

29 GA ROOFING +
SIDING

2x4 RAFTERS
2' O.C.

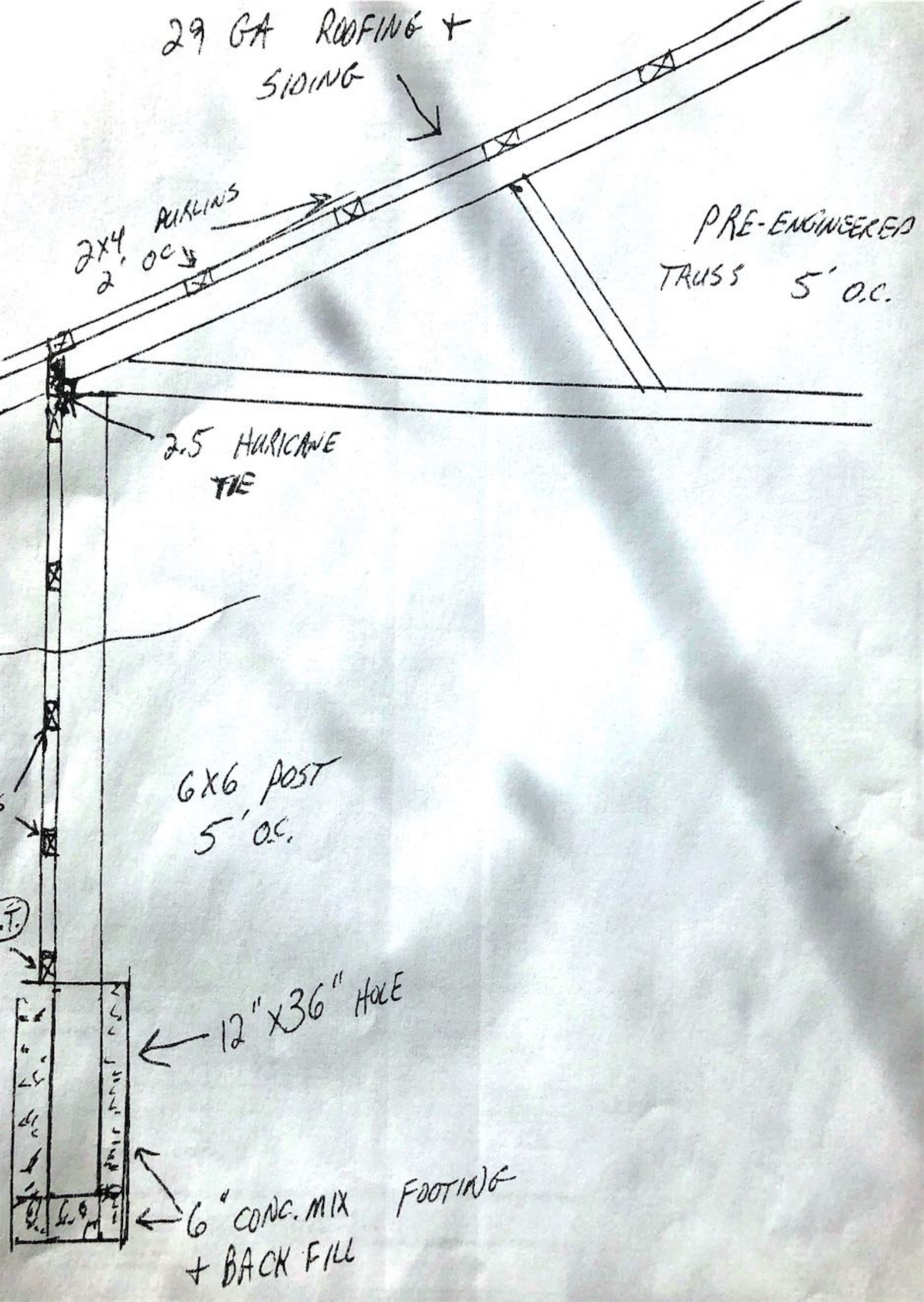
PRE-ENGINEERED
TRUSS 5' O.C.

2.5 HURICANE
TIE

6x6 POST
5' O.C.

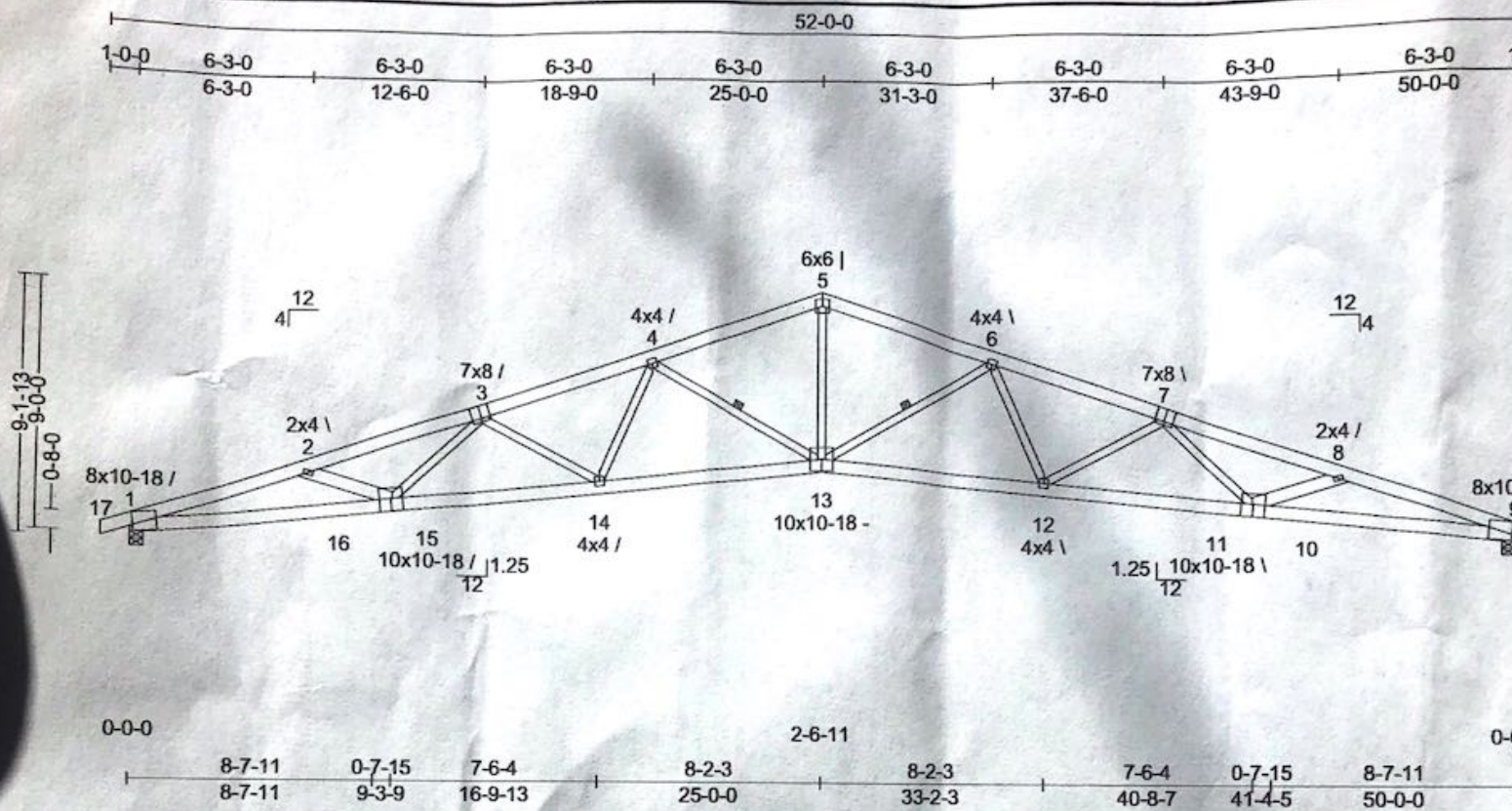
12" x 36" HOLE

6" CONC. MIX
+ BACK FILL
FOOTING



Component Solutions™
Truss
Version: 2018.2 [Build 6]
Material Database version: 1568

Span 50-0-0	Pitch 4/12	Qty 17	OHL 1-0-0	OHR 1-0-0	CANT L 0-0-0	CANT R 0-0-0	PLIES 1	Spacing 60 in
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Loading	General	CSI Summary	Deflection	L/	(loc)	Allowed
Load (psf) TCLL: 20 TCDL: 5 BCLL: 0 BCDL: 5	Bldg Code: IBC 2006/ TP1-2002 Rep.Mbr Increase: No D.O.L.: 125%	TC: 0.82 (8-9) BC: 0.69 (9-11) Web: 0.73 (3-14)	Vert TL: 1.8 in Vert LL: 1.15 in Horz TL: 0.78 in	L / 328 L / 510	(12-13) 13 9	L / 180 L / 240
Plate Offsets (Jnt X, Y, Ang): (1-6-0,3-8,5) (2-0-0,5-13,16) (3-0-10,3-14,18) (4-0-0,5-13,18) (5-0-0,5-13,90) (6-0-0,5-13,18) (7-0-10,3-14,18) (8-0-0,5-13,16) (9-6-0,3-8,5) (11-0-8,5-0,5) (12-0-0,5-8,5) (13-0-0,5-8,0) (14-0-0,5-8,5) (15-0-8,5-0,5)						

Reaction Summary									
JT	Type	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max Wind Uplift	Max Uplift	Max Horiz
1	Pin (Wall)	1	5.5 in	3.19 in	3,875 lbs	-	-776 lbs	-776 lbs	75 lbs
9	H Roll (Wall)	1	5.5 in	3.19 in	3,875 lbs	-	-776 lbs	-776 lbs	0 lbs

Material Summary			Bracing Summary		
TC	SP 2400/2.0 2x6		TC Bracing	Purlins at 24" o.c., Purlin design by Others.	
BC	SP 2400/2.0 2x6		BC Bracing	Purlins at 6'-4"-0, Purlin design by Others.	
Webs	SP (ALSC6-2013) #3/Stud 2x4	except:			
2-15	SP (ALSC6-2013) #2 2x6	5-13	SP (ALSC6-2013) #1 2x4	8-11	SP (ALSC6-2013) #2 2x6

Loads Summary

1) This truss has been designed for the effects of wind loads in accordance with ASCE 7 - 05 with the following user defined input: 100 mph nominal, Exposure C, Fully Enclosed, Gable/Hip, Building Category 1 (I=0.87), Overall Bldg Dims 50 ft x 100 ft, h = 18 ft, Not End Zone Truss, Neither end web considered. DOL = 1.60, CC Zone Width 0 ft.

2) Minimum storage attic loading in accordance with IBC Table 1607.1 has not been applied.

3) In accordance with IBC Table 1607.1, minimum BCLL's do not apply.

4) This truss is designed as an agricultural truss which for the purposes of this program is defined as a structure that represents a low hazard to people and property. See BCSI-10 for installation and temporary bracing.

5) Dead loads have not been reduced when calculating the uplift reaction as allowed by ANSI/TP1, Chapter 6.

Member Forces Summary											
Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force)											
TC	17-1	0.035	40 lbs	4-5	0.529	-8,405 lbs	8-9	0.824	-12,606 lbs		
	1-2	0.824	-12,606 lbs	5-6	0.529	-8,405 lbs	9-18	0.035	40 lbs		
	2-3	0.821	-12,167 lbs	6-7	0.706	-10,871 lbs					
	3-4	0.706	-10,871 lbs	7-8	0.821	-12,167 lbs					
BC	9-11	0.692	11,808 lbs (-2,462 lbs)	12-13	0.596	9,811 lbs (-1,710 lbs)	14-15	0.676	11,413 lbs (-2,185 lbs)		
	11-12	0.676	11,412 lbs (-2,185 lbs)	13-14	0.596	9,812 lbs (-1,710 lbs)	15-1	0.692	11,808 lbs (-2,462 lbs)		
Webs	2-15	0.070	-491 lbs	4-13	0.719	-2,294 lbs	7-12	0.734	-1,405 lbs		
	3-15	0.145	381 lbs (-2 lbs)	5-13	0.692	4,541 lbs (-886 lbs)	7-11	0.145	381 lbs		
	3-14	0.734	-1,405 lbs	6-13	0.719	-2,294 lbs	8-11	0.070	-492 lbs		
	4-14	0.476	1,250 lbs (-111 lbs)	6-12	0.476	1,250 lbs (-111 lbs)					

- Notes:**
- A quality control factor, Cq = 1.19, has been applied to the plate lateral resistance design values as specified by the Truss Manufacturer per ANSI/TP1-2002 Section 2.4.2.
 - Building Designer shall verify self weight of the truss and other dead load materials do not exceed TCCL 5 psf.
 - Building Designer shall verify self weight of the truss and other dead load materials do not exceed BCDL 5 psf.
 - ☒ Indicates continuous lateral restraint (CLR) required at the location(s) shown. Rows of CLR must be adequately diagonally braced, see D-WEBCLRBACE. For alternatives to CLR's and diagonal bracing, see D-WEBREINFORCE.
 - Listed wind uplift reactions based on MWFRS Only loading.