McGee Consulting Associates, Inc.

consultants@mcgeecons.com Phone: 704-841-1550 Fax: 704-841-1406

March 3, 2022

Mr. Cody Jackson The Welding Machine, LLC 62 Judi Lee Road Lillington, NC 27456



RE: Residential Garage – *Review of Plans and Recommendations* MCA #2202

Dear Mr. Jackson:

McGee Consulting Associates, Inc. (MCA) was asked to review whether the Residential Garage plans prepared by you meet the requirements of the current North Carolina Building Code (2018 NCBC).

MCA determined that the columns and footings will need to support a load of 3,800 lbs based on the proposed layout. The HSS3x3x1/4 columns will support a load of 20,460 lbs, so the columns will sufficiently support the actual loads. The 18"x18"x14" footings will support a load of 4,500 lbs based on nominal 2,000 psf bearing capacity soil recommended in the 2018 NCBC, so the footings will support the actual loads.

MCA analyzed the proposed beams and determined that the beams have sufficient strength to support the loads, but the beams will over-deflect under the code prescribed loads. MCA has determined the beams must be an HSS4x3x1/4. See the attached sketch for beam size adjustments.

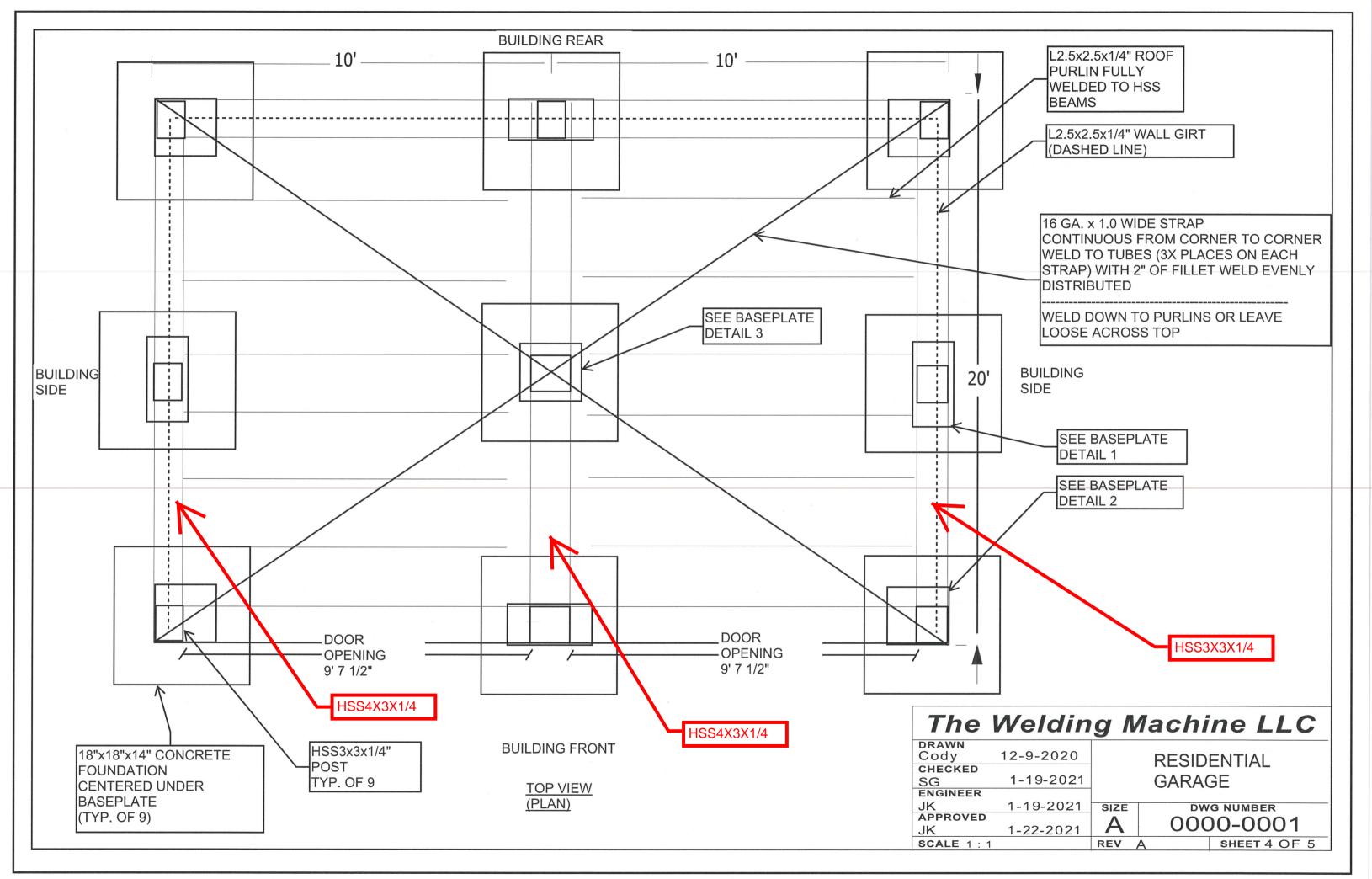
MCA approves of the overall design intent with the above-mentioned adjustments. No resubmission of the plans to MCA will be required.

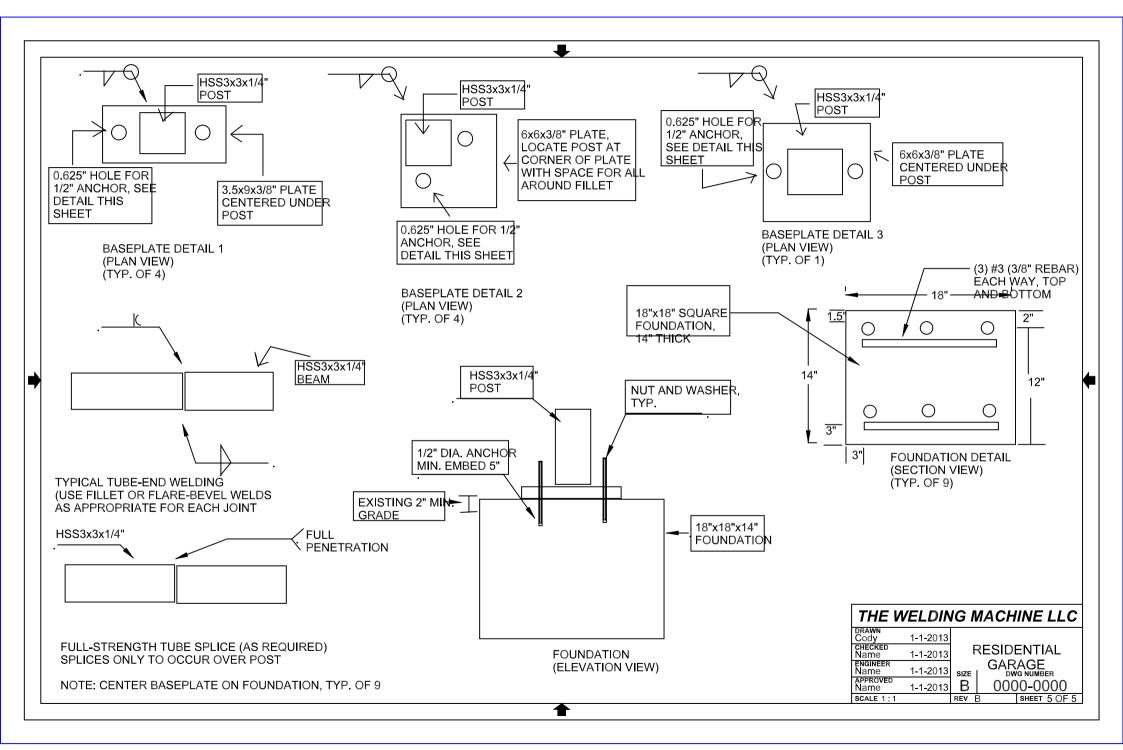
Please review this information and let me know if there are any questions or concerns.

Cordially,

Frank E. Henry, PE MCA COA #C-2716







L.
NOTES:
1. DRAWINGS ARE FOR A STANDALONE GARAGE TO BE CONSTRUCTED AT A RESIDENTIAL PROPERTY LOCATED AT: 62 JUDI LEE ROAD
LILLINGTON, NC 27546
2. ALL CONSTRUCTION TO BE IN ACCORDANCE WITH 2018 NCBC (2015 IBC WITH NC AMENDMENTS)
3. SCOPE: FRAME AND FOUNDATION DESIGN ARE INCLUDED WALLS, ROOFS, DOORS, AND OTHER COMPONENTS ARE BY OTHERS
4. LOADS
4.1 DEAD, ROOF = 3 PSF
4.2 LIVE, ROOF = 20 PSF
4.3 WIND, V = 117 MPH, RISK CATEGORY = 2
4.4 SNOW = 10 PSF
4.5 SEISMIC DESIGN CATEGORY =C , SITE CLASS = D, Sds = 0.211, Sd1 = 0.145,
4.6 LOCATION IS NOT IN A FLOOD HAZARD ZONE
5. FOUNDATIONS AND ANCHORS
5.1 USE NORMAL WEIGHT CONCRETE, f'c = MINIMUM 2,500 PSI @ 28 DAYS
5.2 MIX CONCRETE IN ACCORDANCE WITH ASTM C-94 5.2.1 CALCIUM CHLORIDE ADMIXTURES ARE PROHIBITED
5.2.2 PORTLAND CEMENT SHALL CONFORM TO ASTM C-150, TYPE 1
5.2.3 CONCRETE AGGREGATES SHALL CONFORM TO ASTM C-33
5.3 SUBGRADE SOILS ARE TO BE UNIFORM, DENSE, WELL-DRAINED, AND FREE OF FOREIGN OBJECTS AND DELETERIOUS MATERIAL
SUBGRADE PREPARATION IS AS REQUIRED BY THE JURISDICTION OR A GEOTECHNICAL ENGINEER, AS NEEDED
5.3.1 ALLOWABLE SOIL BEARING PRESSURE = 2000 PSF (ASSUMED), MODULUS OF SUBGRADE REACTION = 90 PCI
5.4 POST BASEPLATES TO BE CENTERED ON FOUNDATIONS
5.5 ANCHORS TO BE CAST IN PLACE OR POST INSTALLED, MIN. 5" EMBED, INSTALL PER MANUFACTURER'S INSTRUCTIONS
5.5.1 CAST IN PLACE = 1/2" DIA. HEX HEAD, F 1554 GR. 36
5.5.2 POST-INSTALLED = 1/2" DIA. A36 THREADED ROD WITH HILTI HIT-HY 200 EPOXY (OR SIMILAR)
6. STEEL
6.1 HSS = A500 GR C, Fy = 50 KSI MIN
6.2 PLATES, ANGLES, STRAPS / FLAT STOCK = A36, Fy = 36 KSI 7. WELDING
7.1 WELDING 7.1 WELDING TO BE IN ACCORDANCE WITH AWS D1.1
7.4 MIN WELD SIZE = THICKNESS OF STEEL FOR ROOF STRAP AND FLAT STOCK MIN WELD SIZE = 3/1
FOR ALL TUBES.
PLATES, AND ANGLES
APPROVED NPROVED 1-1-2013 B 0000-0000 SCALE 1:1 REV B SHEET 1 OF 5
$\mathbf{\uparrow}$

