## APPROVAL NOTES

1. Approval of Olympia Steel drawings and/or calculations indicate that Olympia Steel has correctly interpreted the contract requirements. This approval constitutes the customer acceptance of the Olympia Steel design, concepts, assumptions, and loadings.
2. Failure to respond to clouded areas and areas to verify may result in additional costs and/or schedule delays for which Olympia Steel will not be responsible.
3. Any changes made after the Olympia Steel customer has signed and returned the Olympia Steel drawings and/or calculations and the project is released for fabrication shall be billed to the Olympia Steel customer including material, engineering, and other cost. An additional fee may be charged if the project must be moved from the fabrication and/or the shipping schedule.
4. It is the responsibility of the customer to field verify all existing conditions prior to fabrication.
5. It is imperative that any changes to these drawings:
   5.1. Be made in contrasting ink.
   5.2. Be legible and unambiguous.
   5.3. Have all instances of changes clearly indicated.
6. A dated signature, in the designated areas, is required on all pages. The signature must be from the person authorized on the contract or a person authorized, in writing, by the Olympia Steel customer.
7. Olympia Steel reserves the right to resubmit drawings with extensive or complex changes required to avoid misfabrication. This may impact the delivery schedule.
8. Any changes noted on the drawings not in conformance with the terms and requirements of the contract between Olympia Steel and its customer are not binding on Olympia Steel unless subsequently specifically acknowledged and agreed to in writing by change order or separate documentation.
9. The customer approves of all notes and conditions on the drawings and/or calculations by signing an Approval Drawing Waiver Form.

## GENERAL NOTES

1. Wall and liner panels are an integral part of the structural system. Unauthorized removal of panels or cutting panels for framed openings not shown is prohibited.
2. Oil-canning, a perceived waviness inherent to light gauge metal, may exist. This condition does not affect the structural integrity or the finish of the panel, and therefore is not a cause for rejection.
3. The primer for all cold-formed structural framing members contain a "wax-type" lubricant to facilitate roll-forming. Hair-line crazing which may occur during forming operations is considered normal and is not a cause for rejection.
4. All other primed structural members are given one shop coat (1.0 mils) of Olympia Steel's standard red-oxide primer designed for short term field protection.
5. All bolted connections for cold-formed secondary members use ASTM A307 $\frac{1}{2}$" x $1\frac{1}{4}$" bolts without washers unless noted otherwise on these drawings. Refer to erection drawings for specific framing connections for nonstandard conditions.
6. All bolted connections for primary framing members use ASTM A325 bolts without washers unless noted otherwise on these drawings. High strength bolts shall be installed according to the *Specifications for Structural Joints Using ASTM A325 or A490 Bolts*, Research Council on Structural Connections, December 31, 2009, and shall be tightened as Snug-Tight or Pretension as specified on these drawings.
7. Any type of suspended or load inducing system(s) is prohibited if zero collateral and zero sprinkler loads are designated on the contract. This would include lights, duct work, piping, insulation types other than 3" standard duty fiberglass blanket insulation, etc.

## RESPONSIBILITIES

1. The Olympia Steel Customer, hereafter referred to as the "customer", obtains and pays for all building permits, licenses, public assessments, paving or utility pro rata, utility connections, occupancy fees and other fees required by any governmental authority or utility in connection with the work provided for in the Contract Documents. The customer provides at his expense all plans and specifications required to obtain a building permit. It is the customer's responsibility to ensure that all plans and specifications comply with the applicable requirements of any governing building authorities.
2. The customer is responsible for identifying all applicable building codes, zoning codes, or other regulations applicable to the Construction Project, including the metal building system.
3. It is the responsibility of the customer to interpret all aspects of the End User's specifications and incorporate the appropriate specifications, design criteria, and design loads into the Order Documents submitted to Olympia Steel.
4. It is the responsibility of Olympia Steel to furnish the metal building system to meet the specifications including the design criteria and design loads incorporated by the Contractor into the Order Documents. Olympia Steel is not responsible for making an independent determination of any local codes or any other requirements not part of the Order Documents.
5. Olympia Steel Buildings' standard specifications apply unless stipulated otherwise in the Contract Documents. Olympia Steel design, fabrication, quality criteria, standards, practice, methods and tolerances shall govern the work any other interpretations to the contrary notwithstanding. It is understood by both parties that the customer is responsible for clarifications of inclusions or exclusions from the Architectural plans.
6. In case of discrepancies between Olympia Steel's structural steel plans and plans for other trades, Olympia Steel's shall govern per *Code of Standard Practice for Steel Buildings and Bridges* in the AISC 330-10; Section 3.3.
7. The customer is responsible for overall project coordination. All interface, compatibility and design considerations concerning any materials not furnished by Olympia Steel and Olympia Steel's steel system are to be considered and coordinated by the customer. Specific design criteria concerning this interface between materials must be furnished before release for fabrication or Olympia Steel's assumptions will govern.
8. Anchor rods and foundation embedment are designed, furnished, and set by the customer in accordance with an approved drawing. Dimensional accuracy shall satisfy the requirements of Section 7.5.1 of *Code of Standard Practice for Steel Buildings and Bridges* in the AISC 330-10.
9. All other embedded items or connection materials between the structural steel and the work of other trades are located and set by the customer in accordance with approved location on erection drawings. Accuracy of these items must satisfy the erection tolerance requirements.
10. Olympia Steel does not investigate the influence of the metal building system on existing buildings or structures. The End Customer assures that such buildings and structures are adequate to resist snow drifts, wind loads, or other conditions as a result of the presence of the metal building system.

OLYMPIA STEEL BUILDINGS

| Drawing Index | | |
|---|---|---|
| Issue | Date | Drawing Description |
| 0 | 09-09-20 | C1 - Cover Sheet |
| 0 | 09-09-20 | F1 - Anchor Bolt Plan |
| 0 | 09-09-20 | F2 - Anchor Bolt Reactions |
| 0 | 09-09-20 | F3 - Anchor Bolt Reactions |
| 0 | 09-09-20 | P1 - Rigid Frame Elevation |
| 0 | 09-09-20 | P2 - Rigid Frame Elevation |
| 0 | 09-09-20 | P3 - Rigid Frame Elevation |
| 0 | 09-09-20 | P4 - Rigid Frame Elevation |
| 0 | 09-09-20 | E1 - Roof Framing Plan |
| 0 | 09-09-20 | E2 - Framing Elevation |
| 0 | 09-09-20 | E3 - Framing Elevation |
| 0 | 09-09-20 | E4 - Framing Elevation |
| 0 | 09-09-20 | E5 - Framing Elevation |
| 0 | 09-09-20 | E6 - Framing Elevation |
| 0 | 09-09-20 | E7 - Walls Liner |
| 0 | 09-09-20 | D1 - Standard Detail Page |
| 0 | 09-09-20 | D2 - Standard Detail Page |

**ROOF PANEL**
Profile: Super Span X Gauge: 26 Color: Burnished Slate
UL580 Class 90: Yes
Clip Type if Standing Seam:

**WALL PANEL**
Profile: Super Span X Gauge: 26 Color: Burnished Slate

**BUILDING DESIGN LOADS**
Building Code: North Carolina Building Code 2018
Steel Specification: AISC 360-10
Cold-Formed Specification: AISI S100-12

**GENERAL LOADS**

| | | |
|---|---|---|
| Roof Dead Load: | Metal Bldg. Only | |
| Roof Collateral Load: | 1.00 | psf |
| Sprinkler Load: | 0.00 | psf |
| Roof Live Load: | 20.00 | psf |
| Tributary Live Load Reduction: | Yes | |
| Rainfall Intensity (5 Min Duration): | 8.04 | in/hr |

**WIND LOAD**

| | | | |
|---|---|---|---|
| Wind Speed, ult (3-sec gust): | 115 | mph | |
| Wind Speed, asd: | 89 | mph | |
| Wind Speed, service: | 76 | mph | |
| Wind Exposure Factor: | B | | |
| Wind Condition: | Enclosed | | Building-A |
| | Partially Enclosed | | Building-B |
| Edge Zone Width: | 6.00 | ft | Building-A |
| | 3.00 | ft | Building-B |
| Internal Pressure Coefficient (GCpi): | ± 0.18 | | Building-A |
| | ± 0.55 | | Building-B |

**SNOW LOAD**

| | | | |
|---|---|---|---|
| Ground Snow Load (Pg): | 15.00 | psf | |
| Roof Snow Load: | 11.00 | psf | Building-A |
| | 12.60 | psf | Building-B |
| Snow Load Importance Factor (Is): | 1.00 | | |
| Snow Exposure Factor (Ce): | 1.00 | | |
| Thermal Factor (Ct): | 1.00 | | Building-A |
| | 1.20 | | Building-B |
| Slope Factor: | 1.00 | | |

**DEFLECTION CRITERIA**

| | | | |
|---|---|---|---|
| Main Frames Lateral: | H/60 | Roof Panels: | L/60 |
| Main Frames Vertical: | L/ 180 | Purlins: | L/180 |
| Bearing Frame Rafter: | L/ 180 | Wall Panels: | L/60 |
| Endwall Columns: | L/ 180 | Girts: | L/90 |

**ERECTOR NOTE**
**ONLY USE DRAWINGS ISSUED "FOR ERECTION" TO ERECT BUILDING**

DRAWING STATUS
☐ FOR CONSTRUCTION PERMIT:
These drawings are by definition not final in that, as a minimum, piece markings are not identified. Only drawings issued "For Erection" can be considered final.
☒ FOR CONSTRUCTION:
These drawings are used for anchor bolt setting. Piece markings are not identified.
☐ FOR ERECTION:
For final / erector installation.

**PRIMARY FRAMING**
Built-Up & Hot-Rolled: Gray Oxide Primer

**SECONDARY FRAMING**
Purlins, Eave Struts: Pre-Galvanized
Girts, Light Gage Columns: Pre-Galvanized
Light Gage Jambs & Headers: Pre-Galvanized

Hot-Dip Galvanizing conforms to the ASTM A123 specification.
Pre-Galvanized members conform to the ASTM A653, Grade 50, Coating G-90 specification.

**SEISMIC LOAD**

| | | | | |
|---|---|---|---|---|
| Risk Category: | II - Normal | | | |
| Seismic Importance Factor (Ie): | 1.00 | | | |
| Spectral Response Acceleration (Ss): | 0.1690 | | | |
| Spectral Response Acceleration (S1): | 0.0820 | | | |
| Site Class: | D | | | |
| Spectral Response Coefficients (Sds): | 0.1803 | | | |
| Spectral Response Coefficients (Sd1): | 0.1312 | | | |
| Seismic Design Category: | B | | | |
| Framing Direction: | Lateral | Longitudinal | | |
| Structural Syst: | 'Structural Steel Systems Not Specifically Detailed for Seismic Resistance' | | | |
| Response Modification Factor(s) (R) : | 3.0 | 3.0 | | |
| Deflection Amplification Factor(s) : | 3.0 | 3.0 | | |
| Seismic Response Coefficient(s) (Cs): | 0.0601 | 0.0601 | | |
| Total Design Base Shear : | 1.96 Kips | 2.64 Kips | | Building-A |
| | 0.52 Kips | 0.00 Kips | | Building-B |
| Analysis Procedure : | Equivalent Lateral Force | | | |

Other Loads: - Building A supports Building B

For components, claddings, and MWFRS deflections involving wind are based on 10 year serviceability wind pressures.

NOTICE TO CONTRACTOR
All construction must comply with current NC Building Codes and is subject to field inspection and verification.
APPROVED
Limited building only review
Permit holder responsible for full compliance with the code
11/20/2020
Harnett COUNTY NORTH CAROLINA

## ERECTION NOTES

1. All bracing shown and provided by Olympia Steel for this building is required and shall be installed by the erector as a permanent part of the structure per *Code of Standard Practice for Steel Buildings and Bridges* in AISC 330-10 Section 7.10.
2. Temporary supports, such as guys, braces, false-work, cribbing or other elements required for the erection operation shall be determined and funished by the erector per *Code of Standard Practice for Steel Buildings and Bridges* in AISC 330-10 Section 7.10.
3. Normal erection operations include the correction of minor misfits by moderate amounts of reaming, chipping, or cutting and the drawing of elements into line through use of drift pins. Errors which require major changes in the member configuration are to be reported immediately to Olympia Steel by the customer to enable whoever is responsible either to correct the error or to approve the most efficient and economic method of correction to be used by others per *Code of Standard Practice for Steel Buildings and Bridges* in AISC 330-10; Section 7.14.
4. Erection tolerances are set forth per *Code of Standard Practice for Steel Buildings and Bridges* AISC 330-10 Section 7.13 except that individual members are considered plumb, level and aligned if the deviation does not exceed 1:500. Variations in finished overall dimensions of structural steel framing are deemed within the limits of good practice when they do not exceed the cumulative effect of rolling, fabricating, and erection tolerances.
   4.1. When crane support systems are part of the metal building system erection tolerances, then Chapter 9 of Common Industry Practices in the 2012 MBMA Metal Building Systems Manual shall apply. To achieve the required tolerances, grouting of the columns and shimming of the runway beam may be required. The customer shall provide grout if required. The contractor erecting the runway beams is responsible for shimming, plumbing, and leveling of the runway system. When aligning the runway beams, the alignment shall be with respect to the beam webs so the center of the aligned rail is over the runway web.
5. As a general rule, field welding is not used to assemble a metal building system. In cases where the drawings indicate field welding and in cases where approved corrections are to be made by field welding the following requirements shall be met:
   5.1. Welders must be qualified by an independent testing agency, with suitable documentation to AWS D1.1 Structural Welding Code - Steel or AWS D1.3 Structural Welding Code - Sheet Steel as applicable, for the processes, positions, and materials involved.
   5.2. All welds must be made in conformance to a documented and approved Welding Procedure Specification (WPS). All joints which are not pre-qualified must be supported by a certified Procedure Qualification Record (PQR) by an independent testing agency.
6. All documentation and records shall be the responsibility of the customer.
7. Any claims or shortages by buyer must be made to Olympia Steel within seven (7) working days after delivery, or such claims will be considered to have been waived by the customer and disallowed. All claims should be directed to Olympia Steel Buildings Customer Service Department.
8. Claims for correction of alleged misfits will be disallowed unless Olympia Steel shall have received prior notice thereof and allowed reasonable inspection of such misfits. Ordinary inaccuracies of shop work shall not be construed as misfits. No part of the building may be returned or charges assessed for alleged misfits without prior approval from Olympia Steel.
9. Neither Olympia Steel nor the customer will cut, drill or otherwise alter their work, or the work of other trades to accommodate other trades unless such work is clearly specified in the contract documents. Whenever such work is specified the customer is responsible for furnishing complete information as to materials, size, location, and number of alterations prior to preparation of shop drawings per *Code of Standard Practice for Steel Buildings and Bridges* in the AISC 330-10; Section 7.14.)
10. Olympia Steel Buildings Field Modifications Policy:
    10.1 Olympia Steel will only be responsible for the field-modified parts designed and approved by the Olympia Steel Engineering Department.
    10.2 Any field modifications designed by third parties may not be approved by Olympia Steel and may limit Olympia Steel warranty and liability.
    10.3 Olympia Steel makes no warranty and hereby disclaims any responsibility with respect to the design, engineering, or construction of any field-modified parts performed by third parties.

## WARNING

Some panels and/or trims are furnished with a protective, peel-off film. This film must be removed prior to field installation. Do not expose the panels and/or trims to sunlight without immediate film removal. Film must be removed from non-exposed panels and/or trims within six months after film application or irreparable damage will occur to the panel surface. Claims will not be accepted for this problem.

**TRIM COLOR**

| | | | |
|---|---|---|---|
| Shadow Gutter: | Burnished Slate | Gauge: | 26 |
| Shadow Rake: | Burnished Slate | Gauge: | 26 |
| Corner: | Burnished Slate | Gauge: | 26 |
| Accessory: | Burnished Slate | Gauge: | 26 |
| Downspouts: | Burnished Slate | Gauge: | 26 |
| Base: | Burnished Slate | Gauge: | 26 |

The Engineer whose seal and signature appear on these documents represent Whirlwind Steel Buildings, Inc., and is not the Engineer of Record for the overall project. The Engineer's responsibility is limited to material designed and manufactured by Whirlwind Steel Buildings, Inc., and excludes parts such as doors, windows, foundation design, and erection of the building.

APPROVAL/REVIEWING AUTHORITY: PLEASE REVIEW APPROVAL DRAWINGS CAREFULLY

UNLESS NOTED OTHERWISE, IT WILL BE ASSUMED THAT ALL INFORMATION SHOWN ON THESE DRAWINGS HAS THE AFFIRMATION OF THE APPROVAL/REVIEW AUTHORITY. FAILURE TO RESPOND TO CLOUDED AREAS AND AREAS TO VERIFY MAY RESULT IN ADDITIONAL COSTS AND/OR SCHEDULE DELAYS FOR WHICH OLYMPIA WILL NOT BE RESPONSIBLE. FIELD VERIFY ALL EXISTING CONDITIONS PRIOR TO FABRICATION. ALL SUBSEQUENT CHANGES AFTER THE FIRST SUBMITTAL WILL BE CONSIDERED AS CONTRACTUAL CHANGES.

OLYMPIA STEEL BUILDINGS

| REV | DATE | DESCRIPTION | BY | CHK |
|---|---|---|---|---|
| 0 | 09.09.20 | FOR CONSTRUCTION | MBS | TVN |

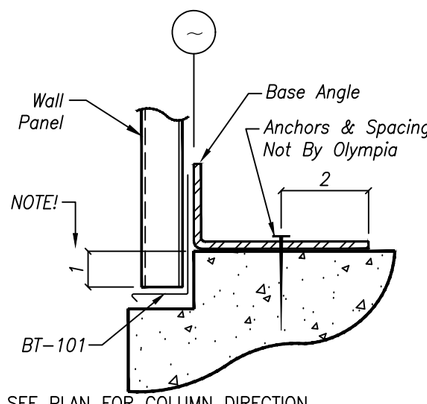
DESC: COVER SHEET
BLDG. SIZE: VARIES
CUSTOMER: WYNN SITE DEVELOPMENT
LOCATION: Holly Springs, NC 27504
REFERENCE: WYNN SITE DEVELOPMENT
JOBSITE: Holly Springs, NC 27540
COUNTY: Wake
DWN: MBS | CHK: . | DATE: 9/ 8/20 | ENG: JG | JOB NO: 6707-23738 | DWG NO: C1 | ISSUE 0

NORTH CAROLINA PROFESSIONAL SEAL 037025 ENGINEER RONALD H. MAYS

Sep 13, 2020

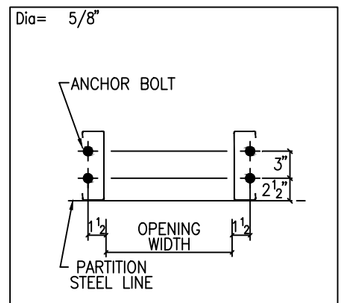
NTS

*Horizontal leg of recess must remain flat or slope away from the building. Notch area indicates a recess for metal wall panels. Panels must not touch the bottom of the recess, which will void the warranty.*

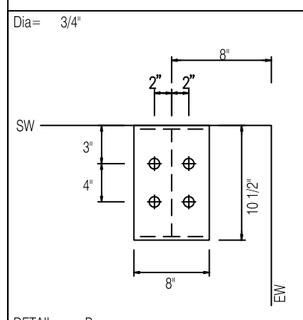



SEE PLAN FOR COLUMN DIRECTION

DETAIL @ SHEETING RECESS




DETAIL S

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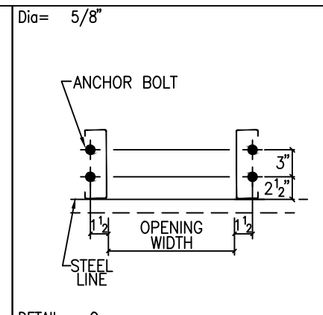


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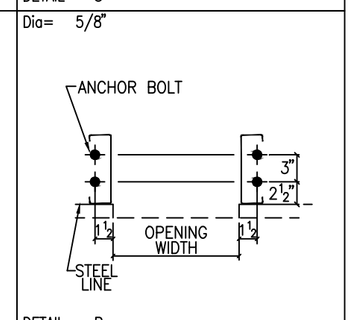


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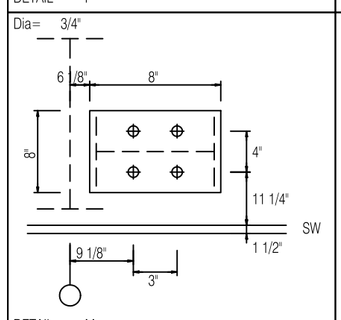


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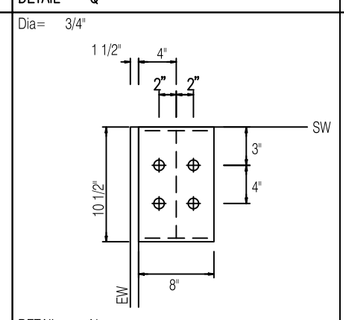


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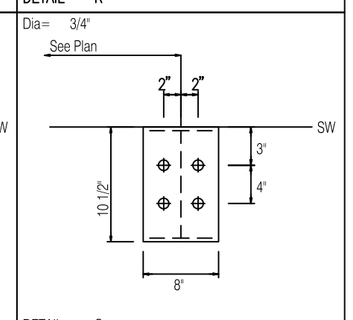
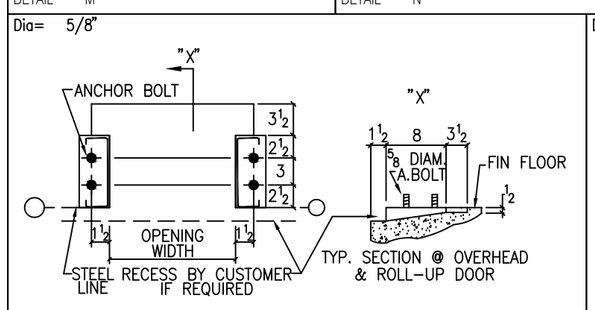
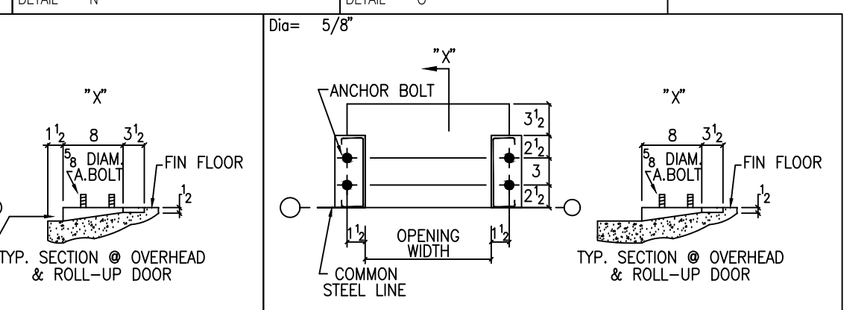



DETAIL O



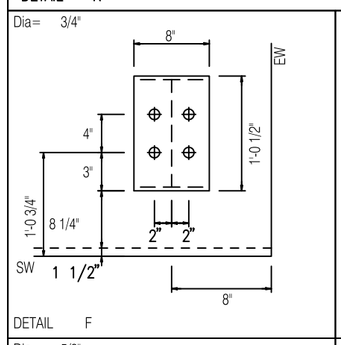

DETAIL K







DETAIL L

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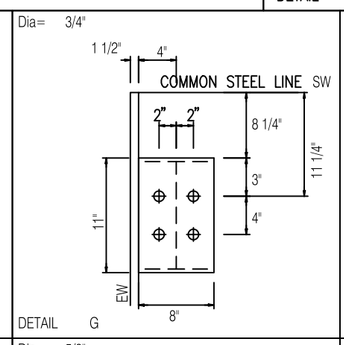


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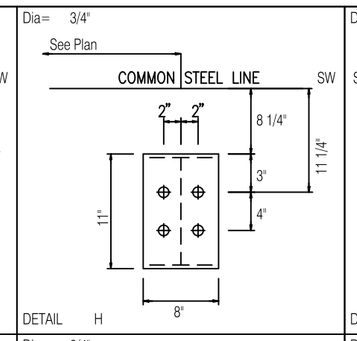


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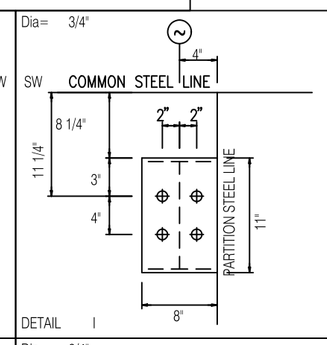


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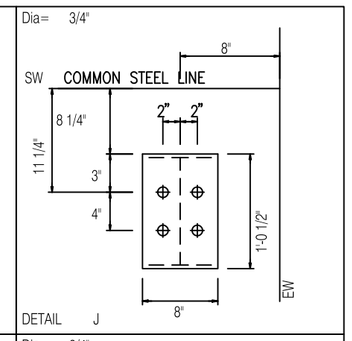


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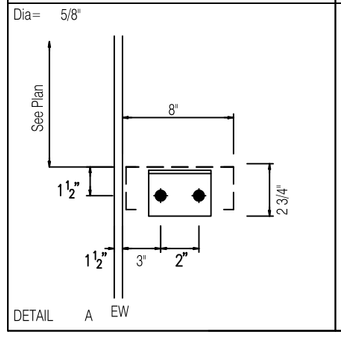


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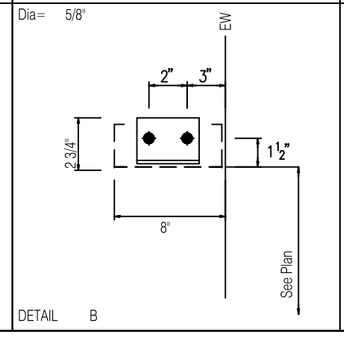


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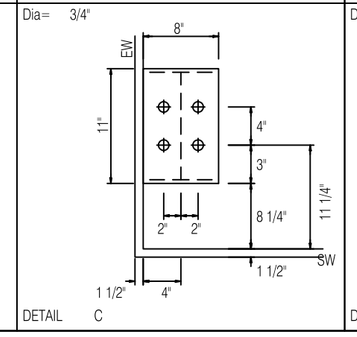


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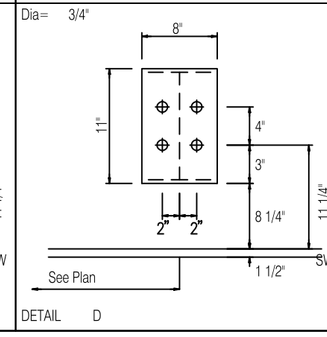


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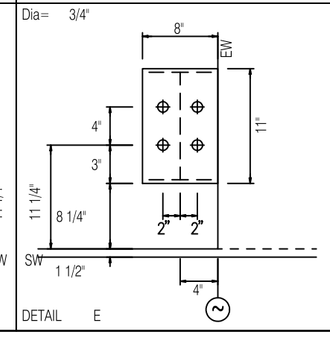


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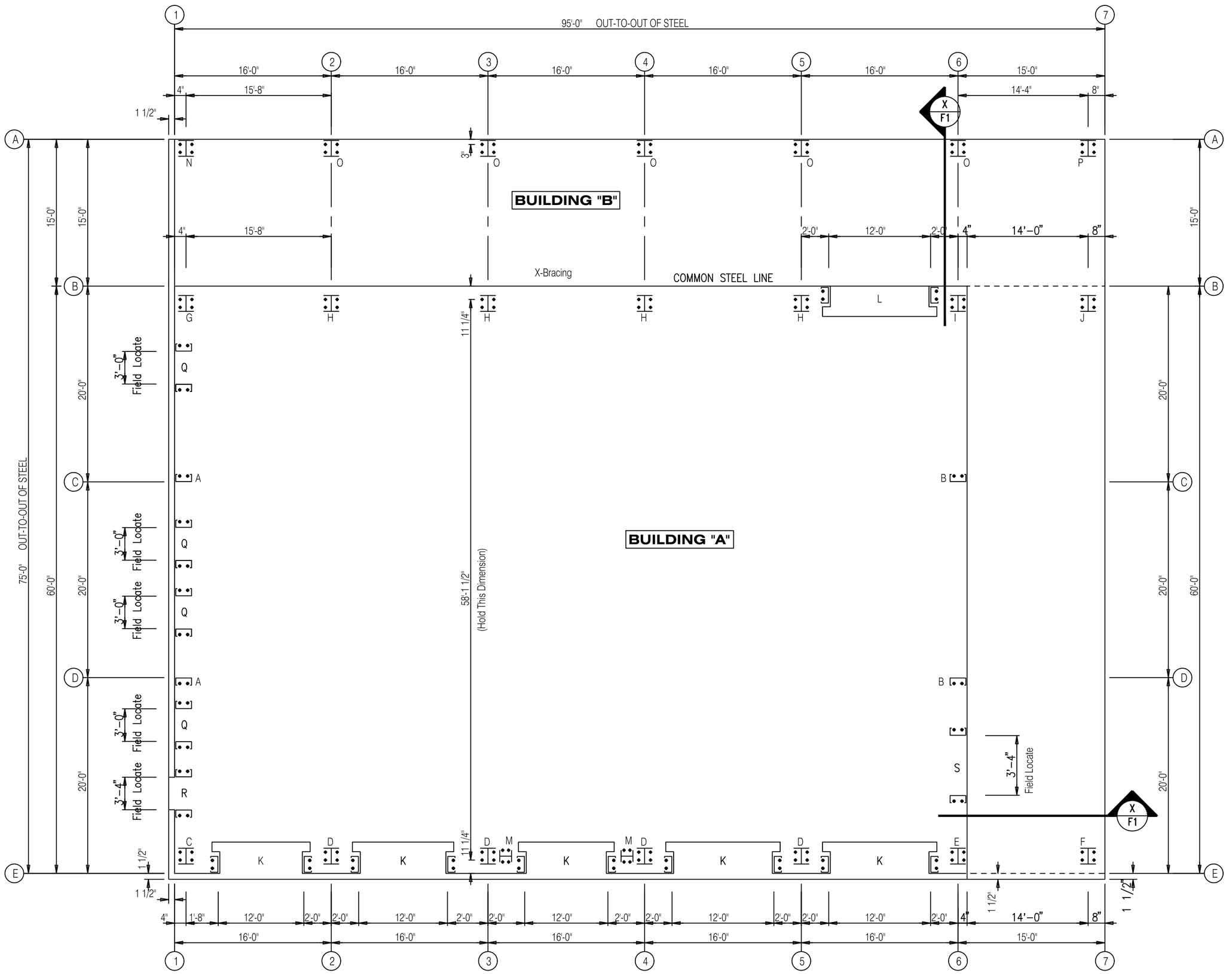
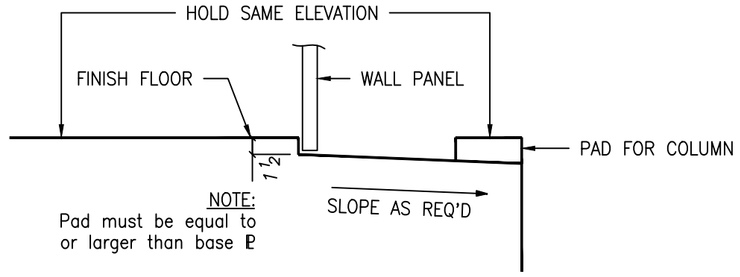



## ANCHOR BOLT PLAN

NOTE: All Base Plates @ FINISH FLOOR ELEVATION (U.N.)




## SECTION "X"

ERECTOR NOTE: ONLY USE DRAWINGS ISSUED "FOR ERECTION" TO ERECT BUILDING

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OLYMPIA STEEL BUILDINGS.

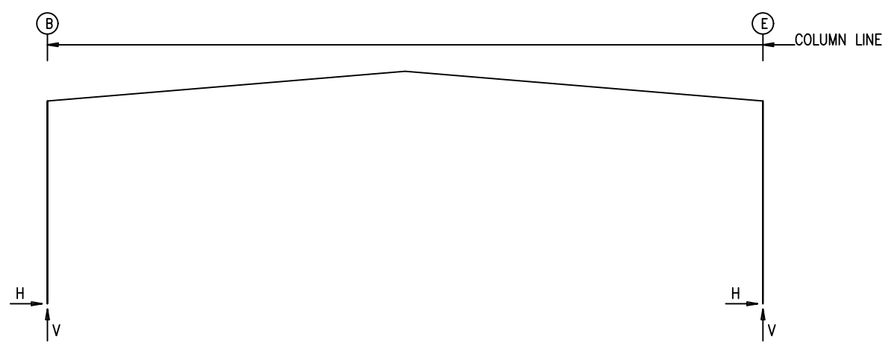
| REV | DATE | DESCRIPTION | BY | CHK |
|---|---|---|---|---|
| 0 | 09.09.20 | FOR CONSTRUCTION | MBS | TVN |

| | | | |
|---|---|---|---|
| DESC: ANCHOR BOLT REACTIONS | | BLDG. SIZE: VARIES | |
| CUSTOMER: WYNN SITE DEVELOPMENT | | LOCATION: Holly Springs, NC 27504 | |
| REFERENCE: WYNN SITE DEVELOPMENT | | | |
| JOBSITE: Holly Springs, NC 27540 | | COUNTY: Wake | |
| DWN: MBS | CHK: . | DATE: 9/ 8/20 | ENG: JG |
| JOB NO: 6707-23738 | DWG NO: F2 | ISSUE 0 | |

NORTH CAROLINA PROFESSIONAL SEAL 037025 ENGINEER RONALD H. MAYS

Sep 13, 2020

FRAME LINES: 1 2 3 4 5 6 7




## RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

| Frm Line | Col Line | Column_Reactions(k) Load Id | Hmax H | V Vmax | Load Id | Hmin H | V Vmin | Bolt(in) Qty | Dia | Base_Plate(in) Width | Length | Thick | Elev. (in) |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| 1 | B | 9 | 2.8 | 5.3 | 5 | −2.2 | −2.2 | 4 | 0.750 | 8.000 | 11.00 | 0.375 | 0.0 |
| | | 1 | 2.6 | 6.3 | 3 | −1.9 | −3.7 | | | | | | |
| 1 | E | 4 | 2.5 | −3.0 | 9 | −2.8 | 5.1 | 4 | 0.750 | 8.000 | 11.00 | 0.375 | 0.0 |
| | | 9 | −2.8 | 5.1 | 4 | 2.5 | −3.0 | | | | | | |

## RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

| Frm Line | Col Line | Column_Reactions(k) Load Id | Hmax H | V Vmax | Load Id | Hmin H | V Vmin | Bolt(in) Qty | Dia | Base_Plate(in) Width | Length | Thick | Elev. (in) |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| 2* | B | 9 | 5.3 | 9.8 | 5 | −3.4 | −2.5 | 4 | 0.750 | 8.000 | 11.00 | 0.375 | 0.0 |
| | | 1 | 4.3 | 10.6 | 6 | 0.3 | −7.9 | | | | | | |
| 2* | E | 4 | 3.8 | −4.4 | 9 | −5.3 | 9.4 | 4 | 0.750 | 8.000 | 11.00 | 0.375 | 0.0 |
| | | 9 | −5.3 | 9.4 | 4 | 3.8 | −4.4 | | | | | | |

2* Frame lines: 2 3 4 5

## RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

| Frm Line | Col Line | Column_Reactions(k) Load Id | Hmax H | V Vmax | Load Id | Hmin H | V Vmin | Bolt(in) Qty | Dia | Base_Plate(in) Width | Length | Thick | Elev. (in) |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| 6 | B | 9 | 5.3 | 9.4 | 5 | −3.9 | −3.4 | 4 | 0.750 | 8.000 | 11.00 | 0.375 | 0.0 |
| | | 1 | 4.3 | 10.3 | 3 | −2.7 | −4.5 | | | | | | |
| 6 | E | 4 | 3.8 | −3.6 | 9 | −5.3 | 9.0 | 4 | 0.750 | 8.000 | 11.00 | 0.375 | 0.0 |
| | | 9 | −5.3 | 9.0 | 8 | 2.7 | −4.2 | | | | | | |

## RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

| Frm Line | Col Line | Column_Reactions(k) Load Id | Hmax H | V Vmax | Load Id | Hmin H | V Vmin | Bolt(in) Qty | Dia | Base_Plate(in) Width | Length | Thick | Elev. (in) |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| 7 | B | 2 | 2.8 | 5.5 | 5 | −1.2 | −2.2 | 4 | 0.750 | 8.000 | 12.50 | 0.375 | 0.0 |
| | | 1 | 2.3 | 6.3 | 7 | −0.6 | −2.8 | | | | | | |
| 7 | E | 7 | 1.8 | −2.6 | 9 | −2.3 | 4.8 | 4 | 0.750 | 8.000 | 12.50 | 0.375 | 0.0 |
| | | 9 | −2.3 | 4.8 | 7 | 1.8 | −2.6 | | | | | | |

## RIGID FRAME: BASIC COLUMN REACTIONS (k)

| Frame Line | Column Line | Dead Horiz | Dead Vert | Collateral Horiz | Collateral Vert | Live Horiz | Live Vert | Snow Horiz | Snow Vert | Wind_Left1 Horiz | Wind_Left1 Vert | Wind_Right1 Horiz | Wind_Right1 Vert |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| 1 | B | 0.5 | 1.3 | 0.1 | 0.3 | 2.0 | 4.7 | 1.5 | 3.3 | −3.7 | −7.4 | −0.5 | −4.4 |
| 1 | E | −0.5 | 1.1 | −0.1 | 0.2 | −2.0 | 3.5 | −1.6 | 2.7 | 1.5 | −3.8 | 4.7 | −6.2 |

| Frame Line | Column Line | Wind_Left2 Horiz | Wind_Left2 Vert | Wind_Right2 Horiz | Wind_Right2 Vert | Wind_Long1 Horiz | Wind_Long1 Vert | Wind_Long2 Horiz | Wind_Long2 Vert | Seismic_Left Horiz | Seismic_Left Vert | Seismic_Right Horiz | Seismic_Right Vert |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| 1 | B | −4.1 | −5.0 | −0.9 | −1.9 | −0.7 | −6.4 | −0.9 | −5.2 | −0.1 | −0.1 | 0.1 | 0.1 |
| 1 | E | 0.1 | −1.5 | 3.3 | −3.9 | 2.9 | −4.7 | 2.7 | −5.9 | −0.1 | 0.1 | 0.1 | −0.1 |

| Frame Line | Column Line | Seismic_Long Horiz | Seismic_Long Vert | MIN_SNOW Horiz | MIN_SNOW Vert | F1UNB_Sl_L Horiz | F1UNB_Sl_L Vert | F1UNB_Sl_R Horiz | F1UNB_Sl_R Vert |
|---|---|---|---|---|---|---|---|---|---|
| 1 | B | −0.1 | 0.0 | 2.2 | 3.7 | 1.3 | 2.7 | 1.5 | 1.7 |
| 1 | E | 0.0 | 0.0 | −2.2 | 3.7 | −1.3 | 1.6 | −1.5 | 3.1 |

| Frame Line | Column Line | Dead Horiz | Dead Vert | Collateral Horiz | Collateral Vert | Live Horiz | Live Vert | Snow Horiz | Snow Vert | Wind_Left1 Horiz | Wind_Left1 Vert | Wind_Right1 Horiz | Wind_Right1 Vert |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| 2* | B | 0.8 | 2.0 | 0.3 | 0.6 | 3.3 | 8.0 | 3.0 | 6.6 | −5.3 | −10.9 | 0.2 | −6.6 |
| 2* | E | −0.8 | 1.7 | −0.3 | 0.5 | −3.3 | 5.7 | −3.1 | 5.3 | 1.6 | −5.6 | 7.2 | −9.1 |

| Frame Line | Column Line | Wind_Left2 Horiz | Wind_Left2 Vert | Wind_Right2 Horiz | Wind_Right2 Vert | Wind_Long1 Horiz | Wind_Long1 Vert | Wind_Long2 Horiz | Wind_Long2 Vert | Seismic_Left Horiz | Seismic_Left Vert | Seismic_Right Horiz | Seismic_Right Vert |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| 2* | B | −6.4 | −6.1 | −0.9 | −1.8 | −0.3 | −15.1 | −0.6 | −13.7 | −0.2 | −0.1 | 0.2 | 0.1 |
| 2* | E | −0.8 | −1.2 | 4.8 | −4.6 | 4.0 | −7.1 | 3.8 | −8.6 | −0.2 | 0.1 | 0.2 | −0.1 |

| Frame Line | Column Line | Seismic_Long Horiz | Seismic_Long Vert | MIN_SNOW Horiz | MIN_SNOW Vert | F2UNB_Sl_L Horiz | F2UNB_Sl_L Vert | F2UNB_Sl_R Horiz | F2UNB_Sl_R Vert |
|---|---|---|---|---|---|---|---|---|---|
| 2* | B | −0.1 | −1.6 | 4.2 | 7.2 | 2.6 | 5.3 | 3.0 | 3.4 |
| 2* | E | 0.0 | 0.0 | −4.2 | 7.2 | −2.6 | 3.1 | −3.0 | 6.0 |

| Frame Line | Column Line | Dead Horiz | Dead Vert | Collateral Horiz | Collateral Vert | Live Horiz | Live Vert | Snow Horiz | Snow Vert | Wind_Left1 Horiz | Wind_Left1 Vert | Wind_Right1 Horiz | Wind_Right1 Vert |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| 6 | B | 0.8 | 2.0 | 0.3 | 0.6 | 3.2 | 7.7 | 3.0 | 6.3 | −5.2 | −9.5 | 0.1 | −7.3 |
| 6 | E | −0.8 | 1.7 | −0.3 | 0.5 | −3.3 | 5.4 | −3.0 | 5.0 | 1.8 | −6.4 | 7.1 | −7.7 |

| Frame Line | Column Line | Wind_Left2 Horiz | Wind_Left2 Vert | Wind_Right2 Horiz | Wind_Right2 Vert | Wind_Long1 Horiz | Wind_Long1 Vert | Wind_Long2 Horiz | Wind_Long2 Vert | Seismic_Left Horiz | Seismic_Left Vert | Seismic_Right Horiz | Seismic_Right Vert |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| 6 | B | −7.2 | −7.6 | −2.0 | −3.2 | −1.7 | −9.4 | −1.9 | −8.7 | −0.2 | −0.1 | 0.2 | 0.1 |
| 6 | E | 0.3 | −2.6 | 5.6 | −6.1 | 5.3 | −8.1 | 5.2 | −8.7 | −0.2 | 0.1 | 0.2 | −0.1 |

| Frame Line | Column Line | Seismic_Long Horiz | Seismic_Long Vert | MIN_SNOW Horiz | MIN_SNOW Vert | F3UNB_Sl_L Horiz | F3UNB_Sl_L Vert | F3UNB_Sl_R Horiz | F3UNB_Sl_R Vert |
|---|---|---|---|---|---|---|---|---|---|
| 6 | B | −0.1 | 0.0 | 4.2 | 6.8 | 2.6 | 5.0 | 3.0 | 3.2 |
| 6 | E | 0.0 | 0.0 | −4.2 | 6.8 | −2.6 | 2.9 | −3.0 | 5.7 |

| Frame Line | Column Line | Dead Horiz | Dead Vert | Collateral Horiz | Collateral Vert | Live Horiz | Live Vert | Snow Horiz | Snow Vert | Wind_Left1 Horiz | Wind_Left1 Vert | Wind_Right1 Horiz | Wind_Right1 Vert |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| 7 | B | 0.4 | 1.3 | 0.1 | 0.3 | 1.7 | 4.7 | 1.3 | 3.2 | −1.6 | −4.4 | −0.8 | −4.7 |
| 7 | E | −0.4 | 1.1 | −0.1 | 0.2 | −1.8 | 3.5 | −1.3 | 2.6 | 1.8 | −4.2 | 2.6 | −3.2 |

| Frame Line | Column Line | Wind_Left2 Horiz | Wind_Left2 Vert | Wind_Right2 Horiz | Wind_Right2 Vert | Wind_Long1 Horiz | Wind_Long1 Vert | Wind_Long2 Horiz | Wind_Long2 Vert | Seismic_Left Horiz | Seismic_Left Vert | Seismic_Right Horiz | Seismic_Right Vert |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| 7 | B | −2.3 | −4.9 | −1.9 | −2.9 | −1.4 | −5.9 | 2.1 | 1.0 | −0.1 | −0.1 | 0.1 | 0.1 |
| 7 | E | 1.1 | −2.5 | 1.5 | −3.8 | 3.4 | −5.4 | −0.1 | 1.5 | −0.1 | 0.1 | 0.1 | −0.1 |

| Frame Line | Column Line | Seismic_Long Horiz | Seismic_Long Vert | MIN_SNOW Horiz | MIN_SNOW Vert | F4UNB_Sl_L Horiz | F4UNB_Sl_L Vert | F4UNB_Sl_R Horiz | F4UNB_Sl_R Vert |
|---|---|---|---|---|---|---|---|---|---|
| 7 | B | −0.1 | 0.0 | 1.8 | 3.5 | 1.1 | 2.6 | 1.3 | 1.6 |
| 7 | E | 0.0 | 0.0 | −1.8 | 3.5 | −1.1 | 1.5 | −1.3 | 3.0 |

2* Frame lines: 2 3 4 5

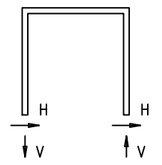
## ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

| Frm Line | Col Line | Dead Vert | Wind Press Horz | Wind Suct Horz |
|---|---|---|---|---|
| 1 | C | 0.1 | −3.0 | 3.3 |
| 1 | D | 0.1 | −3.0 | 3.3 |
| 6 | D | 0.1 | −3.0 | 3.3 |
| 6 | C | 0.1 | −3.0 | 3.3 |

## ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

| Frm Line | Col Line | Column_Reactions(k) Load Id | Hmax H | V Vmax | Load Id | Hmin H | V Vmin | Bolt(in) Qty | Dia | Base_Plate(in) Width | Length | Thick | Elev. (in) |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| 1 | C | 10 | 2.0 | 0.1 | 11 | −1.8 | 0.1 | 2 | 0.625 | 2.750 | 6.000 | 0.375 | 0.0 |
| | | 12 | 2.0 | 0.1 | | | | | | | | | |
| 1 | D | 10 | 2.0 | 0.1 | 11 | −1.8 | 0.1 | 2 | 0.625 | 2.750 | 6.000 | 0.375 | 0.0 |
| | | 12 | 2.0 | 0.1 | | | | | | | | | |
| 6 | D | 10 | 2.0 | 0.1 | 11 | −1.8 | 0.1 | 2 | 0.625 | 2.750 | 6.000 | 0.375 | 0.0 |
| | | 12 | 2.0 | 0.1 | | | | | | | | | |
| 6 | C | 10 | 2.0 | 0.1 | 11 | −1.8 | 0.1 | 2 | 0.625 | 2.750 | 6.000 | 0.375 | 0.0 |
| | | 12 | 2.0 | 0.1 | | | | | | | | | |

## WIND BENT REACTIONS




| Wall Loc | Wall Line | Col Line | ± Reactions Wind(k) Horz | Wind(k) Vert | Seismic(k) Horz | Seismic(k) Vert | Bolt(in) Qty | Dia | Base_Plate(in) Width | Length | Thick |
|---|---|---|---|---|---|---|---|---|---|---|---|
| F_SW | E | 3 | 2.1 | 4.6 | 0.5 | 1.1 | 4 | 0.750 | 8.000 | 8.000 | 0.375 |
| F_SW | E | 4 | 2.1 | 4.6 | 0.5 | 1.1 | 4 | 0.750 | 8.000 | 8.000 | 0.375 |

## BUILDING BRACING REACTIONS

| Wall Loc | Wall Line | Col Line | ± Reactions(k) Wind Horz | Wind Vert | Seismic Horz | Seismic Vert | Panel_Shear (lb/ft) Wind | Seis | Note |
|---|---|---|---|---|---|---|---|---|---|
| L_EW | 1 | | | | | | | | (h) |
| F_SW | E | 3,4 | | | | | | | (a) |
| R_EW | 7 | | | | | | | | (h) |
| B_SW | B | 4,3 | 6.3 | 5.9 | 1.6 | 1.5 | | | |

(a)Wind bent in bay
(h)Rigid frame at endwall

## ANCHOR BOLT SUMMARY

**BUILDING "A" & "B"**

| Qty | Locate | Dia (in) | Type | Proj (in) |
|---|---|---|---|---|
| ● 48 | Jamb | 5/8" | A307 | 2.50 |
| ● 8 | Endwall | 5/8" | A307 | 2.50 |
| ⊕ 84 | Frame | 3/4" | A307 | 3.00 |
| ⊕ 8 | WindCol | 3/4" | A307 | 3.00 |

## NOTES FOR REACTIONS

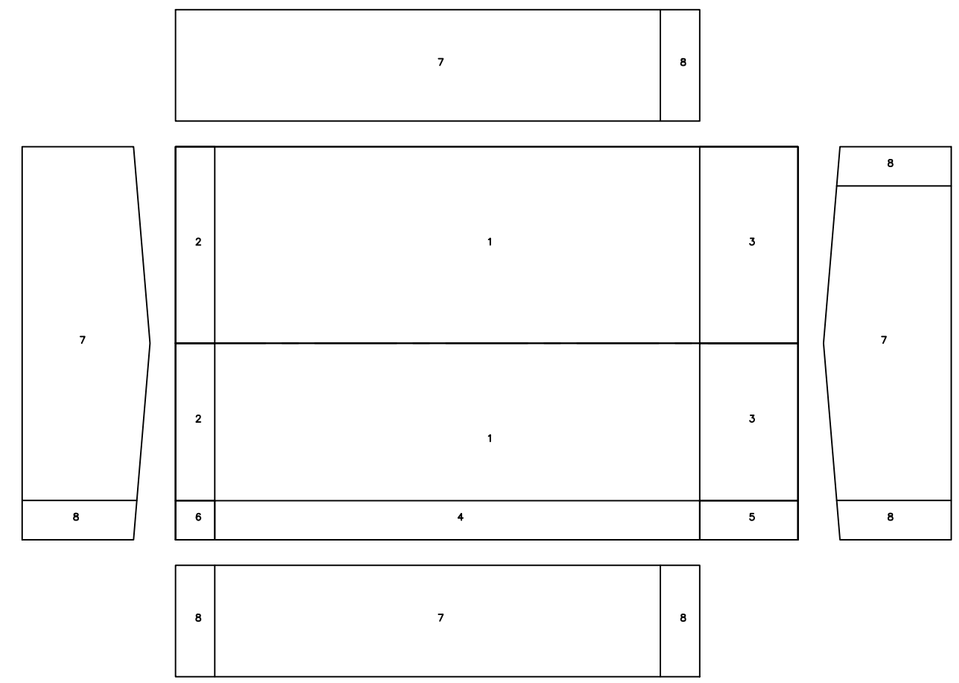
1. All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
2. Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
3. Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
4. Loading conditions are:
   1 Dead+Collateral+Live
   2 Dead+Collateral+0.75Live+0.45Wind_Long2R
   3 0.6Dead+0.6Wind_Left1
   4 0.6Dead+0.6Wind_Right1
   5 0.6Dead+0.6Wind_Left2
   6 0.6Dead+0.6Wind_Long1L
   7 0.6Dead+0.6Wind_Long1R
   8 0.6Dead+0.6Wind_Long2R
   9 Dead+Collateral+MIN_SNOW
   10 0.6Dead+0.6Wind_Right2+0.6Wind_Suction
   11 0.6Dead+0.6Wind_Pressure+0.6Wind_Long2L
   12 Dead+0.6Wind_Right2+0.6Wind_Suction

## GENERAL NOTES

1. All anchor bolts (by others) to have nuts and flat washers.
2. All anchor bolts are designed to full S.A.E. diameters with cut threads. No substitutions are allowed.
3. Olympia is not responsible for the design, materials and workmanship of the foundation. Anchor bolt plans prepared by Olympia are intended to show only location, diameter, and projection of anchor bolts required to attach the Metal Building System to the foundation. Olympia is responsible for providing to the Builder the loads imposed by the Metal Building System on the foundation. It is the responsibility of the End Customer to ensure that adequate provisions are made for specifying bolt embedment, bearing angles, tie rods, and/or other associated items embedded in the concrete foundation, as well as foundation design for the loads imposed by the Metal Building System, other imposed loads, and the bearing capacity of the soil and other conditions of the building site. This is typically the responsibility of the Design Professional or Engineer of Record, which is another reason that their involvement in the Construction Project from the outset is highly recommended.
   (2012 MBMA Metal Building Systems Manual, Section 3.2.2)

Components & Cladding

| Zone | Width (ft) | Length (ft) | Pressure(psf) Member | Pressure(psf) Panel | Suction(psf) Member | Suction(psf) Panel |
|---|---|---|---|---|---|---|
| 1 | | | 16.00 | 16.00 | −21.76 | −23.77 |
| 2 | | 6.00 | 16.00 | 16.00 | −25.68 | −39.93 |
| 3 | | 15.00 | 16.00 | 16.00 | −27.85 | −34.23 |
| 4 | 6.00 | | 16.00 | 16.00 | −25.68 | −39.93 |
| 5 | 6.00 | 15.00 | 16.00 | 16.00 | −27.85 | −56.33 |
| 6 | 6.00 | 6.00 | 16.00 | 16.00 | −25.68 | −60.14 |
| 7 | | | 17.60 | 21.80 | −19.40 | −23.60 |
| 8 | 6.00 | | 17.60 | 21.80 | −20.76 | −29.03 |




Panel Zone: Wind 1

**BUILDING "A"**

ERECTOR NOTE: ONLY USE DRAWINGS ISSUED "FOR ERECTION" TO ERECT BUILDING

The Engineer whose seal and signature appear on these documents represent Whirlwind Steel Buildings, Inc., and is not the Engineer of Record for the overall project. The Engineer's responsibility is limited to material designed and manufactured by Whirlwind Steel Buildings, Inc., and excludes parts such as doors, windows, foundation design, and erection of the building.

APPROVAL/REVIEWING AUTHORITY: PLEASE REVIEW APPROVAL DRAWINGS CAREFULLY

UNLESS NOTED OTHERWISE, IT WILL BE ASSUMED THAT ALL INFORMATION SHOWN ON THESE DRAWINGS HAS THE AFFIRMATION OF THE APPROVAL/REVIEW AUTHORITY. FAILURE TO RESPOND TO CLOUDED AREAS AND AREAS TO VERIFY MAY RESULT IN ADDITIONAL COSTS AND/OR SCHEDULE DELAYS FOR WHICH OLYMPIA WILL NOT BE RESPONSIBLE. FIELD VERIFY ALL EXISTING CONDITIONS PRIOR TO FABRICATION. ALL SUBSEQUENT CHANGES AFTER THE FIRST SUBMITTAL WILL BE CONSIDERED AS CONTRACTUAL CHANGES.

OLYMPIA STEEL BUILDINGS.

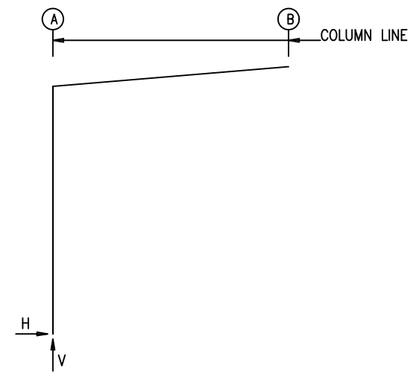
| REV | DATE | DESCRIPTION | BY | CHK |
|---|---|---|---|---|
| 0 | 09.09.20 | FOR CONSTRUCTION | MBS | TVN |

DESC: ANCHOR BOLT REACTIONS
BLDG. SIZE: VARIES
CUSTOMER: WYNN SITE DEVELOPMENT
LOCATION: Holly Springs, NC 27504
REFERENCE: WYNN SITE DEVELOPMENT
JOBSITE: Holly Springs, NC 27540
COUNTY: Wake
DWN: MBS | CHK: . | DATE: 9/8/20 | ENG: JG | JOB NO: 6707-23738 | DWG NO: F2 | ISSUE: 0

NORTH CAROLINA PROFESSIONAL SEAL 037025 ENGINEER RONALD H. MAYS

Sep 13, 2020

FRAME LINES: 1 2 3 4 5 6 7




## RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

| Frm Line | Col Line | Column_Reactions(k) Load Id | Hmax H | V Vmax | Load Id | Hmin H | V Vmin | Bolt(in) Qty | Dia | Base_Plate(in) Width | Length | Thick | Elev. (in) |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| 1* | A | 5 | 0.7 | −0.6 | 3 | −0.8 | 0.0 | 4 | 0.750 | 8.000 | 10.50 | 0.375 | 0.0 |
| | | 1 | 0.0 | 1.7 | 4 | 0.7 | −1.0 | | | | | | |

1* Frame lines: 1 7

## RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

| Frm Line | Col Line | Column_Reactions(k) Load Id | Hmax H | V Vmax | Load Id | Hmin H | V Vmin | Bolt(in) Qty | Dia | Base_Plate(in) Width | Length | Thick | Elev. (in) |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| 2* | A | 5 | 1.4 | −1.2 | 3 | −1.3 | 0.2 | 4 | 0.750 | 8.000 | 10.50 | 0.375 | 0.0 |
| | | 1 | 0.1 | 3.0 | 4 | 1.4 | −1.7 | | | | | | |

2* Frame lines: 2 3 4 5 6

## RIGID FRAME: BASIC COLUMN REACTIONS (k)

| Frame Line | Column Line | Dead Horiz | Dead Vert | Collateral Horiz | Collateral Vert | Live Horiz | Live Vert | Snow Horiz | Snow Vert | Wind_Left1 Horiz | Wind_Left1 Vert | Wind_Right1 Horiz | Wind_Right1 Vert |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| 1* | A | 0.0 | 0.4 | 0.0 | 0.1 | 0.0 | 1.3 | 0.0 | 0.8 | 0.0 | −1.9 | 1.1 | −1.4 |

| Frame Line | Column Line | Wind_Left2 Horiz | Wind_Left2 Vert | Wind_Right2 Horiz | Wind_Right2 Vert | Wind_Long1 Horiz | Wind_Long1 Vert | Wind_Long2 Horiz | Wind_Long2 Vert | Seismic_Left Horiz | Seismic_Left Vert | Seismic_Right Horiz | Seismic_Right Vert |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| 1* | A | −1.3 | −0.4 | −0.2 | 0.1 | 1.2 | −2.0 | 1.2 | −1.5 | 0.0 | 0.0 | 0.0 | 0.0 |

| Frame Line | Column Line | Seismic_Long Horiz | Seismic_Long Vert | MIN_SNOW Horiz | MIN_SNOW Vert |
|---|---|---|---|---|---|
| 1* | A | 0.0 | 0.0 | 0.0 | 1.0 |

| Frame Line | Column Line | Dead Horiz | Dead Vert | Collateral Horiz | Collateral Vert | Live Horiz | Live Vert | Snow Horiz | Snow Vert | Wind_Left1 Horiz | Wind_Left1 Vert | Wind_Right1 Horiz | Wind_Right1 Vert |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| 2* | A | 0.0 | 0.5 | 0.0 | 0.1 | 0.1 | 2.4 | 0.0 | 1.6 | 0.3 | −3.2 | 1.9 | −2.5 |

| Frame Line | Column Line | Wind_Left2 Horiz | Wind_Left2 Vert | Wind_Right2 Horiz | Wind_Right2 Vert | Wind_Long1 Horiz | Wind_Long1 Vert | Wind_Long2 Horiz | Wind_Long2 Vert | Seismic_Left Horiz | Seismic_Left Vert | Seismic_Right Horiz | Seismic_Right Vert |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| 2* | A | −2.2 | −0.1 | −0.6 | 0.5 | 2.3 | −3.4 | 2.3 | −2.6 | 0.0 | 0.0 | 0.0 | 0.0 |

| Frame Line | Column Line | Seismic_Long Horiz | Seismic_Long Vert | MIN_SNOW Horiz | MIN_SNOW Vert |
|---|---|---|---|---|---|
| 2* | A | 0.0 | 0.0 | 0.0 | 1.9 |

1* Frame lines: 1 7
2* Frame lines: 2 3 4 5 6

## BUILDING BRACING REACTIONS

| Wall Loc | Wall Line | Col Line | Wind ± Reactions(k) Horz | Wind Vert | Seismic ± Reactions(k) Horz | Seismic Vert | Panel_Shear (lb/ft) Wind | Seis | Note |
|---|---|---|---|---|---|---|---|---|---|
| L_EW | 1 | | | | | | | | (h) |
| F_SW | B | | | | | | | | (e) |
| R_EW | 7 | | | | | | | | (h) |
| B_SW | A | Torsional Bracing Used | | | | | | | |

(e)Bracing loads must be applied to supporting building
(h)Rigid frame at endwall

## NOTES FOR REACTIONS

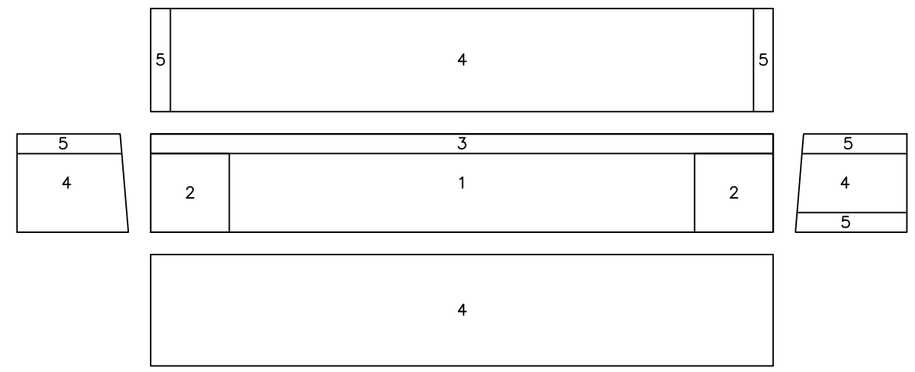
1. All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
2. Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
3. Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
4. Loading conditions are:
   1 Dead+Collateral+Live
   2 0.6Dead+0.6Wind_Left1
   3 0.6Dead+0.6Wind_Left2
   4 0.6Dead+0.6Wind_Long1R
   5 0.6Dead+0.6Wind_Long2R

## GENERAL NOTES

1. All anchor bolts (by others) to have nuts and flat washers.
2. All anchor bolts are designed to full S.A.E. diameters with cut threads. No substitutions are allowed.
3. Olympia is not responsible for the design, materials and workmanship of the foundation. Anchor bolt plans prepared by Olympia are intended to show only location, diameter, and projection of anchor bolts required to attach the Metal Building System to the foundation. Olympia is responsible for providing to the Builder the loads imposed by the Metal Building System on the foundation. It is the responsibility of the End Customer to ensure that adequate provisions are made for specifying bolt embedment, bearing angles, tie rods, and/or other associated items embedded in the concrete foundation, as well as foundation design for the loads imposed by the Metal Building System, other imposed loads, and the bearing capacity of the soil and other conditions of the building site. This is typically the responsibility of the Design Professional or Engineer of Record, which is another reason that their involvement in the Construction Project from the outset is highly recommended. (2012 MBMA Metal Building Systems Manual, Section 3.2.2)

Components & Cladding

| Zone | Width (ft) | Length (ft) | Pressure(psf) Member | Pressure(psf) Panel | Suction(psf) Member | Suction(psf) Panel |
|---|---|---|---|---|---|---|
| 1 | | | 16.00 | 17.12 | −33.24 | −33.24 |
| 2 | | 12.00 | 16.00 | 17.12 | −43.21 | −63.49 |
| 3 | 3.00 | | 16.00 | 17.12 | −35.23 | −37.23 |
| 4 | | | 25.10 | 29.20 | −26.90 | −31.00 |
| 5 | 3.00 | | 25.10 | 29.20 | −28.24 | −36.27 |




Panel Zone: Wind 1

**BUILDING "B"**

ERECTOR NOTE: ONLY USE DRAWINGS ISSUED "FOR ERECTION" TO ERECT BUILDING

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OLYMPIA STEEL BUILDINGS.

| REV | DATE | DESCRIPTION | BY | CHK |
|---|---|---|---|---|
| 0 | 09.09.20 | FOR CONSTRUCTION | MBS | TVN |

DESC: ANCHOR BOLT REACTIONS | BLDG. SIZE: VARIES
CUSTOMER: WYNN SITE DEVELOPMENT | LOCATION: Holly Springs, NC 27504
REFERENCE: WYNN SITE DEVELOPMENT
JOBSITE: Holly Springs, NC 27540 | COUNTY: Wake
DWN: MBS | CHK: . | DATE: 9/ 8/20 | ENG: JG | JOB NO: 6707-23738 | DWG NO: F3 | ISSUE 0

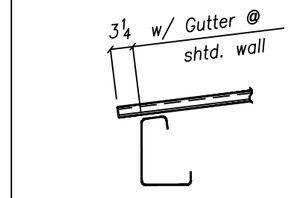
NORTH CAROLINA PROFESSIONAL SEAL 037025 ENGINEER RONALD H. MAYS

Sep 13, 2020

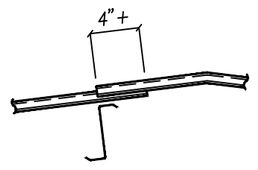
| SPLICE BOLT TABLE | | | | | | |
|---|---|---|---|---|---|---|
| Mark | Qty Top | Bot | Int | Type | Dia | Length |
| SP–1 | 4 | 4 | 0 | A325 | 3/4" | 1 3/4" |
| SP–2 | 4 | 0 | 0 | A325 | 3/4" | 1 3/4" |

| BASE PLATE TABLE | | | |
|---|---|---|---|
| Col Mark | Plate Size Width | Thick | Length |
| BP–1 | 8" | 3/8" | 11" |
| BP–2 | 8" | 3/8" | 10 1/2" |

▽FLANGE BRACES: FBxx (1 or 2)
xx=length(in)
(1) One Side; (2) Two Sides
A – FBN2214

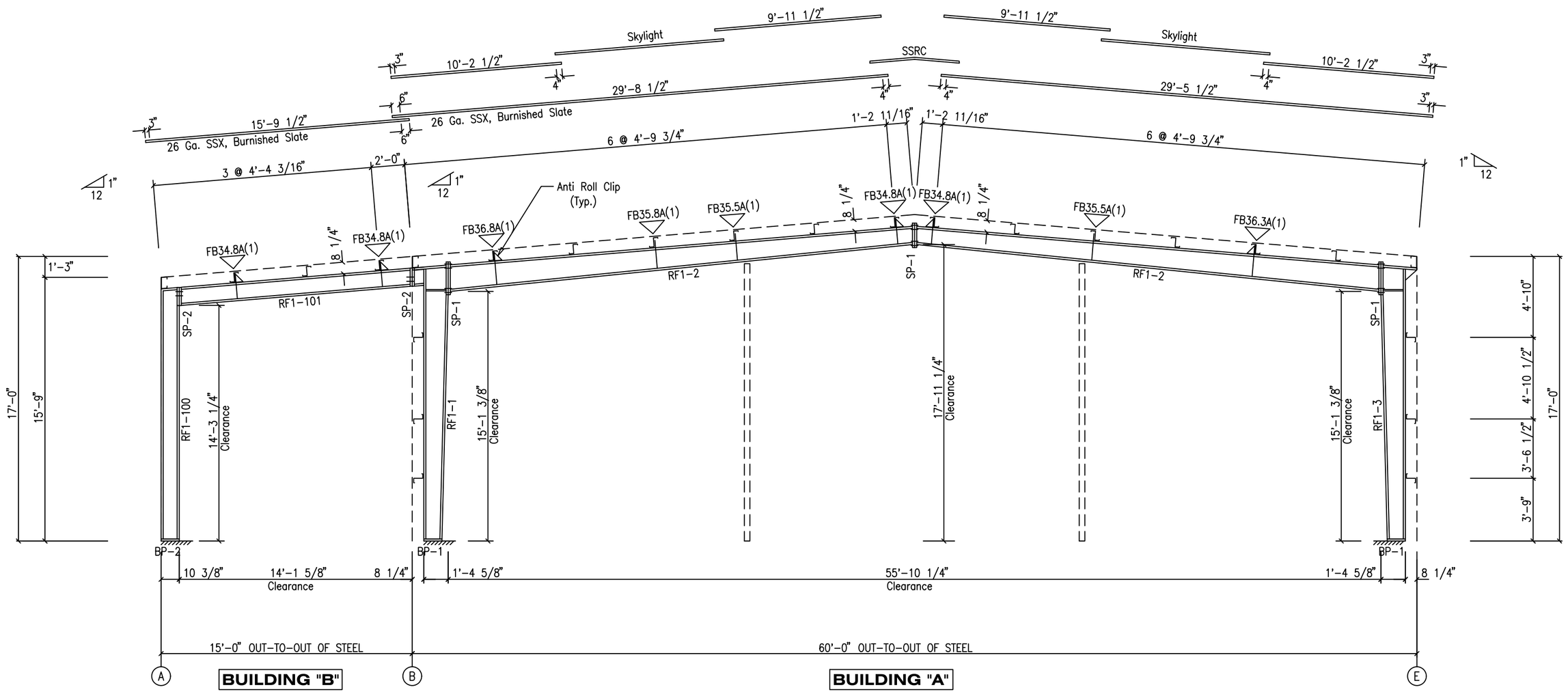



DETAIL @ PANEL END




DETAIL @ RIDGE CAP

| MEMBER TABLE | | | | |
|---|---|---|---|---|
| Mark | Web Depth Start/End | Web Plate Thick | Outside Flange W x Thk | Inside Flange W x Thk |
| RF1–1 | 10.0/16.0 | 0.135 | 6 x 5/16"<br>6 x 3/16" | 6 x 5/16" |
| RF1–2 | 16.0/11.7 | 0.135 | 5 x 3/16" | 5 x 3/16" |
| | 11.7/10.0 | 0.135 | 5 x 3/16" | 5 x 3/16" |
| RF1–3 | 16.0/10.0 | 0.135 | 6 x 3/16"<br>6 x 5/16" | 6 x 5/16" |
| RF1–100 | 10.0/10.0 | 0.135 | 5 x 3/16"<br>6 x 3/16" | 5 x 3/16" |
| RF1–101 | 10.0/10.0 | 0.135 | 5 x 3/16" | 5 x 3/16" |




RIGID FRAME ELEVATION: FRAME LINE 1
(NON–EXPANDABLE FRAME LINE 1)
NOT DESIGNED FOR FUTURE EXPANSION

**Primary structural members are Gray Oxide Primer.**
**Secondary structural members are Pre-Galvanized.**

WARNING!
Skylights or plastic translucent roof panels must not be used for foot traffic nor should they support the undistributed weight of any individual. Roofing ladders or 1" x 12" planks must be used during installation, maintenance, or repair procedures.

The Metal Building Supplier nor the Engineer whose seal appears on these documents are responsible for any injury resulting from the skylight or plastic translucent roof panel failing from improper use.

TURN–OF–NUT TIGHTENING
Connections for rigid frames must be properly pre–tensioned. The Specification for Structural Joints Using ASTM A325 or A490 Bolts dated November 13, 1985 (future reference to this section is to be called the Code) recognizes four methods to properly tighten the bolts; 1) "Turn–of–Nut", 2) calibrated wrench, 3) alternate design bolts and 4) direct tension indicator. All of these methods require special bolts and/or equipment to install, except the Turn–of–Nut Method. This is why Olympia specifies this method for bolt installation.

According to paragraph 7(c) of the Code, washers are not required to be installed.

Excerpts from the Code for installation come from paragraph 8(d): "Bolts shall be installed in all holes of the connection and brought to a snug–tight condition. Snug–tight is defined as the tightness that exist when the plies of the joint are in firm contact. This may be attained by a few impacts of an impact wrench or the full effort of a man using an ordinary spud wrench. Snug tightening shall progress systematically from the most rigid part of the connection to the free edges, and then the bolts of the connection shall be retightened in a similar systematic manner as necessary until all bolts are simultaneously snug tight and the connection is fully compacted. Following this initial operation, all bolts in the connection shall be tightened further by the applicable amount of rotation specified in table 5..."

Table 5. Nut Rotation From Snug–Tight Condition.

Bolt Length (from under side of head to end of bolt)
Up to and including 4 diameters ..... 1/3 turn
Over 4 diameters, but not exceeding 8 diameters ..... 1/2 turn
Over 8 diameters, but not exceeding 12 diameters ..... 2/3 turn

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OLYMPIA STEEL BUILDINGS

| REV | DATE | DESCRIPTION | BY | CHK |
|---|---|---|---|---|
| 0 | 09.09.20 | FOR CONSTRUCTION | MBS | TVN |

DESC: RIGID FRAME ELEVATION
BLDG. SIZE: VARIES
CUSTOMER: WYNN SITE DEVELOPMENT
LOCATION: Holly Springs, NC 27504
REFERENCE: WYNN SITE DEVELOPMENT
JOBSITE: Holly Springs, NC 27540
COUNTY: Wake

| DWN: | CHK: | DATE: | ENG: | JOB NO: | DWG NO: | ISSUE |
|---|---|---|---|---|---|---|
| MBS | . | 9/ 8/20 | JG | 6707–23738 | P1 | 0 |

NORTH CAROLINA PROFESSIONAL SEAL 037025 ENGINEER RONALD H. MAYS

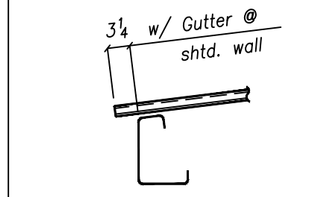
Sep 13, 2020

| SPLICE BOLT TABLE | | | | | | |
|---|---|---|---|---|---|---|
| Mark | Qty Top | Bot | Int | Type | Dia | Length |
| SP-1 | 4 | 4 | 0 | A325 | 3/4" | 2" |
| SP-2 | 4 | 4 | 0 | A325 | 3/4" | 1 3/4" |
| SP-3 | 4 | 0 | 0 | A325 | 3/4" | 1 3/4" |

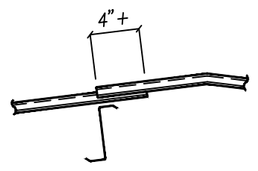
| BASE PLATE TABLE | | | |
|---|---|---|---|
| Col Mark | Plate Size Width | Thick | Length |
| BP-1 | 8" | 3/8" | 11" |
| BP-2 | 8" | 3/8" | 10 1/2" |

▽ FLANGE BRACES: FBxx (1 or 2)
xx=length(in)
(1) One Side; (2) Two Sides

A - FBN2214

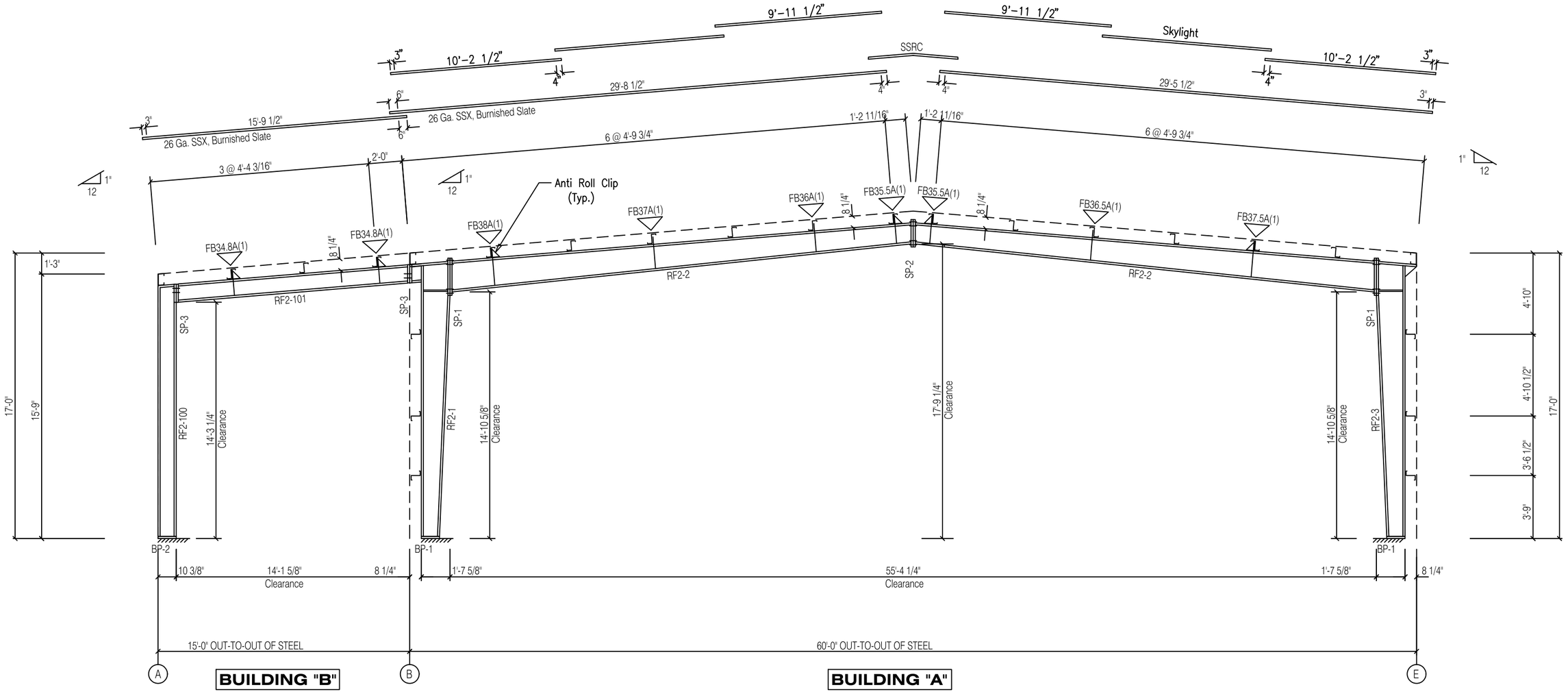



DETAIL @ PANEL END




DETAIL @ RIDGE CAP

| MEMBER TABLE | | | | |
|---|---|---|---|---|
| Mark | Web Depth Start/End | Web Plate Thick | Outside Flange W x Thk | Inside Flange W x Thk |
| RF2-1 | 10.0/19.0 | 0.135 | 6 x 5/16" | 6 x 5/16" |
| | 19.0/19.0 | 0.164 | 6 x 3/16" | |
| RF2-2 | 19.0/14.0 | 0.135 | 5 x 3/16" | 5 x 1/4" |
| | 14.0/12.0 | 0.135 | 5 x 1/4" | 5 x 3/16" |
| RF2-3 | 19.0/19.0 | 0.164 | 6 x 3/16" | 6 x 5/16" |
| | 19.0/10.0 | 0.135 | 6 x 5/16" | |
| RF2-100 | 10.0/10.0 | 0.135 | 5 x 3/16" | 5 x 3/16" |
| | | | 6 x 3/16" | |
| RF2-101 | 10.0/10.0 | 0.135 | 5 x 3/16" | 5 x 3/16" |




RIGID FRAME ELEVATION: FRAME LINE 2 3 4 5

**Primary structural members are Gray Oxide Primer.**
**Secondary structural members are Pre-Galvanized.**

WARNING!
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Connections for rigid frames must be properly pre–tensioned. The Specification for Structural Joints Using ASTM A325 or A490 Bolts dated November 13, 1985 (future reference to this section is to be called the Code) recognizes four methods to properly tighten the bolts; 1) "Turn–of–Nut", 2) calibrated wrench, 3) alternate design bolts and 4) direct tension indicator. All of these methods require special bolts and/or equipment to install, except the Turn–of–Nut Method. This is why Olympia specifies this method for bolt installation.

According to paragraph 7(c) of the Code, washers are not required to be installed.

Excerpts from the Code for installation come from paragraph 8(d): "Bolts shall be installed in all holes of the connection and brought to a snug–tight condition. Snug–tight is defined as the tightness that exist when the plies of the joint are in firm contact. This may be attained by a few impacts of an impact wrench or the full effort of a man using an ordinary spud wrench. Snug tightening shall progress systematically from the most rigid part of the connection to the free edges, and then the bolts of the connection shall be retightened in a similar systematic manner as necessary until all bolts are simultaneously snug tight and the connection is fully compacted. Following this initial operation, all bolts in the connection shall be tightened further by the applicable amount of rotation specified in table 5..."

Table 5. Nut Rotation From Snug–Tight Condition.

Bolt Length (from under side of head to end of bolt)
Up to and including 4 diameters ..... 1/3 turn
Over 4 diameters, but not exceeding 8 diameters ..... 1/2 turn
Over 8 diameters, but not exceeding 12 diameters ..... 2/3 turn

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OLYMPIA STEEL BUILDINGS.

| REV | DATE | DESCRIPTION | BY | CHK |
|---|---|---|---|---|
| 0 | 09.09.20 | FOR CONSTRUCTION | MBS | TVN |

DESC: RIGID FRAME ELEVATION | BLDG. SIZE: VARIES
CUSTOMER: WYNN SITE DEVELOPMENT | LOCATION: Holly Springs, NC 27504
REFERENCE: WYNN SITE DEVELOPMENT
JOBSITE: Holly Springs, NC 27540 | COUNTY: Wake

| DWN: | CHK: | DATE: | ENG: | JOB NO: | DWG NO: | ISSUE |
|---|---|---|---|---|---|---|
| MBS | . | 9/ 8/20 | JG | 6707-23738 | P2 | 0 |

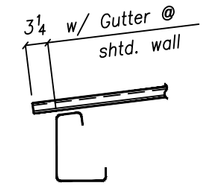
NORTH CAROLINA PROFESSIONAL SEAL 037025 ENGINEER RONALD H. MAYS

Sep 13, 2020

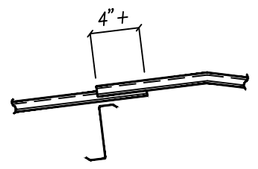
| SPLICE BOLT TABLE | | | | | | |
|---|---|---|---|---|---|---|
| Mark | Qty Top | Bot | Int | Type | Dia | Length |
| SP-1 | 4 | 4 | 0 | A325 | 3/4" | 2" |
| SP-2 | 4 | 4 | 0 | A325 | 3/4" | 1 3/4" |
| SP-3 | 4 | 0 | 0 | A325 | 3/4" | 1 3/4" |

| BASE PLATE TABLE | | | |
|---|---|---|---|
| Col Mark | Plate Size Width | Thick | Length |
| BP-1 | 8" | 3/8" | 11" |
| BP-2 | 8" | 3/8" | 10 1/2" |

▽ FLANGE BRACES: FBxx (1 or 2)
xx=length(in)
(1) One Side; (2) Two Sides
A - FBN2214

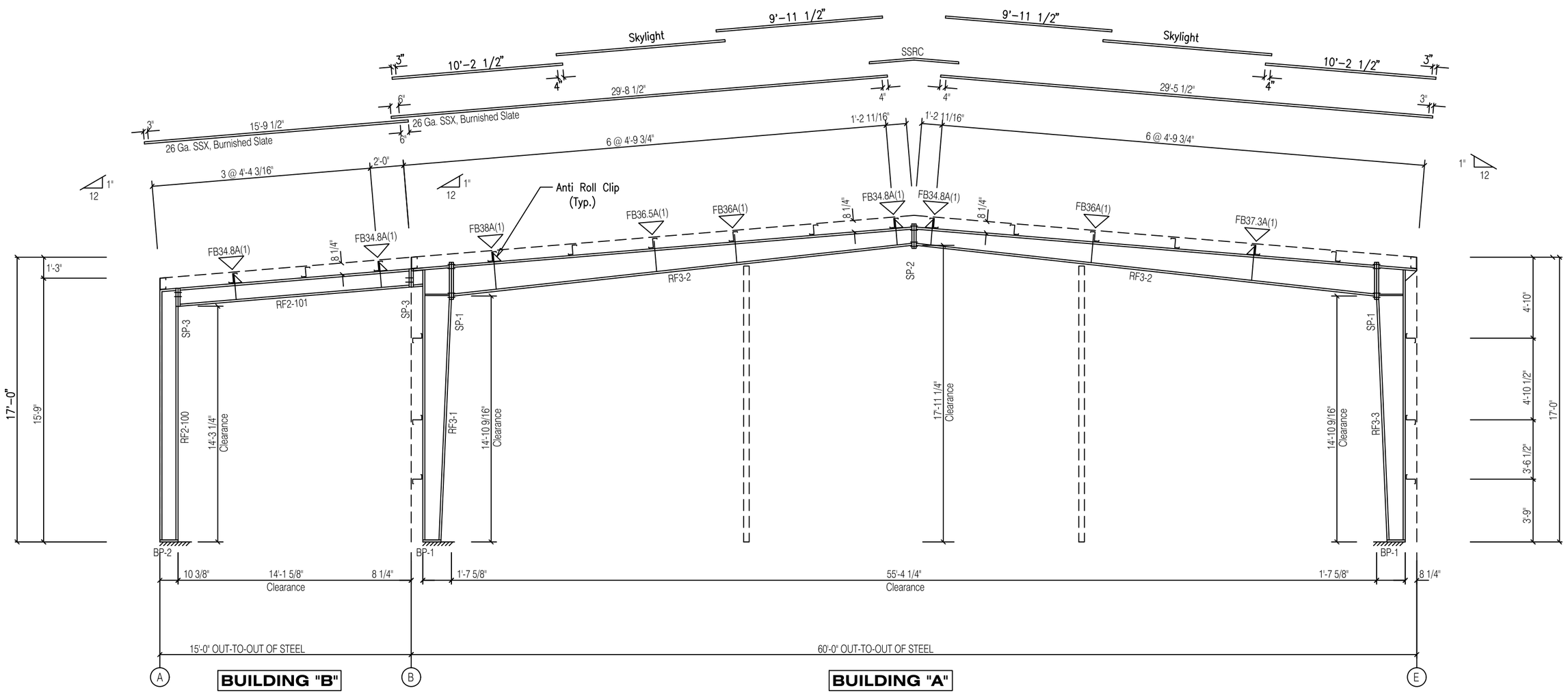



DETAIL @ PANEL END




DETAIL @ RIDGE CAP

| MEMBER TABLE | | | | |
|---|---|---|---|---|
| Mark | Web Depth Start/End | Web Plate Thick | Outside Flange W x Thk | Inside Flange W x Thk |
| RF3-1 | 10.0/19.0 | 0.164 | 8 x 5/16"<br>8 x 1/4" | 8 x 5/16" |
| RF3-2 | 19.0/12.5<br>12.5/10.0 | 0.135<br>0.135 | 5 x 3/16"<br>5 x 1/4" | 5 x 1/4"<br>5 x 3/16" |
| RF3-3 | 19.0/10.0 | 0.164 | 8 x 1/4"<br>8 x 5/16" | 8 x 5/16" |
| RF2-100 | 10.0/10.0 | 0.135 | 5 x 3/16"<br>6 x 3/16" | 5 x 3/16" |
| RF2-101 | 10.0/10.0 | 0.135 | 5 x 3/16" | 5 x 3/16" |




RIGID FRAME ELEVATION: FRAME LINE 6

**Primary structural members are Gray Oxide Primer.**
**Secondary structural members are Pre-Galvanized.**

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TURN–OF–NUT TIGHTENING
Connections for rigid frames must be properly pre–tensioned. The Specification for Structural Joints Using ASTM A325 or A490 Bolts dated November 13, 1985 (future reference to this section is to be called the Code) recognizes four methods to properly tighten the bolts; 1) "Turn–of–Nut", 2) calibrated wrench, 3) alternate design bolts and 4) direct tension indicator. All of these methods require special bolts and/or equipment to install, except the Turn–of–Nut Method. This is why Olympia specifies this method for bolt installation.

According to paragraph 7(c) of the Code, washers are not required to be installed.

Excerpts from the Code for installation come from paragraph 8(d): "Bolts shall be installed in all holes of the connection and brought to a snug–tight condition. Snug–tight is defined as the tightness that exist when the plies of the joint are in firm contact. This may be attained by a few impacts of an impact wrench or the full effort of a man using an ordinary spud wrench. Snug tightening shall progress systematically from the most rigid part of the connection to the free edges, and then the bolts of the connection shall be retightened in a similar systematic manner as necessary until all bolts are simultaneously snug tight and the connection is fully compacted. Following this initial operation, all bolts in the connection shall be tightened further by the applicable amount of rotation specified in table 5..."

Table 5. Nut Rotation From Snug–Tight Condition.

Bolt Length (from under side of head to end of bolt)
Up to and including 4 diameters ..... 1/3 turn
Over 4 diameters, but not exceeding 8 diameters ..... 1/2 turn
Over 8 diameters, but not exceeding 12 diameters ..... 2/3 turn

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OLYMPIA STEEL BUILDINGS.

| REV | DATE | DESCRIPTION | BY | CHK |
|---|---|---|---|---|
| 0 | 09.09.20 | FOR CONSTRUCTION | MBS | TVN |

DESC: RIGID FRAME ELEVATION
BLDG. SIZE: VARIES
CUSTOMER: WYNN SITE DEVELOPMENT
LOCATION: Holly Springs, NC 27504
REFERENCE: WYNN SITE DEVELOPMENT
JOBSITE: Holly Springs, NC 27540
COUNTY: Wake

| DWN: | CHK: | DATE: | ENG: | JOB NO: | DWG NO: | ISSUE |
|---|---|---|---|---|---|---|
| MBS | . | 9/ 8/20 | JG | 6707–23738 | P3 | 0 |

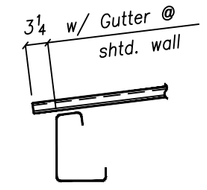
NORTH CAROLINA PROFESSIONAL SEAL 037025 ENGINEER RONALD H. MAYS

Sep 13, 2020

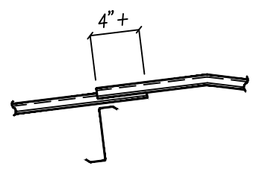
| SPLICE BOLT TABLE | | | | | | |
|---|---|---|---|---|---|---|
| Mark | Qty Top | Bot | Int | Type | Dia | Length |
| SP-1 | 4 | 4 | 0 | A325 | 3/4" | 2" |
| SP-2 | 4 | 4 | 0 | A325 | 3/4" | 1 3/4" |
| SP-3 | 4 | 0 | 0 | A325 | 3/4" | 1 3/4" |

| BASE PLATE TABLE | | | |
|---|---|---|---|
| Col Mark | Plate Size Width | Thick | Length |
| BP-1 | 8" | 3/8" | 1'-0 1/2" |
| BP-2 | 8" | 3/8" | 10 1/2" |

▽ FLANGE BRACES: FBxx (1 or 2)
xx=length(in)
(1) One Side; (2) Two Sides
A - FBN2214

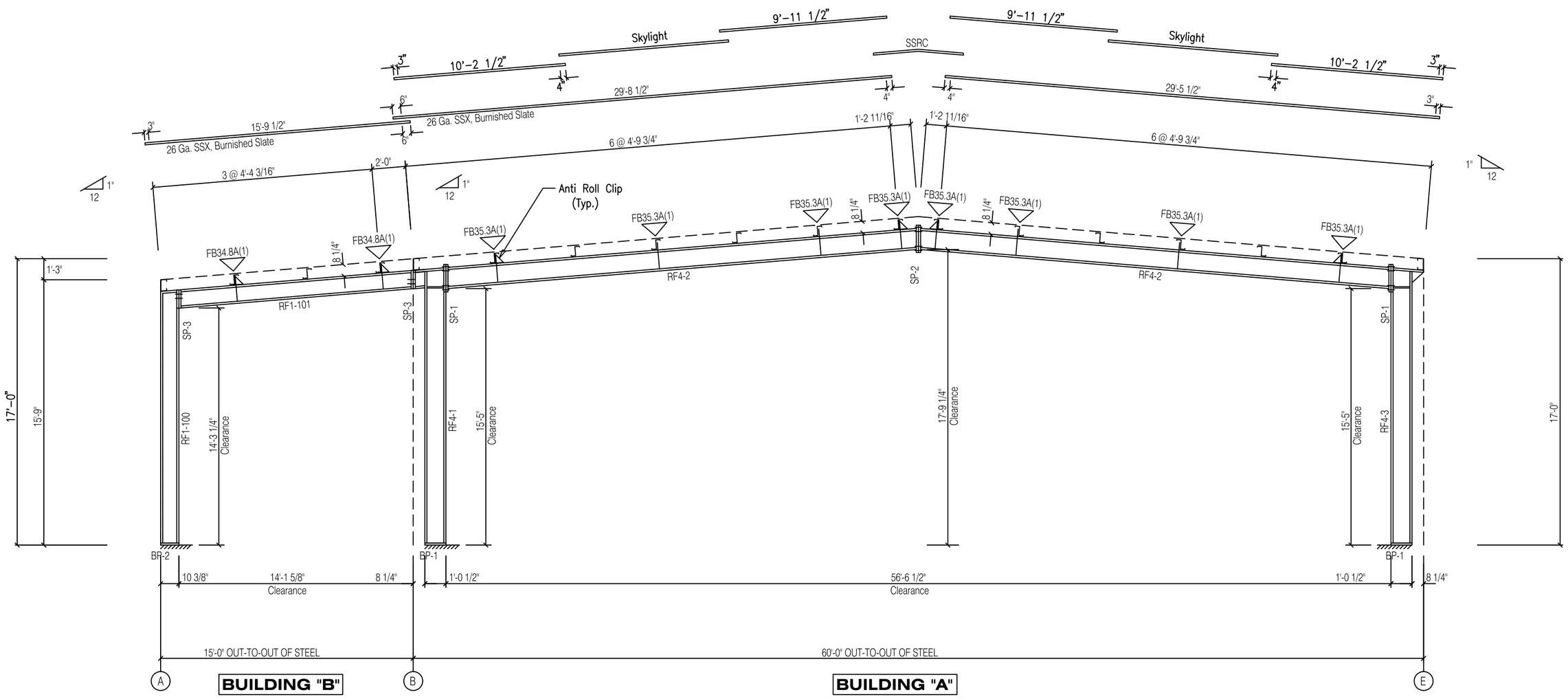



DETAIL @ PANEL END




DETAIL @ RIDGE CAP

| MEMBER TABLE | | | | |
|---|---|---|---|---|
| Mark | Web Depth Start/End | Web Plate Thick | Outside Flange W x Thk | Inside Flange W x Thk |
| RF4-1 | 12.0/12.0 | 0.135 | 6 x 3/16" | 6 x 5/16" |
| | 12.0/12.0 | 0.188 | 6 x 3/16" | |
| RF4-2 | 12.0/12.0 | 0.135 | 5 x 3/16" | 5 x 1/4" |
| | 12.0/12.0 | 0.135 | 5 x 3/16" | 5 x 3/16" |
| RF4-3 | 12.0/12.0 | 0.188 | 6 x 3/16" | 6 x 5/16" |
| | 12.0/12.0 | 0.135 | 6 x 3/16" | |
| RF1-100 | 10.0/10.0 | 0.135 | 5 x 3/16" | 5 x 3/16" |
| | | | 6 x 3/16" | |
| RF1-101 | 10.0/10.0 | 0.135 | 5 x 3/16" | 5 x 3/16" |




RIGID FRAME ELEVATION: FRAME LINE 7

(NON–EXPANDABLE FRAME LINE 1)
NOT DESIGNED FOR FUTURE EXPANSION

**Primary structural members are Gray Oxide Primer.**
**Secondary structural members are Pre-Galvanized.**

WARNING!
Skylights or plastic translucent roof panels must not be used for foot traffic nor should they support the undistributed weight of any individual. Roofing ladders or 1" x 12" planks must be used during installation, maintenance, or repair procedures.

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TURN–OF–NUT TIGHTENING
Connections for rigid frames must be properly pre–tensioned. The Specification for Structural Joints Using ASTM A325 or A490 Bolts dated November 13, 1985 (future reference to this section is to be called the Code) recognizes four methods to properly tighten the bolts; 1) "Turn–of–Nut", 2) calibrated wrench, 3) alternate design bolts and 4) direct tension indicator. All of these methods require special bolts and/or equipment to install, except the Turn–of–Nut Method. This is why Olympia specifies this method for bolt installation.

According to paragraph 7(c) of the Code, washers are not required to be installed.

Excerpts from the Code for installation come from paragraph 8(d): "Bolts shall be installed in all holes of the connection and brought to a snug–tight condition. Snug–tight is defined as the tightness that exist when the plies of the joint are in firm contact. This may be attained by a few impacts of an impact wrench or the full effort of a man using an ordinary spud wrench. Snug tightening shall progress systematically from the most rigid part of the connection to the free edges, and then the bolts of the connection shall be retightened in a similar systematic manner as necessary until all bolts are simultaneously snug tight and the connection is fully compacted. Following this initial operation, all bolts in the connection shall be tightened further by the applicable amount of rotation specified in table 5..."

Table 5. Nut Rotation From Snug–Tight Condition.

Bolt Length (from under side of head to end of bolt)
Up to and including 4 diameters ..... 1/3 turn
Over 4 diameters, but not exceeding 8 diameters ..... 1/2 turn
Over 8 diameters, but not exceeding 12 diameters ..... 2/3 turn

ERECTOR NOTE: ONLY USE DRAWINGS ISSUED "FOR ERECTION" TO ERECT BUILDING

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OLYMPIA STEEL BUILDINGS

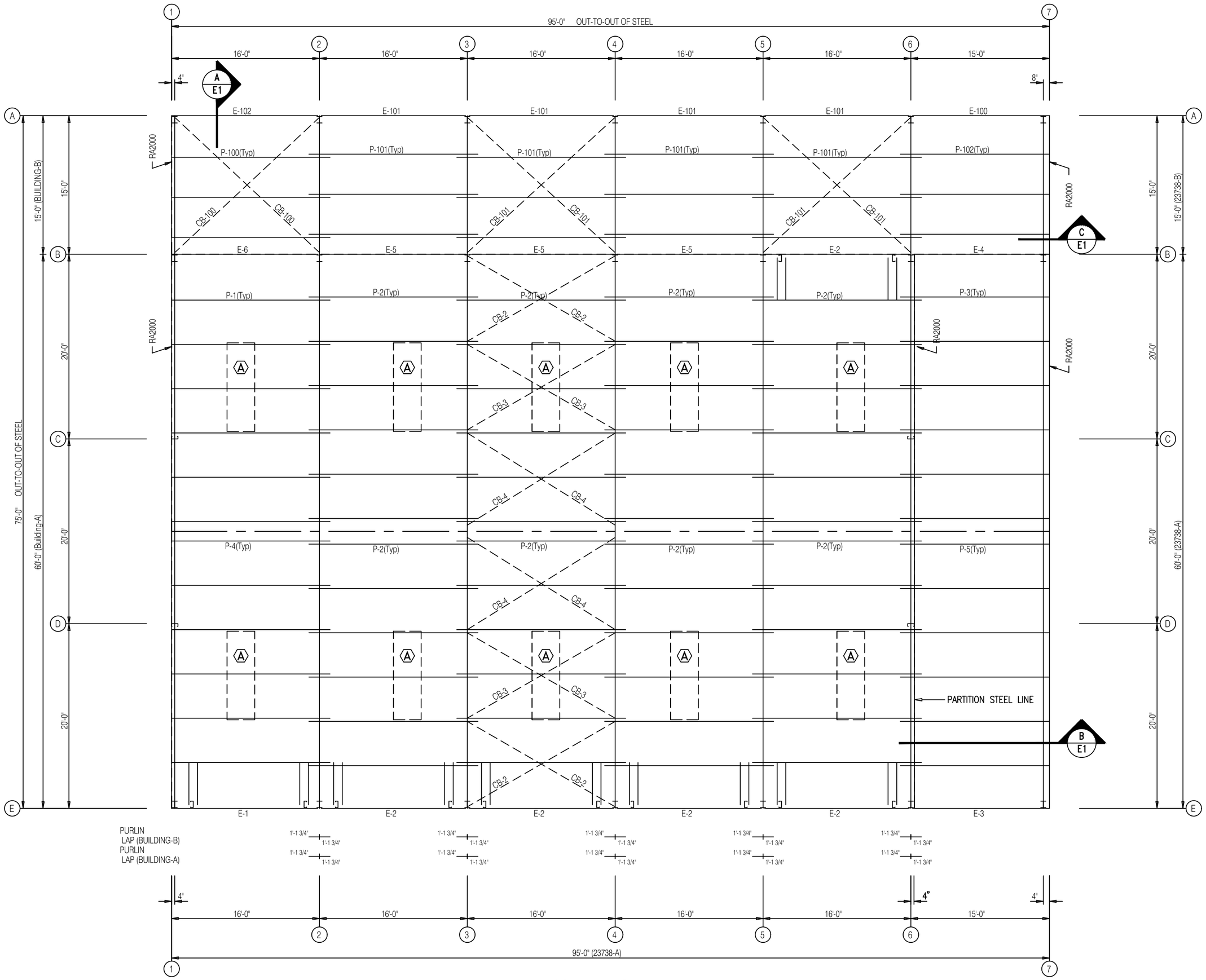
| REV | DATE | DESCRIPTION | BY | CHK |
|---|---|---|---|---|
| 0 | 09.09.20 | FOR CONSTRUCTION | MBS | TVN |

DESC: RIGID FRAME ELEVATION
BLDG. SIZE: VARIES
CUSTOMER: WYNN SITE DEVELOPMENT
LOCATION: Holly Springs, NC 27504
REFERENCE: WYNN SITE DEVELOPMENT
JOBSITE: Holly Springs, NC 27540
COUNTY: Wake

| DWN: | CHK: | DATE: | ENG: | JOB NO: | DWG NO: | ISSUE |
|---|---|---|---|---|---|---|
| MBS | . | 9/ 8/20 | JG | 6707–23738 | P1 | 0 |

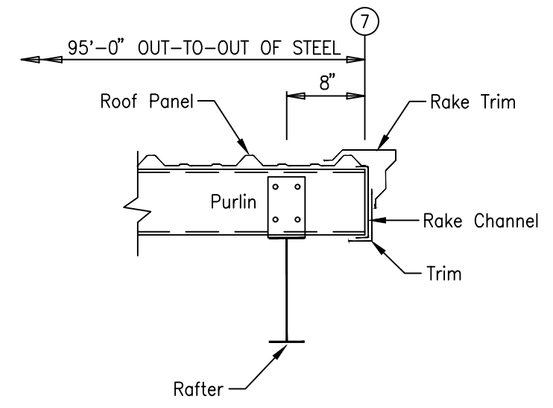
NORTH CAROLINA PROFESSIONAL SEAL 037025 ENGINEER RONALD H. MAYS

Sep 13, 2020

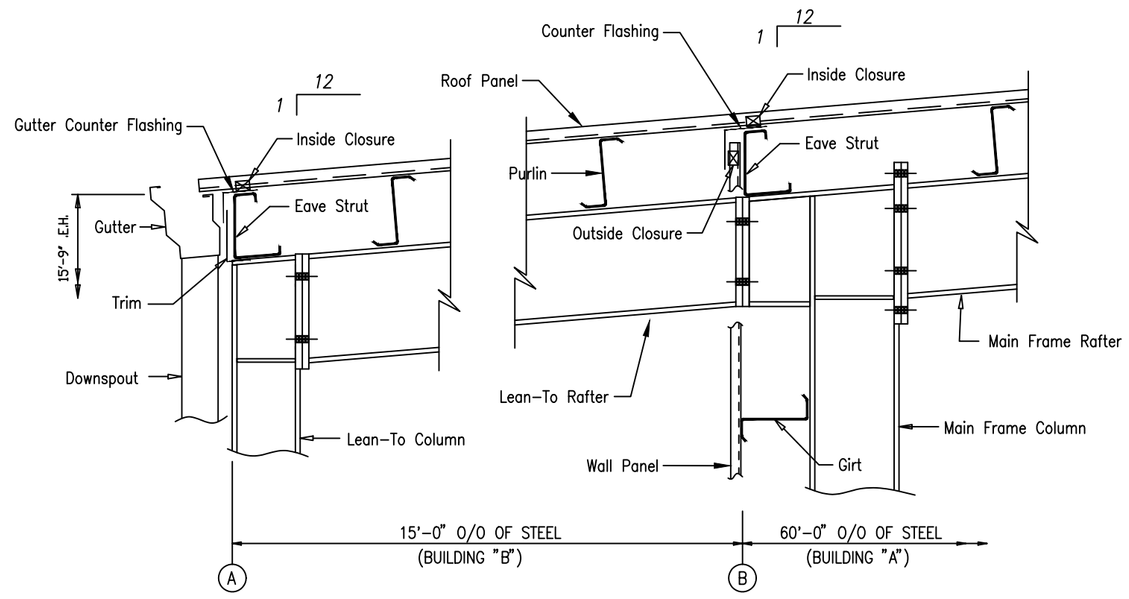



ROOF FRAMING PLAN

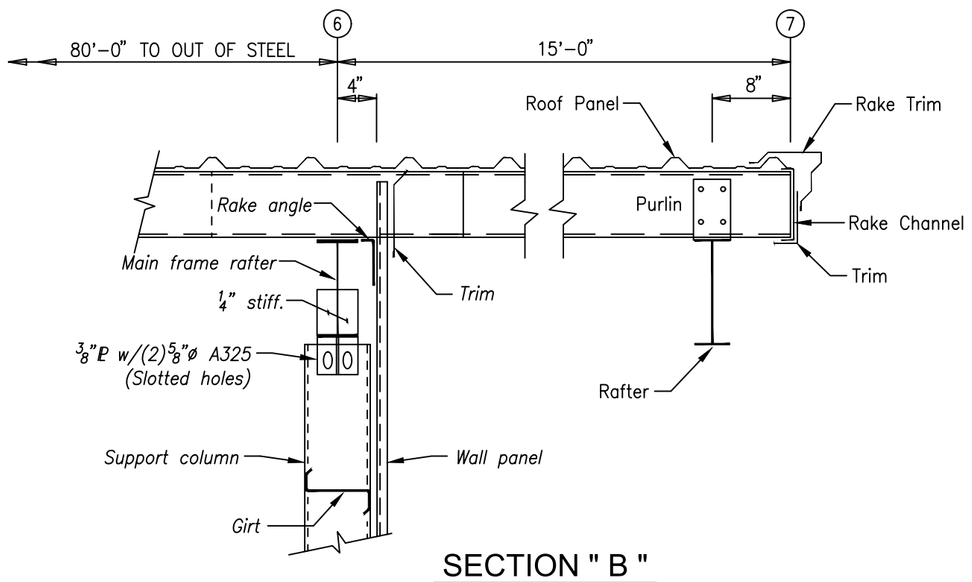
| MEMBER TABLE ROOF PLAN | |
|---|---|
| MARK | PART |
| BUILDING-A | |
| P-1 | 8X25Z16 |
| P-2 | 8X25Z16 |
| P-3 | 8X25Z16 |
| P-4 | 8X25Z16 |
| P-5 | 8X25Z16 |
| E-1 | 8ES141 |
| E-2 | 8ES141 |
| E-3 | 8ES141 |
| E-4 | 8ES141 |
| E-5 | 8ES141 |
| E-6 | 8ES141 |
| CB-2 | 0.25_CBL |
| CB-3 | 0.25_CBL |
| CB-4 | 0.25_CBL |
| BUILDING-B | |
| P-100 | 8X25Z16 |
| P-101 | 8X25Z16 |
| P-102 | 8X25Z16 |
| E-100 | 8ES141 |
| E-101 | 8ES141 |
| E-102 | 8ES141 |
| CB-100 | 0.50_ROD |
| CB-101 | 0.50_ROD |




SECTION " C "




SECTION " A "




SECTION " B "

WARNING!
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**Primary structural members are Gray Oxide Primer.**
**Secondary structural members are Pre-Galvanized.**

UL580, CLASS 90 CONST. NUMBERS
SUPER SPAN = 167

| ACCESSORIES | | | WALKDOORS AND WINDOWS | | | | FRAMED OPENINGS | | | | |
|---|---|---|---|---|---|---|---|---|---|---|---|
| MK | QTY | SKYLIGHTS - WHITE | MK. | QTY. | FINISH | DESCRIPTION | MK. | QTY. | WIDTH | HEIGHT | OPENING FOR: |
| A | 10 | Super Span @ 10'-8" Skylight | · | · | · | . | B | 4 | 3'-0" | 4'-6" | WINDOWS BY OTHERS |
| | | | · | · | · | . | C | 2 | 3'-4" | 7'-2" | DOOR BY OTHERS |
| | | | · | · | · | . | D | 6 | 12'-0" | 14'-0" | OVERHEAD DOOR BY OTHERS |
| MK | QTY | VENTILATORS | · | · | · | . | · | · | · | · | . |
| | | NO VENTILATORS | · | · | · | . | · | · | · | · | . |
| | | | · | · | · | . | · | · | · | · | . |
| MK | QTY | LOUVERS | · | · | · | . | · | · | · | · | . |
| | | NO VENTILATORS | · | · | · | . | · | · | · | · | . |
| | | | · | · | · | . | · | · | · | · | . |

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OLYMPIA STEEL BUILDINGS.

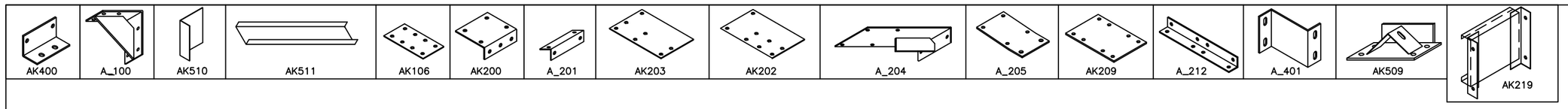
| REV | DATE | DESCRIPTION | BY | CHK |
|---|---|---|---|---|
| 0 | 09.09.20 | FOR CONSTRUCTION | MBS | TVN |

DESC: ROOF FRAMING PLAN | BLDG. SIZE: VARIES
CUSTOMER: WYNN SITE DEVELOPMENT | LOCATION: Holly Springs, NC 27504
REFERENCE: WYNN SITE DEVELOPMENT
JOBSITE: Holly Springs, NC 27540 | COUNTY: Wake

| DWN: | CHK: | DATE: | ENG: | JOB NO: | DWG NO: | ISSUE |
|---|---|---|---|---|---|---|
| MBS | . | 9/ 8/20 | JG | 6707-23738 | E1 | 0 |

NORTH CAROLINA PROFESSIONAL SEAL 037025 ENGINEER RONALD H. MAYS

Sep 13, 2020




BOLT TABLE
FRAME LINE 1

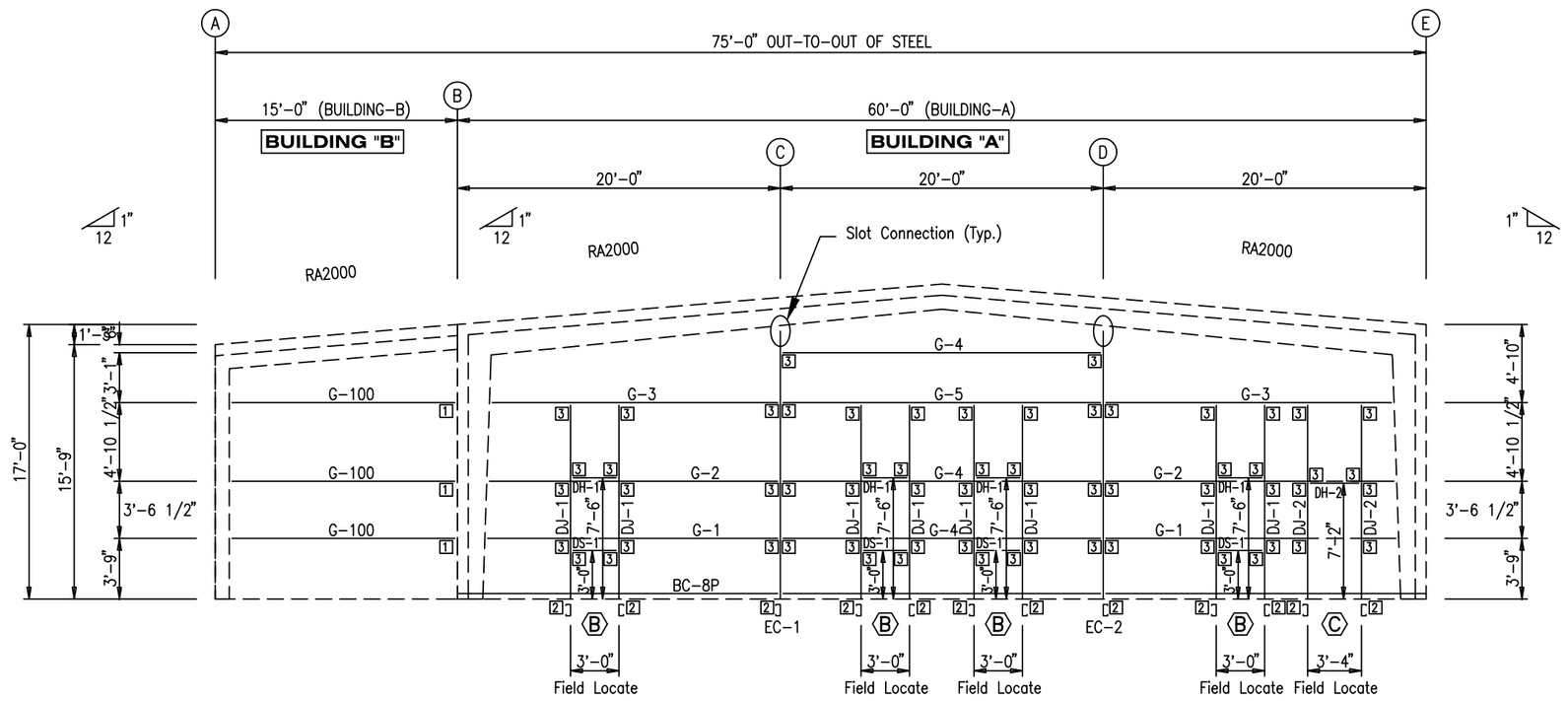
| LOCATION | QUAN | TYPE | DIA | LENGTH |
|---|---|---|---|---|
| BUILDING-A | | | | |
| Columns/Raf | 2 | A325 | 5/8" | 1 1/2" |

MEMBER TABLE
FRAME LINE 1

| MARK | PART |
|---|---|
| BUILDING-A | |
| EC-1 | 8M35C12 |
| EC-2 | 8M35C12 |
| DJ-1 | 8M35C14 |
| DJ-2 | 8M35C14 |
| DH-1 | 8M25C14 |
| DH-2 | 8M25C14 |
| DS-1 | 8M25C14 |
| G-1 | 8X25Z16 |
| G-2 | 8X25Z16 |
| G-3 | 8X25Z12 |
| G-4 | 8X25Z16 |
| G-5 | 8X25Z12 |
| BUILDING-B | |
| G-100 | 8X25Z16 |

CONNECTION PLATES
FRAME LINE 1

| ID | MARK/PART |
|---|---|
| 1 | r100 |
| 2 | AK400 |
| 3 | AK200 |




ENDWALL FRAMING: FRAME LINE 1

FIELD CUT GIRTS




ENDWALL SHEETING & TRIM: FRAME LINE 1

PANELS: 26 Ga. SSX – Burnished Slate

**Primary structural members are Gray Oxide Primer.**
**Secondary structural members are Pre-Galvanized.**

## GENERAL SHEETING & TRIM NOTES

1. Refer to erection drawings for rake angle locations.
2. Roof member screws are at 12" o.c. Eave end lap and peak screws are as shown.
3. Wall member screws are at 6" o.c. at the base member and 12" o.c. at all remaining members.
4. Roof stitch screws are located at each member with two between members (20" max. spacing).
5. Wall stitch screws are located at each member with one between members (20" max. spacing).
6. Skylight stitch screws are at 6" o.c.
7. Start endwall panels at centerline of bldg. unless noted.
8. Gutter, rake, & eave trim lap 2". All other trims lap 1".
9. Field cut or lap panels as required to fit.
10. Field cut panels for all openings.
11. Pop rivet gutter counterflashing to wall panel on 3'-0 centers and caulk all laps.
12. Gutter support strap spacing: Super Span 3'-0, Super Seam 2'-0, Weather Lok-16 2'-8".
13. Corner and/or peak boxes are not furnished with special rake or gutter profiles. Field miter as req'd.
14. Downspout strap spacing: 4" x 4" 8'-0 o.c. maximum, larger downspouts 5'-0 o.c maximum.
15. Hot-rolled or built-up members must be pre-drilled before attaching members screws.
16. Metal shavings must be swept from the roof each day to avoid surface rusting.
17. Windows and louvers must be installed before sheeting the walls.
18. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.

## GENERAL FRAMING NOTES

1. Angles are marked by their length in feet and inches.
2. Field cut or lap angles as required to fit.
3. Flange braces are marked by their length in decimal inches.
4. Outside flange of girt turns down unless noted.
5. Endwall girts and eave struts do not lap.
6. Field cut and self-tap girts at walk doors.
7. Field slot girts for brace rods or cables.
8. Field locate windows and walk doors.
9. Field weld all splices at 14 gauge valley gutters.
10. Field bolt AK400 base clip to endwall columns:
    (2) 5/8" x 1-1/2" A325 bolts if (1) AK400 req'd
    (2) 5/8" x 1-3/4" A325 bolts if (2) AK400 req'd
11. Locate top of roof framed openings flush with the pan of the roof panel.
12. Some field drilling at framed openings may be required. Field drill 9/16" diameter holes.
13. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.
14. Sub-jambs for overhead doors, if required, are not furnished by Olympia.

"SEE DRAWING D1 FOR BUILT-UP SECTION LEGEND"

ERECTOR NOTE: ONLY USE DRAWINGS ISSUED "FOR ERECTION" TO ERECT BUILDING



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OLYMPIA STEEL BUILDINGS.

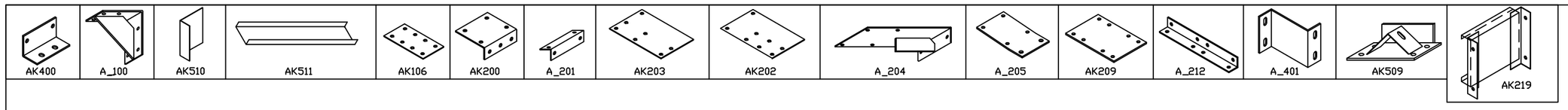
| REV | DATE | DESCRIPTION | BY | CHK |
|---|---|---|---|---|
| 0 | 09.09.20 | FOR CONSTRUCTION | MBS | TVN |

DESC: FRAMING ELEVATION
BLDG. SIZE: VARIES
CUSTOMER: WYNN SITE DEVELOPMENT
LOCATION: Holly Springs, NC 27504
REFERENCE: WYNN SITE DEVELOPMENT
JOBSITE: Holly Springs, NC 27540
COUNTY: Wake

| DWN: | CHK: | DATE: | ENG: | JOB NO: | DWG NO: | ISSUE |
|---|---|---|---|---|---|---|
| MBS | . | 9/ 8/20 | JG | 6707-23738 | E2 | 0 |

NORTH CAROLINA PROFESSIONAL SEAL 037025 ENGINEER RONALD H. MAYS

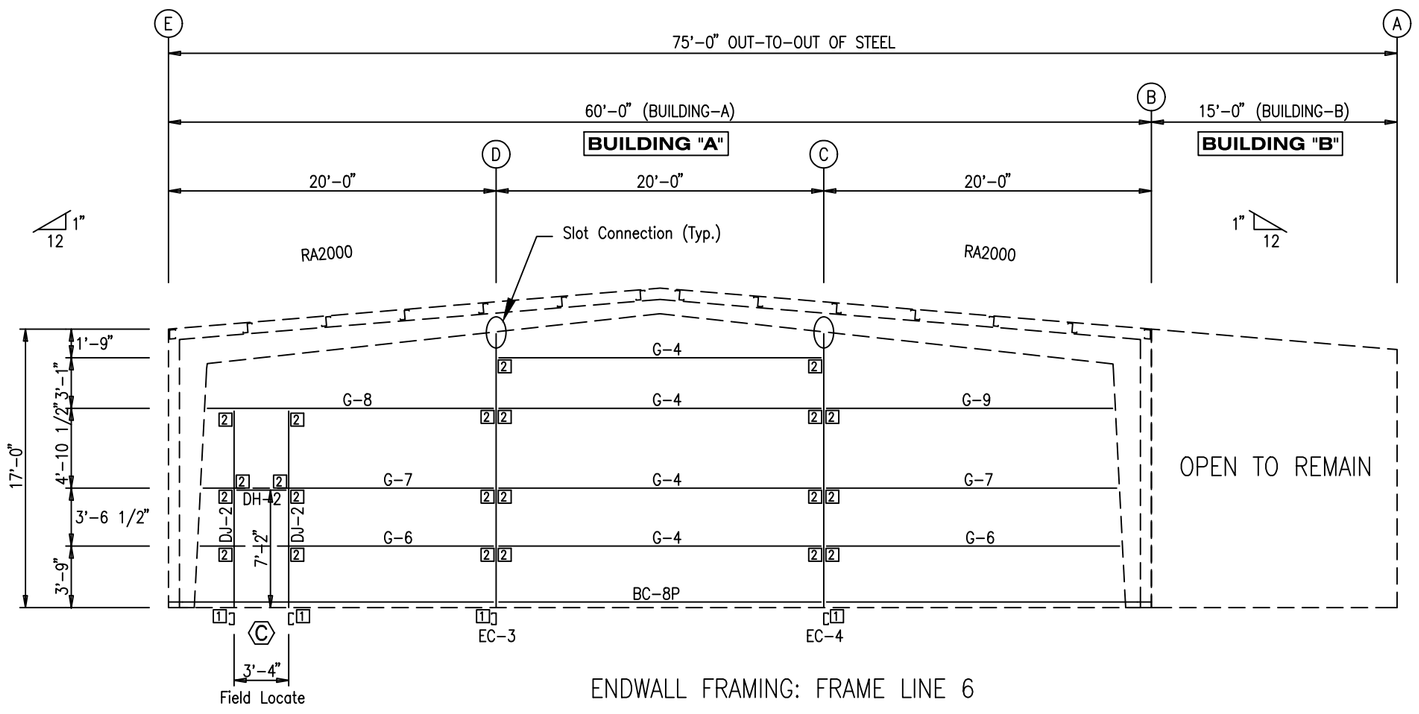
Sep 13, 2020



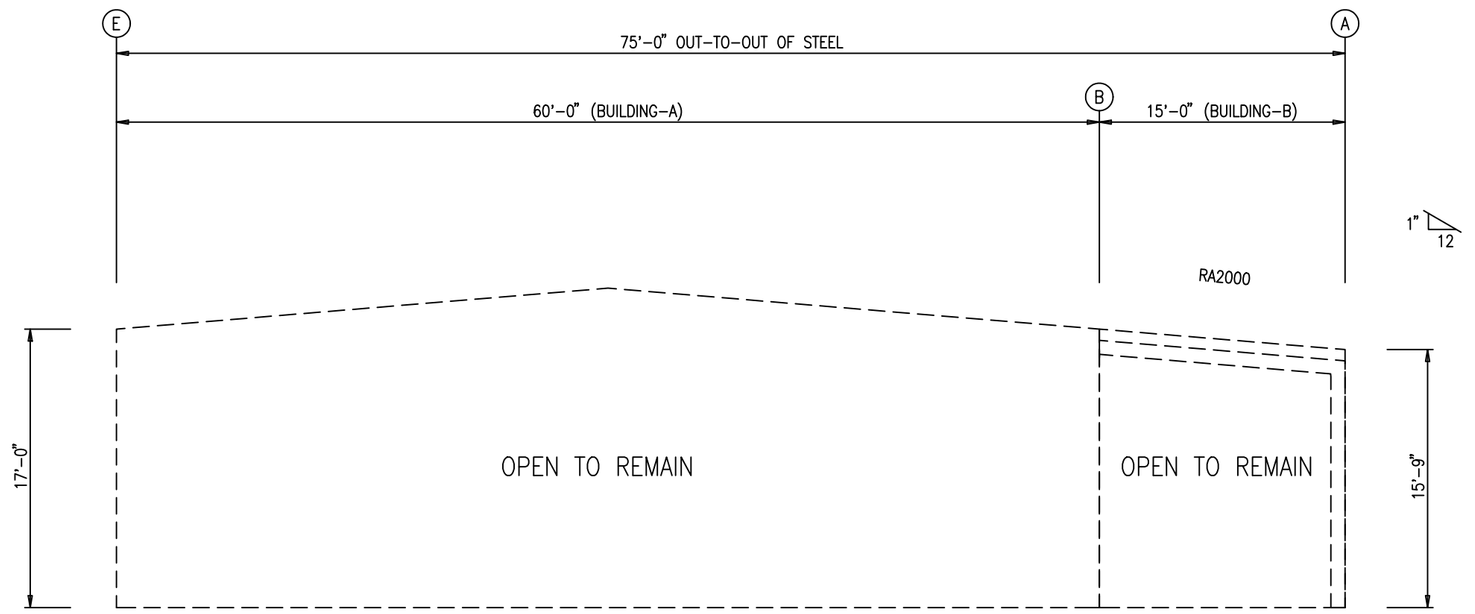

| BOLT TABLE FRAME LINE 7 | | | | |
|---|---|---|---|---|
| LOCATION | QUAN | TYPE | DIA | LENGTH |
| Columns/Raf | 2 | A325 | 5/8" | 1 1/2" |

| MEMBER TABLE FRAME LINE 7 | |
|---|---|
| MARK | PART |
| EC-3 | 8M35C12 |
| EC-4 | 8M35C12 |
| DJ-2 | 8M35C14 |
| DH-2 | 8M25C14 |
| G-4 | 8X25Z16 |
| G-6 | 8X25Z16 |
| G-7 | 8X25Z16 |
| G-8 | 8X25Z12 |
| G-9 | 8X25Z16 |

| CONNECTION PLATES FRAME LINE 7 | |
|---|---|
| ☐ID | MARK/PART |
| 1 | AK400 |
| 2 | AK200 |

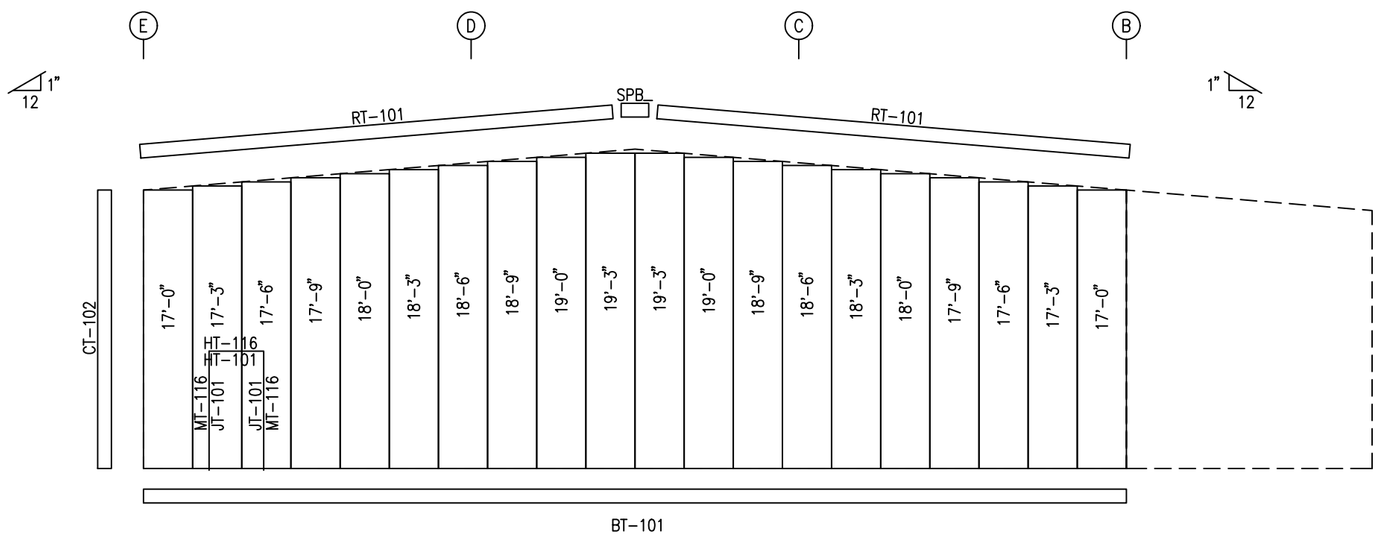



ENDWALL FRAMING: FRAME LINE 6




ENDWALL FRAMING: FRAME LINE 7

FIELD CUT GIRTS

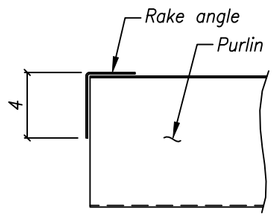



ENDWALL SHEETING & TRIM: FRAME LINE 6

PANELS: 26 Ga. SSX – Burnished Slate


ENDWALL SHEETING & TRIM: FRAME LINE 7

**Primary structural members are Gray Oxide Primer.**
**Secondary structural members are Pre-Galvanized.**




Detail at Rake Angle

## GENERAL SHEETING & TRIM NOTES

1. Refer to erection drawings for rake angle locations.
2. Roof member screws are at 12" o.c. Eave end lap and peak screws are as shown.
3. Wall member screws are at 6' o.c. at the base member and 12' o.c. at all remaining members.
4. Roof stitch screws are located at each member with two between members (20' max. spacing).
5. Wall stitch screws are located at each member with one between members (20' max. spacing).
6. Skylight stitch screws are at 6' o.c.
7. Start endwall panels at centerline of bldg. unless noted.
8. Gutter, rake, & eave trim lap 2'. All other trims lap 1'.
9. Field cut or lap panels as required to fit.
10. Field cut panels for all openings.
11. Pop rivet gutter counterflashing to wall panel on 3'-0 centers and caulk all laps.
12. Gutter support strap spacing: Super Span 3'-0, Super Seam 2'-0, Weather Lok-16 2'-8".
13. Corner and/or peak boxes are not furnished with special rake or gutter profiles. Field miter as req'd.
14. Downspout strap spacing: 4' x 4' 8'-0 o.c. maximum, larger downspouts 5'-0 o.c maximum.
15. Hot-rolled or built-up members must be pre-drilled before attaching members screws.
16. Metal shavings must be swept from the roof each day to avoid surface rusting.
17. Windows and louvers must be installed before sheeting the walls.
18. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.

## GENERAL FRAMING NOTES

1. Angles are marked by their length in feet and inches.
2. Field cut or lap angles as required to fit.
3. Flange braces are marked by their length in decimal inches.
4. Outside flange of girt turns down unless noted.
5. Endwall girts and eave struts do not lap.
6. Field cut and self-tap girts at walk doors.
7. Field slot girts for brace rods or cables.
8. Field locate windows and walk doors.
9. Field weld all splices at 14 gauge valley gutters.
10. Field bolt AK400 base clip to endwall columns:
    (2) 5/8" x 1-1/2" A325 bolts if (1) AK400 req'd
    (2) 5/8" x 1-3/4" A325 bolts if (2) AK400 req'd
11. Locate top of roof framed openings flush with the pan of the roof panel.
12. Some field drilling at framed openings may be required. Field drill 9/16" diameter holes.
13. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.
14. Sub-jambs for overhead doors, if required, are not furnished by Olympia.

'SEE DRAWING D1 FOR BUILT-UP SECTION LEGEND'

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OLYMPIA STEEL BUILDINGS.

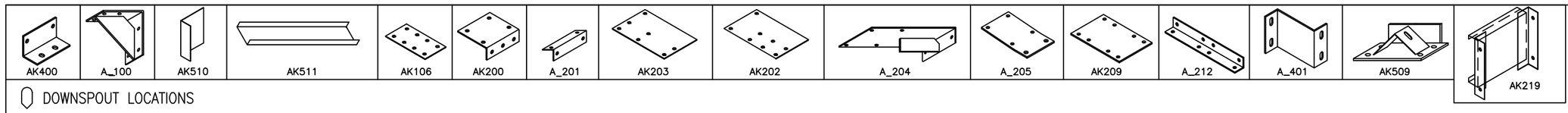
| REV | DATE | DESCRIPTION | BY | CHK |
|---|---|---|---|---|
| 0 | 09.09.20 | FOR CONSTRUCTION | MBS | TVN |

DESC: FRAMING ELEVATION | BLDG. SIZE: VARIES
CUSTOMER: WYNN SITE DEVELOPMENT | LOCATION: Holly Springs, NC 27504
REFERENCE: WYNN SITE DEVELOPMENT
JOBSITE: Holly Springs, NC 27540 | COUNTY: Wake

| DWN: | CHK: | DATE: | ENG: | JOB NO: | DWG NO: | ISSUE |
|---|---|---|---|---|---|---|
| MBS | . | 9/ 8/20 | JG | 6707-23738 | E3 | 0 |

NORTH CAROLINA PROFESSIONAL SEAL 037025 ENGINEER RONALD H. MAYS

Sep 13, 2020

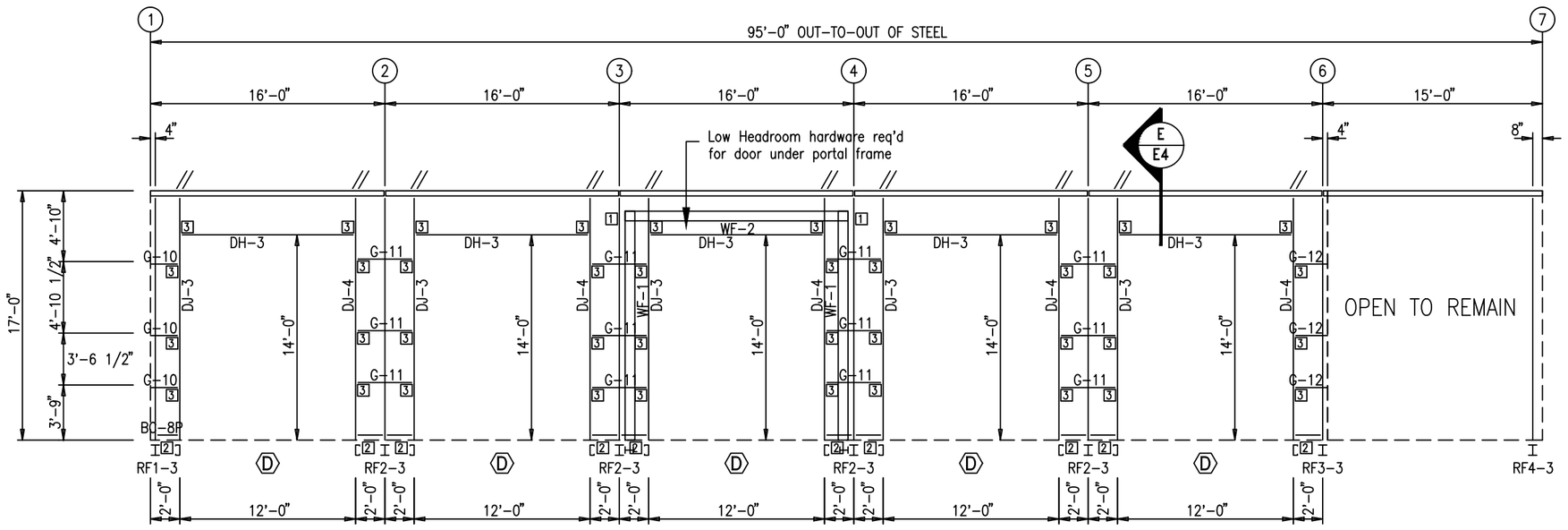



DOWNSPOUT LOCATIONS

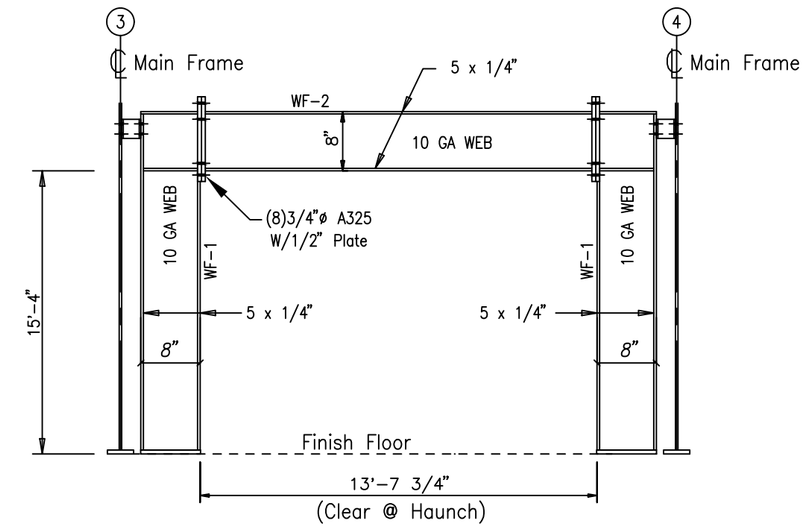
| BOLT TABLE FRAME LINE E | | | | |
|---|---|---|---|---|
| LOCATION | QUAN | TYPE | DIA | LENGTH |
| WF-1 – WF-2 | 8 | A325 | 3/4" | 2" |
| WF-1 – RF2-3 | 8 | A325 | 5/8" | 1 3/4" |

| MEMBER TABLE FRAME LINE E | |
|---|---|
| MARK | PART |
| WF-1 | SEE DETAIL |
| WF-2 | SEE DETAIL |
| DJ-3 | 8M35C14 |
| DJ-4 | 8M35C14 |
| DH-3 | 8M35C14 |
| G-10 | 8X25Z16 |
| G-11 | 8X25Z16 |
| G-12 | 8X25Z16 |

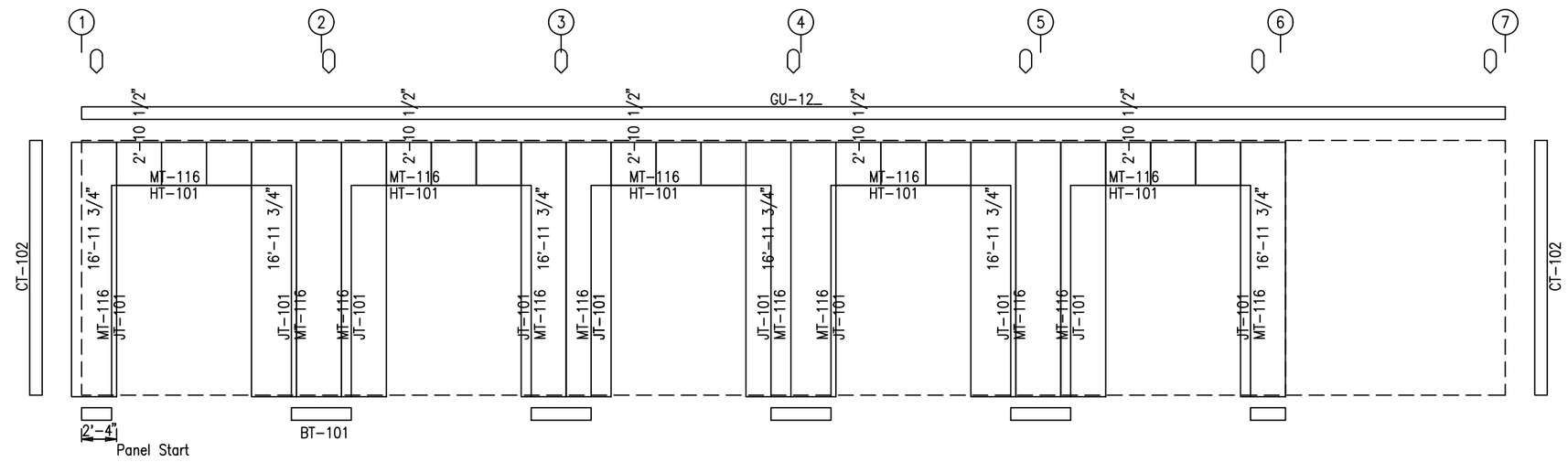
| CONNECTION PLATES FRAME LINE E | |
|---|---|
| ☐ID | MARK/PART |
| 1 | AK508 |
| 2 | AK400 |
| 3 | AK200 |




SIDEWALL FRAMING: FRAME LINE E



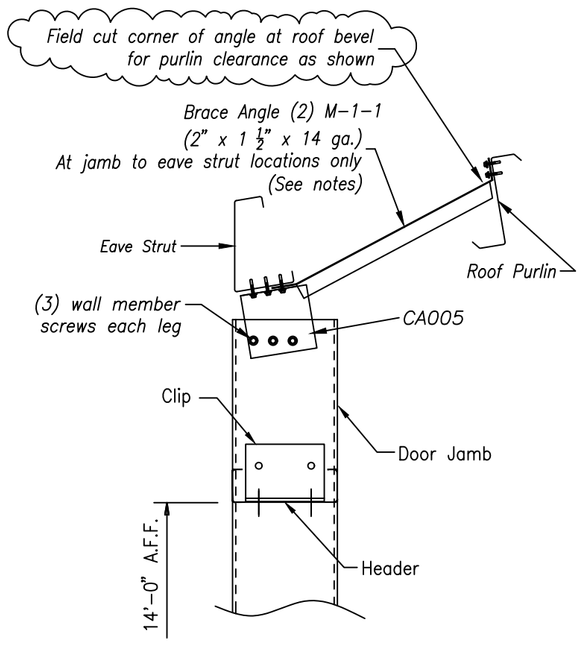

DETAIL @ PORTAL FRAME




SIDEWALL SHEETING & TRIM: FRAME LINE E

PANELS: 26 Ga. SX – Burnished Slate

**Primary structural members are Gray Oxide Primer.**
**Secondary structural members are Pre-Galvanized.**




SECTION " E "

## GENERAL SHEETING & TRIM NOTES

1. Refer to erection drawings for rake angle locations.
2. Roof member screws are at 12" o.c. Eave end lap and peak screws are as shown.
3. Wall member screws are at 6" o.c. at the base member and 12" o.c. at all remaining members.
4. Roof stitch screws are located at each member with two between members (20" max. spacing).
5. Wall stitch screws are located at each member with one between members (20" max. spacing).
6. Skylight stitch screws are at 6" o.c.
7. Start endwall panels at centerline of bldg. unless noted.
8. Gutter, rake, & eave trim lap 2". All other trims lap 1".
9. Field cut or lap panels as required to fit.
10. Field cut panels for all openings.
11. Pop rivet gutter counterflashing to wall panel on 3'–0 centers and caulk all laps.
12. Gutter support strap spacing: Super Span 3'–0, Super Seam 2'–0, Weather Lok–16 2'–8".
13. Corner and/or peak boxes are not furnished with special rake or gutter profiles. Field miter as req'd.
14. Downspout strap spacing: 4" x 4" 8'–0 o.c. maximum, larger downspouts 5'–0 o.c maximum.
15. Hot–rolled or built–up members must be pre–drilled before attaching members screws.
16. Metal shavings must be swept from the roof each day to avoid surface rusting.
17. Windows and louvers must be installed before sheeting the walls.
18. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw–down type roof for additional installation instructions.

## GENERAL FRAMING NOTES

1. Angles are marked by their length in feet and inches.
2. Field cut or lap angles as required to fit.
3. Flange braces are marked by their length in decimal inches.
4. Outside flange of girt turns down unless noted.
5. Endwall girts and eave struts do not lap.
6. Field cut and self–tap girts at walk doors.
7. Field slot girts for brace rods or cables.
8. Field locate windows and walk doors.
9. Field weld all splices at 14 gauge valley gutters.
10. Field bolt AK400 base clip to endwall columns:
    (2) 5/8" x 1–1/2" A325 bolts if (1) AK400 req'd
    (2) 5/8" x 1–3/4" A325 bolts if (2) AK400 req'd
11. Locate top of roof framed openings flush with the pan of the roof panel.
12. Some field drilling at framed openings may be required. Field drill 9/16" diameter holes.
13. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw–down type roof for additional installation instructions.
14. Sub–jambs for overhead doors, if required, are not furnished by Olympia.

"SEE DRAWING D1 FOR BUILT–UP SECTION LEGEND"

ERECTOR NOTE: ONLY USE DRAWINGS ISSUED "FOR ERECTION" TO ERECT BUILDING



APPROVAL/REVIEWING AUTHORITY: PLEASE REVIEW APPROVAL DRAWINGS CAREFULLY

UNLESS NOTED OTHERWISE, IT WILL BE ASSUMED THAT ALL INFORMATION SHOWN ON THESE DRAWINGS HAS THE AFFIRMATION OF THE APPROVAL/REVIEW AUTHORITY. FAILURE TO RESPOND TO CLOUDED AREAS AND AREAS TO VERIFY MAY RESULT IN ADDITIONAL COSTS AND/OR SCHEDULE DELAYS FOR WHICH OLYMPIA WILL NOT BE RESPONSIBLE. FIELD VERIFY ALL EXISTING CONDITIONS PRIOR TO FABRICATION. ALL SUBSEQUENT CHANGES AFTER THE FIRST SUBMITTAL WILL BE CONSIDERED AS CONTRACTUAL CHANGES.

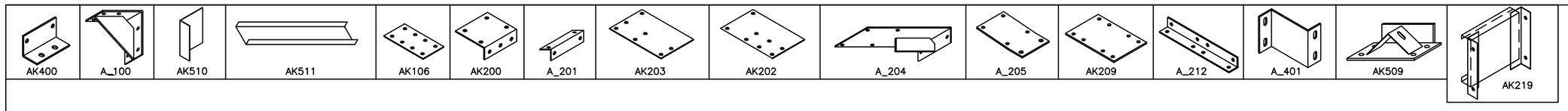
OLYMPIA STEEL BUILDINGS.

| REV | DATE | DESCRIPTION | BY | CHK |
|---|---|---|---|---|
| 0 | 09.09.20 | FOR CONSTRUCTION | MBS | TVN |

| | | | | | |
|---|---|---|---|---|---|
| DESC: FRAMING ELEVATION | | | | BLDG. SIZE: VARIES | |
| CUSTOMER: WYNN SITE DEVELOPMENT | | | | LOCATION: Holly Springs, NC 27504 | |
| REFERENCE: WYNN SITE DEVELOPMENT | | | | | |
| JOBSITE: Holly Springs, NC 27540 | | | | COUNTY: Wake | |
| DWN: MBS | CHK: . | DATE: 9/ 8/20 | ENG: JG | JOB NO: 6707-23738 | DWG NO: E4 / ISSUE 0 |

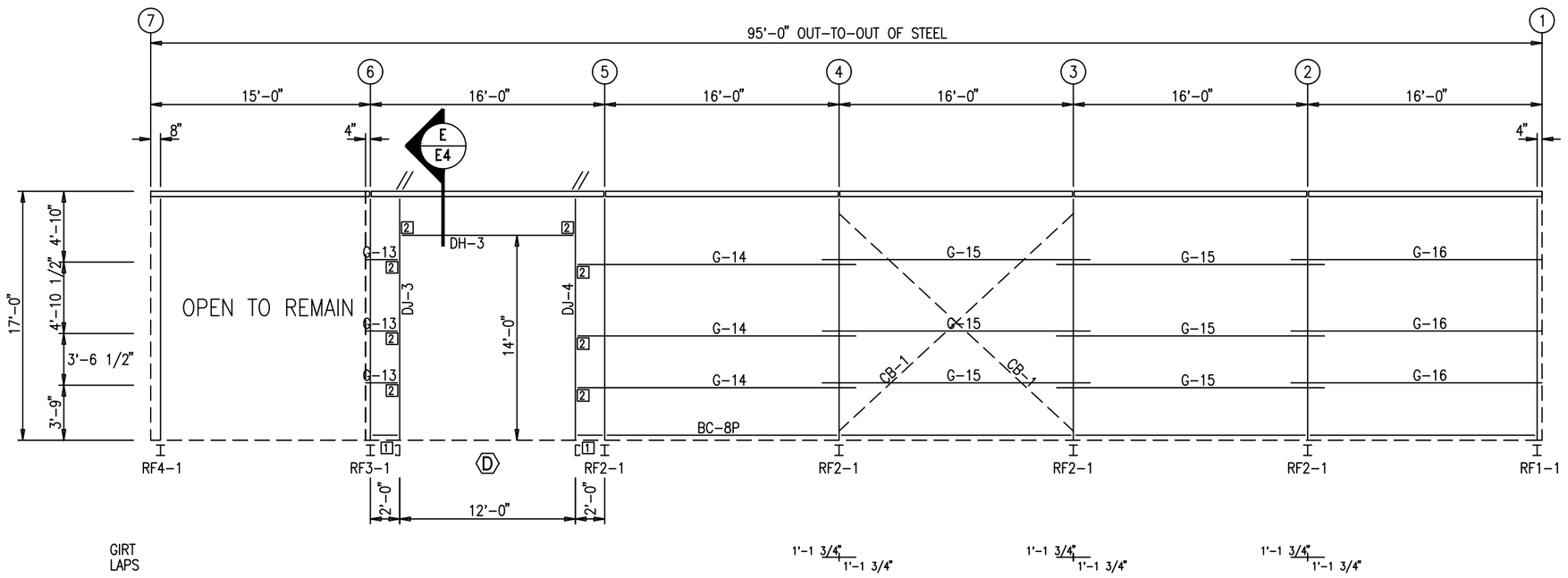
NORTH CAROLINA PROFESSIONAL SEAL 037025 ENGINEER RONALD H. MAYS

Sep 13, 2020

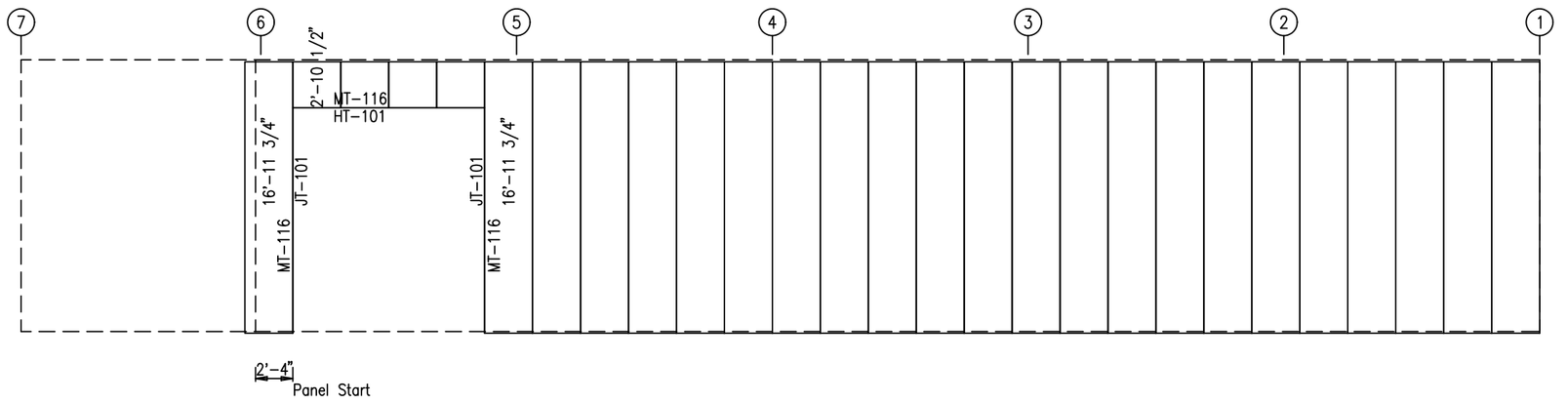



| MEMBER TABLE FRAME LINE B | |
|---|---|
| MARK | PART |
| DJ-3 | 8M35C14 |
| DJ-4 | 8M35C14 |
| DH-3 | 8M35C14 |
| G-13 | 8X25Z16 |
| G-14 | 8X25Z16 |
| G-15 | 8X25Z16 |
| G-16 | 8X25Z16 |
| CB-1 | 0.31_CBL |

| CONNECTION PLATES FRAME LINE B | |
|---|---|
| ☐ID | MARK/PART |
| 1 | AK400 |
| 2 | AK200 |




SIDEWALL FRAMING: FRAME LINE B




SIDEWALL SHEETING & TRIM: FRAME LINE B

PANELS: 26 Ga. SX – Burnished Slate

**Primary structural members are Gray Oxide Primer.**
**Secondary structural members are Pre-Galvanized.**

## GENERAL SHEETING & TRIM NOTES

1. Refer to erection drawings for rake angle locations.
2. Roof member screws are at 12" o.c. Eave end lap and peak screws are as shown.
3. Wall member screws are at 6" o.c. at the base member and 12" o.c. at all remaining members.
4. Roof stitch screws are located at each member with two between members (20" max. spacing).
5. Wall stitch screws are located at each member with one between members (20" max. spacing).
6. Skylight stitch screws are at 6" o.c.
7. Start endwall panels at centerline of bldg. unless noted.
8. Gutter, rake, & eave trim lap 2". All other trims lap 1".
9. Field cut or lap panels as required to fit.
10. Field cut panels for all openings.
11. Pop rivet gutter counterflashing to wall panel on 3'-0 centers and caulk all laps.
12. Gutter support strap spacing: Super Span 3'-0, Super Seam 2'-0, Weather Lok-16 2'-6".
13. Corner and/or peak boxes are not furnished with special rake or gutter profiles. Field miter as req'd.
14. Downspout strap spacing: 4" x 4" 8'-0 o.c. maximum, larger downspouts 5'-0 o.c maximum.
15. Hot-rolled or built-up members must be pre-drilled before attaching members screws.
16. Metal shavings must be swept from the roof each day to avoid surface rusting.
17. Windows and louvers must be installed before sheeting the walls.
18. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.

## GENERAL FRAMING NOTES

1. Angles are marked by their length in feet and inches.
2. Field cut or lap angles as required to fit.
3. Flange braces are marked by their length in decimal inches.
4. Outside flange of girt turns down unless noted.
5. Endwall girts and eave struts do not lap.
6. Field cut and self-tap girts at walk doors.
7. Field slot girts for brace rods or cables.
8. Field locate windows and walk doors.
9. Field weld all splices at 14 gauge valley gutters.
10. Field bolt AK400 base clip to endwall columns:
    (2) 5/8" x 1-1/2" A325 bolts if (1) AK400 req'd
    (2) 5/8" x 1-3/4" A325 bolts if (2) AK400 req'd
11. Locate top of roof framed openings flush with the pan of the roof panel.
12. Some field drilling at framed openings may be required. Field drill 9/16" diameter holes.
13. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.
14. Sub-jambs for overhead doors, if required, are not furnished by Olympia.

"SEE DRAWING D1 FOR BUILT-UP SECTION LEGEND"

ERECTOR NOTE: ONLY USE DRAWINGS ISSUED "FOR ERECTION" TO ERECT BUILDING



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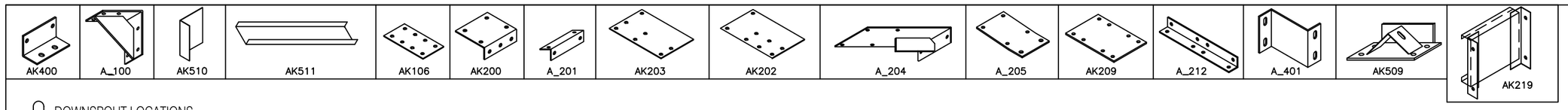
OLYMPIA STEEL BUILDINGS.

| REV | DATE | DESCRIPTION | BY | CHK |
|---|---|---|---|---|
| 0 | 09.09.20 | FOR CONSTRUCTION | MBS | TVN |

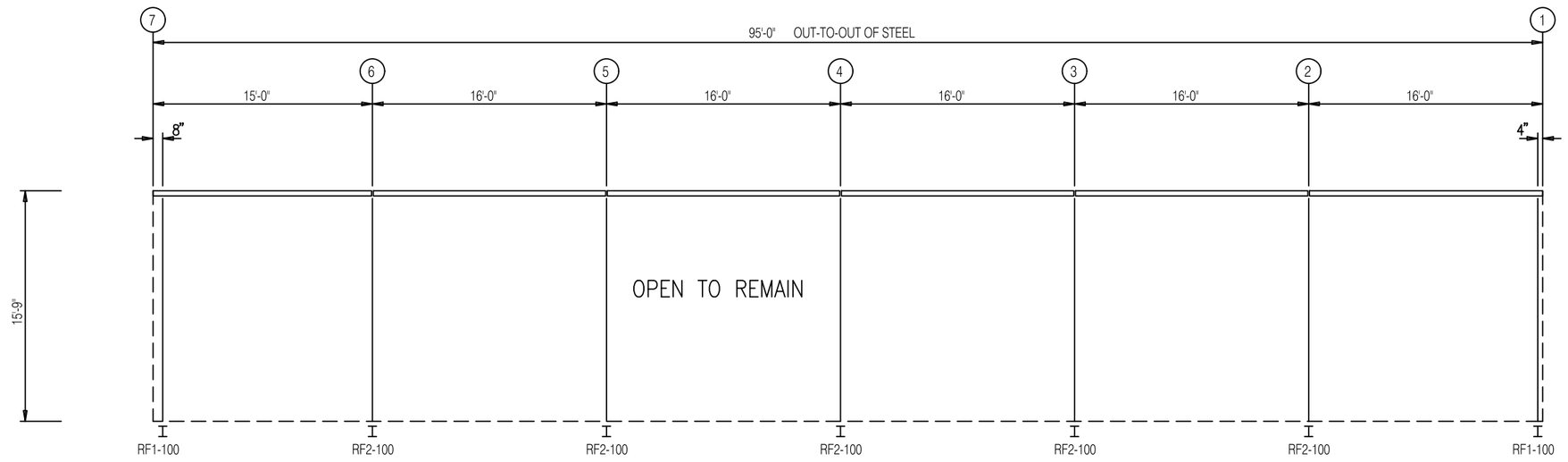
DESC: FRAMING ELEVATION | BLDG. SIZE: VARIES
CUSTOMER: WYNN SITE DEVELOPMENT | LOCATION: Holly Springs, NC 27504
REFERENCE: WYNN SITE DEVELOPMENT
JOBSITE: Holly Springs, NC 27540 | COUNTY: Wake
DWN: MBS | CHK: . | DATE: 9/ 8/20 | ENG: JG | JOB NO: 6707-23738 | DWG NO: E5 | ISSUE 0

NORTH CAROLINA PROFESSIONAL SEAL 037025 ENGINEER RONALD H. MAYS

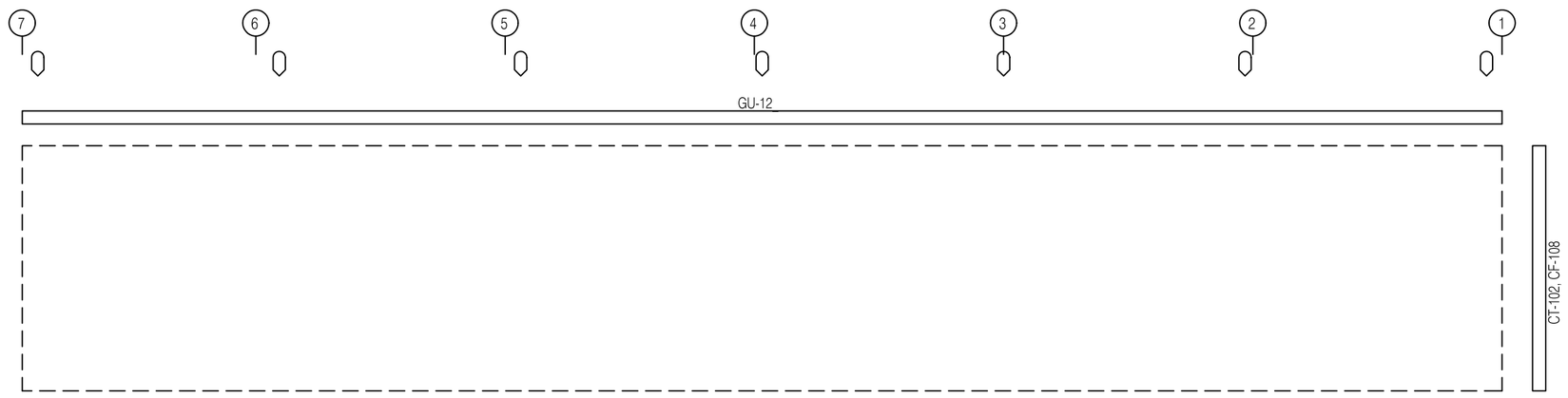
Sep 13, 2020




DOWNSPOUT LOCATIONS




SIDEWALL FRAMING: FRAME LINE A




SIDEWALL SHEETING & TRIM: FRAME LINE A

**Primary structural members are Gray Oxide Primer.**
**Secondary structural members are Pre-Galvanized.**

## GENERAL SHEETING & TRIM NOTES

1. Refer to erection drawings for rake angle locations.
2. Roof member screws are at 12" o.c. Eave end lap and peak screws are as shown.
3. Wall member screws are at 6" o.c. at the base member and 12" o.c. at all remaining members.
4. Roof stitch screws are located at each member with two between members (20" max. spacing).
5. Wall stitch screws are located at each member with one between members (20" max. spacing).
6. Skylight stitch screws are at 6" o.c.
7. Start endwall panels at centerline of bldg. unless noted.
8. Gutter, rake, & eave trim lap 2". All other trims lap 1".
9. Field cut or lap panels as required to fit.
10. Field cut panels for all openings.
11. Pop rivet gutter counterflashing to wall panel on 3'–0 centers and caulk all laps.
12. Gutter support strap spacing: Super Span 3'–0, Super Seam 2'–0, Weather Lok–16 2'–8".
13. Corner and/or peak boxes are not furnished with special rake or gutter profiles. Field miter as req'd.
14. Downspout strap spacing: 4" x 4" 8'–0 o.c. maximum, larger downspouts 5'–0 o.c maximum.
15. Hot–rolled or built–up members must be pre–drilled before attaching members screws.
16. Metal shavings must be swept from the roof each day to avoid surface rusting.
17. Windows and louvers must be installed before sheeting the walls.
18. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw–down type roof for additional installation instructions.

## GENERAL FRAMING NOTES

1. Angles are marked by their length in feet and inches.
2. Field cut or lap angles as required to fit.
3. Flange braces are marked by their length in decimal inches.
4. Outside flange of girt turns down unless noted.
5. Endwall girts and eave struts do not lap.
6. Field cut and self–tap girts at walk doors.
7. Field slot girts for brace rods or cables.
8. Field locate windows and walk doors.
9. Field weld all splices at 14 gauge valley gutters.
10. Field bolt AK400 base clip to endwall columns:
    (2) 5/8" x 1–1/2" A325 bolts if (1) AK400 req'd
    (2) 5/8" x 1–3/4" A325 bolts if (2) AK400 req'd
11. Locate top of roof framed openings flush with the pan of the roof panel.
12. Some field drilling at framed openings may be required. Field drill 9/16" diameter holes.
13. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw–down type roof for additional installation instructions.
14. Sub–jambs for overhead doors, if required, are not furnished by Olympia.

"SEE DRAWING D1 FOR BUILT–UP SECTION LEGEND"

ERECTOR NOTE: ONLY USE DRAWINGS ISSUED "FOR ERECTION" TO ERECT BUILDING



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OLYMPIA STEEL BUILDINGS.

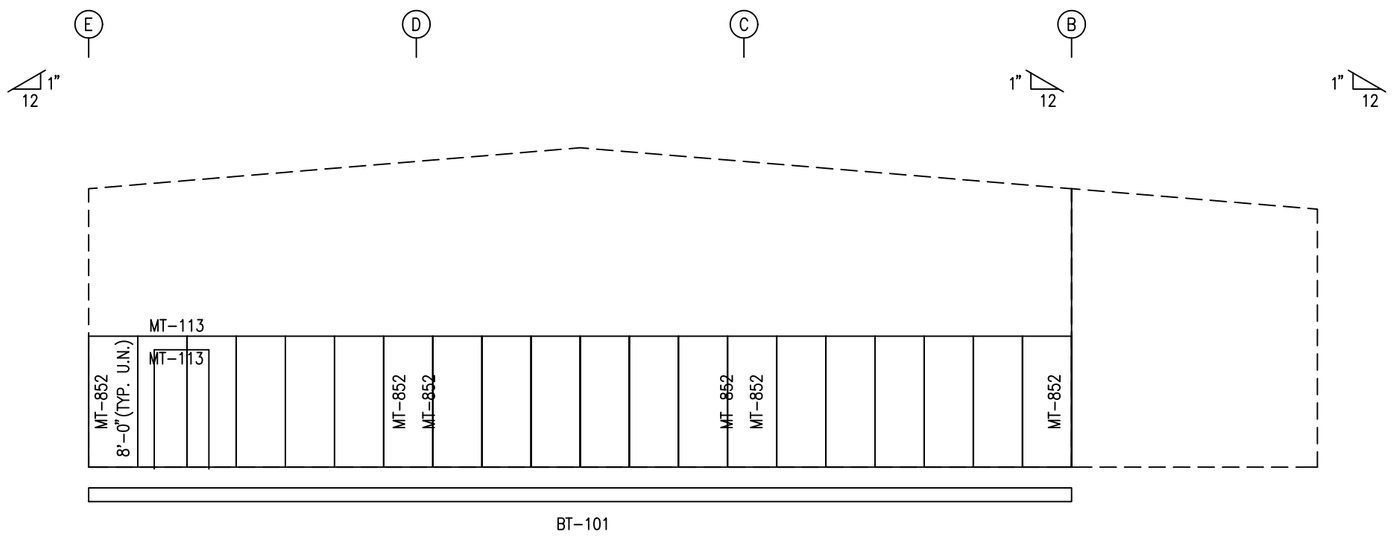
| REV | DATE | DESCRIPTION | BY | CHK |
|---|---|---|---|---|
| 0 | 09.09.20 | FOR CONSTRUCTION | MBS | TVN |

DESC: FRAMING ELEVATION | BLDG. SIZE: VARIES
CUSTOMER: WYNN SITE DEVELOPMENT | LOCATION: Holly Springs, NC 27504
REFERENCE: WYNN SITE DEVELOPMENT
JOBSITE: Holly Springs, NC 27540 | COUNTY: Wake

| DWN: | CHK: | DATE: | ENG: | JOB NO: | DWG NO: | ISSUE |
|---|---|---|---|---|---|---|
| MBS | . | 9/ 8/20 | JG | 6707–23738 | E6 | 0 |

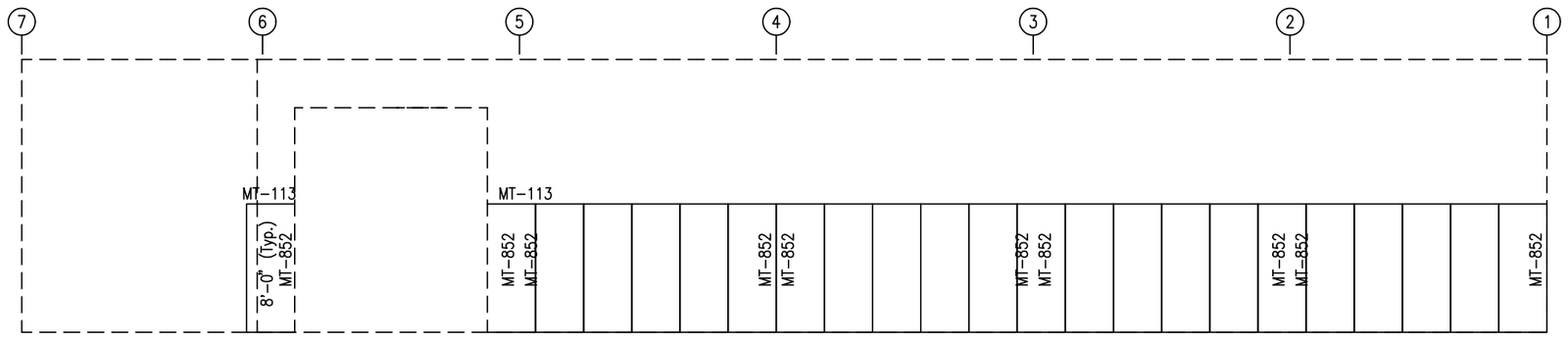
NORTH CAROLINA PROFESSIONAL SEAL 037025 ENGINEER RONALD H. MAYS

Sep 13, 2020



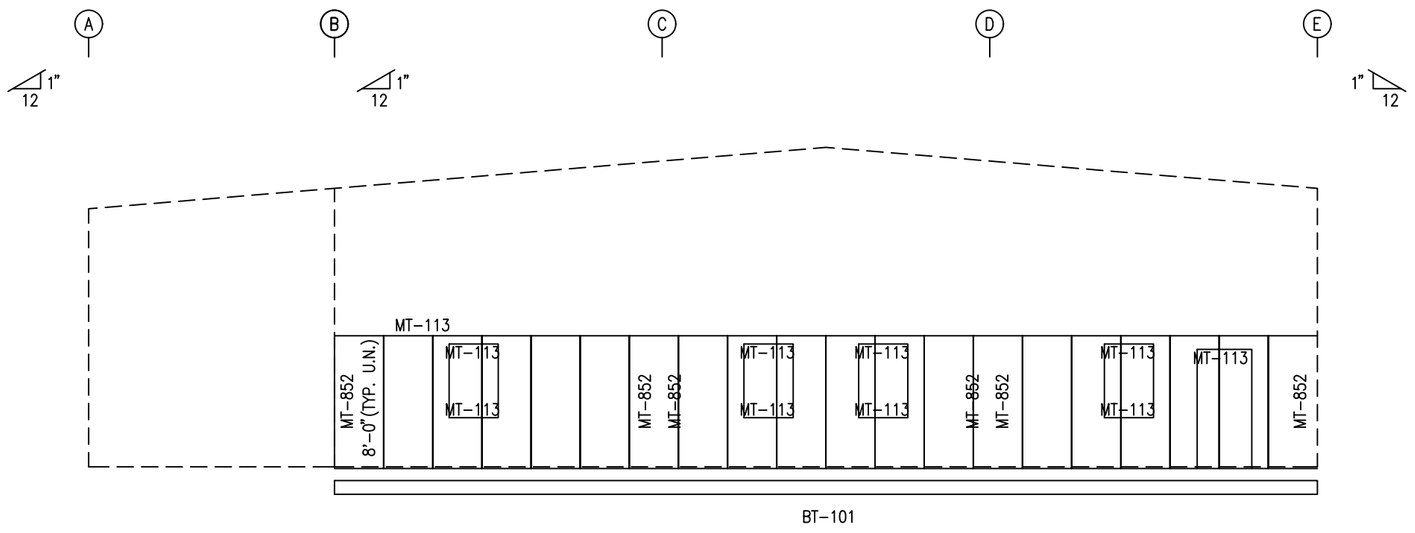

ENDWALL LINER SHEETING & TRIM: FRAME LINE 6

PANELS: 26 Ga. SSX – Polar White



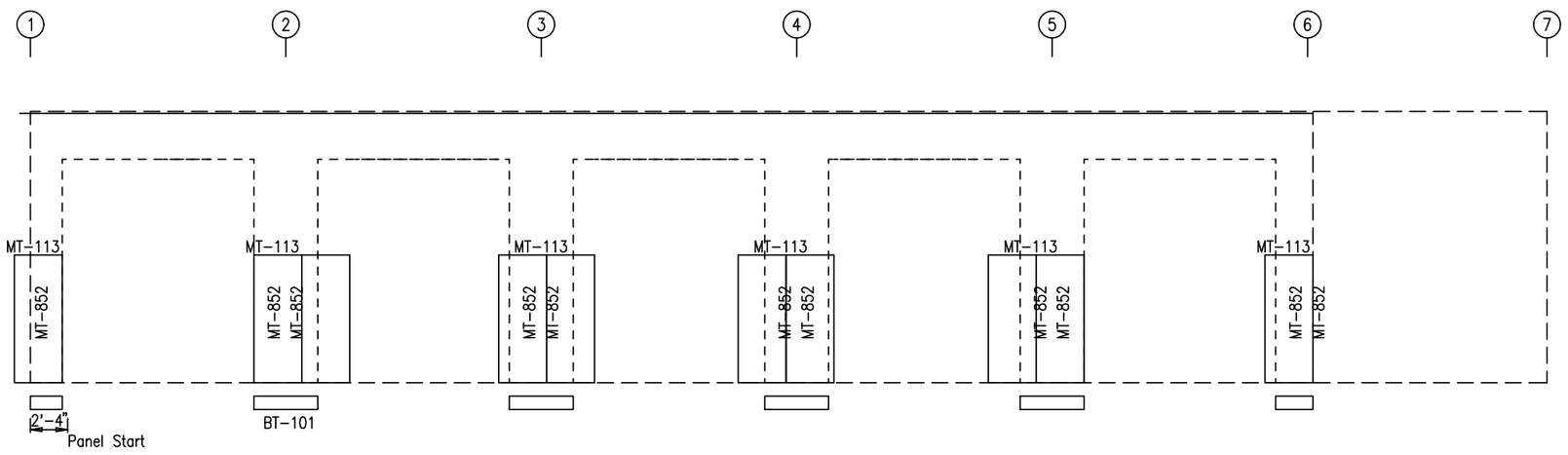

SIDEWALL LINER SHEETING & TRIM: FRAME LINE B

PANELS: 26 Ga. SX – Polar White



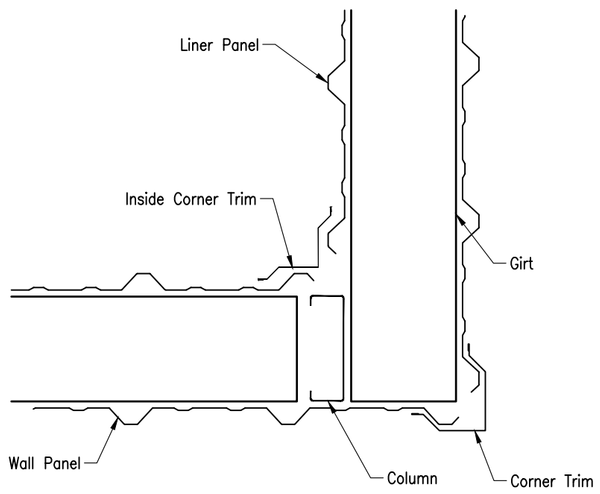

ENDWALL LINER SHEETING & TRIM: FRAME LINE 1

PANELS: 26 Ga. SSX – Polar White

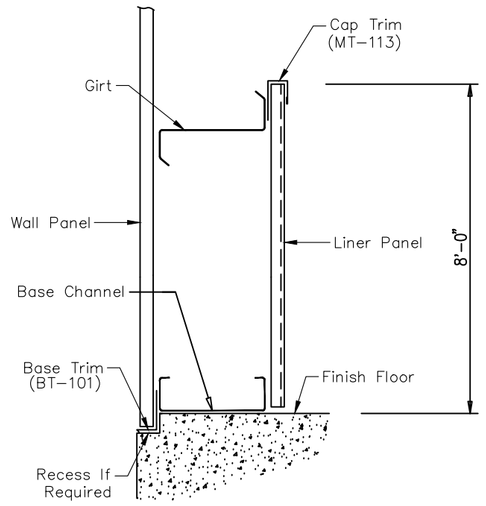



SIDEWALL LINER SHEETING & TRIM: FRAME LINE E

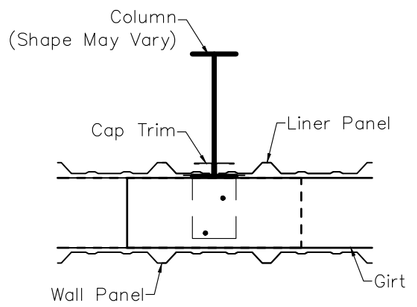
PANELS: 26 Ga. SSX – Polar White



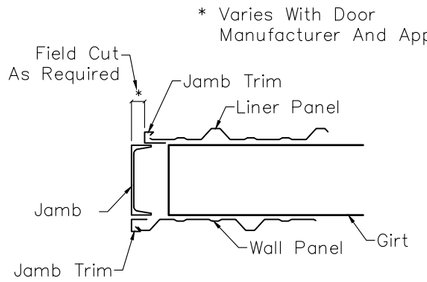

LINER DETAIL AT CORNER




LINER DETAIL AT BASE




LINER DETAIL AT BYPASS COLUMN




DETAIL AT JAMB

ERECTOR NOTE: ONLY USE DRAWINGS ISSUED "FOR ERECTION" TO ERECT BUILDING

The Engineer whose seal and signature appear on these documents represent Whirlwind Steel Buildings, Inc., and is not the Engineer of Record for the overall project. The Engineer's responsibility is limited to material designed and manufactured by Whirlwind Steel Buildings, Inc., and excludes parts such as doors, windows, foundation design, and erection of the building.

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OLYMPIA STEEL BUILDINGS.

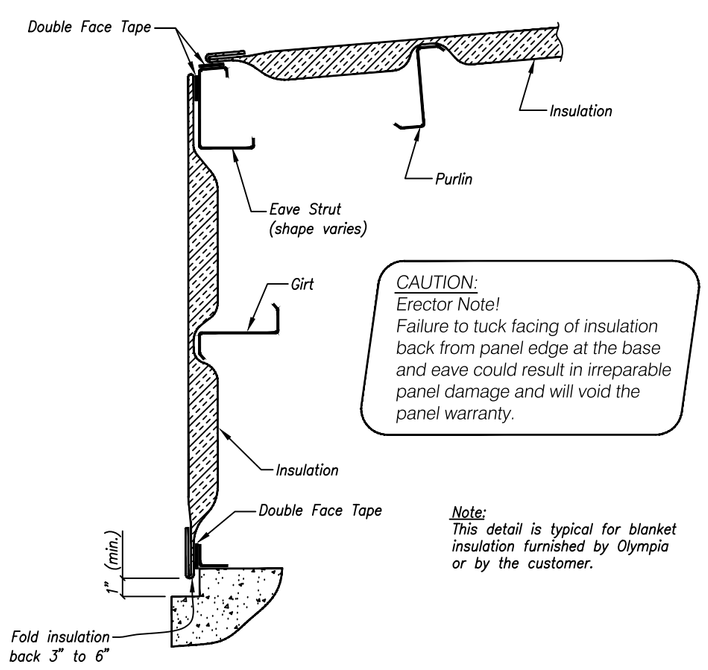
| REV | DATE | DESCRIPTION | BY | CHK |
|---|---|---|---|---|
| 0 | 09.09.20 | FOR CONSTRUCTION | MBS | TVN |

DESC: WALLS LINER
BLDG. SIZE: VARIES
CUSTOMER: WYNN SITE DEVELOPMENT
LOCATION: Holly Springs, NC 27504
REFERENCE: WYNN SITE DEVELOPMENT
JOBSITE: Holly Springs, NC 27540
COUNTY: Wake

| DWN: | CHK: | DATE: | ENG: | JOB NO: | DWG NO: | ISSUE |
|---|---|---|---|---|---|---|
| MBS | . | 9/ 8/20 | JG | 6707-23738 | E7 | 0 |

NORTH CAROLINA PROFESSIONAL SEAL 037025 ENGINEER RONALD H. MAYS

Sep 13, 2020




CAUTION:
Erector Note!
Failure to tuck facing of insulation back from panel edge at the base and eave could result in irreparable panel damage and will void the panel warranty.

Note:
This detail is typical for blanket insulation furnished by Olympia or by the customer.

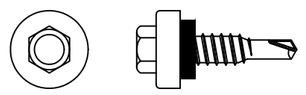
STANDARD DETAIL @ BLANKET INSULATION

Description: ¼–14 x ⅞ Hex Head Undercut #1 Point Self–Drilling Lap Long–Life Zinc Die Cast Head (#14 x ⅞ Long–Life Lap–Tek S.D.S.)

Seating Torque: 30 to 60 in–lbs

Recommended Driving Tool: 1800 RPM electric screw gun with depth sensing nosepiece to prevent overdriving and stripout

Suggested Pre–Drill: None



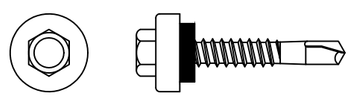
Actual Size

Description: 12–14 x 1¼ Hex Head Undercut #3 Long Pilot Point Self–Drilling Long–Life Zinc Die Cast Head (#12 x 1¼ Long–Life S.D.S.)

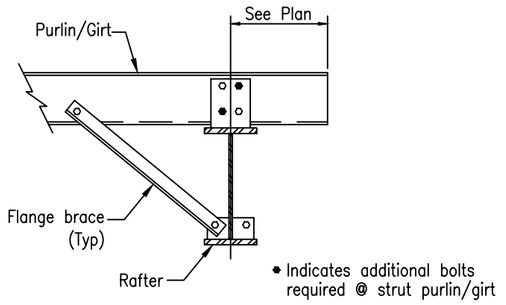
Seating Torque: 30 to 60 in–lbs

Recommended Driving Tool: 1800 RPM electric screw gun with depth sensing nosepiece to prevent overdriving and stripout

Suggested Pre–Drill: None



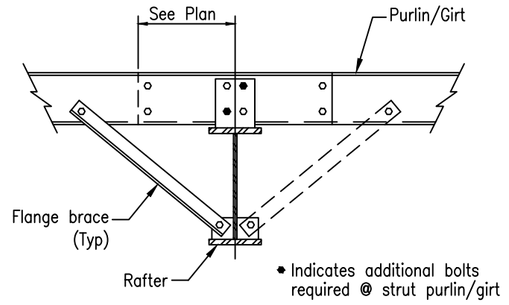
Actual Size




(2) ½"ø A–307 machine bolts required at standard laps.
(4) ½"ø A–307 machine bolts required at strut members.
(A strut member is a purlin or girt at the bracing points.
The struts continue across the length of the building.)
See E–drawing for exceptions to size and quantity of bolts.

END DETAIL

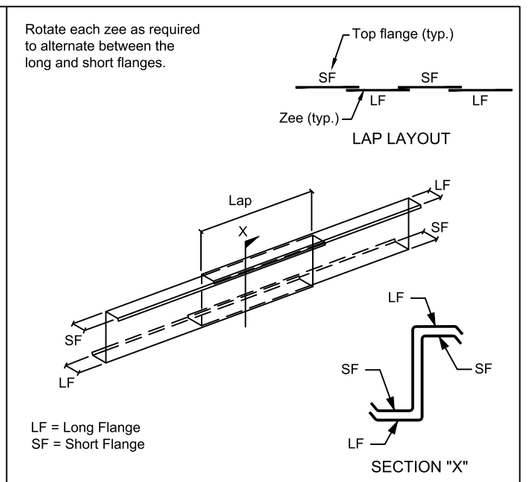
Note: Flange braces may be on one or both sides.
See frame cross–section for locations.



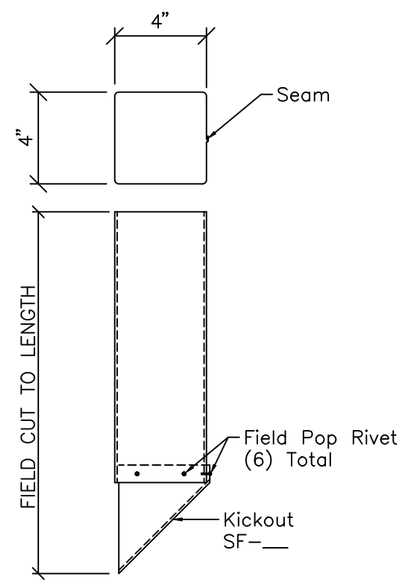

(6) ½"ø A–307 machine bolts required at standard laps.
(8) ½"ø A–307 machine bolts required at strut members.
(A strut member is a purlin or girt at the bracing points.
The struts continue across the length of the building.)
See E–drawing for exceptions to size and quantity of bolts.

LAP DETAIL

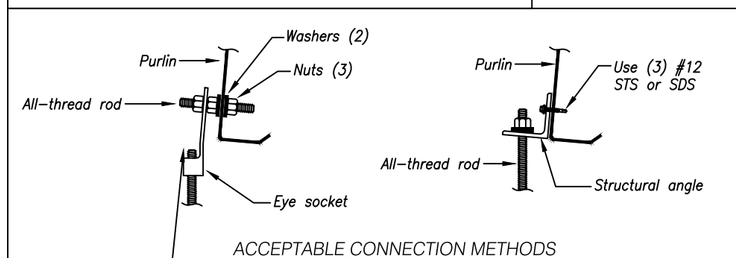
Rotate each zee as required to alternate between the long and short flanges.



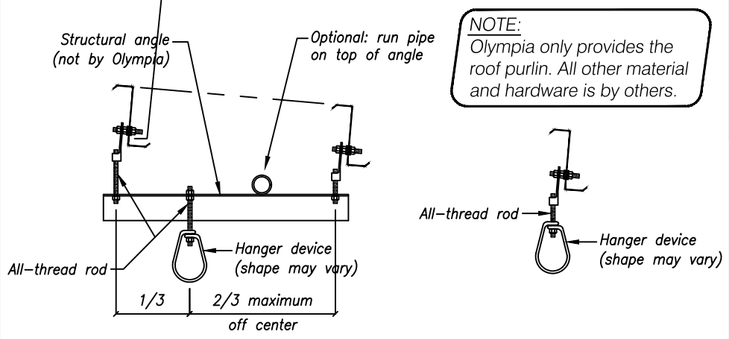

ZEE LAP AT LONG AND SHORT FLANGES




DOWNSPOUT w/SCUPPER
Field Install Scupper If Req'd



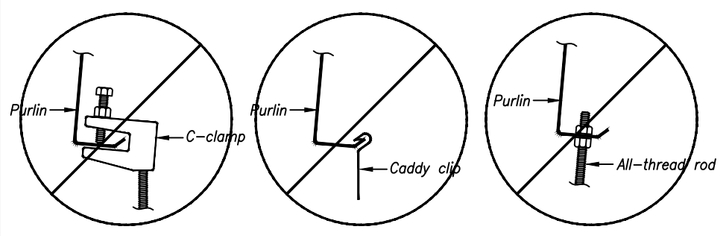

ACCEPTABLE CONNECTION METHODS




NOTE:
Olympia only provides the roof purlin. All other material and hardware is by others.

TRAPEZE HANGER ~ 4" MAX. PIPE

HANGER ~ 2-1/2" MAX. PIPE



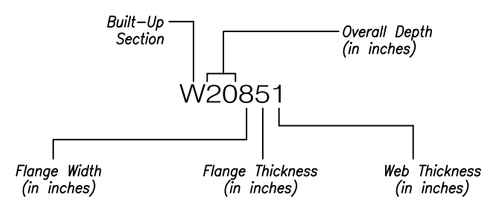

Flange C-Clamp is not an acceptable connection

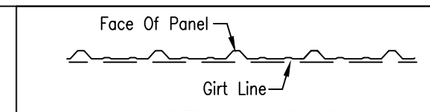
Caddy clip attached to lip is not an acceptable connection

Connection through the flange is not acceptable

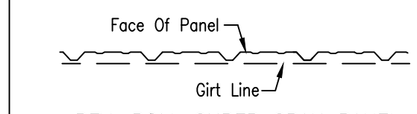
ACCEPTABLE CONNECTIONS FOR ALL COLLATERAL LOADS FOR HANGER ATTACHMENT

BUILT-UP SECTION LEGEND

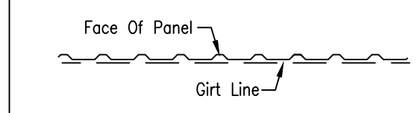


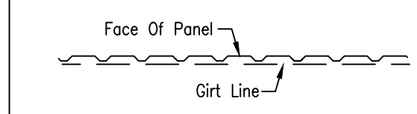

SUPER SPAN PANEL




REV. ROLL SUPER SPAN PANEL




LOW RIB PANEL




REV. ROLL LOW RIB PANEL

PANEL LAYOUT DETAIL
See E1 For Panel Profile Specified For Project

ERECTOR NOTE: ONLY USE DRAWINGS ISSUED "FOR ERECTION" TO ERECT BUILDING

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OLYMPIA STEEL BUILDINGS.

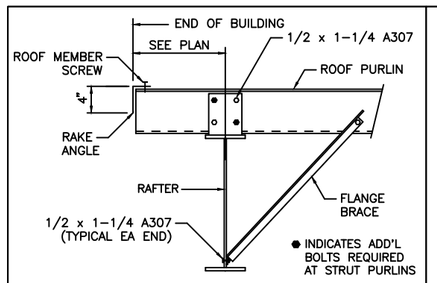
| REV | DATE | DESCRIPTION | BY | CHK |
|---|---|---|---|---|
| 0 | 09.09.20 | FOR CONSTRUCTION | MBS | TVN |

DESC: STANDARD DETAIL PAGE
BLDG. SIZE: VARIES
CUSTOMER: WYNN SITE DEVELOPMENT
LOCATION: Holly Springs, NC 27504
REFERENCE: WYNN SITE DEVELOPMENT
JOBSITE: Holly Springs, NC 27540
COUNTY: Wake

| DWN: | CHK: | DATE: | ENG: | JOB NO: | DWG NO: | ISSUE |
|---|---|---|---|---|---|---|
| MBS | . | 9/ 8/20 | JG | 6707–23738 | D1 | 0 |

NORTH CAROLINA PROFESSIONAL SEAL 037025 ENGINEER RONALD H. MAYS

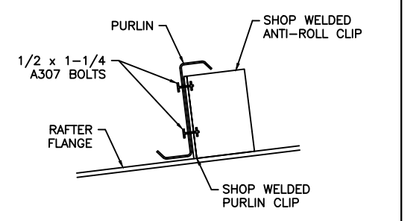
Sep 13, 2020




(2) 1/2 x 1-1/4 A307 REQUIRED AT STANDARD LAPS
(4) 1/2 x 1-1/4 A307 REQUIRED AT STRUT MEMBERS

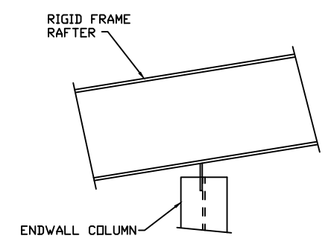
A STRUT MEMBER IS A PURLIN LOCATED AT THE BRACE POINTS. SEE PLANS FOR EXCEPTION TO SIZE & QTY OF BOLTS.

A10 ROOF PURLIN CONNECTION AT MAIN FRAME ENDWALL

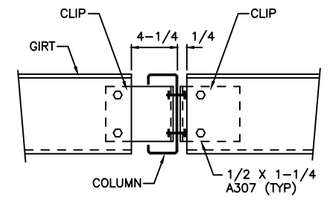



SOME PURLIN CLIPS WILL HAVE AN ADDITIONAL ANTI-ROLL CLIP ATTACHED. THE QUANTITY AND SPACING OF THESE CLIPS IS DETERMINED BY THE DESIGN FOR EACH SPECIFIC BUILDING.

ANTI DETAIL AT ANTI-ROLL CLIP

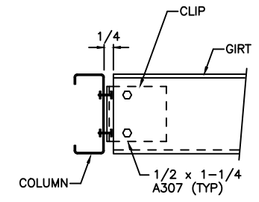



B19 COLUMN TO RIGID FRAME RAFTER



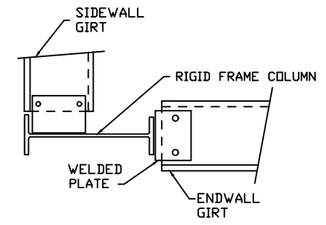

- CLIP LEGS TURN DOWN
- SEE ERECTION PLANS FOR CLIP PART MARKS

C1 GIRT TO COLUMN

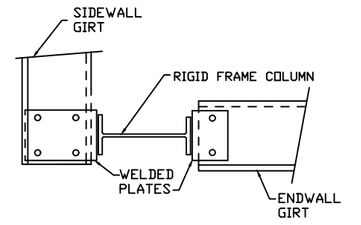



- CLIP LEGS TURN DOWN
- SEE ERECTION PLANS FOR CLIP PART MARKS

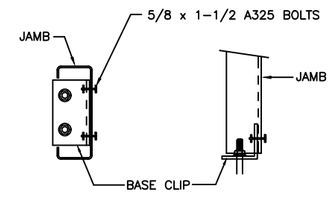
C34 GIRT/HEADER TO CEE COLUMN




D13 CORNER COLUMN TO WALL GIRT

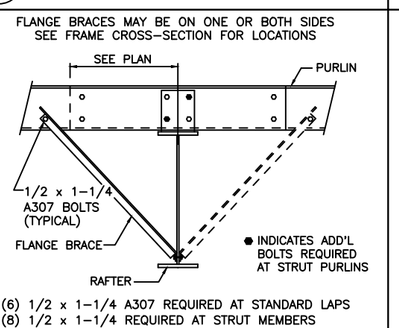



D15 CORNER COLUMN TO WALL GIRT




- ALL ANCHOR RODS (BY OTHERS) TO HAVE NUTS AND FLAT WASHERS.
- SEE ERECTION PLANS FOR CLIP PART MARKS.

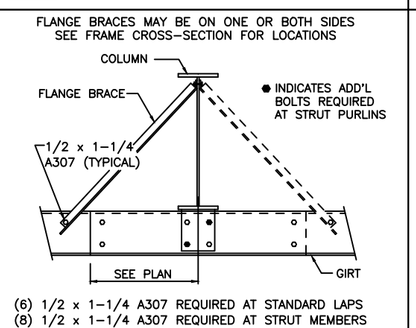
E6 BASE PLATE FOR DOOR JAMB




(6) 1/2 x 1-1/4 A307 REQUIRED AT STANDARD LAPS
(8) 1/2 x 1-1/4 REQUIRED AT STRUT MEMBERS

A STRUT MEMBER IS A PURLIN LOCATED AT THE BRACE POINTS. SEE PLANS FOR EXCEPTION TO SIZE & QTY OF BOLTS.

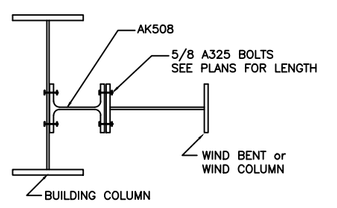
G2 ROOF PURLIN TO INTERIOR FRAME RAFTER



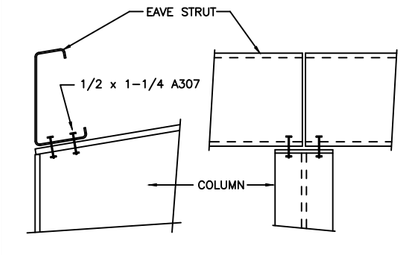

(6) 1/2 x 1-1/4 A307 REQUIRED AT STANDARD LAPS
(8) 1/2 x 1-1/4 A307 REQUIRED AT STRUT MEMBERS

A STRUT MEMBER IS A PURLIN LOCATED AT THE BRACE POINTS. SEE PLANS FOR EXCEPTION TO SIZE & QTY OF BOLTS.

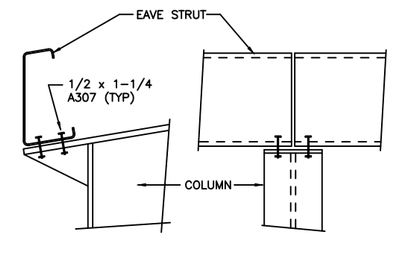
H2 WALL GIRT TO FRAME COLUMN



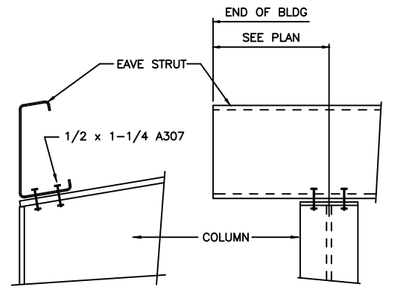

H10 WIND BENT OR WIND COLUMN TO BUILDING COLUMN



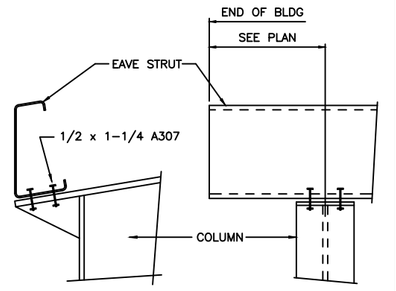

J1 EAVE STRUT TO RIGID FRAME



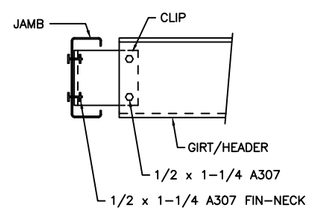

J2 EAVE STRUT TO RIGID FRAME




J23 EAVE STRUT TO RIGID FRAME

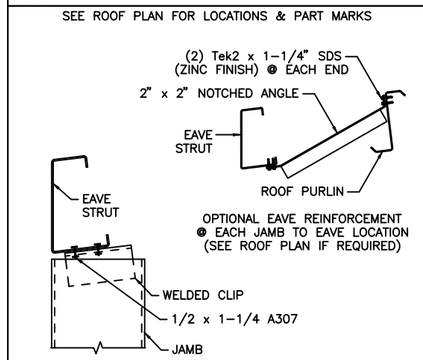



J24 EAVE STRUT TO RIGID FRAME

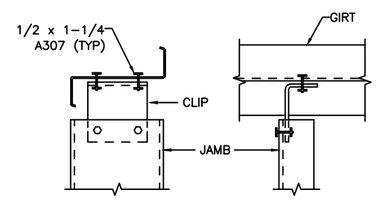



- CLIP LEG TURN DOWN
- SEE ERECTION PLANS FOR CLIP PART MARKS

K2 WALL GIRT TO DOOR JAMB

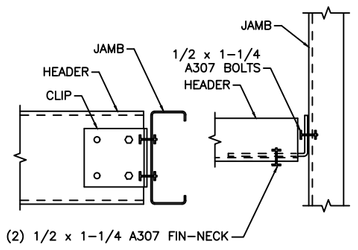



L3 DOOR JAMB TO EAVE STRUT (OPTIONAL REINFORCEMENT SHOWN)



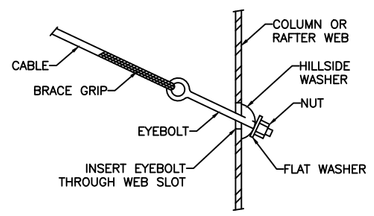

SEE ERECTION PLANS FOR CLIP PART MARKS (TYP)

L6 DOOR JAMB TO WALL GIRT

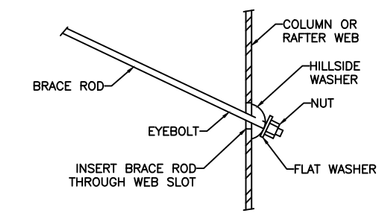



SEE ERECTION PLANS FOR CLIP PART MARKS (TYP)

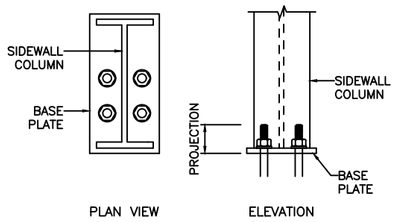
M1 HEADER TO CEE JAMB




Q2 DIAGONAL CABLE INSTALLATION

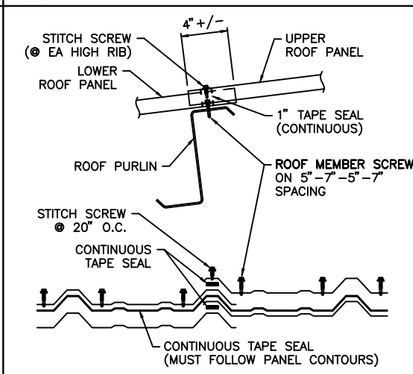



Q3 DIAGONAL BRACE ROD INSTALLATION

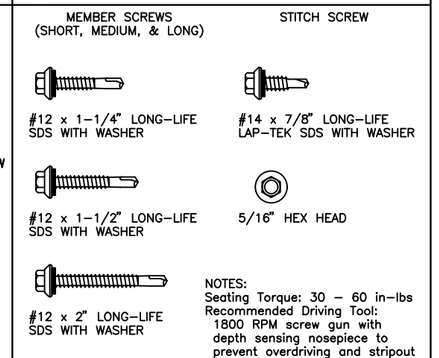



- ALL ANCHOR RODS (BY OTHERS) TO HAVE NUTS AND FLAT WASHERS.
- SEE BOLT SETTING PLAN FOR ACTUAL BOLT QTY.

R2 ANCHOR BOLTS AT SIDEWALL COLUMN

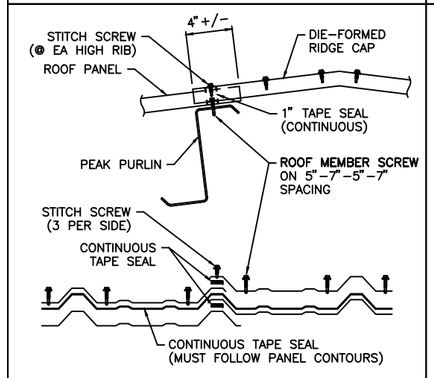



SCREW_2 TYPICAL ROOF PANEL ENDLAP SUPER SPAN X ROOF

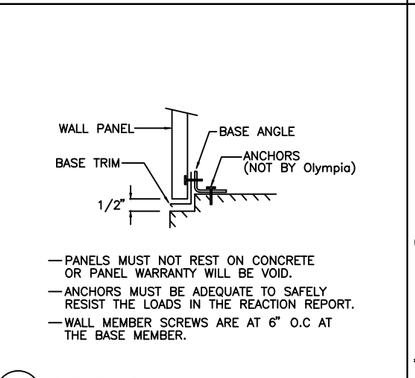



NOTES:
Seating Torque: 30 – 60 in-lbs
Recommended Driving Tool:
1800 RPM screw gun with depth sensing nosepiece to prevent overdriving and stripout

SCREW_4 STANDARD FASTENERS LONG-LIFE SELF-DRILLING

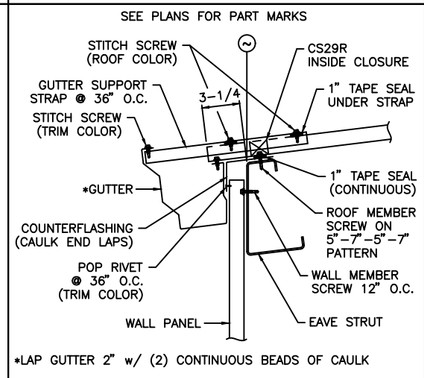



SCREW_8 TYPICAL DIE-FORMED RIDGECAP ENDLAP – SUPER SPAN X ROOF



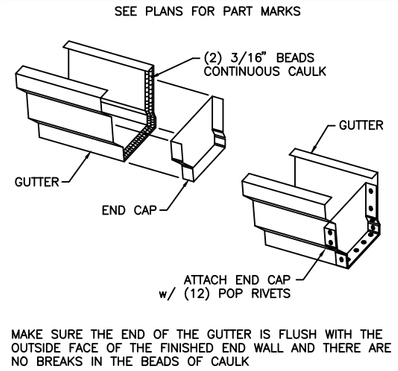

- PANELS MUST NOT REST ON CONCRETE OR PANEL WARRANTY WILL BE VOID.
- ANCHORS MUST BE ADEQUATE TO SAFELY RESIST THE LOADS IN THE REACTION REPORT.
- WALL MEMBER SCREWS ARE AT 6" O.C AT THE BASE MEMBER.

T1 SECTION THRU WALL PANEL AND CONCRETE FOUNDATION



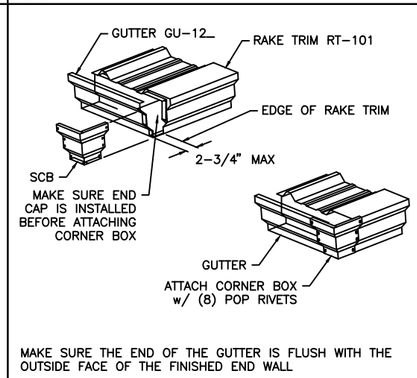

*LAP GUTTER 2" w/ (2) CONTINUOUS BEADS OF CAULK

TRIM_2 TYPICAL GUTTER DETAIL SUPER SPAN X ROOF & WALLS



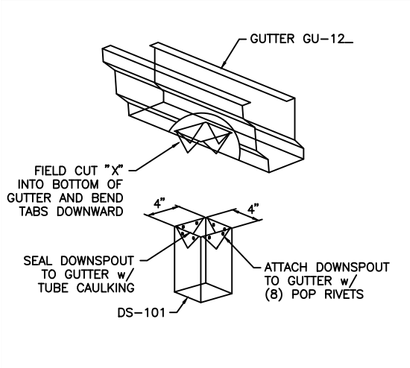

MAKE SURE THE END OF THE GUTTER IS FLUSH WITH THE OUTSIDE FACE OF THE FINISHED END WALL AND THERE ARE NO BREAKS IN THE BEADS OF CAULK

TRIM_4 GUTTER END CAP ATTACHMENT

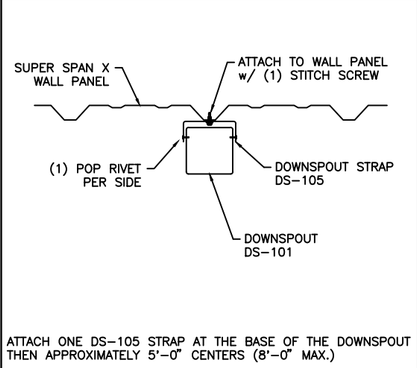



MAKE SURE THE END OF THE GUTTER IS FLUSH WITH THE OUTSIDE FACE OF THE FINISHED END WALL

TRIM_5 CORNER BOX ATTACHMENT

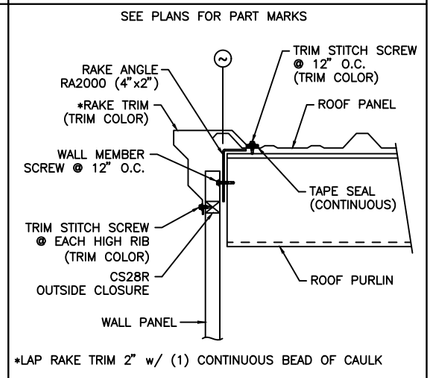



TRIM_6 DOWNSPOUT ATTACHMENT @ GUTTER



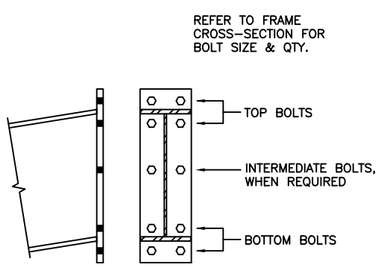

ATTACH ONE DS-105 STRAP AT THE BASE OF THE DOWNSPOUT THEN APPROXIMATELY 5'-0" CENTERS (8'-0" MAX.)

TRIM_7 DOWNSPOUT STRAP ATTACHMENT

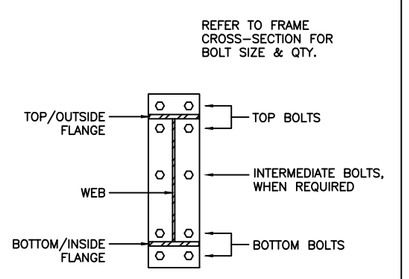



*LAP RAKE TRIM 2" w/ (1) CONTINUOUS BEAD OF CAULK

TRIM_18 TYPICAL RAKE DETAIL SUPER SPAN X ROOF & WALLS




U2 BOLTED END PLATE CONNECTION AT BUILDING PEAK




U3 BOLTS FOR RAFTER TO COLUMN CONNECTION

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OLYMPIA STEEL BUILDINGS.

| REV | DATE | DESCRIPTION | BY | CHK |
|---|---|---|---|---|
| 0 | 09.09.20 | FOR CONSTRUCTION | MBS | TVN |

DESC: STANDARD DETAIL PAGE | BLDG. SIZE: VARIES
CUSTOMER: WYNN SITE DEVELOPMENT | LOCATION: Holly Springs, NC 27504
REFERENCE: WYNN SITE DEVELOPMENT
JOBSITE: Holly Springs, NC 27540 | COUNTY: Wake

| DWN: | CHK: | DATE: | ENG: | JOB NO: | DWG NO: | ISSUE |
|---|---|---|---|---|---|---|
| MBS | . | 9/ 8/20 | JG | 6707-23738 | D2 | 0 |

NORTH CAROLINA PROFESSIONAL SEAL 037025 ENGINEER RONALD H. MAYS

Sep 13, 2020

## STRUCTURAL ABBREVIATIONS

| | |
|---|---|
| A.B. | ANCHOR BOLT |
| ABV. | ABOVE |
| ADD'L | ADDITIONAL |
| ADJ. | ADJACENT |
| ALUM | ALUMINUM |
| APA | AMERICAN PLYWOOD ASSOCIATION |
| ALT. | ALTERNATE |
| APPRX. | APPROXIMATELY |
| ARCH. | ARCHITECTURAL |
| & | AND |
| @ | AT |
| BEL. | BELOW |
| B.F. | BRACED FRAME |
| BLDG. | BUILDING |
| BLK. | BLOCK |
| BLKG. | BLOCKING |
| BM | BEAM |
| B.N. | BOUNDARY NAILING |
| BNDRY. | BOUNDARY |
| B.O.F. | BOTTOM OF FOOTING |
| BRDG. | BRIDGE(ING) |
| BRG. | BEARING |
| BOTT./(B) | BOTTOM |
| BTWN. | BETWEEN |
| B/S | BOTH SIDES |
| CAMB./(C) | CAMBER(ED) |
| CE | CARBON EQUIVALENT |
| CANT. | CANTILEVER(ED) |
| C.F. | CUBIC FEET/(FOOT) |
| C.I.P. | CAST IN PLACE |
| CL. | CENTER LINE |
| CLG. | CEILING |
| CLR. | CLEAR |
| COL. | COLUMN |
| CONC. | CONCRETE |
| CONN. | CONNECTION |
| CONST. | CONSTRUCTION |
| CONT. | CONTINUOUS |
| CTSK. | COUNTERSINK |
| CTR. | CENTER(ED) |
| C.Y. | CUBIC YARD |
| d | PENNY(NAILS) |
| DBL. | DOUBLE |
| DEPT. | DEPARTMENT |
| D.F. | DOUGLAS FIR |
| DIA. | DIAMETER |
| DIAPH. | DIAPHRAGM |
| DIM. | DIMENSION |
| DN. | DOWN |
| do | DITTO(REPEAT) |
| DP. | DEEP |
| DWG. | DRAWING(S) |
| DWL. | DOWEL(S) |
| EA. | EACH |
| E.F. | EACH FACE |
| E.J. | EXPANSION JOINT |
| EL. | ELEVATION |
| ELEC. | ELECTRICAL |
| ELEV. | ELEVATION |
| EMBD. | EMBED(MENT) |
| E.N. | EDGE NAIL |
| ENG. | ENGINEER |
| EQ. | EQUAL |
| EQPT. | EQUIPMENT |
| EXP. | EXPANSION |
| FAB. | FABRICATION |
| FDN. | FOUNDATION |
| FIN. | FINISHED |
| FLG. | FLANGE |
| FLR. | FLOOR |
| F.N. | FIELD (FACE NAIL) |
| F.O.C | FACE OF CONCRETE |
| F.O.M. | FACE OF MASONRY |
| F.O.S. | FACE OF STUD |
| F.O.W. | FACE OF WALL |
| FRM. | FRAME(ING) |
| F.S. | FAR SIDE |
| FT./(') | FOOT/(FEET) |
| FTG. | FOOTING |
| GA. | GAUGE |
| GALV. | GALVANIZE(D) |
| GB | GRADE BEAM |
| GLB | GLUED LAMINATED BEAM |
| GRD. | GRADE |
| GYPBD | GYPSUM WALLBOARD |
| HD | HOLD DOWN |
| HRD. | HEADER |
| HGR. | HANGER |
| HORIZ./(H) | HORIZONTAL |
| HSB | HIGH STRENGTH BOLTS |
| HT. | HEIGHT |
| I.D. | INSIDE DIAMETER |
| I.E. | INVERT ELEVATION |
| IN./(") | INCH(ES) |
| INT. | INTERIOR |
| J&T. | JOIST |
| JT. | JOINT |
| K | KIPS/(000) |
| LAT. | LATERAL |
| LB/# | POUND(S) |
| L.B. | LAG BOLTS |
| L.F. | LINEAR FEET/FOOT) |
| LGTH. | LENGTH |
| LLH | LONG LEG HORIZ. |
| LG. | LONG(ITUDINAL) |
| LLV | LONG LEG VERT. |
| LT.WT. | LIGHT WEIGHT |
| MAS. | MASONRY |
| MAT'L | MATERIAL |
| MAX. | MAXIMUM |
| M.B. | MACHINE BOLT |
| MECH. | MECHANICAL |
| MEZZ. | MEZZANINE |
| MFR. | MANUFACTURER |
| MISC. | MISCELLANEOUS |
| MIN. | MINIMUM |
| MTL. | METAL |
| NO./(#) | NUMBER |
| N.S. | NEAR SIDE |
| N.T.S. | NOT TO SCALE |
| O.C. | ON CENTER |
| O.D. | OUTSIDE DIAMETER |
| PAR | PARALLEL |
| P/C | PRECAST |
| PERP. | PERPENDICULAR |
| PL | PLATE |
| PLY. | PLYWOOD |
| P.S.F. | POUNDS PER SQUARE FOOT |
| P.S.I. | POUNDS PER SQUARE INCH |
| P.S.L. | PARALLAM BEAM |
| P.T. | PRESSURE TREATED |
| P.T.D.F. | PRESSURE TREATED DOUGLAS FIR |
| QTY. | QUANTITY |
| RAD/(R) | RADIUS |
| R.C.P. | REINFORCED CONCRETE PIPE |
| REF. | REFERENCE |
| R.F. | RIGID FRAME |
| REINF. | REINFORCEMENT/(ING) |
| REQ'D | REQUIRED |
| R.O. | ROUGH OPENING |
| SCH. | SCHEDULE |
| SHT. | SHEET |
| SIM. | SIMILAR |
| SKN. | SKEW(ED) |
| SPC. | SPACE/(ING) |
| SPEC. | SPECIFICATION(S) |
| SP.INSP. | SPECIAL INSPECTION |
| SQ. | SQUARE |
| STD. | STANDARD |
| STGR. | STAGGER(ED) |
| STIF. | STIFFENER(S) |
| STIR. | STIRRUP(S) |
| STL | STEEL |
| STRUC. | STRUCTURAL |
| SUSP. | SUSPENDED(TION) |
| SYMM. | SYMMETRICAL |
| T&B | TOP AND BOTTOM |
| T&G | TONGUE AND GROOVE |
| TEMP. | TEMPERATURE |
| THK. | THICKNESS |
| THRD. | THREADED |
| TMPRY | TEMPORARY |
| T.N. | TOE NAIL |
| T.O.S. | TOP OF SHEATHING |
| T.O.W. | TOP OF WALL |
| T.S. | TOP OF STEEL |
| TRANSV. | TRANSVERSE |
| TYP. | TYPICAL |
| U.O.N. | UNLESS OTHERWISE NOTED |
| VERT./(V) | VERTICAL |
| (W) | WIDE/(WIDTH) |
| w/ | WITH |
| WD. | WOOD |
| W.P. | WORK POINT |
| WPJ. | WEAKENED PLANE JOINT |
| W.S. | WELDED STUD(S) |
| WT. | WEIGHT |
| WWF | WELDED WIRE FABRIC |
| X-STG | EXTRA STRONG |
| XX-STG | DOUBLE EXTRA STRONG |
| YD. | YARD |

## DESIGN PARAMETERS:

COLUMN REACTIONS PROVIDED BY METAL BUILDING DESIGNER

## GENERAL NOTES:

1. THE CONTRACTOR SHALL VERIFY DIMENSIONS AND SITE CONDITIONS BEFORE STARTING WORK AND THE DESIGNER SHALL BE NOTIFIED IMMEDIATELY OF ANY DISCREPANCIES. IN NO CASE SHALL DIMENSIONS BE SCALED FROM PLANS, SECTIONS, OR DETAILS ON THESE DRAWINGS.
2. ALL OMISSIONS AND CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND OR SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER BEFORE PROCEEDING WITH ANY WORK SO INVOLVED.
3. NO PIPES OR DUCTS SHALL BE PLACED IN WALLS UNLESS SPECIFICALLY SHOWN OR NOTED ON STRUCTURAL DRAWINGS. NO STRUCTURAL MEMBER SHALL BE CUT FOR PIPES, DUCTS, ETC., UNLESS NOTED.
4. THE CONTRACTOR SHALL DETERMINE THE LOCATION OF EXISTING UTILITY SERVICES IN THE AREA TO BE EXCAVATED PRIOR TO BEGINNING OF EXCAVATION.
5. ALL WORKMANSHIP AND MATERIALS SHALL CONFORM TO THE REQUIREMENTS OF THE 2018 EDITION OF THE "NORTH CAROLINA STATE BUILDING CODE".
6. THESE DRAWINGS REPRESENT THE FINISHED STRUCTURE AND DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR IS RESPONSIBLE FOR ALL TEMPORARY BRACING, SHORING AND SUPPORT NECESSARY TO ACHIEVE THE FINISHED STRUCTURE.

## FOUNDATION NOTES:

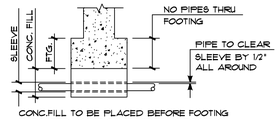
1. MAXIMUM DESIGN SOIL PRESSURE:
   - CONTINUOUS FOOTINGS: 2,000 PSF
   - PAD FOOTINGS: 2,000 PSF
2. SEE SOILS REPORT BY: CONTRACTOR'S GEOTECHNICAL ENGINEER
   - PROJECT NO.: -
   - DATED: -
3. ALL FOOTINGS TO BE A MINIMUM OF: 18" BELOW NATURAL GRADE
   18" BELOW FINISH GRADE
4. SOILS COMPACTION AND SITE PREPARATION TO BE IN ACCORDANCE WITH SOILS REPORT. ALL WORK TO BE DONE UNDER THE DIRECT SUPERVISION OF THE GEOTECHNICAL ENGINEER (AS APPLICABLE).
5. CONTRACTOR/OWNER IS TO CONSULT W/ A GEOTECHNICAL ENGINEER FOR FOUNDATION DESIGN REVIEW AND ADDITIONAL RECOMMENDATIONS IF UNSUITABLE SOILS ARE ENCOUNTERED.
6. FINISH EXCAVATION FOR FOUNDATION SHALL BE NEAT AND TRUE TO LINE WITH LOOSE MATERIAL REMOVED FROM EXCAVATION.
7. THE FOOTING EXCAVATIONS SHALL BE KEPT FREE FROM LOOSE MATERIAL AND STANDING WATER AND, BEFORE ANY FOOTING CONCRETE IS PLACED, SHALL BE CHECKED AND APPROVED BY A QUALIFIED GEOTECHNICAL ENGINEER TO INSURE COMPLIANCE WITH THE REQUIREMENTS.
8. SIDE OF FOUNDATION MAY BE POURED AGAINST STABLE EARTH (U.O.N.)
9. METHOD OF SUPPORTING REINFORCING PIPE SLEEVES MUST BE APPROVED BY THE STRUCTURAL ENGINEER.
10. CONTRACTOR SHALL PROTECT ALL UTILITY LINES, ETC., ENCOUNTERED DURING EXCAVATION AND BACKFILLING.
11. FOOTING BACKFILL AND UTILITY TRENCH BACKFILL WITHIN BUILDING AREA SHALL BE MECHANICALLY COMPACTED IN LAYERS TO THE APPROVAL OF THE GEOTECHNICAL OR STRUCTURAL ENGINEER. FLOODING WILL NOT BE PERMITTED.(U.O.N.)
12. ALL HOLDOWNS SHALL BE TIED IN PLACE PRIOR TO FOUNDATION INSPECTION, AS APPLICABLE.
13. FOR ADDITIONAL DIMENSIONS SEE MANUFACTURER'S DRAWINGS.

## CONCRETE NOTES:

1. CONCRETE IN ALL WORK SHALL HAVE THE FOLLOWING ULTIMATE COMPRESSIVE STRENGTH AT 28 DAYS EXCEPT AS MODIFIED BY THESE NOTES:
   - A. SLAB ON GRADE (NO SP. INSP.) 3,000 PSI
   - B. FOOTINGS 3,000 PSI
   - C. WALLS ____ PSI
   - D. CIP BEAMS & STRUCTURAL SLABS ____ PSI
   - E. COLUMNS ____ PSI
   - F. POST TENSION SLAB ____ PSI
   - G. GRADE BEAMS (GB'S) ____ PSI
   - H. CONCRETE OVER STEEL DECKS ____ PSI
   - I. PRECAST CONCRETE TOPPING SEE NOTES IN APPROPRIATE SECTION.
   - J. TILT-UP CONCRETE SEE NOTES IN APPROPRIATE SECTION.
   - K. CONCRETE TOPPING OVER PLYWOOD:
     - A. STRUCTURAL LIGHT-WEIGHT: 3,000 PSI (SEE NOTE BELOW)
     - B. NORMAL WEIGHT: 3,000 PSI
     - C. CELLULAR: 1,000 PSI (SEE NOTE 2 BELOW)
2. LIGHT WEIGHT CONCRETE SHALL BE AIR ENTRAINED AND HAVE A DENSITY RANGE OF 115 PCF MAXIMUM AND 100 PCF MINIMUM.
3. COMPRESSIVE STRENGTH TEST REPORTS SHOULD BE RETAINED BY THE CONTRACTOR WHEN TESTING IS REQUIRED.
4. CEMENT SHALL CONFORM TO ASTM C-150, TYPE I OR TYPE II.
5. AGGREGATES FOR NORMAL WEIGHT CONCRETE SHALL CONFORM TO ASTM C-33. AGGREGATE FOR LIGHT WEIGHT CONCRETE SHALL CONFORM TO
6. READY MIX CONCRETE SHALL BE MIXED AND DELIVERED IN ACCORDANCE WITH ASTM C-94-81.
7. ADMIXTURES MAY BE USED WITH THE PRIOR APPROVAL OF THE ENGINEER. ADMIXTURE (COMPLYING WITH ASTM A494) USED TO INCREASE THE WORKABILITY OF THE CONCRETE SHALL NOT BE CONSIDERED TO REDUCE THE SPECIFIED MINIMUM CEMENT CONTENT. (CALCIUM CHLORIDE SHALL NOT BE USED).
8. WATER SHALL BE CLEAN, FREE FROM DELETERIOUS AMOUNT OF ACIDS, ALKALIS OR ORGANIC MATERIALS.
9. REFER TO ARCHITECTURAL DRAWINGS (AS APPLICABLE) FOR MOLDS, GROOVES, ORNAMENTS, CLIPS OR GROUNDS REQUIRED TO BE CAST INTO CONCRETE AND FOR EXTENT OF DEPRESSIONS, CURBS, AND RAMPS.
10. DOWELS BETWEEN FOOTINGS AND WALLS SHALL BE THE SAME SIZE, GRADE AND SPACING AS VERTICAL WALL REINFORCING. (U.O.N.).
11. SLUMPS: THE MAXIMUM SLUMP SHALL NOT EXCEED 4" FOR FOOTINGS, SLABS ON EARTH AND MASS CONCRETE AND 5" FOR OTHER CONCRETE. DURING TEMPERATURES ABOVE 80 F, MAXIMUM OF 6" SLUMP IS PERMISSIBLE PROVIDED THE MIX DESIGN IS REVISED ACCORDINGLY BY THE TESTING LABORATORY. MEASURE SLUMP IN ACCORDANCE WITH "METHOD OF TEST FOR SLUMP" OF PORTLAND CEMENT CONCRETE ASTM C143.
12. NO SLEEVES OR CHASES SHALL BE PLACED IN BEAMS, SLABS OR WALLS EXCEPT AS SHOWN ON PLANS. CONTRACTOR SHALL OBTAIN PRIOR APPROVAL FOR INSTALLATION OF ANY ADDITIONAL SLEEVES OR CHASES. NO CONDUIT SHALL BE PLACED IN THE CONCRETE TOPPING OVER STEEL DECKING.
13. SLEEVE PLUMBING OPENINGS IN CONCRETE WALLS AND SLABS BEFORE PLACING CONCRETE. BEND REINFORCEMENT AROUND SLEEVES.
14. PROJECTING CORNERS OF SLABS, BEAMS, WALLS, COLUMNS, ETC. SHALL BE FORMED WITH A ¾" CHAMFER.
15. IN THE EVENT OF AN UNINTENTIONAL COLD JOINT, THE CONTRACTOR SHALL REMOVE ALL LAITANCE AND DELETERIOUS MATERIAL TO PROVIDE A SOUND, CLEAN, ROUGH SURFACE AND USE A BONDING AGENT THAT PRODUCES A HIGHER STRENGTH JOINT THAN THE CONCRETE USED - F'c + 25% MINIMUM.
16. CONCRETE COVER SHALL BE IN ACCORDANCE WITH SECTION 7.7 OF ACI 318.
17. DO NOT DISPLACE REBAR FROM THEIR INTENDED POSITIONS DURING CONCRETING.

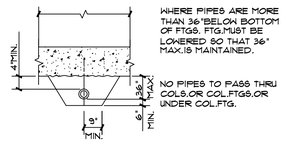
## REINFORCING STEEL NOTES:

1. BAR REINFORCEMENT SHALL BE: GR 40 = #4 & SMALLER
   ASTM A615 GR. 60 = #5 & LARGER
2. THE "CARBON EQUIVALENT" OF ANY REINFORCING TO BE WELDED SHALL BE DETERMINED BY A TESTING LAB. IF THE "CARBON EQUIVALENT" (C.E.) IS LESS THAT 0.55, THEN THE REINFORCING STEEL MAY BE WELDED WITHOUT PREHEATING. IF THE CARBON EQUIVALENT EXCEEDS 0.55± NOTIFY THE STRUCTURAL ENGINEER FOR THE PREHEATING REQUIREMENTS. WELDING WILL NOT BE ALLOWED FOR A CARBON EQUIVALENT ABOVE 0.75. WELDING OF REINFORCING STEEL SHALL BE IN ACCORDANCE WITH UBC STANDARD NO. 19-1. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185 AND SHALL BE LAPPED 12 INCHES MINIMUM.
   E70XX ELECTRODES SHALL BE USED IN WELDING GRADE 40 REBAR.
   E90XX ELECTRODES SHALL BE USED IN WELDING GRADE 60 REBAR.
3. VERTICAL BARS IN WALLS SHALL BE ACCURATELY POSITIONED AT THE CENTER OF WALL (U.O.N.) AND SHALL BE TIED IN POSITION AT TOP AND BOTTOM AND AT INTERVALS PER CHAPTERS 19 AND 21 OF THE 1997 UBC.
4. REINFORCING DETAILING AND PLACING SHALL BE IN ACCORDANCE WITH THE CONCRETE REINFORCING STEEL INSTITUTE "MANUAL OF STANDARD PRACTICE" LATEST EDITION.
5. ALL REINFORCING STEEL, ANCHOR BOLTS, DOWELS, AND INSERTS SHALL BE WELL SECURED IN POSITION PRIOR TO PLACING CONCRETE.
6. REINFORCING STEEL SHALL BE PROVIDED WITH THE FOLLOWING AMOUNTS OF CONCRETE COVER:
   - FOOTINGS (CONC. DEPOSITED AGAINST EARTH).....3"
   - CONC. SURFACE (FORMED) EXPOSED TO EARTH OR WEATHER
     - #6 THROUGH #18 BARS:.....2"
     - #5 & SMALLER:.....1 1/2"
   - CONC. NOT EXPOSED TO EARTH OR WEATHER:
     - SLABS, WALLS & JOIST:
       - #14 & #18 BARS:.....1 1/2"
       - #11 BAR & SMALLER:..... 3/4"
     - BEAMS, COLUMNS :
       - PRIMARY REINFORCEMENT TIES STIRRUPS, SPIRALS:1 1/2"

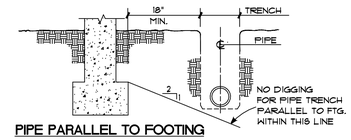



CONC.FILL TO BE PLACED BEFORE FOOTING IS POURED. MAKE SAME WIDTH AS FOOTING AND POUR FULL WIDTH OF PIPE TRENCH

PIPE PERPENDICULAR TO FOOTING



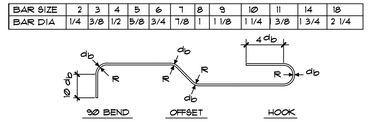

PASSING UNDER FOOTING




PIPE PARALLEL TO FOOTING

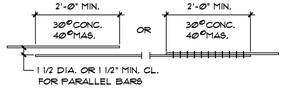
TYPICAL DETAIL OF PIPING AT FOOTINGS AND FOUNDATION WALLS — 1

| BAR SIZE | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 14 | 18 |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| BAR DIA | 1/4 | 3/8 | 1/2 | 5/8 | 3/4 | 7/8 | 1 | 1 1/8 | 1 1/4 | 3/8 | 1 3/4 | 2 1/4 |



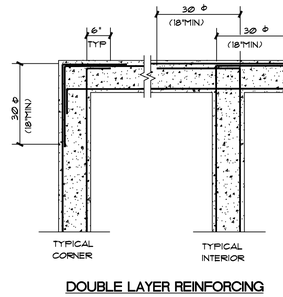

$d_b$ = DIAMETER OF BAR
R = $3d_b$ FOR #2 TO #8
R = $4d_b$ FOR #9 TO #11

TYPICAL BAR BEND

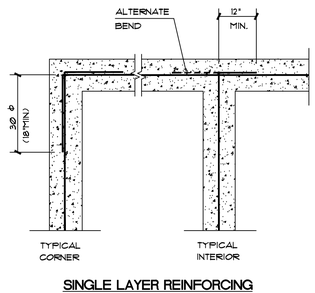



TYPICAL SPLICE

REINF. BAR BEND AND SPLICE DETAIL — 2

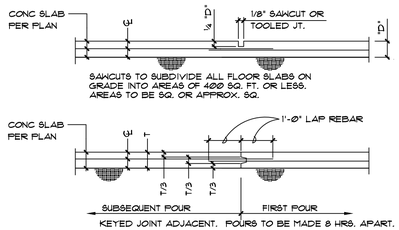



DOUBLE LAYER REINFORCING




SINGLE LAYER REINFORCING

TYP. REINF. AT INTERSECTION OF CONCRETE FOOTINGS — 3




KEYED JOINT ADJACENT. POURS TO BE MADE 8 HRS. APART.

NOTE: SLAB UNDERLAYMENT PREPRATION BY OTHERS

CONSTRUCTION JOINTS

SLAB ON GRADE — 4


STONEWALL
STRUCTURAL ENGINEERING
4800 Falls of Neuse Rd, Suite #120
Raleigh, NC 27609
(919)407-8663
stonewalleng.com Lic. # P-0951


NORTH CAROLINA PROFESSIONAL SEAL 025914 ENGINEER MATTHEW G. BRUSHABER

10-30-20

| REVISIONS | BY |
|---|---|
| | |

ICN Partners LLC
540 Farabow Drive
Holly Springs, NC 27540

DATE 9-9-20
SCALE AS SHOWN
DRAWN A. VALLIN
JOB 20-2049
SHEET SP1
OF SHEETS

# NOTES:

1. ASSUMED SOIL BEARING CAPACITY IS 2000 PSF. CONTRACTOR MUST CONTACT A SOILS ENGINEER IF UNSUITABLE SOILS ARE ENCOUNTERED.
2. ADEQUATE DRAINAGE SHALL BE PROVIDED FOR THE SURFACE AREA ADJACENT TO THE STRUCTURE SUCH THAT WASTER DRAINS AWAY FROM STRUCTURE.
3. CONTRACTOR TO VERIFY ALL DIMENSIONS W/ METAL BUILDING FLOOR PLAN & ANCHOR BOLT LAYOUT PRIOR TO WORK.
4. CONTRACTOR TO COORDINATE BUILDING LOCATION & ORIENTATION W/ OWNER.
5. METAL BUILDING DESIGN BY OTHERS.
6. FOR ADDITIONAL NOTES SEE SHEET SP1.
7. ANCHOR BOLTS SHALL BE GR. 36 INSTALLED PER DETAILS. FOR EXACT ANCHOR BOLT LOCATION SEE ANCHOR BOLT PLAN (BY BLDG MANUFACTURER)
8. SEE ANCHOR BOLT PLAN (BY BLDG MANUFACTURER) FOR SLOPE IN SLAB AT OVERHEAD & ROLL-UP DOOR.

# LEGEND:

INDICATES CONTINUOUS FOOTING PER PLAN

INDICATES PAD FOOTING PER PLAN

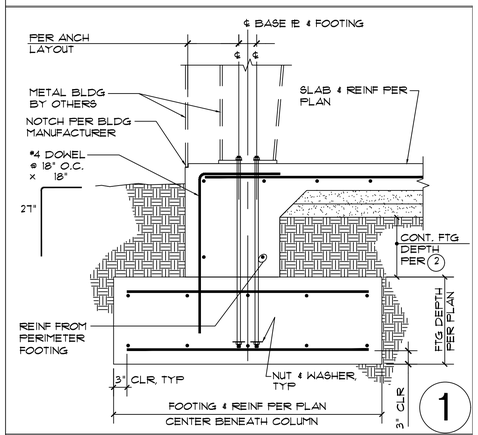
INDICATES COLUMN & BASE PLATE BY OTHERS

INDICATES CRACK CONTROL JOINT PER 4/SP1. CONTRACTOR TO INSTALL ADDITIONAL CONTROL JOINTS AS NEEDED TO PREVENT SLAB CRACKS

INDICATES DOOR JAMB CONNECTOR CLIP BY BUILDING MANUFACTURER W/ (2) $\frac{1}{2}$"Φ F1554 Gr. 36 ANCHOR BOLTS, DILLED 6" THROUGH THE TOP SURFACE OF THE SLAB & EPOXIED W/ SIMPSON "AT-XP" PER EPOXY MFR'S GUIDELINES. JAMB CHANNELS TO BE FIELD LOCATED U.O.N

# FOOTING SCHEDULE:

| FOOTING | SIZE | REINF | NOTES |
|---|---|---|---|
| F1 | 5'-6"x5'-6"x18" | (4) #5 EA WAY T&B | DETAIL 1 |
| F2 | 4'-0"x4'-0"x18" | (3) #5 EA WAY T&B | DETAIL 1 |
| F3 | 3'-6"x3'-6"x18" | (3) #5 EA WAY T&B | DETAIL 1 |
| F4 | 4'-0"x4'-0"x16" | (5) #5 EW WAY @ BOTT | DETAIL 4 |

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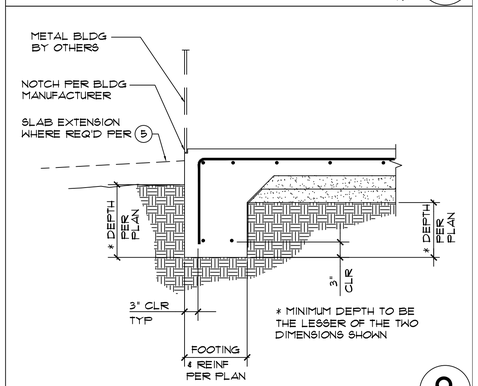


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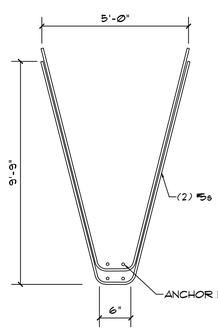
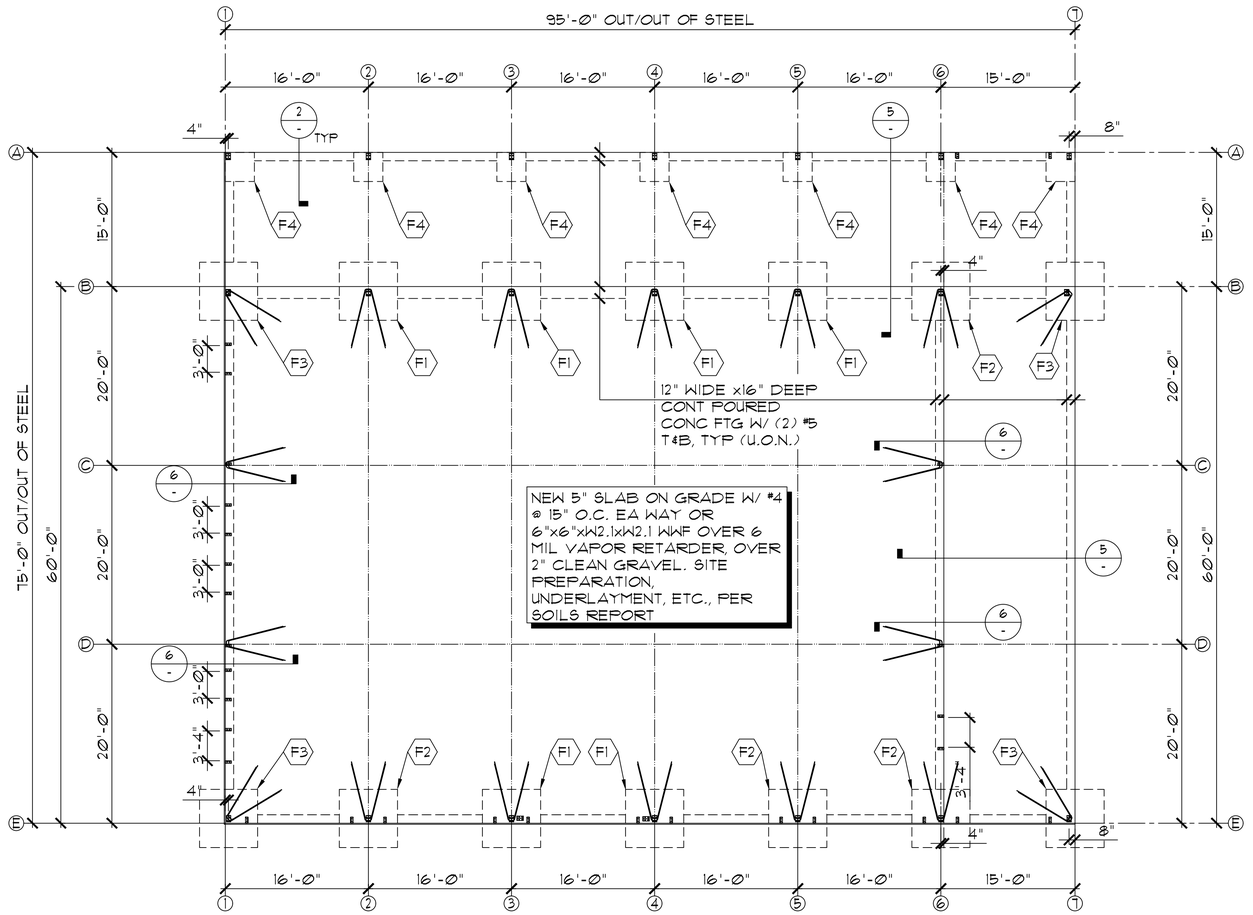


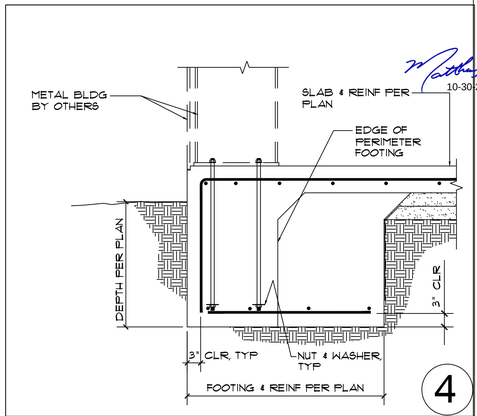

TYP. #5 HAIRPIN

3

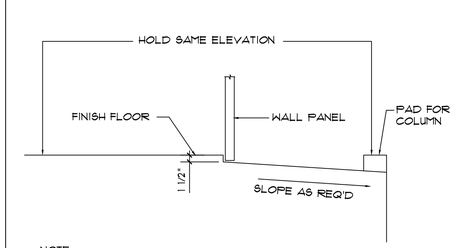



## FOUNDATION PLAN

SCALE: 1/8"=1'-0"

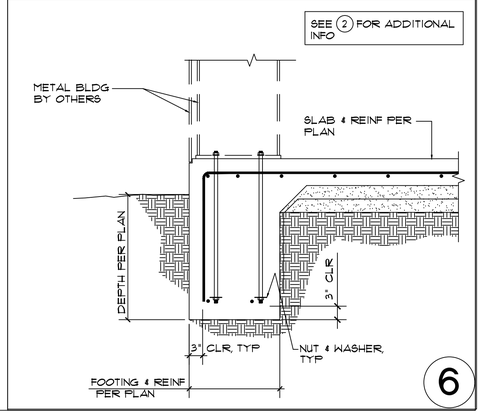



4




NOTE:
PAD MUST BE EQUAL TO OR LARGER THAN BASE PL

5

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REVISIONS | BY


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10-30-2020

ICN Partners LLC
540 Farabow Drive
Holly Springs, NC 27540

DATE 9-9-20
SCALE AS SHOWN
DRAWN A. VALLIN
JOB 20-2049
SHEET S1
OF SHEETS