GENERAL REQUIREMENTS

REFER TO SUBSEQUENT PLAN AND DETAIL NOTES FOR VARIATIONS AND REQUIREMENTS SPECIFIC TO REFERENCED PROJECT.

NOTES ON DRAWINGS TAKE PRECEDENCE OVER GENERAL STRUCTURAL NOTES

REMEDIAL FOUNDATION REPAIR ACKNOWLEDGEMENT:
IT IS OUR UNIDERSTANDING THAT THE INTENT OF THIS REPAIR IS TO STABILIZE, AND POTENTIALLY LIFT THE
WORST CASE PORTIONS OF THE STRUCTURE, BY SUPPORTING THE FOUNDATION WITH STEEL UNDERPINNING
FOUNDED BELOW THE ACTIVE EXPANSION ZONE. IT SHOULD BE NOTED THAT THIS PROJECT IS VOLUNTARY
AND RECOMMENDATIONS FOR REPAIRS ARE NOT MANDATORY. IF THE RECOMMENDATIONS ARE NOT
ECONOMICALLY OR OTHERWISE FEASIBLE AT THIS TIME, THE HOMEOWHER MAY CHOOSE TO REDUCE THE
SCOPE OF THE REPAIR TO ADDRESS THE WORSE CASE PORTIONS OF THE FOUNDATION. HOWEVER, THE
HOMEOWNER SHOULD UNDERSTAND THAT THE NEW FOUNDATION SYSTEM WILL SUPPORT THE STRUCTURE
ONLY IN THE AREAS ADDRESSED. STRUCTURAL CONCERNS AND ADDITIONAL SETILEMENT MAY CONTINUE IN
THE REMAINDER OF THE FOUNDATION. IN THIS SCENARIO, THE HOME SHOULD BE CLOSELY MONITORED FOR
ADDITIONAL DAMAGE IN AREAS WHERE THE FOUNDATION IS NOT REPAIRED.

DESIGN CRITERIA

BUILDING CODE CONFORMANCE (NEETS OR EXCEEDS REQUIREMENTS)

2015 INTERNATIONAL RESIDENTIAL CODE (IRC)
2018 NORTH CAROLINA BUILDING CODE (NCBC)
2018 NORTH CAROLINA RESIDENTIAL CODE (NCRC) 2015 INTERNATIONAL BUILDING CODE (IBC)

DECK DEAD LOAD WOOD WALL DEAD BRICK DEAD LOAD DEAD LOADS: ROOF DEAD LOAD FLOOR DEAD LOAD

15 PSF 12 PSF 12 PSF 39 PSF 50 PCF

60 PSE PSE

ROOF LIVE

FLOOR LIVE LOAD (RESIDENTIAL) 6

ROOF LIVE L

CONCRETE

LOAO

HELICAL PIERS

BRACKET PLATES — ASTM A36
(MIN YIELD STRESS, Fy = 36 KS) / MIN TENSILE STRESS, Fu
PIER TUBES — ASTM A500 GRADE B OR C
(MIN YIELD STRESS, Fy = 50 KS) / MIN TENSILE STRESS, Fu
EXTERNAL SLEEVE — ASTM A500 GRADE B OR C ii = 58 KSI) 55 KSI)

(MIN YIELD STRESS, Fy = 50 KSI) PIER CAP - ASTM A529 GRADE 50 / MIN TENSILE STRESS, Fu = 62 KSI)

125 KSI)

(MIN YIELD STRESS, Fy = 50 KS) / MIN TENSILE STRESS, Fu = 65 KS!)
COIL ROD - ASTM A193 GRADE B7
(MIN YIELD STRESS, Fy = 125 KS
STEEL ANGLE SHAPES - ASTM A36
(MIN YIELD STRESS, Fy = 36 KS) / MIN TENSILE STRESS, Fu = 58 KS!)
SHAFT COUPLER - ASTM A513 TYPE 5

87 KSI)

(MIN YIELD STRESS, Fy = 70 KS) / MIN TENSILE STRESS, Fu = SHAFT COUPLING HARDWARE — GRADE 8 BOLTS WITH NUTS HELIX PLATES (ROUND SHAFT) — ASTM A572 GRADE 50

Fy = 50 KSI / MIN TENSILE STRESS, 2 65 KSI)

WELDING NOTES:
CONFORM TO AWS D1.1. WELDERS SHALL BE CERTIFIED IN ACCORDANCE WITH AWS REQUIREMENTS. USE E70 ELECTRODES OF TYPE REQUIRED FOR MATERIALS TO BE WELDED.

SCALE: NTS

ICINITY MAP

HELICAL PIERS (CONT.)

WELDING NOTES:
CONFORM TO AWS D1.1. W
WITH AWS REQUIREMENTS. U
MATERIALS TO BE WELDED. WELDERS SHALL BE CERTIFIED IN ACCORDANCE USE E70 ELECTRODES OF TYPE REQUIRED FOR

CORROSION PROTECTION:
SACRIFICIAL DESIGN THICKNESS – CAPACITIES INCLUDE A SCHEDULED LOSS
IN STEEL THICKNESS DUE TO CORROSION FOR BLACK, UNCOATED STEEL,
ANCHORS ARE DESIGNED FOR 50-YEAR SCHEDULED SACRIFICIAL THICKNESS
LOSS IN ACCORDANCE WITH ICC-ES AC358.



510

<u>INSTALLATION:</u>
SYSTEM TO BE INSTALLED PER MANUFACTURERS RECOMMENDATIONS.
TO BE DETERMINED BY THE FOLLOWING EQUATION: MINIMUM INSTALLATION PRESSURE IS

PIER INSTALLATION TORQUE (FT-LB): WORKING LOAD] \times [FS = 2] / [EMPERICAL TORQUE CORRELEATION FACTOR, KT = 9 FT-1]

HELICAL

MINIMUM INSTALLATION DEPTH IS 10"-0" UNO

NOTIFY ENGINEER IF MINIMUM INSTALLATION CONDITIONS CANNOT BE ACHIEVED

EXISTING UTILITY LINES: CONTRACTOR TO REPAIR

UTILITY LINES THAT MAY BE DAMAGED DURING INSTALLATION.

<u>HELICAL PIER SPLICING:</u> HELICAL LEAD AND EXTENSIONS ARE TO BE MECHANICALLY SPLICED WITH GRADE 8 BOLTS WITH NUTS.





REVISIONS

GENERAL NOTES

ROMANOSKI RESIDENCE FOUNDATION UNDERPINNING 409 S 16TH ST

ERWIN, NC 28339

SHEET NO:

DATE: 02-11-2021 CHECKED BY: PROJECT NO: SE21-023 DESIGNED BY: DH

DHAWN BY:

TESTING & INSPECTION

CONTINUOUS SPECIAL INSPECTION IS REQUIRED DURING INSTALLATION PER 2015 IBC SECTION 1810.4.12. THE SPECIAL INSPECTOR IS RESPONSIBLE FOR VERIFYING AND RECORDING THE FOLLOWING:

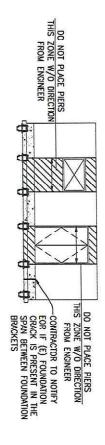
- PROJECT DESCRIPTION (ADDRESS, INSTALLATION DATE, PERMIT NUMBER PILE AND BRACKET CONFIGURATION
- PART DESCRIPTION (PRODUCT MANUFACTURER, BRACKET TYPE, PIER TYPE, PIER OUTSIDE DIAMETER,
- PIER WALL THICKNESS)
 PIER INCLINATION, LOCATION, DEPTH, AND INSTALLATION PRESSURE ACHIEVED

LOAD TESTING SHALL BE PERFORMED IN ACCORDANCE WITH ASTM METHOD D1143 (QUICK METHOD) ON 20 PERCENT OF PIERS AND WILL BE SELECTED BY THE SPECIAL INSPECTOR. AN ALICNMENT LOAD (AL) SHALL BE APPULED TO THE PILE PRIOR TO SETTING THE DEFLECTION MEASURING EQUIPMENT TO ZERO OR A REFERENCE POSITION. THE AL SHALL BE NO MORE THAN 10% OF THE DESIGN LOAD. INCREMENTAL LOADING SHALL BE IN ACCORDANCE WITH THE FOLLOWING SCHEDULE.

AF (10 0.25 pt 0.75 pt 1.25 pt TEST LOADING SCHEDULE DL MAX) HOLD TIME
O MIN.
UNTIL STABLE
UNTIL STABLE TEST (SEE BELOW) 0.04 INCHES MAX DEFLECTION

LOAD TESTING CREEP ACCEPTANCE CRITERIA SHALL BE NO GREATER THAN 0.04 INCHES WITHIN A 10 MINUTE PERIOD. IF MOVEMENT IS OBSERVED GREATER THAN 0.04 INCHES WITHIN THE 10 MINUTE PERIOD THE LOAD TEST SHALL BE HELD FOR AN ADDITIONAL 50 MINUTES, THE PIER IS TO BE DEEPERED AND REPLACED WITH A NEW PIER. IF THE LOAD TEST IS TO BE HELD THE PIER MOVEMENTS SHALL BE MEASURED AT 15, 20, 30, 40, 50, AND 60 MINUTES. THE CREEP VERSUS THE LOGARITHM OF TIME SHALL BE PLOTTED. IF THE CREEP RATE IS LESS THAN 0.080 INCHES BETWEEN 6 AND 60 MINUTES, THE LOAD TEST SHALL BE CONSIDERED SUCCESSFUL.

push pier test pressure (PSI): [design load] x 1.5 / [area of hydraulic ram]. Helical pier test pressure (PSI): [design load] x 1.5 / [area of hydraulic ram].



NO PIER PLACEMENT ZONE

SCALE: NTS

ROMANOSKI RESIDENCE FOUNDATION UNDERPINNING 409 S 16TH ST **ERWIN, NC 28339**



EXPIRES:

12/31/21

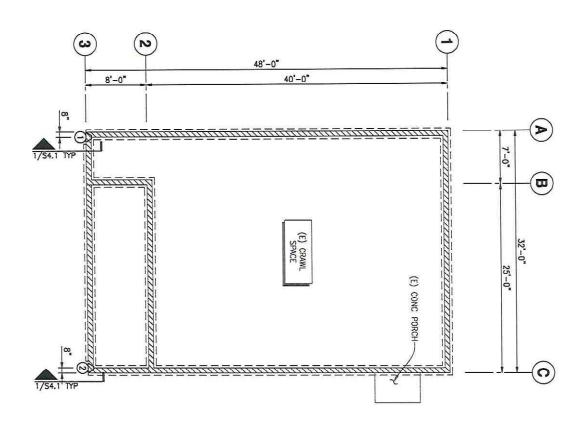
SHEET NO:

DATE: 02-11-2021

PROJECT NO: SE21-023 DESIGNED BY: DH CHECKED BY: DRAWN BY:

REVISIONS

GENERAL NOTES



(E) FOUNDATION/(N) PIER LAYOUT PLAN NOTES:

- REFERENCE S1.1 FOR GENERAL REQUIREMENTS
- CONTRACTOR TO NOTIFY ENGINEER OF RECORD OF DISCREPANCIES BETWEEN FIELD CONDITIONS & THOSE SHOWN IN THESE DOCUMENTS PRIOR TO WORK TYP
- INDICATES (E) CMU STEMWALL ON (E) CONC FOOTING (CONTRACTOR TO VERIFY 8"Wx3"-0"H (E) CMU STEMWALL AND 2"-0"Wx12"DP (E) CONC FOOTING MIN TYP (NOTIFY ENGINEER OF RECORD IF FIELD CONDITIONS DIFFER IN THE AREA OF WORK))



X/SX.X SECTION CUT - DETAIL NUMBER/SHEET NUMBER

INDICATES LOCATION OF FSI 288 HELICAL PIER W/ FSI FS288BL FOUNDATION BRACKET PER DETAILS ON S4.1 ((2) TOTAL)

- 9

- HELICAL PIER INSTALLATION NOTES:

 MAX LOAD TO ANCHOR = 6,135 LBS

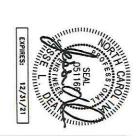
 2.875% PIPE PILE W/ 0.276" THICK WALL

 3.5%x24" LONG PIPE SLEEVE W/ 0.216" WALL

 0.375" THICK 10/12" HELIX W/ 1/4" FILLET WELDS EACH SIDE OF HELIX TO PIER

 MINIMUM 10'-O" INSTALLATION DEPTH & 1,500 FT-LB INSTALLATION TORQUE
- ALL CONSTRUCTION MATERIALS ON PLANS, ELEVATIONS & DETAILS ARE (N) UNO
- CONTRACTOR TO NOTIFY ENGINEER OF RECORD IF (E) FOUNDATION CRACK IS PRESENT IN THE SPAN BETWEEN FOUNDATION BRACKETS

PIER SPACING SHALL BE AS INDICATED ON PLAN



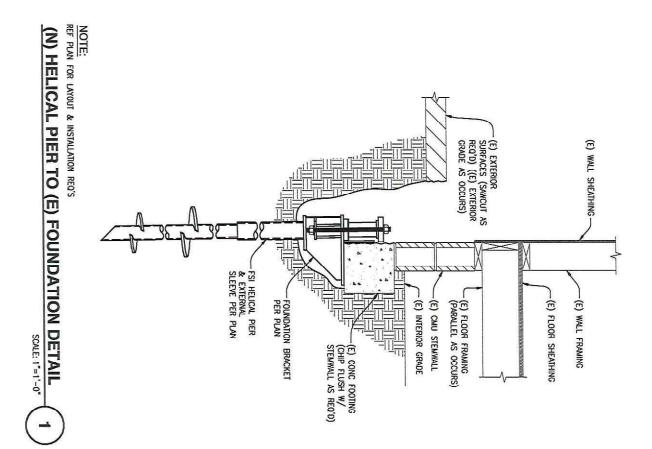
SHEET NO: S2.1

DATE: 02-11-2021

(E) FOUNDATION/ (N) PIER LAYOUT PLAN SNOISIVE

ROMANOSKI RESIDENCE FOUNDATION UNDERPINNING 409 S 16TH ST **ERWIN, NC 28339**







SHEET NO:

PROJECT NO: SE21-023 DESIGNED BY: DH DRAWN BY: DH CHECKED BY: JLD DATE: 02-11-2021

REVISIONS PIER DETAIL

ROMANOSKI RESIDENCE FOUNDATION UNDERPINNING 409 S 16TH ST ERWIN, NC 28339

