ARE 8" x 16" x 8" NOMINAL SIZE, HOLLOW CELL LOAD BEARING CMU'S MANUFACTURED IN CONFORMANCE WITH ASTM C90, GRADE "N". OPEN CELLS ARE ALIGNED VERTICALLY.THE PIERS SHALL BE LAID IN RUNNING BOND WITH TYPE M OR S MORTAR OR APPROVED ALTERNATE (SEE GENERAL NOTE 12). SINGLE STACKED BLOCKS TO BE LAID WITH LONG SIDE PERPENDICULAR TO MATE LINE RIM JOISTS. DOUBLE STACKED BLOCK IS LAID WITH EACH LAYER AT A RIGHT ANGLE TO THE PREVIOUS LAYER. THE TOP COURSE OR THE CAP BLOCKS SHALL BE PERPENDICULAR TO THE MATE LINE RIM JOISTS.

3. CAP BLOCKS SHALL BE 4" SOLID CONCRETE OR MASONRY BLOCK. 2x NOMINAL HARDWOOD OR 1/2" STEEL MAY BE USED AS A CAP BLOCK IF THE TOP COURSE OF THE PIER IS SOLID MASONRY OR CONCRETE OR IF THE TOP COURSE OF A HOLLOW PIER IS FILLED WITH CONCRETE OR GROUT. OPTIONAL FILLER MATERIAL MAY BE 2x NOMINAL HARDWOOD OR 2" OR 4" NOMINAL SOLID CONCRETE BLOCK. ALL CAPS AND FILLER SHALL BE OF THE SAME NOMINAL DIMENSIONS AS THE PIERS THEY REST UPON. INDIVIDUAL LENGTHS OF CAP BLOCKS AND FILLER SHALL BE PERPENDICULAR TO THE MATE LINE RIM JOISTS.

4. SHIMS SHALL BE OF HARDWOOD, AT LEAST 3 1/2" WIDE AND 6" LONG AND ARE NOT TO EXCEED ONE INCH IN THICKNESS. SHIMS SHALL BE PERPENDICULAR TO MATE LINE, FITTED AND DRIVEN TIGHT BETWEEN CAP BLOCKS OR FILLER AND MATE LINE RIM JOISTS.

5. MARRIAGE LINE PIERS SHALL SUPPORT THE MARRIAGE WALL AND COLUMNS WHERE OCCURS PER MODEL SPECIFIC FOUNDATION PLAN. MAXIMUM PIER SPACING PER TABLE "M".

6. SEE GENERAL NOTES FOR DRAINAGE AND OTHER FOUNDATION REQUIREMENTS.

# **Schult**

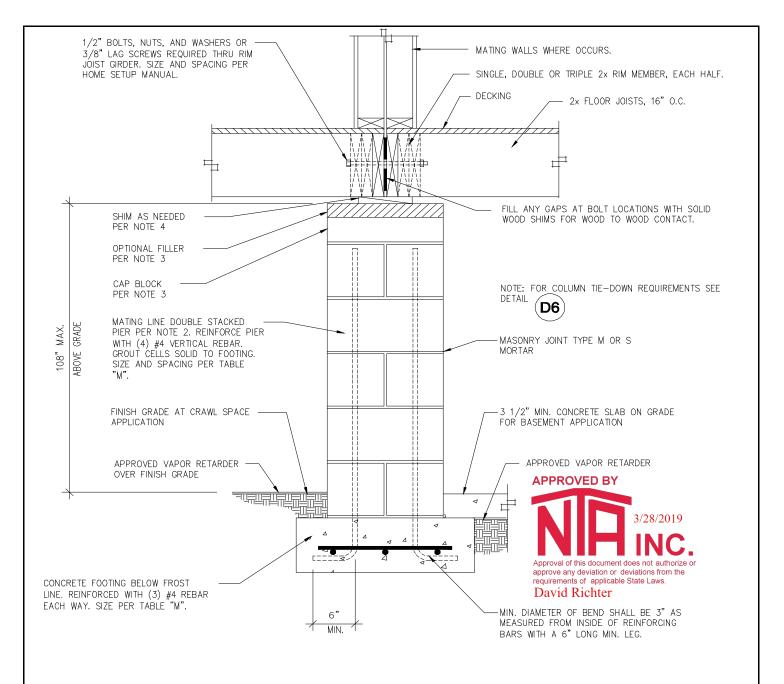
NON-REINFORCED MATING WALL COLUMN SUPPORT PIER - CRAWLSPACE ONLY -DETAIL - D3

DATE: 06/13/07

958I-14.R.J.E.22.22.117(\_)

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# REINFORCED MATING WALL OR COLUMN SUPPORT PIER BASEMENT OR CRAWL SPACE (PIER SPACING AND FOOTER SIZE PER TABLE M)

### NOTES:

1. FOOTINGS MUST BE LEVEL IN ALL DIRECTIONS. PIERS ARE TO BE PLACED CENTERED ON THE FOOTING SO THAT THE FOOTING PROJECTION FROM THE PIER IS EQUAL FROM SIDE-TO-SIDE AND FRONT-TO-BACK. PIERS MUST BE LEVEL VERTICALLY ON ALL SIDES AND SQUARE WITH THE FOOTING.
2. CONCRETE BLOCKS FOR PIERS ARE 8" x 16" x 8" NOMINAL SIZE, HOLLOW CELL LOAD BEARING CMU'S

2. CONCRETE BLOCKS FOR PIERS ARE 8" x 16" x 8" NOMINAL SIZE, HOLLOW CELL LOAD BEARING CMU'S MANUFACTURED IN CONFORMANCE WITH ASTM C90, GRADE "N". OPEN CELLS ARE ALIGNED VERTICALLY. THE PIERS SHALL BE LAID IN RUNNING BOND WITH TYPE M OR S MORTAR. SINGLE STACKED BLOCKS TO BE LAID WITH LONG SIDE PERPENDICULAR TO MATE LINE RIM JOISTS. DOUBLE STACKED BLOCK IS LAID WITH EACH LAYER AT A RIGHT ANGLE TO THE PREVIOUS LAYER. THE TOP COURSE OR THE CAP BLOCKS SHALL BE PERPENDICULAR TO THE MATE LINE RIM JOISTS.

3. CAP BLOCKS SHALL BE 4" SOLID CONCRETE OR MASONRY BLOCK. 2x NOMINAL HARDWOOD OR 1/2" STEEL MAY BE USED AS A CAP BLOCK IF THE TOP COURSE OF THE PIER IS SOLID MASONRY OR CONCRETE OR IF THE TOP COURSE OF A HOLLOW PIER IS FILLED WITH CONCRETE OR GROUT. OPTIONAL FILLER MATERIAL MAY BE 2x NOMINAL HARDWOOD OR 2" OR 4' NOMINAL SOLID CONCRETE BLOCK. ALL CAPS AND FILLER SHALL BE OF THE SAME NOMINAL DIMENSIONS AS THE PIERS THEY REST UPON. INDIVIDUAL LENGTHS OF CAP BLOCKS AND FILLER SHALL BE PERPENDICULAR TO THE MATE LINE RIM JOISTS.

4. SHIMS SHALL BE OF HARDWOOD, AT LEAST 3 1/2" WIDE AND 6" LONG AND ARE NOT TO EXCEED ONE INCH IN THICKNESS. SHIMS SHALL BE PERPENDICULAR TO MATE LINE, FITTED AND DRIVEN TIGHT BETWEEN CAP BLOCKS OR FILLER AND MATE LINE RIM JOISTS.

5. MARRIAGE LINE PIERS SHALL SUPPORT THE MARRIAGE WALL AND COLUMNS WHERE OCCURS PER MODEL SPECIFIC FOUNDATION PLAN. MAXIMUM PIER SPACING PER TABLE "M".

6. SEE GENERAL NOTES FOR DRAINAGE AND OTHER FOUNDATION REQUIREMENTS.

### Schult

REINFORCED MATING WALL OR COLUMN SUPPORT PIER -BASEMENT OR CRAWL SPACE DETAIL - D5

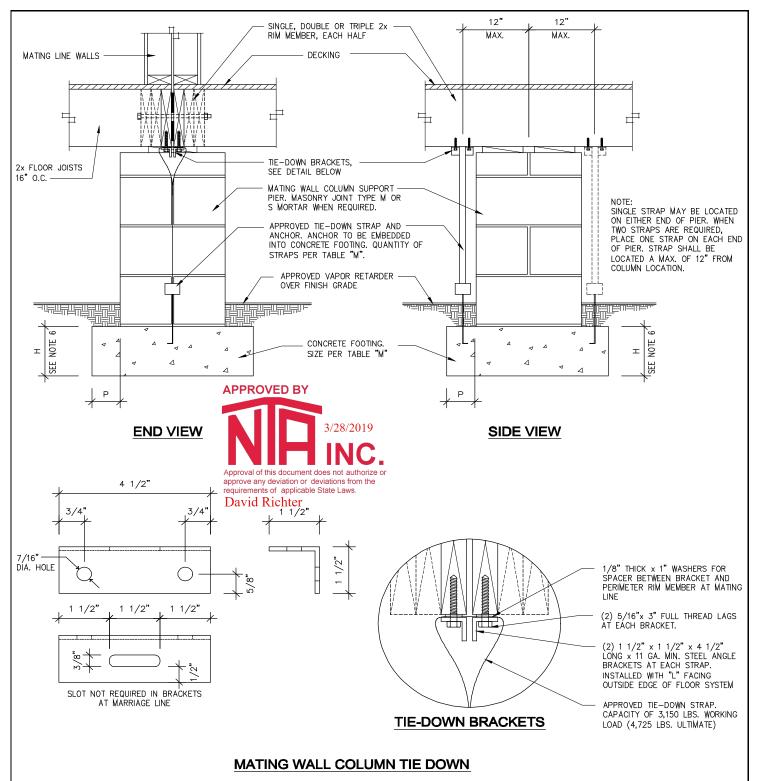
DATE: 06/04/07

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### NOTES:

- 1. ALL MARRIAGE WALL COLUMN LOCATIONS WITH OPENINGS 4 FEET OR GREATER MAY REQUIRE THE INSTALLATION OF COLUMN BRACKETS AND TIE-DOWNS. SEE TABLE "M" FOR REQUIREMENTS.
- 2. EACH BRACKET IS RATED FOR AN ALLOWABLE WORKING LOAD OF 1,719 LBS.
- 3. THE CAPACITY OF BOTH THE TIE-DOWN STRAP AND ANCHOR MUST BE 3,150 LBS. WORKING LOAD (4,725 LBS. ULTIMATE)
- 4. USE A RADIUS CLIP FOR ALL BRACKET APPLICATIONS BY THREADING A PIECE OF STRAP OVER THE BRACKETS BEFORE LOOPING THE TIE-DOWN STRAP AROUND THE BRACKET.
- 5. GROUND ANCHORS WHICH ARE LISTED FOR THE REQUIRED CAPACITY ABOVE MAY BE USED IN LIEU OF CONCRETE ANCHOR.
- 6. DISTANCE FROM EDGE OF FOOTING TO FACE OF FOUNDATION WALL (P) SHALL NOT BE LESS THAN 2" AND SHALL NOT EXCEED THE FOOTING THICKNESS (H). FOOTING THICKNESS MAY BE 10" IF GROUND ANCHORS WITH AN UPLIFT CAPACITY OF 3,150 LBS. ARE USED IN PLACE OF CONCRETE ANCHORS.
- 7. FOOTING SIZES PER TABLE "M" HAVE BEEN DESIGNED ASSUMING CONCRETE ANCHORS WILL BE UTILIZED. IF GROUND ANCHORS ARE UTILIZED TO TRANSMIT UPLIFT INTO GROUND SOIL, THE DEPTH OF THE FOOTING MAY BE REDUCED TO (P). WHERE (P) IS EQUAL TO THE GREATEST DISTANCE FROM EDGE OF FOOTING TO EDGE OF PIER. MINIMUM DEPTH IS 9".

### **Schult**

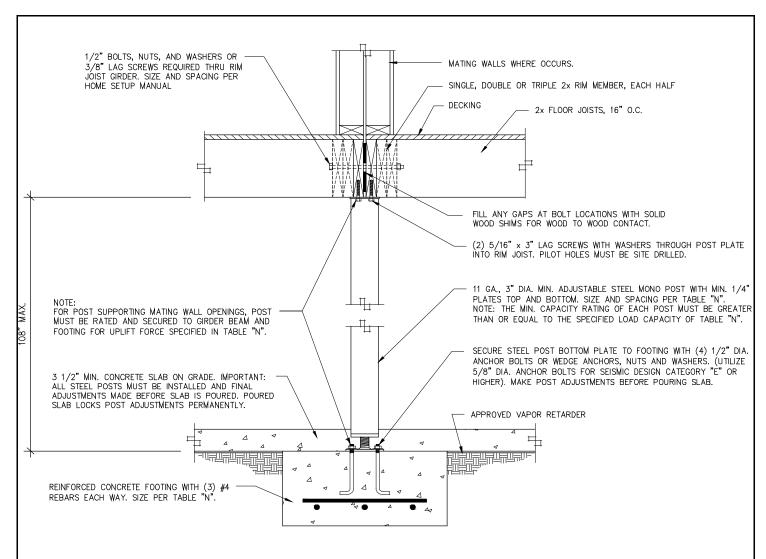
# MATING WALL COLUMN TIE DOWN - <u>DETAIL - D6</u>

DATE: 06/29/07

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ALTERNATE POST INSTALLATION: STEEL POSTS MAY BE INSTALLED WITH SCREW JACK ASSEMBLY AT THE TOP OR BOTTOM. STEEL POSTS INSTALLED WITHOUT THE SCREW JACK ASSEMBLY AT THE BOTTOM AND ENCASED IN CONCRETE ARE SUBJECT TO LOAD REDUCTIONS, VERIFY THE CAPACITY OF THE STEEL POST BASED ON THE INSTALLATION METHOD PRIOR TO INSTALLATION OF THE POST.



# ADJUSTABLE STEEL COLUMN POST BASEMENT OR CRAWL SPACE (MAXIMUM POST SPACING PER TABLE N)

### NOTES:

- 1. FOOTINGS MUST BE LEVEL IN ALL DIRECTIONS. STEEL POSTS ARE TO BE PLACED CENTERED ON THE FOOTING SO THAT THE FOOTING PROJECTION FROM THE POST IS EQUAL FROM SIDE-TO-SIDE AND FRONT-TO-BACK. COLUMN POSTS MUST BE LEVEL VERTICALLY ON ALL SIDES AND SQUARE WITH THE FOOTING.
- 2. MARRIAGE LINE STEEL POSTS SHALL SUPPORT THE MARRIAGE WALL AND COLUMNS WHERE OCCURS PER
- 3. SEE GENERAL NOTES FOR DRAINAGE AND OTHER FOUNDATION REQUIREMENTS.

### Schult

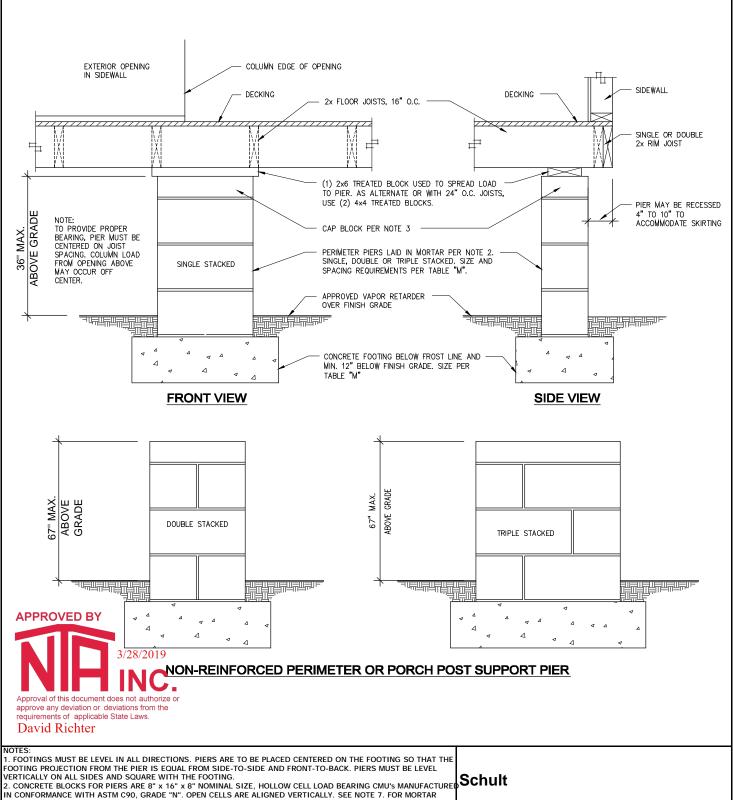
ADJUSTABLE STEEL COLUMN POST - BASEMENT OR CRAWL SPACE - DETAIL - D7

DATE: 06/08/07

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2. CONCRETE BLOCKS FOR PIERS ARE 8" x 16" x 8" NOMINAL SIZE, HOLLOW CELL LOAD BEARING CMU'S MANUFACTUREI IN CONFORMANCE WITH ASTM C90, GRADE "N". OPEN CELLS ARE ALIGNED VERTICALLY. SEE NOTE 7. FOR MORTAR REQUIREMENT. SINGLE STACKED BLOCKS TO BE LAID WITH LONG SIDE PERPENDICULAR TO MATE LINE RIM JOISTS. DOUBLE STACKED BLOCK IS LAID WITH EACH LAYER AT A RIGHT ANGLE TO THE PREVIOUS LAYER. THE TOP COURSE O

THE CAP BLOCKS SHALL BE PERPENDICULAR TO THE MATE LINE RIM JOISTS. CAP BLOCKS SHALL BE 4" SOLID CONCRETE OR MASONRY BLOCK. 2x NOMINAL HARDWOOD OR 1/2" STEEL MAY BE USED AS A CAP BLOCK IF THE TOP COURSE OF THE PIER IS SOLID MASONRY OR CONCRETE OR IF THE TOP COURSE OF HOLLOW PIER IS FILLED WITH CONCRETE OR GROUT. OPTIONAL FILLER MATERIAL MAY BE 2x NOMINAL HARDWOOD OR 2" OR 4' NOMINAL SOLID CONCRETE BLOCK ALL CAPS AND FILLER SHALL BE OF THE SAME NOMINAL DIMENSIONS AS THE PIERS THEY REST UPON. INDIVIDUAL LENGTHS OF CAP BLOCKS AND FILLER SHALL BE PERPENDICULAR TO THE MATE LINE RIM JOISTS

4. SHIMS SHALL BE OF HARDWOOD, AT LEAST 3 1/2" WIDE AND 6" LONG AND ARE NOT TO EXCEED ONE INCH IN THICKNESS. SHIMS SHALL BE PERPENDICULAR TO MATE LINE, FITTED AND DRIVEN TIGHT BETWEEN CAP BLOCKS OR

5. MARRIAGE LINE PIERS SHALL SUPPORT THE MARRIAGE WALL AND COLUMNS WHERE OCCURS PER MODEL SPECIFIC FOUNDATION PLAN. MAXIMUM PIER SPACING PER TABLE "M"

6. SEE GENERAL NOTES FOR DRAINAGE AND OTHER FOUNDATION REQUIREMENTS

THE PIERS SHALL BE LAID IN RUNNING BOND WITH TYPE M OR S MORTAR OR DRY STACKED ABOVE FIRST COARSE WITH SURFACE BONDING AGENT APPLIED THAT MEETS ASTM C887 WHEN ACCEPTABLE TO LOCAL AUTHORITY. BONDING AGENT MUST BE INTENDED FOR USE/APPLICATION AND SHALL BE INSTALLED PER MANUFACTURES SPECIFICATINS

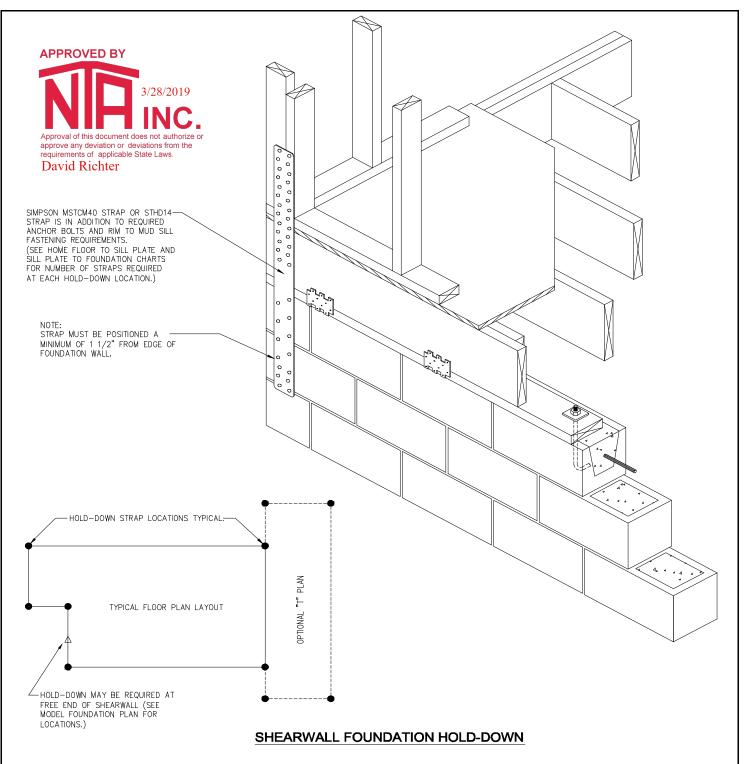
## NON-REINFORCED PERIMETER/ **PORCH POST SUPPORT PIER -DETAIL - D15**

DATE: 07/18/07

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### NOTES:

- 1. WHERE REQUIRED AT FREE-END HOLD-DOWNS (AS LOCATED ON THE FOUNDATION LAYOUT) OR AT BUILDING CORNERS PER THE FASTENING TABLES INCLUDED WITHIN THIS FOUNDATION DESIGN PACKAGE, THE FOUNDATION HOLD-DOWN STRAPS ARE THE RESPONSIBILITY OF OTHERS AND ARE NOT PROVIDED BY CLAYTON HOME BUILDING GROUP OR SUBSIDIARIES.
- 2. SIMPSON MSTCM40 SHALL BE FASTENED TO WALL STUD WITH (26) 16d NAILS AND TO FOUNDATION WALL WITH (14) 1/4" x 2 1/4" TITAN SCREWS.
- 3. SIMPSON MSTCM40 OR STHD14 STRAP MAY BE PLACED ON ENDWALL OR SIDEWALL.
  MINIMUM EDGE DISTANCE OF TITAN SCREW TO CONCRETE OR MASONRY BLOCK CORNER OF 1
  1/2" MUST BE MAINTAINED.
- 4. SIMPSON MSTCM40 STRAP IS IN ADDITION TO THE REQUIRED ANCHOR BOLTS AND RIM TO MUD SILL FASTENING REQUIREMENTS.
- 5. SIMPSON STHD14 STRAP (POURED WALLS) MUST BE FASTENED TO WALL STUD WITH (38) .148X 3 1/4" NAILS.)
- 6. DESIGN STRAP CAPACITY: MSTCM40=4250# AND STHD14= 5025#

### **Schult**

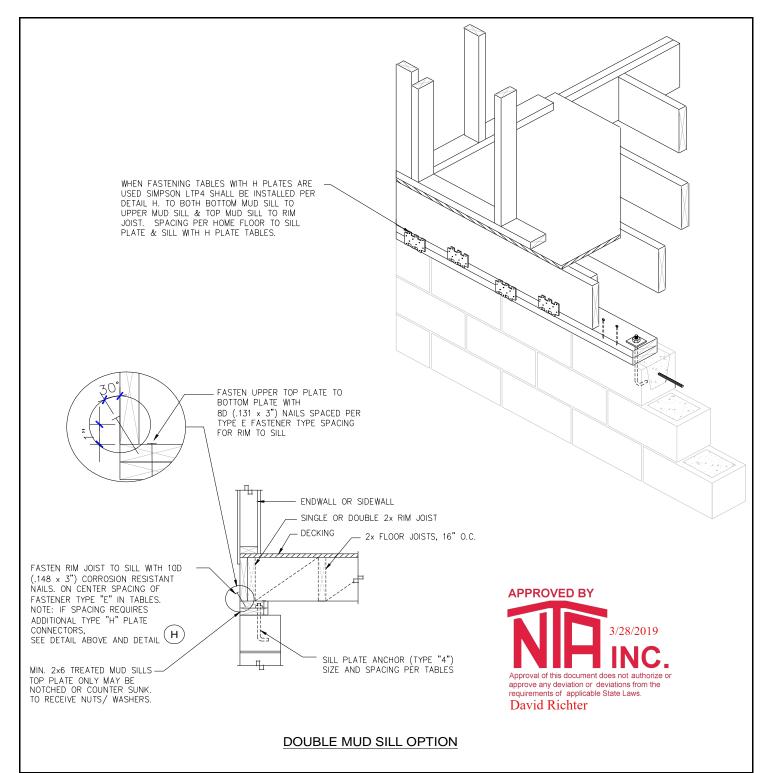
## SHEARWALL FOUNDATION HOLD-DOWN - DETAIL - D18

DATE: 06/13/07

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### NOTES:

- 1. MUD SILL TO FOUNDATION ANCHORS:
  - TYPE 4:1/2" DIAMETER STEEL ANCHOR BOLTS EMBEDDED 7" MIN. INTO CONCRETE FOUNDATION WALL
    OR CLOSE CELL CMU WITH 2"x2"x1/8" WASHERS AND NUTS. BOLT HEADS SHALL NOT BE
    RECESSED INTO BOTTOM MUD SILL PLATE.
- UPPER MUD SILL MUST BE FASTENED TO LOWER MUD SILL WITH .131"X3" NAILS SPACED PER RIM JOIST TO MUD SILL SPACING TABLE FOR TYPE E FASTENERS.
- 4. WHEN FASTENING TABLES WITH H PLATES ARE USED, SIMPSON LPT4 PLATES MUST BE INSTALLED FROM LOWER MUD SILL TO UPPER MUD SILL AND FROM UPPER MUD SILL TO RIM JOIST PER FASTENING SPACED PER RIM TO MUD SILL SPACING TABLES.

### **Schult**

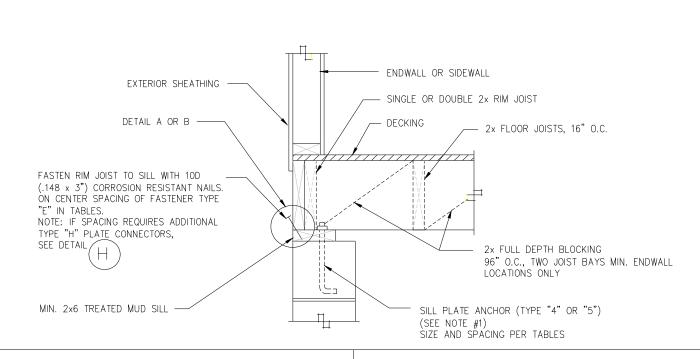
# DOUBLE MUD SILL FOUNDATION WALL <u>DETAIL - D34</u>

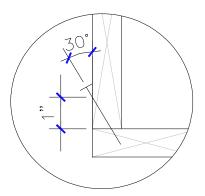
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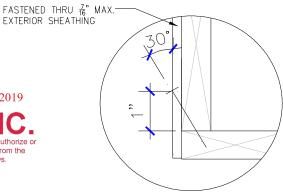
APPROVED BY

3/28/2019

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Approval of this document does not authorize or approve any deviation of deviations from the requirements of applicable State Laws.

David Richter



ALTERNATE FASTENER:

THE FOLLOWING ALTERNATE FASTENERS MAY BE USED WHEN SPACING IN CHART IS MULTIPLIED BY THE FOLLOWING FASTENER MULTIPLIER.

8D (.131  $\times$  3") NAIL = .82 16D (.162  $\times$  3 1/2") NAIL = 1.2 #8  $\times$  3" WOOD SCREW = .78 ALTERNATE FASTENER:

FASTENERS MAY BE INSTALLED THROUGH  $\frac{7}{16}$ " MAXIMUM THICK WALL SHEATHING WHEN SPACING IN CHARTS ARE REDUCED BY MULTIPLYING BY THE FOLLOW: 10d (.148"X3") NAIL = .68

8D (.131 x 3") NAIL = .66 8D (.131 x 3") NAIL = .55 16D (.162 x 3 1/2") NAIL = .816 #8 x 3" WOOD SCREW = .53

**DETAIL A- DIRECT RIM TO SILL FASTENING** 

DETAIL B- THRU SHEATHING RIM TO SILL FASTENING

### FLOOR TO SILL PLATE FASTENING -TYPE "E" -ENDWALL OR SIDEWALL

### NOTES:

1) MUD SILL TO FOUNDATION ANCHORS:

TYPE 4: DIAMETER STEEL ANCHOR BOLTS EMBEDDED 7" MIN. INTO CONCRETE FOUNDATION WALL OR CLOSE CELL CMU WITH WASHERS AND NUTS. BOLT HEADS SHALL NOT BE RECESSED INTO SINGLE SILL PLATE.

TYPE 5: SIMPSON MAB OR MASA MUD SILL ANCHOR INSTALLED PER INSTALLATION INSTRUCTIONS

- 2) RIM TO MUD SILL FASTENING AND SILL TO FOUNDATION ANCHOR SPACING SHALL BE THE MINIMUM OF:
  - SPACING GIVEN IN APPLICABLE TABLES FOR UNIT CONFIGURATION AND WIND SPEED
  - b) SPACING GIVEN IN BACKFILL/ SIDEWALL TABLES FOR GIVEN UNIT CONFIGURATION, MAXIMUM BASEMENT WALL HEIGHT, BACKFILL DEPTH, AND LOCAL SOIL CLASSIFICATION.

### **Schult**

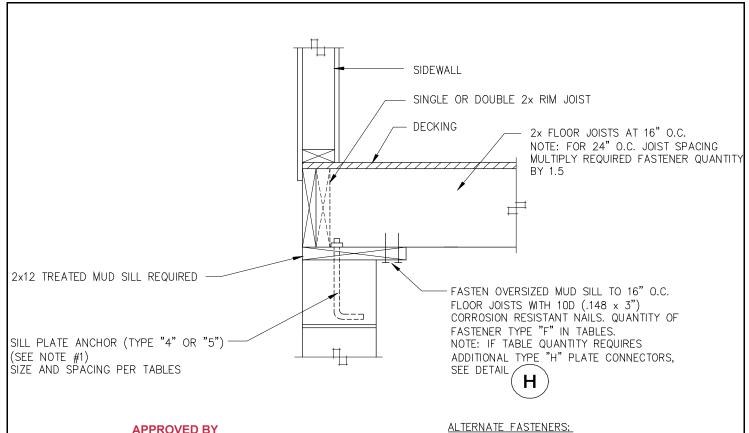
FLOOR TO SILL PLATE FASTENING - ENDWALL OR SIDEWALL - DETAIL - E

DATE: 04/17/07

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APPROVED BY

3/28/2019

3/28/2019

Approval of this document does not authorize or approve any deviation or deviations from the requirements of applicable State Laws.

David Richter

THE FOLLOWING ALTERNATE FASTENERS MAY BE USED WHEN QUANTITY IN TABLE IS MULTIPLIED BY THE FOLLOWING FASTENER MULTIPLIER.

8D (.131 x 3") NAIL = 1.21 16D (.162 x 3 1/2") NAIL = .83 #8 x 3" WOOD SCREW = 1.28

NOTE: THIS DETAIL FOR TYPE "F" FASTENING IS APPLICABLE TO SIDEWALL CONNECTIONS ONLY AND CAN NOT BE USED FOR ENDWALLS. SEE FASTENING TYPE "E" OR TYPE "G" FOR ENDWALL APPLICATION.

## FLOOR TO SILL PLATE FASTENING - TYPE "F" - SIDEWALL ONLY

### NOTES:

1) MUD SILL TO FOUNDATION ANCHORS:

TYPE 4: DIAMETER STEEL ANCHOR BOLTS EMBEDDED 7" MIN. INTO CONCRETE FOUNDATION WALL OR CLOSE CELL CMU WITH WASHERS AND NUTS. BOLT HEADS SHALL NOT BE RECESSED INTO SINGLE SILL PLATE.

TYPE 5: SIMPSON MAB OR MASA MUD SILL ANCHOR INSTALLED PER INSTALLATION INSTRUCTIONS

- 2) RIM TO MUD SILL FASTENING AND SILL TO FOUNDATION ANCHOR SPACING SHALL BE THE MINIMUM OF:
  - SPACING GIVEN IN APPLICABLE TABLES FOR UNIT CONFIGURATION AND WIND SPEED.
  - b) SPACING GIVEN IN BACKFILL/ SIDEWALL TABLES FOR GIVEN UNIT CONFIGURATION, MAXIMUM BASEMENT WALL HEIGHT, BACKFILL DEPTH, AND LOCAL SOIL CLASSIFICATION.

### **Schult**

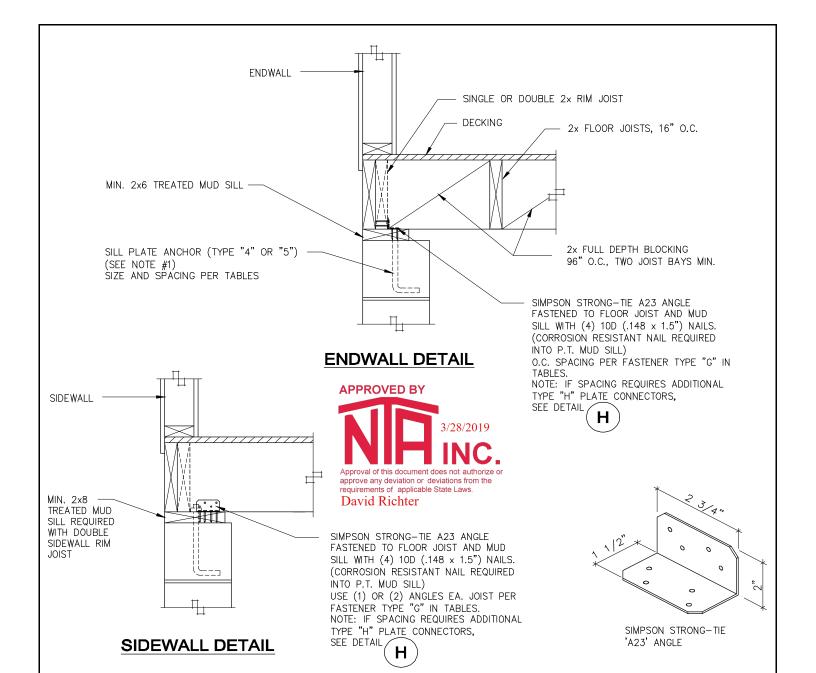
# FLOOR TO SILL PLATE FASTENING - SIDEWALL ONLY DETAIL - F

DATE: 04/17/07

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### FLOOR TO SILL PLATE FASTENING - TYPE "G" -ENDWALL OR SIDEWALL

### NOTES:

- 1) MUD SILL TO FOUNDATION ANCHORS:
  - TYPE 4: DIAMETER STEEL ANCHOR BOLTS EMBEDDED 7" MIN. INTO CONCRETE FOUNDATION WALL OR CLOSE CELL CMU WITH WASHERS AND NUTS. BOLT HEADS SHALL NOT BE RECESSED INTO SINGLE SILL PLATE.
  - TYPE 5: SIMPSON MAB OR MASA MUD SILL ANCHOR INSTALLED PER INSTALLATION INSTRUCTIONS
- 2) RIM TO MUD SILL FASTENING AND SILL TO FOUNDATION ANCHOR SPACING SHALL BE THE MINIMUM OF:
  - SPACING GIVEN IN APPLICABLE TABLES FOR UNIT CONFIGURATION AND WIND SPEED.
  - b) SPACING GIVEN IN BACKFILL/ SIDEWALL TABLES FOR GIVEN UNIT CONFIGURATION, MAXIMUM BASEMENT WALL HEIGHT, BACKFILL DEPTH, AND LOCAL SOIL CLASSIFICATION.

### **Schult**

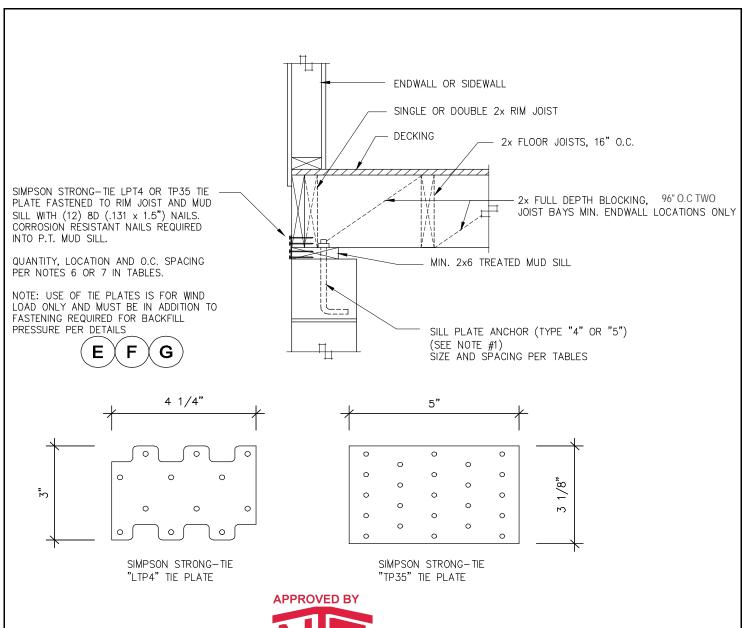
FLOOR TO SILL PLATE FASTENING - ENDWALL OR SIDEWALL - <u>DETAIL - G</u>

DATE: 05/25/07

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# FLOOR TO SILL PLATE FASTENING - TYPE "H" - ENDWALL OR SIDEWALL

### NOTES:

- 1) MUD SILL TO FOUNDATION ANCHORS:
  - TYPE 4: DIAMETER STEEL ANCHOR BOLTS EMBEDDED 7" MIN. INTO CONCRETE FOUNDATION WALL OR CLOSE CELL CMU WITH WASHERS AND NUTS. BOLT HEADS SHALL NOT BE RECESSED INTO SINGLE SILL PLATE.
  - TYPE 5: SIMPSON MAB OR MASA MUD SILL ANCHOR INSTALLED PER INSTALLATION INSTRUCTIONS
- 2) RIM TO MUD SILL FASTENING AND SILL TO FOUNDATION ANCHOR SPACING SHALL BE THE MINIMUM OF:
  - SPACING GIVEN IN APPLICABLE TABLES FOR UNIT CONFIGURATION AND WIND SPEED.
  - b) SPACING GIVEN IN BACKFILL/ SIDEWALL TABLES FOR GIVEN UNIT CONFIGURATION, MAXIMUM BASEMENT WALL HEIGHT, BACKFILL DEPTH, AND LOCAL SOIL CLASSIFICATION.

## **Schult**

FLOOR TO SILL PLATE FASTENING - ENDWALL OR SIDEWALL - DETAIL - H

DATE: 04/17/07

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# Home Floor to Sill Plate & Sill Plate to Foundation WITH TYPE H PLATE CONNECTORS (See note 6 & 7) SOIL CLASSES GW, GP, SW AND SP SOILS

Unit Width: 29.67' to 29.67' Max.

Unit Length: 76' Max. Roof Pitch: 3/12 to 6/12

Max. Roof Overhang: 12 "
Max. Sidewall Height: 9 '

\*Wind Speed (3s): 100 Seismic Zone C



		M	AXIMUM F	ASTENER	<b>SPACING</b>	OR FASTE	NERS PE	R JOIST SF	PACING 2,3	& 5	# REQ'D
		SI	DEWALL F	ASTENIN	G SPACIN	3 <sup>1</sup>	E	ND WALL	FASTENIN	IG	S/W HDS
Foundati	on Wall <sup>10</sup>	F	Rim to Sill <sup>o</sup>	6	Sill to F	nd. Wall	Rim t	o Sill <sup>7</sup>	Sill to Fnd. Wall		SEE
Wall	Backfill	Fa	astener Typ		Anchor	Spacing	Fasten	er Type	Anchor	D18	
Height	Depth	Е	F⁴	G⁴	4	5	Е	G	4	5	/CORNER
24 "	16 "	9.6" o.c.	1	1	72" o.c.	72" o.c.	80" o.c.	269" o.c.	57" o.c.	30" o.c.	0
32 "	24 "	15.3" o.c.	1	1	72" o.c.	72" o.c.	32" o.c.	106" o.c.	56" o.c.	30" o.c.	0
40 "	32 "	15.3" o.c.	1	1	72" o.c.	72" o.c.	17" o.c.	56" o.c.	54" o.c.	30" o.c.	0
3.833 '	3.33 '	9.9" o.c.	1	1	72" o.c.	72" o.c.	10" o.c.	33" o.c.	50" o.c.	29" o.c.	0
7 '	4 '	10.4" o.c.	1	1	72" o.c.	72" o.c.	10" o.c.	35" o.c.	51" o.c.	29" o.c.	0
7 '	5 '	5.3" o.c.	2	1	46" o.c.	51" o.c.	5" o.c.	18" o.c.	40" o.c.	26" o.c.	0
7 '	6'	3.1" o.c.	3	1	26" o.c.	29" o.c.	3" o.c.	10" o.c.	26" o.c.	21" o.c.	0
8 '	4 '	11.9" o.c.	1	1	72" o.c.	72" o.c.	12" o.c.	40" o.c.	52" o.c.	29" o.c.	0
8 '	5 '	6.1" o.c.	2	1	52" o.c.	58" o.c.	6" o.c.	20" o.c.	43" o.c.	27" o.c.	0
8 '	6 '	3.5" o.c.	3	1	30" o.c.	33" o.c.	4" o.c.	12" o.c.	30" o.c.	23" o.c.	0
8 '	7 '	NA	5	1	19" o.c.	21" o.c.	NA	7" o.c.	19" o.c.	17" o.c.	0
9 '	3 '	15.3" o.c.	1	1	72" o.c.	72" o.c.	32" o.c.	106" o.c.	56" o.c.	30" o.c.	0
9 '	4 '	13.4" o.c.	1	1	72" o.c.	72" o.c.	13" o.c.	45" o.c.	53" o.c.	29" o.c.	0
9 '	5 '	6.8" o.c.	2	1	59" o.c.	65" o.c.	7" o.c.	23" o.c.	45" o.c.	27" o.c.	0
9 '	6'	4.0" o.c.	3	1	34" o.c.	38" o.c.	4" o.c.	13" o.c.	33" o.c.	24" o.c.	0
9 '	7 '	NA	4	1	21" o.c.	24" o.c.	NA	8" o.c.	21" o.c.	19" o.c.	0
9 '	8 '	NA	6	2	14" o.c.	16" o.c.	NA	6" o.c.	14" o.c.	14" o.c.	0

### NOTES:

- 1. Fastener Types A,B,C & D are not reflected in charts and are available prescriptively per table R404.1(1) in 2006 IRC.
- 2. See details for additional fastener options.
- 3. All fastener spacing must start within 12" maximum of each corner or half specified spacing (lesser of two).
- 4. Type F & G connectors are qty. per 16" oc. Joist spacing.
- 5. Fastener Type Key:
- "Type E"- Fasteners toe-nailed through rim joist into sill plate (Refer to Detail E)
- "Type F"- Fasteners direct nailed from sill plate into each floor joist (Applicable at Sidewalls only) (Refer to Detail F)
- "Type G"- Number of Simpson A23 angles fastened to sill plate and each 16" OC. (2x8 min. sill plate) (Refer to Detail G)
- "Type H"- Simpson LPT4 or TP35 plate fastened to rim joist and mud sill with (12) 8dx1.5" treated nails. (Refer to Detail H) Anchor Types:
  - "Type 4"- 1/2" x10" Anchor Bolt with 2"x2"x1/8" Washer between plate and nut.
  - "Type 5"- Simpson MAB15 (concrete) or MAB23 (concrete block) or MASA
- 6. Fasteners are in addition to (2) Type H tie plates spaced within 6' of corners & 96" oc. elsewhere along sidewalls.(See note 3)
- 7. Fasteners are in addition to Type H tie plates spaced at 33" oc. along endwall.
- 8. Three options (E,F,& G) for rim to sill fastening and two options (4 & 5) for sill plate to foundation anchorage

have been provided in chart. Any combination of rim sill connectors and mud sill anchors maybe used.

- 9. All connection hardware, anchor bolts, straps, hold-downs, washers and fasteners shall be galvanized or stainless when in contact with PT sill plates or other PT lumber.
- 10. Maximum foundation wall height and maximum unbalanced backfill.

# Home Floor to Sill Plate & Sill Plate to Foundation WITH TYPE H PLATE CONNECTORS (See note 6 & 7) SOIL CLASSES GM, GC, SM, SM-SC AND ML SOILS

Unit Width: 29.67' to 29.67' Max.

Unit Length: 76' Max. Roof Pitch: 3/12 to 6/12

Max. Roof Overhang: 12 "
Max. Sidewall Height: 9 '
\*Wind Speed (3s): 100

Seismic Zone C



		M	AXIMUM F	ASTENER	SPACING	OR FASTE	NERS PE	R JOIST SF	PACING 2,3	& 5	# REQ'D
		SI	DEWALL F	ASTENIN	G SPACINO	G '	E	ND WALL	FASTENIN	G	S/W HDS
Foundati	on Wall <sup>™</sup>	ı	Rim to Sill	0	Sill to F	nd. Wall	Rim t	o Sill'	Sill to F	nd. Wall	SEE
Wall	Backfill	Fa	astener Typ		Anchor	Spacing	Fasten	er Type	Anchor Space		D18
Height	Depth	Е	F <sup>4</sup>	G⁴	4 5		E G		4	5	/CORNER
24 "	16 "	15.3" o.c.	1	1	72" o.c.	72" o.c.	53" o.c.	656" o.c.	56" o.c.	30" o.c.	0
32 "	24 "	15.3" o.c.	1	1	72" o.c.	72" o.c.	21" o.c.	259" o.c.	55" o.c.	30" o.c.	0
40 "	32 "	11.1" o.c.	1	1	72" o.c.	72" o.c.	11" o.c.	137" o.c.	51" o.c.	29" o.c.	0
3.833 '	3.33 '	6.6" o.c.	2	1	56" o.c.	62" o.c.	7" o.c.	81" o.c.	44" o.c.	27" o.c.	0
7 '	4 '	6.9" o.c.	2	1	60" o.c.	66" o.c.	7" o.c.	85" o.c.	45" o.c.	27" o.c.	0
7 '	5 '	3.5" o.c.	3	1	30" o.c.	34" o.c.	4" o.c.	44" o.c.	30" o.c.	23" o.c.	0
7 '	6 '	NA	5	1	18" o.c.	19" o.c.	NA	25" o.c.	18" o.c.	16" o.c.	0
8 '	4 '	7.9" o.c.	2	1	68" o.c.	72" o.c.	8" o.c.	97" o.c.	47" o.c.	28" o.c.	0
8 '	5	4.1" o.c.	3	1	35" o.c.	38" o.c.	4" o.c.	50" o.c.	34" o.c.	24" o.c.	0
8 '	6	NA	4	1	20" o.c.	22" o.c.	NA	29" o.c.	20" o.c.	18" o.c.	0
8 '	7 '	NA	7	2	13" o.c.	14" o.c.	NA	18" o.c.	13" o.c.	13" o.c.	0
9 '	3 '	15.3" o.c.	1	1	72" o.c.	72" o.c.	21" o.c.	259" o.c.	55" o.c.	30" o.c.	0
9 '	4 '	8.9" o.c.	2	1	72" o.c.	72" o.c.	9" o.c.	109" o.c.	49" o.c.	28" o.c.	0
9 '	5 '	4.6" o.c.	2	1	39" o.c.	43" o.c.	5" o.c.	56" o.c.	36" o.c.	25" o.c.	0
9 '	6'	NA	4	1	23" o.c.	25" o.c.	NA	32" o.c.	23" o.c.	19" o.c.	0
9 '	7 '	NA	6	2	14" o.c.	16" o.c.	NA	20" o.c.	14" o.c.	14" o.c.	0
9 '	8 '	NA	9	0	10" o.c.	11" o.c.	NA	14" o.c.	10" o.c.	10" o.c.	0

### NOTES:

- 1. Fastener Types A,B,C & D are not reflected in charts and are available prescriptively per table R404.1(1) in 2006 IRC.
- 2. See details for additional fastener options.
- 3. All fastener spacing must start within 12" maximum of each corner or half specified spacing (lesser of two).
- 4. Type F & G connectors are qty. per 16" oc. Joist spacing.
- 5. Fastener Type Key:
- "Type E"- Fasteners toe-nailed through rim joist into sill plate (Refer to Detail E)
- "Type F"- Fasteners direct nailed from sill plate into each floor joist (Applicable at Sidewalls only) (Refer to Detail F)
- "Type G"- Number of Simpson A23 angles fastened to sill plate and each 16" OC. (2x8 min. sill plate) (Refer to Detail G)
- "Type H"- Simpson LPT4 or TP35 plate fastened to rim joist and mud sill with (12) 8dx1.5" treated nails. (Refer to Detail H) Anchor Types:
- "Type 4"- 1/2" x10" Anchor Bolt with 2"x2"x1/8" Washer between plate and nut.
- "Type 5"- Simpson MAB15 (concrete) or MAB23 (concrete block) or MASA
- 6. Fasteners are in addition to (2) Type H tie plates spaced within 6' of corners & 96" oc. elsewhere along sidewalls.(See note 3)
- 7. Fasteners are in addition to Type H tie plates spaced at 33" oc. along endwall.
- 8. Three options (E,F,& G) for rim to sill fastening and two options (4 & 5) for sill plate to foundation anchorage have been provided in chart. Any combination of rim sill connectors and mud sill anchors maybe used.
- 9. All connection hardware, anchor bolts, straps, hold-downs, washers and fasteners shall be galvanized or stainless when in contact with PT sill plates or other PT lumber.
- 10. Maximum foundation wall height and maximum unbalanced backfill.

# Home Floor to Sill Plate & Sill Plate to Foundation WITH TYPE H PLATE CONNECTORS (See note 6 & 7) SOIL CLASSES SC, ML-CL AND INORGANIC CL SOILS

Unit Width: 29.67' to 29.67' Max.

Unit Length: 76' Max. Roof Pitch: 3/12 to 6/12

Max. Roof Overhang: 12 "
Max. Sidewall Height: 9 '

\*Wind Speed (3s): 100

Seismic Zone C



		M	AXIMUM F	ASTENER	SPACING	OR FASTE	NERS PE	R JOIST SE	PACING 2,3	& 5	# REQ'D
		SI	DEWALL F	ASTENIN	G SPACING	G <sup>1</sup>	Е	ND WALL	FASTENIN	IG	S/W HDS
Foundati	ion Wall <sup>10</sup>	ı	Rim to Sill	6	Sill to F	nd. Wall	Rim t	o Sill <sup>7</sup>	Sill to F	SEE	
Wall	Backfill	Fa	astener Typ		Anchor	Spacing	Fastener Type		Anchor Spacing		D18
Height	Depth	Е	F ⁴	G⁴	4	5	Е	G	4	5	/CORNER
24 "	16 "	15.3" o.c.	1	1	72" o.c.	72" o.c.	40" o.c.	492" o.c.	56" o.c.	30" o.c.	0
32 "	24 "	15.3" o.c.	1	1	72" o.c.	72" o.c.	16" o.c.	194" o.c.	54" o.c.	29" o.c.	0
40 "	32 "	8.4" o.c.	2	1	72" o.c.	72" o.c.	8" o.c.	102" o.c.	48" o.c.	28" o.c.	0
3.833 '	3.33 '	4.9" o.c.	2	1	42" o.c.	47" o.c.	5" o.c.	61" o.c.	38" o.c.	25" o.c.	0
7 '	4 '	5.2" o.c.	2	1	45" o.c.	49" o.c.	5" o.c.	64" o.c.	39" o.c.	26" o.c.	0
7 '	5 '	NA	4	1	23" o.c.	25" o.c.	NA	33" o.c.	23" o.c.	20" o.c.	0
7 '	6'	NA	6	2	13" o.c.	15" o.c.	NA	19" o.c.	13" o.c.	13" o.c.	0
8 '	4 '	5.9" o.c.	2	1	51" o.c.	56" o.c.	6" o.c.	73" o.c.	42" o.c.	27" o.c.	0
8 '	5 '	3.0" o.c.	3	1	26" o.c.	29" o.c.	3" o.c.	37" o.c.	26" o.c.	21" o.c.	0
8 '	6'	NA	6	2	15" o.c.	17" o.c.	NA	22" o.c.	15" o.c.	15" o.c.	0
8 '	7 '	NA	9	2	10" o.c.	11" o.c.	NA	14" o.c.	10" o.c.	10" o.c.	0
9 '	3 '	15.3" o.c.	1	1	72" o.c.	72" o.c.	16" o.c.	194" o.c.	54" o.c.	29" o.c.	0
9 '	4 '	6.7" o.c.	2	1	57" o.c.	63" o.c.	7" o.c.	82" o.c.	44" o.c.	27" o.c.	0
9 '	5 '	3.4" o.c.	3	1	29" o.c.	32" o.c.	3" o.c.	42" o.c.	29" o.c.	22" o.c.	0
9 '	6'	NA	5	2	17" o.c.	19" o.c.	NA	24" o.c.	17" o.c.	16" o.c.	0
9 '	7 '	NA	8	2	11" o.c.	12" o.c.	NA	15" o.c.	11" o.c.	11" o.c.	0
9 '	8 '	NA	11	NA	7" o.c.	8" o.c.	NA	10" o.c.	7" o.c.	8" o.c.	0

### NOTES:

- 1. Fastener Types A,B,C & D are not reflected in charts and are available prescriptively per table R404.1(1) in 2006 IRC.
- 2. See details for additional fastener options.
- 3. All fastener spacing must start within 12" maximum of each corner or half specified spacing (lesser of two).
- 4. Type F & G connectors are qty. per 16" oc. Joist spacing.
- 5. Fastener Type Key:
- "Type E"- Fasteners toe-nailed through rim joist into sill plate (Refer to Detail E)
- "Type F"- Fasteners direct nailed from sill plate into each floor joist (Applicable at Sidewalls only) (Refer to Detail F)
- "Type G"- Number of Simpson A23 angles fastened to sill plate and each 16" OC. (2x8 min. sill plate) (Refer to Detail G)
- "Type H"- Simpson LPT4 or TP35 plate fastened to rim joist and mud sill with (12) 8dx1.5" treated nails. (Refer to Detail H) Anchor Types:
- "Type 4"- 1/2" x10" Anchor Bolt with 2"x2"x1/8" Washer between plate and nut.
- "Type 5"- Simpson MAB15 (concrete) or MAB23 (concrete block) or MASA
- 6. Fasteners are in addition to (2) Type H tie plates spaced within 6' of corners & 96" oc. elsewhere along sidewalls.(See note 3)
- 7. Fasteners are in addition to Type H tie plates spaced at 33" oc. along endwall.
- 8. Three options (E,F,& G) for rim to sill fastening and two options (4 & 5) for sill plate to foundation anchorage

have been provided in chart. Any combination of rim sill connectors and mud sill anchors maybe used.

- 9. All connection hardware, anchor bolts, straps, hold-downs, washers and fasteners shall be galvanized or stainless when in contact with PT sill plates or other PT lumber.
- 10. Maximum foundation wall height and maximum unbalanced backfill.

# Home Floor to Sill Plate & Sill Plate to Foundation WITHOUT TYPE H PLATE CONNECTORS (See note 6 & 7) SOIL CLASSES GW, GP, SW AND SP SOILS

Unit Width: 29.67' to 29.67' Max.

Unit Length: 76' Max.
Roof Pitch: 3/12 to 6/12

Max. Roof Overhang: 12 "
Max. Sidewall Height: 9 '

\*Wind Speed (3s): 100 Seismic Zone C



		M	AXIMUM F	ASTENER	SPACING	OR FASTE	NERS PER	R JOIST SP	ACING 2,3	& 5	# REQ'D
		SI	DEWALL F	ASTENING	G SPACING	<b>3</b> ¹	E	ND WALL	FASTENIN	G	S/W HDS
Foundati	ion Wall <sup>10</sup>		Rim to Sill <sup>t</sup>	Ó	Sill to F	nd. Wall	Rim t	o Sill'	Sill to F	nd. Wall	SEE
Wall	Backfill	Fa	astener Typ		Anchor	Spacing	Fasten	er Type	Anchor Spacing		D18
Height	Depth	Е	F ⁴	G⁴	4	5	Е	G	4	5	/CORNER
24 "	16 "	9.6" o.c.	1	1	72" o.c.	72" o.c.	8" o.c.	30" o.c.	57" o.c.	30" o.c.	1
32 "	24 "	9.6" o.c.	1	1	72" o.c.	72" o.c.	8" o.c.	30" o.c.	56" o.c.	30" o.c.	1
40 ''	32 "	9.6" o.c.	1	1	72" o.c.	72" o.c.	8" o.c.	28" o.c.	54" o.c.	30" o.c.	1
3.833 '	3.33 '	9.6" o.c.	1	1	72" o.c.	72" o.c.	7" o.c.	25" o.c.	50" o.c.	29" o.c.	1
7 '	4 '	9.6" o.c.	1	1	72" o.c.	72" o.c.	7" o.c.	26" o.c.	51" o.c.	29" o.c.	1
7 '	5 '	5.3" o.c.	2	1	46" o.c.	51" o.c.	5" o.c.	19" o.c.	40" o.c.	26" o.c.	1
7 '	6 '	3.1" o.c.	3	1	26" o.c.	29" o.c.	3" o.c.	12" o.c.	26" o.c.	21" o.c.	1
8 '	4 '	9.6" o.c.	1	1	72" o.c.	72" o.c.	7" o.c.	27" o.c.	52" o.c.	29" o.c.	1
8	5 '	6.1" o.c.	2	1	52" o.c.	58" o.c.	6" o.c.	21" o.c.	43" o.c.	27" o.c.	1
8 '	6 '	3.5" o.c.	3	1	30" o.c.	33" o.c.	4" o.c.	13" o.c.	30" o.c.	23" o.c.	1
8 '	7 '	NA	5	1	19" o.c.	21" o.c.	NA	8" o.c.	19" o.c.	17" o.c.	0
9	3 '	9.6" o.c.	1	1	72" o.c.	72" o.c.	8" o.c.	30" o.c.	56" o.c.	30" o.c.	1
9 '	4 '	9.6" o.c.	1	1	72" o.c.	72" o.c.	7" o.c.	27" o.c.	53" o.c.	29" o.c.	1
9 '	5 '	6.8" o.c.	2	1	59" o.c.	65" o.c.	6" o.c.	22" o.c.	45" o.c.	27" o.c.	1
9 '	6'	4.0" o.c.	3	1	34" o.c.	38" o.c.	4" o.c.	15" o.c.	33" o.c.	24" o.c.	1
9 '	7 '	NA	4	1	21" o.c.	24" o.c.	NA	9" o.c.	21" o.c.	19" o.c.	0
9 '	8 '	NA	6	2	14" o.c.	16" o.c.	NA	6" o.c.	14" o.c.	14" o.c.	0

### NOTES:

- 1. RESERVED
- 2. See details for additional fastener options.
- 3. All fastener spacing must start within 12" maximum of each corner or half specified spacing (lesser of two).
- 4. Type F & G connectors are qty. per 16" oc. Joist spacing.
- Fastener Type Key:
- " Type E"- Fasteners toe-nailed through rim joist into sill plate (Refer to Detail E)
- "Type F"- Fasteners direct nailed from sill plate into each floor joist (Applicable at Sidewalls only) (Refer to Detail F)
- "Type G"- Number of Simpson A23 angles fastened to sill plate and each 16" OC. (2x8 min. sill plate) (Refer to Detail G)
- "Type H"- Simpson LPT4 or TP35 plate fastened to rim joist and mud sill with (12) 8dx1.5" treated nails. (Refer to Detail H) Anchor Types:
  - "Type 4"- 1/2" x10" Anchor Bolt with 2"x2"x1/8" Washer between plate and nut.
  - "Type 5"- Simpson MAB15 (concrete) or MAB23 (concrete block) or MASA
- 6. Fasteners reflected in chart do NOT require "H type" connector plates to be installed along sidewall.
- 7. Fasteners reflected in chart do NOT require "H type" connector plates to be installed along endwall.
- 8. Three options (E,F,& G) for rim to sill fastening and two options (4 & 5) for sill plate to foundation anchorage have been provided in chart. Any combination of rim sill connectors and mud sill anchors maybe used.
- 9. All connection hardware, anchor bolts, straps, hold-downs, washers and fasteners shall be galvanized or stainless when in contact with PT sill plates or other PT lumber.
- 10. Maximum foundation wall height and maximum unbalanced backfill.

# Home Floor to Sill Plate & Sill Plate to Foundation WITHOUT TYPE H PLATE CONNECTORS (See note 6 & 7) SOIL CLASSES GM, GC, SM, SM-SC AND ML SOILS

Unit Width: 29.67' to 29.67' Max.

Unit Length: 76' Max. Roof Pitch: 3/12 to 6/12

Max. Roof Overhang: 12 "
Max. Sidewall Height: 9 '

\*Wind Speed (3s): 100 Seismic Zone C



		M	AXIMUM F	ASTENER	<b>SPACING</b>	OR FASTE	NERS PER	R JOIST SP	ACING 2,3	<u>%</u> 5	# REQ'D
		SI	<b>DEWALL F</b>	ASTENING	G SPACING	<b>3</b> ¹	E	ND WALL	FASTENIN	G	S/W HDS
Foundati	ion Wall <sup>10</sup>		Rim to Sill <sup>t</sup>	ő	Sill to F	nd. Wall	Rim to Sill'		Sill to Fnd. Wall		SEE
Wall	Backfill		astener Typ		Anchor	Spacing	Fasten	er Type	Anchor Spacing		D18
Height	Depth	E <sup>9</sup>	F <sup>4</sup>	G⁴	4	5	Е	G	4	5	/CORNER
24 "	16 "	9.6" o.c.	1	1	72" o.c.	72" o.c.	8" o.c.	30" o.c.	56" o.c.	30" o.c.	1
32 "	24 "	9.6" o.c.	1	1	72" o.c.	72" o.c.	8" o.c.	29" o.c.	55" o.c.	30" o.c.	1
40 ''	32 "	9.6" o.c.	1	1	72" o.c.	72" o.c.	7" o.c.	26" o.c.	51" o.c.	29" o.c.	1
3.833 '	3.33 '	6.6" o.c.	2	1	56" o.c.	62" o.c.	6" o.c.	21" o.c.	44" o.c.	27" o.c.	1
7 '	4 '	6.9" o.c.	2	1	60" o.c.	66" o.c.	6" o.c.	22" o.c.	45" o.c.	27" o.c.	1
7 '	5 '	3.5" o.c.	3	1	30" o.c.	34" o.c.	4" o.c.	13" o.c.	30" o.c.	23" o.c.	1
7 '	6 '	NA	5	1	18" o.c.	19" o.c.	NA	8" o.c.	18" o.c.	16" o.c.	0
8 '	4 '	7.9" o.c.	2	1	68" o.c.	72" o.c.	6" o.c.	23" o.c.	47" o.c.	28" o.c.	1
8 '	5 '	4.1" o.c.	3	1	35" o.c.	38" o.c.	4" o.c.	15" o.c.	34" o.c.	24" o.c.	1
8 '	6 '	NA	4	1	20" o.c.	22" o.c.	NA	9" o.c.	20" o.c.	18" o.c.	0
8 '	7 '	NA	7	2	13" o.c.	14" o.c.	NA	5" o.c.	13" o.c.	13" o.c.	0
9'	3 '	9.6" o.c.	1	1	72" o.c.	72" o.c.	8" o.c.	29" o.c.	55" o.c.	30" o.c.	1
9'	4 '	8.9" o.c.	2	1	72" o.c.	72" o.c.	7" o.c.	25" o.c.	49" o.c.	28" o.c.	1
9'	5 '	4.6" o.c.	2	1	39" o.c.	43" o.c.	5" o.c.	17" o.c.	36" o.c.	25" o.c.	1
9'	6 '	NA	4	1	23" o.c.	25" o.c.	NA	10" o.c.	23" o.c.	19" o.c.	0
9'	7 '	NA	6	2	14" o.c.	16" o.c.	NA	6" o.c.	14" o.c.	14" o.c.	0
9 '	8 '	NA	9	2	10" o.c.	11" o.c.	NA	4" o.c.	10" o.c.	10" o.c.	0

### NOTES:

- 1. RESERVED
- 2. See details for additional fastener options.
- 3. All fastener spacing must start within 12" maximum of each corner or half specified spacing (lesser of two).
- 4. Type F & G connectors are qty. per 16" oc. Joist spacing.
- 5. Fastener Type Key:
- " Type E"- Fasteners toe-nailed through rim joist into sill plate (Refer to Detail E)
- "Type F"- Fasteners direct nailed from sill plate into each floor joist (Applicable at Sidewalls only) (Refer to Detail F)
- "Type G"- Number of Simpson A23 angles fastened to sill plate and each 16" OC. (2x8 min. sill plate) (Refer to Detail G)
- "Type H"- Simpson LPT4 or TP35 plate fastened to rim joist and mud sill with (12) 8dx1.5" treated nails. (Refer to Detail H) Anchor Types:
  - "Type 4"- 1/2" x10" Anchor Bolt with 2"x2"x1/8" Washer between plate and nut.
- "Type 5"- Simpson MAB15 (concrete) or MAB23 (concrete block) or MASA
- 6. Fasteners reflected in chart do NOT require "H type" connector plates to be installed along sidewall.
- 7. Fasteners reflected in chart do NOT require "H type" connector plates to be installed along endwall.
- $8. \ Three \ options \ (\textit{E,F,\& G}) \ for \ \textit{rim to sill fastening and two options} \ (\textit{4\& 5}) \ for \ \textit{sill plate to foundation anchorage}$
- have been provided in chart. Any combination of rim sill connectors and mud sill anchors maybe used.
- 9. All connection hardware, anchor bolts, straps, hold-downs, washers and fasteners shall be galvanized or stainless when in contact with PT sill plates or other PT lumber.
- 10. Maximum foundation wall height and maximum unbalanced backfill.

# Home Floor to Sill Plate & Sill Plate to Foundation WITHOUT TYPE H PLATE CONNECTORS (See note 6 & 7) SOIL CLASSES SC, MH, ML-CL AND INORGANIC CL SOILS

Unit Width: 29.67' to 29.67' Max.

Unit Length: 76' Max.
Roof Pitch: 3/12 to 6/12

Max. Roof Overhang: 12 "
Max. Sidewall Height: 9 '

\*Wind Speed (3s): 100 Seismic Zone C



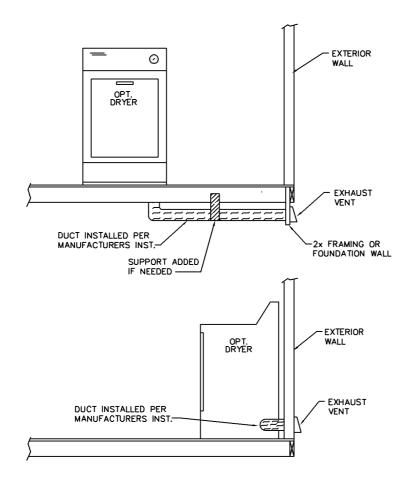
		M	AXIMUM F	ASTENER	<b>SPACING</b>	OR FASTE	NERS PER	R JOIST SP	ACING 2,3	% 5	# REQ'D
		SI	DEWALL F	FASTENING	SPACINO	<b>ì</b> ¹	E	ND WALL	FASTENIN	G	S/W HDS
Foundati	on Wall <sup>10</sup>		Rim to Sill	6	Sill to F	nd. Wall	Rim t	o Sill <sup>′</sup>	Sill to Fnd. Wall		SEE
Wall	Backfill		astener Typ		Anchor	Spacing	Fastener Type		Anchor Spacing		D18
Height	Depth	E	F <sup>4</sup>	G⁴	4 5		E G		4	5	/CORNER
24 "	16 "	9.6" o.c.	1	1	72" o.c.	72" o.c.	8" o.c.	30" o.c.	56" o.c.	30" o.c.	1
32 "	24 "	9.6" o.c.	1	1	72" o.c.	72" o.c.	7" o.c.	28" o.c.	54" o.c.	29" o.c.	1
40 ''	32 "	8.4" o.c.	2	1	72" o.c.	72" o.c.	7" o.c.	24" o.c.	48" o.c.	28" o.c.	1
3.833 '	3.33 '	4.9" o.c.	2	1	42" o.c.	47" o.c.	5" o.c.	18" o.c.	38" o.c.	25" o.c.	1
7 '	4 '	5.2" o.c.	2	1	45" o.c.	49" o.c.	5" o.c.	19" o.c.	39" o.c.	26" o.c.	1
7 '	5 '	NA	4	1	23" o.c.	25" o.c.	NA	10" o.c.	23" o.c.	20" o.c.	0
7'	6 '	NA	6	2	13" o.c.	15" o.c.	NA	6" o.c.	13" o.c.	13" o.c.	0
8 '	4 '	5.9" o.c.	2	1	51" o.c.	56" o.c.	6" o.c.	20" o.c.	42" o.c.	27" o.c.	1
8 '	5 '	3.0" o.c.	3	1	26" o.c.	29" o.c.	3" o.c.	12" o.c.	26" o.c.	21" o.c.	1
8 '	6 '	NA	6	2	15" o.c.	17" o.c.	NA	6" o.c.	15" o.c.	15" o.c.	0
8 '	7 '	NA	9	2	10" o.c.	11" o.c.	NA	4" o.c.	10" o.c.	10" o.c.	0
9 '	3 '	9.6" o.c.	1	1	72" o.c.	72" o.c.	7" o.c.	28" o.c.	54" o.c.	29" o.c.	1
9 '	4 '	6.7" o.c.	2	1	57" o.c.	63" o.c.	6" o.c.	22" o.c.	44" o.c.	27" o.c.	1
9 '	5 '	3.4" o.c.	3	1	29" o.c.	32" o.c.	4" o.c.	13" o.c.	29" o.c.	22" o.c.	1
9 '	6'	NA	5	2	17" o.c.	19" o.c.	NA	7" o.c.	17" o.c.	16" o.c.	0
9 '	7 '	NA	8	2	11" o.c.	12" o.c.	NA	4" o.c.	11" o.c.	11" o.c.	0
9'	8 '	NA	11	NA	7" o.c.	8" o.c.	NA	3" o.c.	7" o.c.	8" o.c.	0

### NOTES:

- 1. RESERVED
- 2. See details for additional fastener options.
- 3. All fastener spacing must start within 12" maximum of each corner or half specified spacing (lesser of two).
- 4. Type F & G connectors are qty. per 16" oc. Joist spacing.
- 5. Fastener Type Key:
- " Type E"- Fasteners toe-nailed through rim joist into sill plate (Refer to Detail E)
- "Type F"- Fasteners direct nailed from sill plate into each floor joist (Applicable at Sidewalls only) (Refer to Detail F)
- "Type G"- Number of Simpson A23 angles fastened to sill plate and each 16" OC. (2x8 min. sill plate) (Refer to Detail G)
- "Type H"- Simpson LPT4 or TP35 plate fastened to rim joist and mud sill with (12) 8dx1.5" treated nails. (Refer to Detail H) Anchor Types:
  - "Type 4"- 1/2" x10" Anchor Bolt with 2"x2"x1/8" Washer between plate and nut.
- "Type 5"- Simpson MAB15 (concrete) or MAB23 (concrete block) or MASA
- 6. Fasteners reflected in chart do NOT require "H type" connector plates to be installed along sidewall.
- 7. Fasteners reflected in chart do NOT require "H type" connector plates to be installed along endwall.
- 8. Three options (E,F,& G) for rim to sill fastening and two options (4 & 5) for sill plate to foundation anchorage

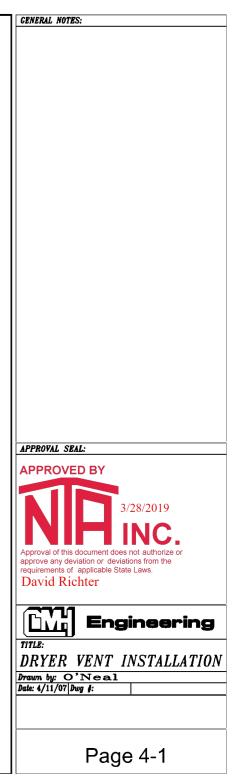
have been provided in chart. Any combination of rim sill connectors and mud sill anchors maybe used.

- 9. All connection hardware, anchor bolts, straps, hold-downs, washers and fasteners shall be galvanized or stainless when in contact with PT sill plates or other PT lumber.
- 10. Maximum foundation wall height and maximum unbalanced backfill.



### INSTALLTION INSTRUCTIONS:

EXHAUST DUCTS FOR DOMESTIC CLOTHES DRYERS SHALL BE CONSTRUCTED OF METAL OR NONCOMBUSTIBLE MATERIAL OF EQUAL STRENGTH AND CORROSION RESISTANCE AND SHALL HAVE A SMOOTH INTERIOR FINISH, NO PART OF THE DUCT TO BE IN CONTACT WITH THE GROUND. THE DUCT TO RUN TO THE OUTSIDE OF THE UNIT AND SHALL NOT TERMINATE UNDERNEATH THE UNIT. A APPROVED DAMPER TO BE INSTALLED ON THE END OF THE DUCT.



# **ELECTRICAL FURNACE DESCRIPTION CHART**

Nortek							nded Wire zes	
Model						NM-B	SEU*	Low Voltage
E Series				Max Over-	Min. Circuit	60°C	60°C	Thermostat Wire
	Supply C	Circuit	Total Amperes	Current Rating	Ampacity	Copper	Copper	Size
010	Single		44.6	60	56	4-2	4-4-6	
012	Single		51.2	70	64	4-2	4-4-6	2-Wire
	Dual	"A"	27.1	40	34	8-2	6-6-10	system max wire
		"B"	24.2	30	30	10-2	8-8-10	lengths:
015	Single		N/A	N/A	N/A			24 Ga. = 55'
	Dual	"A"	44.6	60	56	4-2	4-4-6	22 Ga. = 90'
		"B"	20.8	30	26	10-2	8-8-10	20 Ga. = 140'
017	Single		N/A	N/A	N/A			24 Ga. = 55'
	Dual	"A"	47.9	60	60	4-2	4-4-6	22 Ga. = 90'
		"B"	22.5	30	28	10-2	8-8-10	20 Ga. = 140'
020	Single		N/A	N/A	N/A			18 Ga. = 225'
	Dual	"A"	44.6	60	56	4-2	4-4-6	
		"B"	41.7	60	52	4-2	4-4-6	4 or more-Wire
023	Single		N/A	N/A	N/A			system max wire
	Dual	"A"	45.5	60	57	4-2	4-4-6	lengths:
		"B"	48.0	60	60	4-2	4-4-6	24 Ga. = 25'
						-		22 Ga. = 45'
								20 Ga. = 70'
								18 Ga. = 110'

ELECTRIC FURNACE MODEL NUMBER	OUTPUT CAPACITY (BTU)
E#EB-010H	35,000
E#EB-012H	41,000
E#EB-015H	53,000
E#EB-017H	57,000
E#EB-020H	70,000
E#EB-023H	75,000
E#EB-023H	75,000

# = Series Version

\*- NEC Section 338.10(B)(4)(a)



ELE(	CTRICAL LEGEN	D (NOT	TO SCALE)
$\bigcirc$	LIGHT		PANEL BOX
-CAN-	CAN LIGHT	(T)	THERMOSTAT
	PULL CHAIN LIGHT	-₩-	SWITCH
9	BATH FAN	<u>.</u> ⊕.	3-WAY SWITCH
	FLUORESCENT LIGHT	$\square$	PHONE JACK
TV	CABLE JACK	© CO	CEILING MOUNT C.O. & SMOKE DETECTOR
<del></del>	15 AMP RECEPT FLOOR LEVEL	$\otimes_{co}$	CEILING MOUNT C.O. DETECTOR
	15 AMP RECEPT CABINET LEVEL	<b>(D)</b>	WALL MOUNT SMOKE DETECTOR
	15 AMP RECEPT SIDEWAYS	(E)	CEILING MOUNT SMOKE DETECTOR
	20 AMP RECEPT FLOOR LEVEL	enterminate term total statistically.	SWITCH LEG
e or	20 AMP RECEPT Cabinet Level	GEN	JUNCTION BOX
	20 AMP RECEPT SIDEWAYS		CEILING FAN
	240 VOLT RECEPT		CEILING FAN
₩P GFI	15 AMP Waterproof Recept		POT & PAN RACK
H <sub>WP</sub> GFI	20 AMP WATERPROOF RECEPT		HEAT TAPE RECEPT
	FURNACE	WH	WATER HEATER
A D	ASHED SYMBOL RE	PRESEN1	S AN OPTION
GFI-I	NDICATES A GROUND	FAULT P	ROTECTED RECEPT



PLU	MBING FIXTURE DE	ESCRIPTION CHA	ART
APPLIANCE	MANUFACTURER	MODEL#	ANSI/ASME STANDARD
TOILET	BRISTOL BAY	VCEFB-03B	
SINKS	LYONS	KS01P4-TB	
	EL MUSTICE & SON	#610 UTILITY	
	PREMIUM FLOW	SINGLE BOWL	
	CORESTONE & TEKA	DOUBLE BOWL	
	REVERE	BAR SINK	
LAVITORIES	BRISTOL BAY	VCL-10	
TUB SHOWER	BAYMONT BATHWARE	5118	UL
		5100	
		5109	
SHOWER	BAYMONT BATHWARE	3309	UL
		3308	
		3304	
TUB	BAYMONT BATHWARE	2205 2272	UL





### **Trenco**

818 Soundside Rd Edenton, NC 27932

Re: WPL-913-0315-014\_(16W)
CMH MANUFACTURING - SCHULT (Rich-NC)

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Wood Perfect, Ltd.

Pages or sheets covered by this seal: I33865413 thru I33865426

My license renewal date for the state of North Carolina is December 31, 2018.

North Carolina COA: C-0844





July 3,2018

Galinski, John

**IMPORTANT NOTE:** Truss Engineer's responsibility is solely for design of individual trusses based upon design parameters shown on referenced truss drawings. Parameters have not been verified as appropriate for any use. Any location identification specified is for file reference only and has not been used in preparing design. Suitability of truss designs for any particular building is the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.

Wood Perfect, LLC, Guin, AL 33563

7.640 s Apr 22 2016 MiTek Industries, Inc. Mon Jul 02 15:37:12 2018 Page

Structural wood sheathing directly applied or 6-0-0 oc purlins,

APPROVED BY

David Richter

Rigid ceiling directly applied or 7-0-1 oc bracing.

except end verticals.

1 Brace at Jt(s): 12

1 Row at midpt

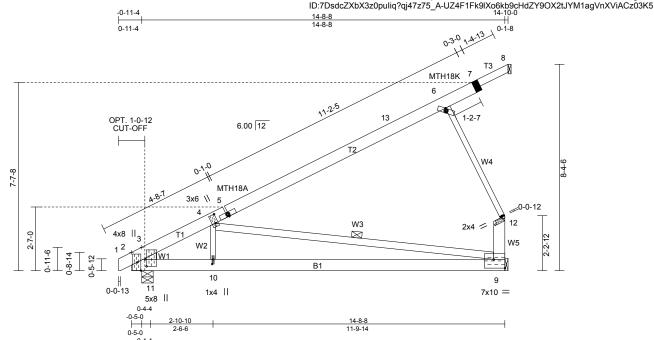


Plate Offset	Plate Offsets (X,Y) [2:0-4-0,0-1-12], [4:0-4-4,0-1-4], [5:0-0-5,0-1-2], [6:0-0-11,0-1-2], [9:0-8-5,1-2-12], [9:Edge,0-4-4], [10:0-2-4,0-0-8], [11:0-3-6,0-0-12]													
SPACING-: LOADING		SPACING-: LOADING		SPACING-	2-0-0	CSI.	0.70	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	23.1	TCLL	34.7	Plate Grip DOL Lumber DOL	1.15 1.15	TC BC	0.70 0.55	Vert(LL) Vert(CT)		9-10 9-10	>999 >551	240 180	MT20 MT18HS	197/144 197/144
(Ground Sn	(Ground Snow=30.0)		ow=45.0)	Rep Stress Incr	YES	WB	0.55	Horz(CT)	0.01	9-10	>ɔɔ ı n/a	n/a	IVII IONS	197/144
TCDL	TCDL 11.0 TCD		16.5			(Matrix)		11012(01)	0.01	3	11/G	TI/ CI	Weight: 80	lh
BCLL	0.0 *	BCLL	0.0 *	Code IBC2015/TPI2014		(iviali	ix)						FT = 0%	ID
BCDL	10.0	BCDL	15.0										F1 - 0%	

TOP CHORD

**BOT CHORD** 

**WEBS** 

**JOINTS** 

I UMRFR-**BRACING-**

TOP CHORD 2x6 SPF No.2 \*Except\*

7-8: 2x4 SPF No.2 2x6 SPF No.2

**BOT CHORD** WEBS

2x3 SPF Stud \*Except\*

4-9: 2x4 SPF No.2, 9-12: 2x6 SPF Stud, 3-11: 2x6 SP No.2

(lb/size) 9=627/Mechanical, 8=0/Mechanical, 2=735/0-5-8 REACTIONS.

Max Horz 8=-103(LC 19), 2=464(LC 12) Max Uplift 9=-506(LC 12), 2=-321(LC 12) Max Grav 9=730(LC 19), 2=771(LC 19)

FORCES. (lb) - Maximum Compression/Maximum Tension

1-2=-1/0, 2-3=-1340/530, 3-4=-1188/355, 4-5=-503/24, 5-13=-474/42, 6-13=-279/57, 6-7=-201/76, 7-8=-118/88, TOP CHORD

9-12=-463/492

2-11=-792/929, 10-11=-792/929, 9-10=-792/929 **BOT CHORD** 

4-10=0/439, 4-9=-779/569, 6-12=-511/543, 3-11=-273/252 **WEBS** 

REQUIRED FIELD JOINT CONNECTIONS - Maximum Compression (lb)/ Maximum Tension (lb)/ Maximum Shear (lb)/ Maximum Moment (lb-in)

7=148/84/60/0, 12=511/545/0/0

### NOTES-

- 1) Wind: ASCE 7-10; Vult=152mph (3-second gust) Vasd=120mph @24in o.c.; TCDL=4.4psf; BCDL=4.0psf; (Alt. 180mph @16in o.c.; TCDL=6.6psf; BCDL=6.0psf); h=22ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pg=30.0 psf (ground snow); Ps=23.1 psf (roof snow); Category II; Exp C; Partially Exp.; Ct=1.1
- 3) Roof design snow load has been reduced to account for slope.
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 18.0 psf or 2.00 times flat roof load of 23.1 psf on overhangs non-concurrent with other live loads.
- 6) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
- 7) All plates are MT20 plates unless otherwise indicated.
- 8) See HINGE PLATE DETAILS for plate placement.
- 9) Provisions must be made to prevent lateral movement of hinged member(s) during transportation.
- 10) All additional member connections shall be provided by others for forces as indicated.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 506 lb uplift at joint 9 and 321 lb uplift at joint 2. Continued on page 2



Approval of this document does not authorize or approve any deviation or deviations from the requirements of applicable State Laws.

July 3,2018

A WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	CMH MANUFACTURING - SCHULT (Rich-NC)	133865413
WPL-913-0315-014_(16W)	9529-15B	HINGED TRUSS	1		M9529 : 6/12 32 WIDE MOD/HUD	133003413

Wood Perfect, LLC, Guin, AL 33563

Job Reference (optional)
7.640 s Apr 22 2016 MiTek Industries, Inc. Mon Jul 02 15:37:12 2018 Page 2
ID:7DsdcZXbX3z0puliq?qj47z75\_A-UZ4F1Fk9lXo6kb9cHdZY9OX2tJYM1agVnXViACz03K5

#### NOTES-

- 14) This truss is designed in accordance with the 2015 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.
  15) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
  16) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.





### **Trenco**

818 Soundside Rd Edenton, NC 27932

Re: WPL-913-0315-014\_(16W)
CMH MANUFACTURING - SCHULT (Rich-NC)

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Wood Perfect, Ltd.

Pages or sheets covered by this seal: I33865459 thru I33865460

My license renewal date for the state of North Carolina is December 31, 2018.

North Carolina COA: C-0844





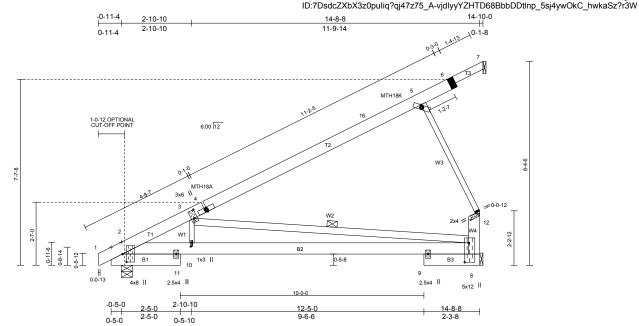
July 3,2018

Galinski, John

**IMPORTANT NOTE:** Truss Engineer's responsibility is solely for design of individual trusses based upon design parameters shown on referenced truss drawings. Parameters have not been verified as appropriate for any use. Any location identification specified is for file reference only and has not been used in preparing design. Suitability of truss designs for any particular building is the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.

Wood Perfect, LLC, Guin, AL 33563

7.640 s Apr 22 2016 MiTek Industries, Inc. Tue Jul 03 07:50:37 2018 Page



[2:0-3-14,0-2-0], [3:0-4-0,0-1-8], [4:0-0-5,0-1-2], [5:0-0-11,0-1-2], [8:0-9-8,0-2-0], [8:0-5-15,0-9-13], [10:0-1-12,0-0-8], [10:0-12,0-0-8], [10:0-12,Plate Offsets (X,Y)--SPACING-: 1-4-0 **SPACING-:** 1-0-0 SPACING-1-4-0 CSI. **DEFL** in I/defl L/d **PLATES GRIP** (loc) LOADING (psf) LOADING (psf) Plate Grip DOL 1.15 TC 0.49 Vert(LL) -0.18 8-10 >939 240 MT20 **TCLL** 23 1 TCLL 30 B

0.58 Vert(CT) -0.37 Lumber DOL 1.15 BC 8-10 >457 180 (Ground Snow=30.0) (Ground Snow=40.0) Rep Stress Incr YES WB 0.42 Horz(CT) 0.05 8 n/a n/a TCDL 11.0 TCDL 14.7 Code IBC2015/TPI2014 (Matrix-M) **BCLL** 0.0 **BCLL** 0.0 BCDI BCDL 10.0

197/144 MT18HS 197/144

Weight: 87 lb FT = 0%

LUMBER-

TOP CHORD 2x6 SPF No.2 \*Except\*

REQUIRED FIELD JOINT CONNECTIONS

6=99/56/40/0, 12=341/363/0/0

6-7: 2x4 SPF No.2

**BOT CHORD** 2x6 SPF No.2

**WEBS** 2x3 SPF Stud \*Except\* 3-8: 2x4 SPF No.2, 8-12: 2x6 SPF Stud **BRACING-**

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals.

**BOT CHORD** Rigid ceiling directly applied or 9-3-4 oc bracing.

**WEBS** 1 Row at midpt **JOINTS** 1 Brace at Jt(s): 12

REACTIONS. (lb/size) 2=497/0-5-8, 8=406/Mechanical, 7=0/Mechanical

> Max Horz 2=306(LC 12), 7=-68(LC 19) Max Uplift 2=-234(LC 12), 8=-334(LC 12) Max Grav 2=521(LC 19), 8=475(LC 19)

FORCES. (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/32, 2-3=-966/314, 3-4=-347/25, 4-16=-328/38, 5-16=-195/48, 5-6=-134/51, 6-7=-79/59, 8-12=-293/312

2-11=-613/799, 10-11=-607/798, 9-10=-589/806, 8-9=-577/778 BOT CHORD

3-10=0/323, 3-8=-683/432, 5-12=-341/362 **WEBS** 

APPROVED BY Approval of this document does not authorize or approve any deviation or deviations from the requirements of applicable State Laws.

**David Richter** - Maximum Compression (lb)/ Maximum Tension (lb)/ Maximum Shear (lb)/ Maximum Moment (lb-in)

## NOTES-

- Wind: ASCE 7-10; Vult=152mph (3-second gust) Vasd=120mph @16in o.c.; TCDL=4.4psf; BCDL=4.0psf; (Alt. 176mph @12in o.c.; TCDL=5.9psf; BCDL=5.3psf); h=22ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-10; Pg=30.0 psf (ground snow); Ps=23.1 psf (roof snow); Category II; Exp C; Partially Exp.; Ct=1.1
- 3) Roof design snow load has been reduced to account for slope.
- 4) Unbalanced snow loads have been considered for this design
- 5) This truss has been designed for greater of min roof live load of 18.0 psf or 2.00 times flat roof load of 23.1 psf on overhangs non-concurrent with other live loads.
- As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
- All plates are MT20 plates unless otherwise indicated.
- 8) See HINGE PLATE DETAILS for plate placement.
- 9) Provisions must be made to prevent lateral movement of hinged member(s) during transportation.
- 10) All additional member connections shall be provided by others for forces as indicated.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 234 lb uplift at joint 2 and 319 lb uplift at joint 8.



July 3,2018

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

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Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	CMH MANUFACTURING - SCHULT (Rich-NC)	133865459
WPL-913-0315-014_(16W)	9529-15	HINGED TRUSS	1		T9529 - 6/12 9' FLAT (RICH) - 15	133603439

Wood Perfect, LLC, Guin, AL 33563

7.640 s Apr 22 2016 MiTek Industries, Inc. Tue Jul 03 07:50:38 2018 Page 2 ID:7DsdcZXbX3z0puliq?qi47z75\_A-NvB79HZB2mLzlLAnnxP\_J1XGc7QBfrzMDLfH6vz?r3V

#### NOTES-

- 14) This truss is designed in accordance with the 2015 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.
  15) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
  16) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

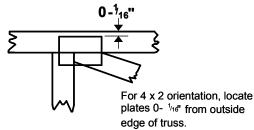


# **Symbols**

### **PLATE LOCATION AND ORIENTATION**



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.





This symbol indicates the required direction of slots in connector plates.

\* Plate location details available in MiTek 20/20 software or upon request.

### PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

### LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

### **BEARING**



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

### **Industry Standards:**

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.

DSB-89: Design Standard for Bracing.

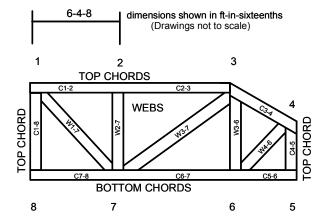
BCSI:

Building Component Safety Information. Guide to Good Practice for Handling,

Installing & Bracing of Metal Plate

Connected Wood Trusses

## **Numbering System**



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

**CHORDS AND WEBS ARE IDENTIFIED BY END JOINT** NUMBERS/LETTERS. APPROVED BY

**PRODUCT CODE APPROVALS** ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988

ER-3907, ESR-2362, ESR-1397, ESR-9282s document does not authorize or approve any deviation or deviations from the requirements of applicable State Laws. **David Richter** 

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015



# **General Safety Notes**

## Failure to Follow Could Cause Property Damage or Personal Injury

- 1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- 2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- 3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- 4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- 5. Cut members to bear tightly against each other.
- 6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- 7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- 8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- 9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- 10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- 11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- 12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- 13. Top chords must be sheathed or purlins provided at spacing indicated on design.
- 14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- 15. Connections not shown are the responsibility of others.
- 16. Do not cut or alter truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- 18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- 19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- 20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

**Project Description** 

Model Number: 3440

Customer:

State(s): NC, SC

Serial Number:

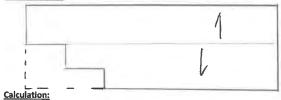
### Objective:

Determine the following elements associated with a simply supported flexible diaphragm:

- 1. Load to supporting shearwalls
- II. Required Diaphragm Capacity
- III. Maximum moment expereinced in diaphragm
- IV. Maximum tension experienced in diaphragm chord
- V. Required diaphragm chord

Wall Height =	9 ft		
Distance between shearwalls =	76 ft		
Diaphragm width at Left S/W =	14.83 ft	Dia Width at Right S/W=	29.67 ft
Roof Pitch (x/12)=	6:12		
End Zone Distance =	6 ft		
Interior Zone Distance =	32 ft		
Available S/W (Right) =	22 ft		
Available S/W (Left)=	8.33 ft		
Diaphragm Width at Max Tension =	29.67 ft		

#### Sketch/Layout:



### I. Determine Load to Shearwalls

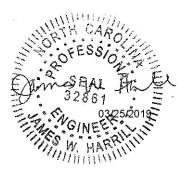
Wind Speed (mph)	MWFRS EZ plf	MWFRS IZ plf	Load (lbs)	Right S/W (plf)	Left S/W (plf)	
90	147	117	4626	211	556 -	1
100	182	144	5700	260	685	1
110	220	174	6888	314	827	2-sided required
120	272	215	8512	387	1022	2-sided required
130	307	243	9618	438	1155	2-sided required
140	356	282	11160	508	1340	2-sided required

### II. Determine required Diaphragm based Upon Min. Width at Shearwall

Wind Speed (mph)	Required Capacity (plf)
90	312
100	385
110	465

Required Capacity
(plf)
156
193
233







### Available Shearwall Designs

125

195

241

320 366

489

756 781

### Double Sided Shearwall Designs (Not included in Manuals)

656

978

1314

1624

120	574
130	649
140	753

120	287
130	325
140	377

Staple	ed Diaphragn	1 Capacitie:	s (Case 1)			
Fastener	Boundary	Edge (in)	Field (in)	ESR 1539	SPF Adj.	Capacity
7/16" x 1 1/2" x 14, 15, 16 ga staples	-	6	12	150	0.82	123
7/16" x 1 1/2" x 14, 15, 16 ga staples	6	6	12	165	0.82	135
7/16" x 1 1/2" x 14, 15, 16 ga staples	4	6	12	225	0.82	185
7/16" x 1 1/2" x 14, 15, 16 ga staples	2 1/2	4	12	335	0.82	275
7/16" x 1 1/2" x 14, 15, 16 ga staples	2	3	12	380	0.82	312

	<b>Nailed Diaphragm</b>	Capacities	(Case 1)			
Fastener	Boundary	Edge (in.)	Field (in.)	ESR 1539	SPF Adi	Capacity
.131 x 2" nails		6	12	320	0.92	294
.131 x 2" nails	6	6	1.2	360	0.92	331
.131 x 2" nails	4	6	12	475	0.92	437
.131 x 2" nails	2 1/2	4	12	705	0.92	649
.131 x 2" nails	2	3	12	805	0.92	741

### III. Determine maximum moment and chord tension

Wind Speed (mph)	Moment from End Zone Area (#-ft)	Moment from Int. Zone Area (#-ft)	Moment (#-ft)	Tension (lbs)
90	25110	59904	85014	2865
100	30924	73728	104652	3527
110	37368	89088	126456	4262
120	46176	110080	156256	5266
130	52182	124416	176598	5952
140	60552	144384	204936	6907

### IV. Diaphragm Chord Capacities

Tension Capacity of Diaphragm Chords (See state approved manual for design)		
Chord Type	Capacity (lbs)	
Type A	8335	
Туре В	7770	
Туре С	6495	
Type D	8970	
Type E	11040	

### V. Select Diaphragm Chord Design based Upon Maximum Tension

Wind Speed (mph)	Required Tension Capacity (lbs)	Available Chord Type(s)
90	2865	All
100	3527	All
110	4262	All
120	5266	All
130	5952	All
140	6907	Types A, B, D, & E

## VI. Determine Extent of Diaphragm Blocking (if necessary) at Left End Shearwall

Wind Speed (mph)	Max. Diaphragm Load (lbs)	Max Unblocked Capacity (lbs)	Max IZ Load (lbs)	Blocking Required	Extent of Blocking (ft)
90	4626	4366	3744	YES	2
100	5700	4366	4608	YES	8



140	11160	4366	9024	YES	23
130	9618	4366	7776	YES	-20 16
120	8512	4366	6880	YES	18-16
110	6888	4366	5568	YES	13

VII. Determine Extent of Diaphragm Blocking (if necessary) at Right End Shearwall

Wind Speed (mph)	Max Diaphragm Load (lbs)	Max. Unblocked Capacity	Max IZ Load (lbs)	Blocking Required	Extent of Blocking (ft)
90	4626	8735	3744	NO	0
100	5700	8735	4608	NO	0
110	6888	8735	5568	NO	0
120	8512	8735	6880	NO	0
130	9618	8735	7776	YES	2.9
140	11160	8735	9024	YES	7.0

disphrage width is 29.67'
seginning e 16' from
1 ST end

Diaphagn: Black 2' from left end of fack that the region of 131x2/2" na, b at 6" edge, 6" panel edger, \$ 12" panel Fields. Fack other areas Summary 90mph:

100 mph:

756 ptf (min.) per construction monual LA+ UM:

Right s/W! 325 plf (min.) per corrtruction manual

Blocket B' from left-end 4" boundary, 6" edge, 12" Field for B' from

Diaphragum: 131x 2"2" unit, a 4" boundary, 6" edge, 12" Field for B' from Diaphragum:

1 et - end. Remaining portion of disphrage to be unblocked & fastened w/. 131x 2"2"
nails a 6"E/12"F. 978 plf (min.) double-sided sheavnell per attached construction chart

110 mph: FAT 2/M: 320 plf (min.) per conduction normal Right SW:

Blacked 13' from left and & faster with . 131x 2"2" neily at Diaphragu : 2"2" along boundary edges, 4" prince adges, \$ 12" panel Fields.

Fastu with . 131 x 2"2" mil C 6" E112" f elsewhere

Right SIW: 120mph:

489 plf (min.) por construction namuel
1314 plf (min.) double-sided shearnell per attached construction chart Left SIW:

Block 16' from left end & fast- with . 131x2"2" noil fartened at 2"5" along boundary edges, 4" panel edges, & 12" panel fields. Farta with Diaphragun =

. 131x 21/2" rail à 6'E/12" E elsewhère.

1314 plf (min.) double-rided sheavned per attached construction chard 130 mph:

19+ 2/M: 489 pt (min.) per construction manual.

Block 16' from left end \$ 3' from right end. Fasta blocked regions Right SIW: With . 131 x 21/2" rail, @ 21/2" tourday edge, 4" edge, \$ 12" field. Fart Diagniragin:

remainder of displayer with . 131 x 21/2" nail c 6"E/12"F.

			S/W TO WALL	, FLOOR & C	EILING FAS	STENING 6		ENDWALL S/V	V HOLI	DDOWNS
						17		WITH SHEAT	HING (	OVERLAP 13,15
SHEAR	WALL CONSTRUCTION:		THRU 1/2" MAI		WOOD 10	WOOD		108		108
SW		PANEL FASTENING 16:	WITHOUT OVE	ERLAP 11	WITH OVE	RLAP 12	# SW 9	(# STRAPS) / 10	# SW 9	(# STRAPS) / 10
	WALL SHEATHING:	(EDGE SPACING/ FIELD SPACING)			#8x3"	.162"X3.5"		(FST/END)	JOIS1	(FST/END)
656.88	BOTH SIDES 7/16" (24/16) PS1/PS2 RATED WITH PANEL LENGTH RUNNING EITHER DIRECTION OF STUDS	.131X2.5" FASTENER AT: (6/12) INCHES O.C. (EDGE/FIELD)	1.3/ 2.2	2.1/ 3.5	1.9/ 3.2	3/ 4.9	3	(4) STRP. W/ (11) FST/END	3	(4) STRP. W/ (11) FST/END
978.88	BOTH SIDES 7/16" (24/16) PS1/PS2 RATED WITH PANEL LENGTH RUNNING EITHER DIRECTION OF STUDS	.131X2.5" FASTENER AT: (4/12) INCHES O.C. (EDGE/FIELD)	0.9/ 1.5	1.4/ 2.3	1.2/ 2.1	2/ 3.3	4	(5) STRP. W/ (13) FST/END	4	(5) STRP. W/ (13) FST/END
1313.8	BOTH SIDES 7/16" (24/16) PS1/PS2 RATED WITH PANEL LENGTH RUNNING EITHER DIRECTION OF STUDS	.131X2.5" FASTENER AT: (3/12) INCHES O.C. (EDGE/FIELD)	0.6/ 1.1	1/ 1.7	0.9/ 1.6	1.5/ 2.4	1	D18 TO FOUNDATION	1	D18 TO FOUNDATION
1674.4	BOTH SIDES 7/16" (24/16) PS1/PS2 RATED WITH PANEL LENGTH RUNNING EITHER DIRECTION OF STUDS	.131X2.5" FASTENER AT: (2/12) INCHES O.C. (EDGE/FIELD)	0.5/ 0.8	0.8/ 1.3	0.8/ 1.3	1.2/ 2	1	D18 TO FOUNDATION	1	D18 TO FOUNDATION
	SIDE 1: 7/16" (24/16) PS1/PS2 RATED WITH PANEL LENGTH RUNNING EITHER DIRECTION OF STUDS. SIDE 2: 1/2" gypsum board (unblocked edges) FASTENED WITH 5d nail/ 16 Ga. staples (7"/7")AT NONE" OC.	.131X2.5" FASTENER AT: (6/12) INCHES O.C. (EDGE/FIELD)	2.4/ 4.1	3.8/ 6.2	5.1/ 8.6	8/ 9.2	2	(2) STRP. W/ (12) FST/END	2	(2) STRP. W/ (12) FST/END
489	SIDE 1: 7/16" (24/16) PS1/PS2 RATED WITH PANEL LENGTH RUNNING EITHER DIRECTION OF STUDS.	.131X2.5" FASTENER AT: (4/12) INCHES O.C. (EDGE/FIELD)	1.8/ 3	2.8/ 4.7	4.4/ 7.4	6.9/ 9.2	2	(3) STRP. W/ (10) FST/END	2	(3) STRP. W/ (10) FST/END
	SIDE 1: 7/16" (24/16) PS1/PS2 RATED WITH PANEL LENGTH RUNNING EITHER DIRECTION OF STUDS. SIDE 2: 1/2" gypsum board (unblocked edges) FASTENED WITH 5d nail/ 16 Ga. staples (7"/7")AT NONE" OC.	.131X2.5" FASTENER AT: (3/12) INCHES O.C. (EDGE/FIELD)	1.1/ 1.9	1.8/ 3	2.4/ 4	3.7/ 6.1	3	(4) STRP. W/ (12) FST/END	3	(4) STRP. W/ (12) FST/END
	SIDE 1: 7/16" (24/16) PS1/PS2 RATED WITH PANEL LENGTH RUNNING EITHER DIRECTION OF STUDS. SIDE 2: 1/2" gypsum board (blocked edges) FASTENED WITH 5d nail/ 16 Ga. staples (7"/7")AT NONE" OC.	.131X2.5" FASTENER AT: (3/12) INCHES O.C. (EDGE/FIELD)	1.1/ 1.9	1.7/ 2.9	2.2/ 3.7	3.5/ 5.7	4	(4) STRP. W/ (12) FST/END	4	(4) STRP. W/ (12) FST/END

16 FASTENER LENGTH MUST BE ADJUSTED AS NECESSARY TO PROVIDE THE FOLLOWING MINIMUM PENETRATIONS INTO FRAMING MEMBERS: 131" NAILS-1 3/8"; .120" NAILS-1 3/8"; 14 GA STAPLES-1 1/2"; 15 GA STAPLES-1 1/4"; 16 GA STAPLES-1".

17 FIRST NUMBER INDICATES SPACE WHEN FASTENER PENETRATES THROUGH 1/2" MAX, GYPSUM AND SECOND SPACING ASSUMES FULL WOOD TO WOOD CONNECTION.







#### GENERAL NOTES:

- MINIMUM SHEARWALL SEGMENT LENGTH WHICH CAN BE CONSIDERED IN TOTAL EFFECTIVE LENGTH WITH CHARTED QTYS (Wind/ seismic catgorizes of D and above or gypsum shearwalls):
- a. 31"/54 " MINIMUM. FOR A MAXIMUM. SIDEWALL HEIGHT OF 108 ".
- 2 SHEARWALL FRAMING TO BE 2X4 MIN. STUDS AT 16" OC. MAX.(install panels either horizontally or vertically)
- 3 ALL PANEL EDGES ARE BACKED BY 2X4 MIN. BLOCKING.
- 4 SEE TRIB. SPAN TABLES FOR MINIMUM EFFECTIVE SHEARWALL LENGTHS BASED ON BOX SIZE AND CONFIGURATOIN
- 5 MINIMUM SHEARWALL HOLDDOWNS ARE REQUIRED AT THE END OF EACH FREE END OF SHEAR WALL SEQUMENT (SEE OTHER DETAILS FOR HOLD DOWN AND FASTENING CONSTRUCTION).
- 6 EACH EFFECTIVE SHEARWALL SEQUMENT SHOULD BE FASTENED TO ADJACENT WALLS, FLOOR AND TRUSSES PER ONE OF THE FASTENER OPTIONS AT SPACING INDICATED IN TABLE.
- 7 WHERE PANELS ARE APPLIED TO BOTH FACES OF A WALL AND FASTENER SPACING IS LESS THAN 6" OC. ON EITHER SIDE, PANEL JOINTS SHALL BE OFFSET OR FRAMING SHALL BE 3" NOMINAL AND FASTENERS ON EACH SIDE SHALL BE STAGGERED.
- 8 FRAMING AT ADJOINING PANEL EDGES SHALL BE 3" NOMINAL AND NAILS STAGGERED WHERE NAILS ARE SPACED 2" OC.
- 9 # SW JOIST: NUMBER OF #2 spf 1.5X9.25 JOIST REQUIRE UNDER SHEARWALL. JOIST MUST BE SECURED TO SUPPORTING FOUNDATION WALL PER FOUNDATION INSTRUCTIONS. MAXIMUM UNIT WIDTH: 2 SECTIONAL 178
- 10 NUMBER OF SIMPSON CS16 REQUIRED AT EACH FREE END OF SW SEGMENTS. (FST/END): NUMBER OF.131\*2.5\* NAILS NAILS REQUIRE PER END OF EACH STRAE, WHEN D18 TO FOUNDATION IS INDICATED IN TABLE A SPECIAL HOLDDOWN PER DETAIL D18 OF FOUNDATION INSTRUCTION MUST BE ATTACHED AT S/W FREE ENDS.
- 11 EXTERIOR SHEATHING DOES NOT OVER LAP CONNECTION JOINT. FASTENER CARRY ALL SHEARWALL LOADS.
- 12 EXTERIOR SHEATHING OVER LAPS CONNECTION JOINT AND IS FASTENED PER SHEARWALL EDGE FASTENING. CHART FASTENER CARRIES EXCESS LOAD ONLY.
- 13 EXTERIOR SHEATHING OVERLAPS WALL TO RIM JOIST JOINT. SEE NOTE 12.
- 14 N.R.: FREE END STRAPS ARE NOT REQUIRED.
- 15 .131"x3" NAILS MAY BE SUBSTATUTED FOR #8X3" WOOD SCREWS.

Clayton home building group

calc. ref. CSW-35.14.\_.\_.22-1.

SHEARWALL CONSTRUCTION & FASTENING

Drawn by: JWH Ver. 17.2

Date: 01/03/19

APPROVAL #:

PAGE 1 of 3 revised May 201  CMH MANUFACTUING INC.  3440  NTA INC.  DAVID RICHTER  Plan Sheet Page # and NOTES
CMH MANUFACTUING INC.  3440  NTA INC.  DAVID RICHTER
NTA INC.  DAVID RICHTER
NTA INC.  3/28/19  DAVID RICHTER
DAVID RICHTER
Plan Sheet Page # and NOTES
single family dwelling - not required
IX-1
1.0
1-0
1-0
1-0
1-0
1-0
1-0
1-0
1-0
1-0
1-0
1-0, HDD on REScheck (attached)
1-0
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1-0.2
TS-1
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Charts on 1-0, calc ref on 1-0
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20-1, 20-2, 1-0.2
20-1, 20-2
locations on floor plan 1-1
1-1
DWV: 8-1; Supply: 9-1
DWV: 8-1; Supply: 9-1
ref to electrical appliances on 1-0
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Design calculations	attached		
Installed unit capacity	attached		
Supply and returns (locations and sizes)	4-4,4-5		
Duct sizes	4-4,4-5		
Specifications (units, ducts)	1-1, 4-4,4-5		
All appliances furnished by mfg. shown on plans	1-1, exhaust fans 11-1		
<u>ELECTRICAL</u>			
Plan	11-1		
_ocation of all electrical boxes	11-1		
Electrical panel location	11-1		
Note regarding main disconnect (if applicable)	-		
xterior lighting and receptacles	11-1		
Ground level receptacles (if applicable)	11-1		
Smoke detector location(s)	11-1		
Electrical load calculations	TS-5		
Electrical panel layout (breaker and wire sizes, circuit schedule)	11-1		
Panel and service entrance sizes	Panel: 1-0a, SE ref in set-up on 1-0		
All fixtures furnished by mfg. shown on plans	11-1	•	
ACCESSIBILITY			
for other than 1 & 2 family dwellings) Entrances and means of egress			
Doors, doorways, and door hardware			
Stairs and handrails			
Foilet rooms, plumbing fixtures, grab bars, etc			
Bathrooms and shower rooms			
Occupancy specific requirements			
Multi-family dwellings: Type A and B units			
FLOOR X-SECTION	4.00		
oist and beam sizes and spacing Materials species and grade	1-0.2		
Pheathing, decking, and concrete as applicable	1-0.2		
fastening, decking, and concrete as applicable fastening instructions	1-0.2		
rastening instructions	1-0.2		
Details as required for clarification	1-0.2, other details ref manual	lon 1 0 2	
recails as required for clarification	1-0.2, other details ref manual	i on 1-0.2	
VALL X-SECTION			
Stud and column sizes and spacing	studs: 1-0.2; column charts: 1-	-0.2	
Materials species and grade	1-0.2		
sheathing and bracing	1-0.2		
leaders and lintels	header charts: 1-0.2		
inishes	1-0.2		
astening instructions	1-0.2		
nsulation Details as required for clarificaiton	1-0.2		

MODULAR PLANS REVIE	PAGE 3 of 3	revised N	
		TOVISCU II	
	Plan Sheet Pa	ge # and NOTES	
CEILING/ROOF X-SECTION			
Truss, rafter, and beam spacing	1-0.2		
Lumber species and grade	1-0.2		
Sheathing and decking	1-0.2		
Finishes	1-0.2		
Fastening instructions	1-0.2		
Insulation	1-0.2		
Details including NC sealed truss designs or manual			
reference	man ref to trusses 1-0.2, o	ther details man ref 1	
FOUNDATION PLAN			
Footings, pier, and curtain wall locations and specifications	21-30 PSF (OFF FRAME)	121-PS/ON ERAME)	
X-sections with dimensions	21-30 PSF (OFF FRAME		
Anchorage - sill plate to piers and curtain wall	21-30 PSF (OFF FRAME		
Anchorage - building to sill plate	21-30 PSF (OFF FRAME	1)21-PS(ON FRAME)	
Anchorage - tie downs (lateral and longitudinal)	21-30 PSF (OFF FRAME		
Soil bearing capacity	21-30 PSF (OFF FRAME)21-PS(ON FRAME)		
Minimum concrete compressive strength	21-30 PSF (OFF FRAME)21-PS(ON FRAME)		
Motar type	21-30 PSF (OFF FRAME)21-PS(ON FRAME		
Ventilation requirements (with and without vapor barrier)	21-30 PSF (OFF FRAME	S)21 DS/ON EDAME)	
Crawl space access requirements	21-30 PSF (OFF FRAME	)21-PS(ON FRAME)	
ENERGY COMPLIANCE			
Demonstrate compliance	PRESCRIPTIVE		
	THE SOUTH THE		
SET-UP INSTRUCTIONS			
Floor and ceiling connections	ref to set-up manual on 1-0	0.2	
Marriage wall connections	ref to set-up manual on 1-0		
Roof set-up connections	ref to set-up manual on 1-0		
Plumbing connections	ref to set-up manual on 1-0		
Mechanical connections	ref to set-up manual on 1-0		
Electrical connections	ref to set-up manual on 1-0	0.2	
Fire stopping	1-0.2		
Air infiltration elimination	ref to set-up manual on 1-0	0.2	
Notice to inspections department attachment if set-up			
instructions are by attachment	1-0.2		
ITEMS NOT INSPECTED IN PLANT			
List of items not inspected by 3rd. Party	1-0.2		
Notice to inspections department	1-0.2		