

Trenco 818 Soundside Rd Edenton, NC 27932

Re: J0216-0966 Price\Campbell Building #12\Harnett

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: E9310002thru E9310056

My license renewal date for the state of North Carolina is December 31, 2016.

North Carolina COA: C-0844

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.



Lassiter, Frank

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdictions(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to Trenco. Any project specific information included is for Trenco's customer's file reference purpose only, and was not taken into account in the preparation of these designs. Trenco has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of the design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



bilding design. Bracting indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety** Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 2314.

Job	Truss	Truss Type	Qty	Ply	Price\Campbell Building #12\Harnett
					E9310002
J0216-0966	A01	Hip Girder	1	2	
					Job Reference (optional)
Comtech, Inc., Fayettevi	le, NC 28309			7.64	40 s Sep 29 2015 MiTek Industries, Inc. Mon Feb 29 15:38:41 2016 Page 2
			ID:ZzXTvI	MvxB55ZL	n?FA7aN0rzLZck-pE6EY83wxL1nnkuxzYvFYx7bGTrAAnc76aBx4ZzaFI

NOTES-

11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 14 lb down and 46 lb up at 7-2-12, 53 lb down and 70 lb up at 9-2-12, 53 lb down and 70 lb up at 11-2-12, 53 lb down and 70 lb up at 13-2-12, 53 lb down and 70 lb up at 15-2-12, 53 lb down and 70 lb up at 17-2-12, 53 lb down and 70 lb up at 19-2-12, 53 lb down and 70 lb up at 21-2-12, 53 lb down and 70 lb up at 21-2-12, 53 lb down and 70 lb up at 23-2-12, 53 lb down and 70 lb up at 25-2-12, 53 lb down and 70 lb up at 26-1-4, and 50 lb down and 63 lb up at 28-2-12, and 50 lb down and 56 lb up at 30-2-12 on top chord, and 132 lb down and 42 lb up at 3-2-12, 132 lb down and 42 lb up at 5-2-12, 122 lb down and 73 lb up at 7-2-12, 37 lb down and 13 lb up at 9-2-12, 37 lb down and 13 lb up at 13-2-12, 37 lb down and 13 lb up at 13-2-12, 37 lb down and 13 lb up at 13-2-12, 37 lb down and 13 lb up at 25-2-12, 37 lb down and 13 lb up at 25-2-12, 37 lb down and 13 lb up at 25-2-12, 37 lb down and 13 lb up at 25-2-12, 37 lb down and 13 lb up at 25-2-12, 37 lb down and 13 lb up at 25-2-12, 37 lb down and 13 lb up at 25-2-12, 37 lb down and 13 lb up at 25-2-12, 37 lb down and 13 lb up at 25-2-12, 37 lb down and 13 lb up at 25-2-12, 37 lb down and 13 lb up at 25-2-12, 37 lb down and 13 lb up at 25-2-12, 37 lb down and 13 lb up at 25-2-12, 37 lb down and 13 lb up at 25-2-12, 37 lb down and 13 lb up at 25-2-12, 37 lb down and 38 lb up at 25-2-12, 37 lb down and 13 lb up at 25-2-12, 37 lb down and 39 lb up at 30-2-12, 37 lb down and 39 lb up at 30-2-12, 37 lb down and 39 lb up at 30-2-12, 37 lb down and 50 lb up at 30-2-12, 37 lb down and 38 lb up at 30-2-12, 12 lb down and 58 lb up at 30-2-12, 37 lb down and 58 lb up at 30-2-12, 37 lb down and 58 lb up at 30-2-12, 37 lb down and 58 lb up at 30-2-12, 37 lb down and 58 lb up at 30-2-12, 37 lb down and 58 lb up at 30-2-12, 37 lb down and 58 lb up at 30-2-12, 37 lb down and 58 lb up at 30-2-12, 37 lb down and 58 lb up at 30-2-12, 37 lb down and 58 lb up at 30-2-12, 37 lb down a

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-60, 3-8=-60, 8-10=-60, 1-13=-20, 12-13=-20, 11-12=-20, 10-11=-20 Concentrated Loads (lb)

Vert: 13=-23(F) 12=-127(F) 17=46(F) 18=-53(F) 19=-53(F) 20=-53(F) 21=-53(F) 22=-53(F) 23=-53(F) 24=-53(F) 25=-53(F) 26=-53(F) 27=-53(F) 28=-50(F) 29=-50(F) 30=-132 31=-132 32=-122(F) 33=-23(F) 35=-23(F) 36=-23(F) 37=-23(F) 38=-23(F) 39=-23(F) 40=-23(F) 41=-23(F) 42=-18(F) 43=-25(F) 44=-127(F) 45=-127(F)



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zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between

the bottom chord and any other members. 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 94 lb uplift at joint 9 and 89 lb uplift at joint

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



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is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
fabrication, storage, delivery, erection and bracing of trusses systems, see
MISI/TPII Quality Criteria, DSB-89 and BCSI Building Component
Safety Information
available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

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2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Interior(1) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between

the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 133 lb uplift at joint 11 and 125 lb uplift at joint 1.

6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



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Job		Truss	Truss Type	Qty	Ply	Price\Campbell Building #12\Harnett
						E9310008
J0216-0966		A08	Roof Special Supported Gable	5	1	
						Job Reference (optional)
Comtech, Inc.,	Fayetteville,	NC 28309			7.64	0 s Sep 29 2015 MiTek Industries, Inc. Mon Feb 29 15:38:45 2016 Page 2
			I	-77XTvM	/xB557I n ⁴	2FA7aN0rzl Zck-h0LlOV6R?aYDGLCiCOzBinHTt4KF6nPi1H98DKzaEle

NOTES-

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 7 lb uplift at joint 23, 27 lb uplift at joint 30, 5 lb uplift at joint 26, 29 lb uplift at joint 35,

40 lb uplift at joint 36, 36 lb uplift at joint 38, 37 lb uplift at joint 39, 35 lb uplift at joint 40, 43 lb uplift at joint 41, 12 lb uplift at joint 42, 129 lb uplift at joint 43, 11 lb uplift at joint 44, 28 lb uplift at joint 33, 40 lb uplift at joint 32, 40 lb uplift at joint 31, 37 lb uplift at joint 29, 35 lb uplift at joint 28, 33 lb uplift at joint 27, 14 lb uplift at joint 25 and 107 lb uplift at joint 24.

9) Non Standard bearing condition. Review required.10) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

11) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



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Job	Truss	Truss Type	Qty	Ply	Price\Campbell Building #12\Harnett		
					E9310009		
J0216-0966	A09	Roof Special	5	1			
					Job Reference (optional)		
Comtech, Inc., Fayetteville, NC 28309 7.640 s Sep 29 2015 MiTek Industries, Inc. Mon Feb 29 15:38:46 2016 Page 2							
		I	D:ZzXTyM	vxB55ZLn	?FA7qN0rzLZck-9Cv7br73mug4uVnum6VQF?qe1Ug1rEqsFxuilmzgFld		

NOTES-

- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1 lb uplift at joint 1, 27 lb uplift at joint 29, 25 lb uplift at joint 22, 18 lb uplift at joint 35, 43 lb uplift at joint 36, 37 lb uplift at joint 37, 36 lb uplift at joint 38, 36 lb uplift at joint 39, 37 lb uplift at joint 40, 29 lb uplift at joint 41, 61 lb uplift at joint 42, 12 lb uplift at joint 32,
- 44 lb uplift at joint 31, 41 lb uplift at joint 30, 38 lb uplift at joint 28, 35 lb uplift at joint 27, 37 lb uplift at joint 26, 25 lb uplift at joint 24 and 67 lb uplift at joint 23.
- 10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 1, 25, 21, 28, 27, 26, 24, 23.
- 11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 12) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



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2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Interior(1) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between

the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 129 lb uplift at joint 11 and 122 lb uplift at joint 1.

6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



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Job	Truss	Truss Type	Qty	Ply	Price\Campbell Building #12\Harnett		
					E9310011		
J0216-0966	A11	Roof Special	4	1			
					Job Reference (optional)		
Comtech, Inc., Fayetteville, NC 28309 7.640 s Sep 29 2015 MiTek Industries, Inc. Mon Feb 29 15:38:48 2016 Page 2							
ID:ZzXTyMvxB55ZLn?FA7qN0rzLZck-5a1u0X9JHVwo7pxHuWXuLQvyoIL5J7D9jFNoq							

NOTES-

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 25 lb uplift at joint 29, 58 lb uplift at joint 25, 23 lb uplift at joint 22, 27 lb uplift at joint 34, 41 lb uplift at joint 35, 36 lb uplift at joint 37, 35 lb uplift at joint 38, 41 lb uplift at joint 39, 19 lb uplift at joint 40, 100 lb uplift at joint 41, 20 lb uplift at joint 42, 29 lb uplift at joint 41, 20 lb uplift at joint 42, 29 lb uplift at joint 42, 40 lb uplift at joint 42,

34, 41 lb uplift at joint 35, 36 lb uplift at joint 37, 35 lb uplift at joint 38, 41 lb uplift at joint 39, 19 lb uplift at joint 40, 100 lb uplift at joint 41, 20 lb uplift at joint 42, 29 lb uplift at joint 32, 40 lb uplift at joint 31, 40 lb uplift at joint 30, 39 lb uplift at joint 28, 35 lb uplift at joint 27, 34 lb uplift at joint 26, 16 lb uplift at joint 24 and 116 lb uplift at joint 23. 9) Non Standard bearing condition. Review required.

10) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

11) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



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Job	Truss	Truss Type	Qty	Ply	Price\Campbell Building #12\Harnett
					E9310012
J0216-0966	A12	Roof Special Supported Gable	4	1	
					Job Reference (optional)
Comtech, Inc., Fayetteville,	NC 28309			7.64	0 s Sep 29 2015 MiTek Industries, Inc. Mon Feb 29 15:38:49 2016 Page 2
			ID:ZzXTyl	MvxB55ZL	n?FA7qN0rzLZck-anbGDt9y2p2flzWTRE27tdS9Cihs2aOIyv7MM5zgFla

NOTES-

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members, with BCDL = 10.0psf.

- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2 lb uplift at joint 1, 33 lb uplift at joint 32, 44 lb uplift at joint 24, 31 lb uplift at joint 38, 39 lb uplift at joint 39, 36 lb uplift at joint 40, 36 lb uplift at joint 41, 36 lb uplift at joint 42, 36 lb uplift at joint 43, 37 lb uplift at joint 44, 36 lb uplift at joint 45, 52 lb uplift at joint 45, 52 lb uplift at joint 46, 50 lb uplift at joint 45, 52 lb uplift at joint 45, 56 lb uplift at jo and 43 lb uplift at joint 25.
- Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 1, 28, 23, 31, 30, 29, 27, 26, 25.
 "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 12) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



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the bottom chord and any other members. 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 119 lb uplift at joint 9 and 128 lb uplift at

joint 1. 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-60, 3-5=-60, 5-8=-60, 1-14=-20, 12-14=-80, 11-12=-20, 10-11=-20, 9-10=-20, 8-9=-20



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Job	Truss	Truss Type	Qty	Ply	Price\Campbell Building #12\Harnett
					E9310017
J0216-0966	A18	Hip Girder	1	2	
					Job Reference (optional)
Comtech, Inc., Fayetteville	, NC 28309			7.64	0 s Sep 29 2015 MiTek Industries, Inc. Mon Feb 29 15:38:52 2016 Page 2
		ID:Z:	zXTvMvxB	55ZLn?FA	7aN0rzLZck- MGOsuCaLkQDcQE27MbaVG4URvdCFtMletL0zQzaFIX

NOTES-

10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 99 lb down and 56 lb up at 2-7-7, 53 lb down and 33 lb up at 4-7-7, 19 lb down and 21 lb up at 6-1-7, 53 lb down and 70 lb up at 7-10-8, 53 lb down and 70 lb up at 10-1-7, 53 lb down and 70 lb up at 12-1-7, 53 lb down and 70 lb up at 14-1-7, 53 lb down and 70 lb up at 14-1-7, 53 lb down and 70 lb up at 16-1-7, 53 lb down and 70 lb up at 18-1-7, 53 lb down and 70 lb up at 20-1-7, 53 lb down and 70 lb up at 22-1-7, 53 lb down and 70 lb up at 24-1-7, 53 lb down and 70 lb up at 28-1-0, 50 lb down and 63 lb up at 28-1-0, 51 lb down and 56 lb up at 30-1-0, 11 lb down and 29 lb up at 32-1-0, and 61 lb down and 31 lb up at 33-7-0, and 61 lb down and 58 lb up at 35-7-0 on top chord, and 48 lb down at 2-7-7, 63 lb down and 20 lb up at 4-7-7, 97 lb down and 46 lb up at 6-1-7, 37 lb down and 13 lb up at 10-1-7, 37 lb down and 13 lb up at 12-1-7, 37 lb down and 13 lb up at 16-1-7, 37 lb down and 13 lb up at 12-1-7, 37 lb down and 13 lb up at 26-1-12, 33 lb down and 13 lb up at 28-1-0, 44 lb down and 23 lb up at 30-1-0, and 45 lb down and 18 lb up at 33-7-0, and 27 lb down and 31 lb up at 30-1-0, and 45 lb down and 18 lb up at 22-1-7, 37 lb down and 13 lb up at 26-1-12, 33 lb down and 18 lb up at 28-1-0, 54 lb down and 23 lb up at 20-1-7, 37 lb down and 18 lb up at 33-7-0, and 27 lb down and 31 lb up at 35-7-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-60, 3-7=-60, 7-10=-60, 1-13=-20, 12-13=-20, 11-12=-20, 10-11=-20

Concentrated Loads (lb)

Vert: 3=-53(B) 15=-23(B) 13=-23(B) 14=-23(B) 17=-59(B) 18=-13(B) 19=21(B) 20=-53(B) 21=-53(B) 22=-53(B) 23=-53(B) 24=-53(B) 25=-53(B) 26=-53(B) 27=-53(B) 28=-53(B) 29=-50(B) 30=-51(B) 31=29(B) 32=-21(B) 33=-21(B) 34=-33(B) 35=-63(B) 36=-97(B) 37=-23(B) 38=-23(B) 39=-23(B) 40=-23(B) 41=-23(B) 42=-23(B) 43=-23(B) 44=-18(B) 45=-24(B) 46=-45(B) 47=-12(B)



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		1-3-8 0-1-12	2-10-4	1 2	-10-4	0-1-12	1-3-8	
LOADIN	G (psf)	SPACING- 2-0-0	CSI.	DEFL. ii	n (loc)	l/defl L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.13	Vert(LL) -0.0		>999 360	MT20	244/190
BCLL	0.0 *	Rep Stress Incr YES	WB 0.04	Horz(TL) -0.0	2 / D 6	>999 240 n/a n/a		
BCDL	10.0	Code IRC2009/TPI2007	(Matrix)	Wind(LL) 0.0	7-8	>999 240	Weight: 36	lb FT = 20%

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 2x4 SP No.3 WEBS

REACTIONS. (lb/size) 8=343/0-3-8, 6=343/0-3-8

Max Horz 8=86(LC 5) Max Uplift8=-27(LC 6), 6=-27(LC 7)

FORCES. (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-69/31, 2-3=-140/70, 3-4=-140/70, 4-5=-69/31

BOT CHORD 1-8=-20/66, 7-8=-19/66, 6-7=-19/66, 5-6=-19/66

WEBS 3-7=-29/50, 2-8=-204/119, 4-6=-204/119

NOTES-

Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Interior(1)

zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between

the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 8 and 27 lb uplift at joint 6.

6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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Plate Of	fsets (X,Y)				
LOADIN	G (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d PLATES GRIP	
TCLL	20.0	Plate Grip DOL 1.15	TC 0.03	Vert(LL) -0.00 1 >999 360 MT20 244/190	
TCDL	10.0	Lumber DOL 1.15	BC 0.02	Vert(TL) -0.00 1-3 >999 240	
BCLL	0.0 *	Rep Stress Incr YES	WB 0.00	Horz(TĽ) -0.00 2 n/a n/a	
BCDL	10.0	Code IRC2009/TPI2007	(Matrix)	Wind(LL) 0.00 1 **** 240 Weight: 15 lb FT = 20%	

LUMBER-

TOP CHORD 2x6 SP No.1 WEDGE

BOT CHORD 2x6 SP No.1 Left: 2x6 SP No.1

REACTIONS. (lb/size) 2=70/Mechanical, 3=23/Mechanical, 1=94/0-3-8 Max Horz 1=41(LC 6) Max Uplift 2=-35(LC 6), 1=-1(LC 6) Max Grav 2=70(LC 1), 3=47(LC 2), 1=94(LC 1)

FORCES. (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=-35/27

BOT CHORD 1-3=0/0

NOTES-

1) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Interior(1)

- zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 35 lb uplift at joint 2 and 1 lb uplift at joint 1.

6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



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BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 2-6-11 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.



LOADIN	G (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d PLATES GRIP							
TCLL	20.0	Plate Grip DOL 1.15	TC 0.19	Vert(LL) -0.00 5 >999 360 MT20 244/190							
TCDL	10.0	Lumber DOL 1.15	BC 0.02	Vert(TL) -0.00 5 >999 240							
BCLL	0.0 *	Rep Stress Incr YES	WB 0.05	Horz(TL) -0.03 3 n/a n/a							
BCDL	10.0	Code IRC2009/TPI2007	(Matrix)	Wind(LL) 0.00 5 **** 240 Weight: 12 lb FT = 20%							

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 WFBS 2x4 SP No.3 WEDGE Left: 2x4 SP No.3

REACTIONS. (lb/size) 3=-39/Mechanical, 5=256/0-3-8, 4=13/Mechanical Max Horz 5=53(LC 6) Max Uplift 3=-45(LC 2) Max Grav 5=256(LC 1), 4=26(LC 2)

FORCES. (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=-46/26, 2-3=-56/0 BOT CHORD 1-5=0/61, 4-5=-5/5 WEBS 2-5=-230/30

NOTES-

1) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Interior(1)

zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 45 lb uplift at joint 3.

6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



Structural wood sheathing directly applied or 2-11-4 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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				1-3-8 0-1-	-12	4-5-4			-1			
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.20	Vert(LL)	-0.01	4-5	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.20	Vert(TL)	-0.03	4-5	>999	240		
BCLL BCDL	0.0 * 10.0	Rep Stress Incr Code IRC2009/TP	YES 12007	WB (Matri	0.05 x)	Horz(TL) Wind(LL)	-0.05 0.03	3 4-5	n/a >999	n/a 240	Weight: 22 lb	FT = 20%

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 WEBS WEDGE

2x4 SP No.3 Left: 2x4 SP No.3

REACTIONS. (lb/size) 3=113/Mechanical, 4=43/Mechanical, 5=309/0-3-8 Max Horz 5=132(LC 6) Max Uplift 3=-78(LC 6), 4=-3(LC 6) Max Grav 3=113(LC 1), 4=77(LC 2), 5=309(LC 1)

FORCES. (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=-195/63, 2-3=-89/48

BOT CHORD 1-5=-5/176, 4-5=0/0 2-5=-249/162 WFBS

NOTES-

1) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Interior(1)

zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between

the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 78 lb uplift at joint 3 and 3 lb uplift at joint 4.

6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



Structural wood sheathing directly applied or 5-10-8 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL TCDL	20.0 10.0	Plate Grip DOL Lumber DOL	1.15 1.15	TC BC	0.07 0.26	Vert(LL) Vert(TL)	-0.02 -0.06	6 5-6	>999 >851	360 240	MT20	244/190
BCLL BCDL	0.0 * 10.0	Rep Stress Incr Code IRC2009/TP	YES 12007	WB (Matr	0.03 ix)	Horz(TL) Wind(LL)	0.08 0.04	4 6	n/a >999	n/a 240	Weight: 24 lb	FT = 20%

TOP CHORD

BOT CHORD

LUMBER-

 TOP CHORD
 2x4 SP No.1

 BOT CHORD
 2x4 SP No.1

 WEBS
 2x4 SP No.3

 WEDGE
 Left: 2x4 SP No.3

Leil. 2x4 SF 110.5

REACTIONS. (lb/size) 4=73/Mechanical, 5=83/Mechanical, 7=309/0-3-8 Max Horz 7=77(LC 6) Max Uplift4=-31(LC 4), 5=-10(LC 6), 7=-10(LC 6) Max Grav 4=73(LC 1), 5=93(LC 2), 7=309(LC 1)

FORCES. (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-97/48, 2-3=-51/27, 3-4=-1/1

BOT CHORD 1-7=-7/108, 6-7=-7/6, 5-6=0/0 WEBS 3-6=-124/105, 2-7=-132/61

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Interior(1)

zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 31 lb uplift at joint 4, 10 lb uplift at joint 5 and 10 lb uplift at joint 7.

8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



Structural wood sheathing directly applied or 5-10-8 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing.

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	Job	Truss	Truss Type	Qty	Ply	Price\Campbell Building #12\Harnett			
	J0216-0966	J06E	Jack-Open Girder	1	1	E9310024			
L						Job Reference (optional)			
Comtech, Inc., Fayetteville, NC 28309 7.640 s Sep 29 2015 MiTek Industries, Inc. Mon Feb 29 15:38:55 2016 Page 2									
			ID:ZzXTyMvxB55ZLn?FA7qN0rzLZck-OxyXUwEiefooTuzdoV9X7ui9n6kdSKOBKragZl;						

LOAD CASE(S) Standard

Concentrated Loads (lb) Vert: 3=-10(F) 8=-10(F) 9=-3(F) 10=-3(F)



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BOT CHORD 2x6 SP No.1 WFBS 2x4 SP No 3 WEDGE Left: 2x4 SP No.3

REACTIONS. (lb/size) 4=39/Mechanical, 5=117/Mechanical, 7=309/0-3-8 Max Horz 7=98(LC 6) Max Uplift4=-17(LC 4), 5=-36(LC 6), 7=-3(LC 6)

FORCES. (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-150/52, 2-3=-61/39, 3-4=-1/1

1-7=-6/135, 6-7=-6/4, 5-6=0/0 BOT CHORD

WFBS 3-6=-120/93. 2-7=-182/141

NOTES-

Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Interior(1)

zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 17 lb uplift at joint 4, 36 lb uplift at joint 5 and 3 lb uplift at joint 7.

8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



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3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 4, 9 lb uplift at joint 8 and 8 lb uplift at joint 5.

8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



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Joh	Truss		Oty Ply Price\Campbell Bui	ilding #12\Harnett
10216 0066		lack Open Circler		E9310027
30210-0900	JUON	Jack-Open Girder	Job Reference (c	optional)
Comtech, Inc., Fayette	ville, NC 28309		7.640 s Sep 29 2015 Mi ID:ZzXTyMvxB55ZLn?FA7qN0rzLZck-s7\	rek Industries, Inc. Mon Feb 29 15:38:56 2016 Page 1 NvhGFLPywf41YpMCgmf6FKiW4xBIHKZVJD6BzgFIT
	2-2-1		5-10-8	· · · · · · · · · · · · · · · ·
	2-2-1		3-8-1	
		4x4 =		Scale = 1:13.6
		3	10	4
	_	A		
	8.00 12 _{2x4}			
	2			
	/			
4		8		14
-11-				
1				
8				
0-0	Y Y			6 5 74
1 1		\mathbf{H}	11	1 1-1
	4x4 🥢	\square		2x4
		6.00 12		
		8 5x8		
		3,0 -		
	1-3-8 1-3-8	0-3-8 0-8-14	<u>5-10-8</u> 3-6-10	I
Plate Offsets (X,Y) [3:0-2-0,0-1-2], [4:0-4-0,0-0-8], [8:0-4-0,0-1-15]		
LOADING (psf)	SPACING- 2-0	-0 CSI .	DEFL. in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.	15 TC 0.09	Vert(LL) 0.00 7 >999 360	MT20 244/190
ICDL 10.0 BCLL 0.0 *	Lumber DOL 1.2 Rep Stress Incr N	0 WB 0.14	Vert(TL) -0.00 6-7 >999 240 Horz(TL) -0.01 4 n/a n/a	
BCDL 10.0	Code IRC2009/TPI200	07 (Matrix)	Wind(LL) 0.00 7 >999 240	Weight: 30 lb FT = 20%
LUMBER-			BRACING-	
TOP CHORD 2x4 SP	No.1 *Except*		TOP CHORD Structural wood sheathin	g directly applied or 5-10-8 oc purlins.
BOT CHORD 2x4 SP	No.1 *Except*		BOT CHORD Rigid ceiling directly appl	led of 6-0-0 oc bracing.
5-9: 2x6	SP No.1			
WEDGE 2X4 SP	10.5			
Left: 2x4 SP No.3				
REACTIONS. (Ib/size)) 8=250/0-3-8, 6=23/Mecha	nical, 4=75/Mechanical		
Max Ho	orz 8=53(LC 5)	, ,		
Max Up Max Gr	av 8=250(LC 5), 4=-68(LC 14) av 8=250(LC 1), 6=61(LC 2),	4=76(LC 10)		
TOP CHORD 1-2=-8	num Compression/Maximum 8/95, 2-3=-46/42, 3-10=-0/1,	1 ension 4-10=-1/1		
BOT CHORD 1-8=-4	46/15, 7-8=-175/0, 7-9=0/141	, 7-11=0/0, 6-11=0/0, 5-6=0/0		
WEBS 8-9=-	102/90, 2-9=-181/87, 4-0=0/0			
NOTES-	laada haya haan aanaidaraa	I for this design		
2) Wind: ASCE 7-05; 10	00mph; TCDL=6.0psf; BCDL	=5.0psf; h=15ft; Cat. II; Exp C; er	nclosed; MWFRS (low-rise); Lumber DOL=1.60 plat	e
grip DOL=1.60		1		
4) This truss has been	designed for a 10.0 psf botto	m chord live load nonconcurrent	with any other live loads.	
5) * This truss has been	n designed for a live load of 2	0.0psf on the bottom chord in all	areas with a clearance greater than 6-0-0 between	TH CARO
6) Refer to girder(s) for	truss to truss connections.			S S ESSION A ST
7) Provide mechanical	connection (by others) of true	ss to bearing plate capable of with	hstanding 49 lb uplift at joint 8 and 68 lb uplift at joir	t Kit Later
4.			analysis and design of this truss.	
8) "Semi-rigid pitchbrea	aks including heels" Member	end fixity model was used in the		
 8) "Semi-rigid pitchbrea 9) Gap between inside 10) Happor(a) as athered 	aks including heels" Member of top chord bearing and firs	end fixity model was used in the a t diagonal or vertical web shall no	t exceed 0.500in.	SEAL
 Semi-rigid pitchbrea Gap between inside Hanger(s) or other 4-3-14 on top chord 	aks including heels" Member of top chord bearing and firs connection device(s) shall be d, and 14 lb up at 2-3-14. an	end fixity model was used in the a diagonal or vertical web shall no provided sufficient to support co d 14 lb up at 4-3-14 on bottom cl	ot exceed 0.500in. Incentrated load(s) 99 lb up at 2-2-1, and 99 lb up a hord. The design/selection of such connection	at 030652
 8) "Semi-rigid pitchbrea 9) Gap between inside 10) Hanger(s) or other 4-3-14 on top chord device(s) is the resp. 	aks including heels" Member of top chord bearing and firs' connection device(s) shall be d, and 14 lb up at 2-3-14, an ponsibility of others.	end fixity model was used in the a diagonal or vertical web shall no provided sufficient to support co d 14 lb up at 4-3-14 on bottom ch	of exceed 0.500in. Incentrated load(s) 99 lb up at 2-2-1, and 99 lb up a hord. The design/selection of such connection	SEAL 030652
 8) "Semi-rigid pitchbrea 9) Gap between inside 10) Hanger(s) or other 4-3-14 on top chorc device(s) is the response 11) In the LOAD CASE 	aks including heels" Member of top chord bearing and firs connection device(s) shall be d, and 14 lb up at 2-3-14, an ponsibility of others. (S) section, loads applied to	end fixity model was used in the a diagonal or vertical web shall no provided sufficient to support co d 14 lb up at 4-3-14 on bottom cl the face of the truss are noted as	of exceed 0.500in. Incentrated load(s) 99 lb up at 2-2-1, and 99 lb up a hord. The design/selection of such connection front (F) or back (B).	at 030652
 8) "Semi-rigid pitchbread 9) Gap between inside 10) Hanger(s) or other 4-3-14 on top chord device(s) is the reside 11) In the LOAD CASE LOAD CASE(S) Stand 	aks including heels" Member of top chord bearing and firs connection device(s) shall be d, and 14 lb up at 2-3-14, an ponsibility of others. (S) section, loads applied to dard	end fixity model was used in the is diagonal or vertical web shall no provided sufficient to support co d 14 lb up at 4-3-14 on bottom ci the face of the truss are noted as	of exceed 0.500in. Incentrated load(s) 99 lb up at 2-2-1, and 99 lb up a hord. The design/selection of such connection front (F) or back (B).	at O30652
 8) "Semi-rigid pitchbrea 9) Gap between inside 10) Hanger(s) or other 4-3-14 on top chord device(s) is the res 11) In the LOAD CASE LOAD CASE(S) Stand 1) Dead + Roof Live (based) 	aks including heels" Member of top chord bearing and firs connection device(s) shall be d, and 14 lb up at 2-3-14, an ponsibility of others. (S) section, loads applied to dard alanced): Lumber Increase=1	end fixity model was used in the a transpond or vertical web shall no e provided sufficient to support co d 14 lb up at 4-3-14 on bottom ct the face of the truss are noted as 1.15, Plate Increase=1.15	of exceed 0.500in. Incentrated load(s) 99 lb up at 2-2-1, and 99 lb up a hord. The design/selection of such connection front (F) or back (B).	at 030652
 8) "Semi-rigid pitchbrea 9) Gap between inside 10) Hanger(s) or other 4-3-14 on top chord device(s) is the resistent 11) In the LOAD CASE LOAD CASE(S) Stand 1) Dead + Roof Live (based) 	aks including heels" Member of top chord bearing and firs connection device(s) shall be d, and 14 lb up at 2-3-14, an ponsibility of others. (S) section, loads applied to dard alanced): Lumber Increase=1	end fixity model was used in the a t diagonal or vertical web shall no e provided sufficient to support co d 14 lb up at 4-3-14 on bottom ch the face of the truss are noted as 1.15, Plate Increase=1.15	of exceed 0.500in. Incentrated load(s) 99 lb up at 2-2-1, and 99 lb up a hord. The design/selection of such connection front (F) or back (B).	At SEAL 030652 NGINEER KR. LASS February 29,2016

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPII Quality Criteria, DSB-89 and BCSI Building Component Safely Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

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ENGINEERING

818 Soundside Road Edenton, NC 27932

ſ	Job	Truss	Truss Type	Qty	Ply	Price\Campbell Building #12\Harnett
	J0216-0966	J06H	Jack-Open Girder	1	1	E9310027
						Job Reference (optional)
	Comtech, Inc., Fayetteville,	NC 28309			7.64	40 s Sep 29 2015 MiTek Industries, Inc. Mon Feb 29 15:38:56 2016 Page 2
			ID:2	ZzXTyMvx	355ZLn?F	A7qN0rzLZck-s7WvhGFLPywf41YpMCgmf6FKiW4xBlHKZVJD6BzgFIT

LOAD CASE(S) Standard

Uniform Loads (plf) Vert: 1-3=-60, 3-4=-60, 1-8=-20, 7-8=-20, 5-7=-20 Concentrated Loads (lb) Vert: 3=46(B) 7=7(B) 10=46(B) 11=7(B)



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February 29,2016

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LOADING (psf)		SPACING- 2-	-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL 1	1.15	TC	0.09	Vert(LL)	-0.02	7	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL 1	1.15	BC	0.20	Vert(TL)	-0.04	7	>999	240		
BCLL	0.0 *	Rep Stress Incr Y	YES	WB	0.05	Horz(TL)	0.05	4	n/a	n/a		
BCDL	10.0	Code IRC2009/TPI20	007	(Matri	ix)	Wind(LL)	0.03	7	>999	240	Weight: 28 lb	FT = 20%

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 WEBS 2x4 SP No.3 WEDGE Left: 2x4 SP No.3

REACTIONS. (lb/size) 4=31/Mechanical, 8=320/0-3-8, 8=320/0-3-8, 5=115/Mechanical Max Horz 8=107(LC 6) Max Uplift4=-13(LC 4), 5=-45(LC 6)

FORCES. (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-18/117, 2-3=-60/42, 3-4=-1/1

- BOT CHORD 1-8=-56/31, 7-8=-142/0, 2-7=0/127, 6-7=-8/6, 5-6=0/0
- WEBS 2-8=-267/112, 3-6=-115/84

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Interior(1)

zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 13 lb uplift at joint 4 and 45 lb uplift at joint 5.

8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



Structural wood sheathing directly applied or 5-10-8 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing, Except:

10-0-0 oc bracing: 5-6.

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LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 WFBS 2x4 SP No 3 WEDGE Left: 2x4 SP No.3

REACTIONS. (lb/size) 3=111/Mechanical, 6=353/0-3-8, 4=44/Mechanical Max Horz 6=132(LC 6) Max Uplift 3=-65(LC 6), 4=-13(LC 6) Max Grav 3=111(LC 1), 6=353(LC 1), 4=84(LC 2)

FORCES. (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/81, 2-3=-84/47

BOT CHORD 1-6=-55/5, 5-6=-204/0, 2-5=0/177, 4-5=0/0

2-6=-303/91 WEBS

NOTES-

1) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Interior(1)

zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between

the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 65 lb uplift at joint 3 and 13 lb uplift at joint 4

6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



February 29,2016

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TOP CHORD BOT CHORD Structural wood sheathing directly applied or 5-10-8 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing.



1-3-8	1⊦7-0	2-7-0	5-10-8
1-3-8	0-3-8	1-0-0	3-3-8

Plate Offsets (X,Y) [6:0-2-8,0-2-4]											
	G (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) l/defl L/d PLATES GRIP							
TCLL	20.0	Plate Grip DOL 1.15	IC 0.19	Vert(LL) -0.01 4-5 >999 360 M120 244/190							
TCDL	10.0	Lumber DOL 1.15	BC 0.17	Vert(TL) -0.03 4-5 >999 240							
BCLL	0.0 *	Rep Stress Incr YES	WB 0.34	Horz(TL) -0.03 3 n/a n/a							
BCDL	10.0	Code IRC2009/TPI2007	(Matrix)	Wind(LL) 0.03 4-5 >999 240 Weight: 23 lb FT = 20%							

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 WFBS 2x4 SP No.3 WEDGE Left: 2x4 SP No.3

REACTIONS. (lb/size) 3=110/Mechanical, 6=340/0-3-8, 4=38/Mechanical Max Horz 6=132(LC 6) Max Uplift 3=-71(LC 6), 4=-9(LC 6) Max Grav 3=110(LC 1), 6=340(LC 1), 4=73(LC 2)

FORCES. (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=-9/74, 2-3=-87/46

- BOT CHORD 1-6=-46/19, 5-6=-366/0, 5-7=0/324, 4-5=0/0
- 6-7=-290/113, 2-7=-255/69 WEBS

NOTES-

1) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Interior(1)

zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 71 lb uplift at joint 3 and 9 lb uplift at joint 4.

6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



Structural wood sheathing directly applied or 5-10-8 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing.

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6 5x5 = 6.00 12

	1-3-8	1-7-0	2-7-14	5-10-8
	1-3-8	0-3-8	1-0-14	3-2-10
Plate Offsets (X,Y) [6:0-2-8,0-2-4]				

LOADIN	G (psf)	SPACING- 2-0-0	CSI.	DEFL. in	(loc)	l/defl	L/d	PLATES GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.19	Vert(LL) -0.01	4-5	>999	360	MT20 244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.16	Vert(TL) -0.03	4-5	>999	240	
BCLL	0.0 *	Rep Stress Incr YES	WB 0.33	Horz(TL) -0.03	3	n/a	n/a	
BCDL	10.0	Code IRC2009/TPI2007	(Matrix)	Wind(LL) 0.03	4-5	>999	240	Weight: 23 lb FT = 20%
LUMBER	र-			BRACING-				

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 WFBS 2x4 SP No 3 WEDGE Left: 2x4 SP No.3

REACTIONS. (lb/size) 3=110/Mechanical, 6=341/0-3-8, 4=38/Mechanical Max Horz 6=132(LC 6) Max Uplift 3=-71(LC 6), 4=-10(LC 6) Max Grav 3=110(LC 1), 6=341(LC 1), 4=73(LC 2)

FORCES. (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=-7/75, 2-3=-86/46 BOT CHORD 1-6=-47/17. 5-6=-350/0. 5-7=0/310. 4-5=0/0 6-7=-291/114, 2-7=-254/70 WEBS

NOTES-

1) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Interior(1)

zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 71 lb uplift at joint 3 and 10 lb uplift at joint 4

6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



Structural wood sheathing directly applied or 5-10-8 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing.

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TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 WFBS 2x4 SP No 3 WEDGE Left: 2x4 SP No.3

REACTIONS. (lb/size) 3=110/Mechanical, 6=353/0-3-8, 4=45/Mechanical Max Horz 6=132(LC 6) Max Uplift 3=-64(LC 6), 4=-13(LC 6) Max Grav 3=110(LC 1), 6=353(LC 1), 4=85(LC 2)

FORCES. (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/81, 2-3=-84/47

BOT CHORD 1-6=-55/5, 5-6=-206/0, 2-5=0/179, 4-5=0/0

2-6=-303/92 WEBS

NOTES-

1) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Interior(1)

zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 64 lb uplift at joint 3 and 13 lb uplift at joint 4

6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



818 Soundside Road Edenton, NC 27932

Structural wood sheathing directly applied or 5-10-8 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing.

February 29,2016

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Plate Off	Plate Offsets (X,Y) [9:0-2-8,0-2-4]											
	G (psf)	SPACING-	2-0-0	CSI.	0.15	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCDL	20.0 10.0	Lumber DOL	1.15	BC	0.15	Vert(LL) Vert(TL)	-0.01	8-9 8-9	>999	240	M120	244/190
BCLL BCDL	0.0 * 10.0	Rep Stress Incr Code IRC2009/TPI	YES 2007	WB (Matri	0.05 x)	Horz(TL) Wind(LL)	0.01 0.01	7 3-8	n/a >999	n/a 240	Weight: 29 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 WFBS 2x4 SP No 3 WEDGE Left: 2x4 SP No.3

REACTIONS. (lb/size) 9=312/0-3-8, 9=312/0-3-8, 7=147/Mechanical Max Horz 9=132(LC 6) Max Uplift 7=-85(LC 6)

FORCES. (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1/53, 2-3=-58/57, 3-4=-70/42, 4-5=-3/0

BOT CHORD 1-9=-10/10, 8-9=-171/2, 3-8=0/152, 7-8=0/0, 6-7=0/0

WFBS 2-9=-243/89. 4-7=-103/77

NOTES-

1) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Interior(1)

zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between

the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 85 lb uplift at joint 7.

6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



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818 Soundside Road Edenton, NC 27932

BRACING-TOP CHORD

Rigid ceiling directly applied or 6-0-0 oc bracing.

BOT CHORD

Structural wood sheathing directly applied or 5-10-8 oc purlins.



LOADING (psf)		SPACING- 2-0-0	CSI.	DEFL. in (lo	oc) l/defl L/d	PLATES GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.16	Vert(LL) -0.01	7 >999 360	MT20 244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.11	Vert(TL) -0.02	7 >999 240	
BCLL	0.0 *	Rep Stress Incr YES	WB 0.05	Horz(TL) -0.01	6 n/a n/a	
BCDL	10.0	Code IRC2009/TPI2007	(Matrix)	Wind(LL) 0.02	7 >999 240	Weight: 29 lb FT = 20%

TOP CHORD

BOT CHORD

LUMBER-

 TOP CHORD
 2x4 SP No.1

 BOT CHORD
 2x4 SP No.1

 WEBS
 2x4 SP No.3

 WEDGE
 Left: 2x4 SP No.3

REACTIONS. (lb/size) 8=312/0-3-8, 6=147/Mechanical Max Horz 8=132(LC 6) Max Uplift6=-85(LC 6)

FORCES. (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/68, 2-3=-82/43, 3-4=-3/0

BOT CHORD 1-8=-30/0, 7-8=-180/0, 2-7=0/164, 6-7=0/0, 5-6=0/0

WEBS 2-8=-250/82. 3-6=-107/86

NOTES-

1) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Interior(1)

zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between

the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 85 lb uplift at joint 6.

6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



Structural wood sheathing directly applied or 5-10-8 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing.

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LOADING	G (psf)	SPACING- 2	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL 1	1.15	TC	0.17	Vert(LL)	-0.01	6-7	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL 1	1.15	BC	0.15	Vert(TL)	-0.02	6-7	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.31	Horz(TL)	-0.01	6	n/a	n/a		
BCDL	10.0	Code IRC2009/TPI2	2007	(Matr	ix)	Wind(LL)	0.02	6-7	>999	240	Weight: 28 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 WFBS 2x4 SP No 3 WEDGE Left: 2x4 SP No.3

Plate Offsets (X,Y)-- [8:0-2-8,0-2-4]

REACTIONS. (lb/size) 8=312/0-3-8, 6=147/Mechanical Max Horz 8=132(LC 6) Max Uplift 6=-85(LC 6)

FORCES. (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-31/61, 2-3=-82/44, 3-4=-3/0

BOT CHORD 1-8=-30/38, 7-8=-324/0, 7-9=0/296, 6-7=0/0, 5-6=0/0

8-9=-263/94, 2-9=-252/80, 3-6=-109/95 WFBS

NOTES-

1) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Interior(1)

zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between

the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 85 lb uplift at joint 6.

6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



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BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 5-10-8 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing.



			⊢	<u>1-3-8</u> 1 ₁ 5	-14	5-10-8				4		
				1-3-8 0-1-	12	4-5-4						
LOADING	(psf)	SPACING-	2-0-0	CSI		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.15	Vert(LL)	-0.01	6	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.15	Vert(TL)	-0.03	6-7	>999	240		
BCLL BCDL	0.0 * 10.0	Rep Stress Incr Code IRC2009/T	YES PI2007	WB (Ma	0.04 trix)	Horz(TL) Wind(LL)	0.00 0.01	5 6	n/a >999	n/a 240	Weight: 29 lb	FT = 20%

TOP CHORD

BOT CHORD

end verticals.

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 WEBS 2x4 SP No.3 WEDGE Left: 2x4 SP No.3

REACTIONS. (lb/size) 7=306/0-3-8, 5=152/0-1-8 Max Horz 7=83(LC 5) Max Uplift7=-12(LC 6), 5=-29(LC 6)

FORCES. (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-69/30, 2-3=-85/28, 3-4=-35/32, 3-6=-64/40 BOT CHORD 1-7=-17/74, 6-7=-17/20, 5-6=0/0

2-7=-188/115, 4-5=-71/43 WEBS

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Interior(1) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2x4 ||

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between

the bottom chord and any other members. 5) Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

6) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 12 lb uplift at joint 7 and 29 lb uplift at joint 5

8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



Structural wood sheathing directly applied or 5-10-8 oc purlins, except

Rigid ceiling directly applied or 10-0-0 oc bracing.

February 29,2016

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			1	1-3-8 0-1-	12	3-10-14		'c)-6-6 '			
	G (psf)	SPACING- 2-	0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL 1.	.15	TC	0.17	Vert(LL)	-0.01	6-7	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL 1.	.15	BC	0.16	Vert(TL)	-0.04	6-7	>999	240		
BCLL	0.0 *	Rep Stress Incr Y	ES	WB	0.04	Horz(TL)	-0.06	4	n/a	n/a		
BCDL	10.0	Code IRC2009/TPI20	007	(Matri	x)	Wind(LL)	0.03	6-7	>999	240	Weight: 27 lb	FT = 20%

TOP CHORD

BOT CHORD

LUMBER-

 TOP CHORD
 2x4 SP No.1

 BOT CHORD
 2x4 SP No.1

 WEBS
 2x4 SP No.3

 WEDGE
 Left: 2x4 SP No.3

REACTIONS. (lb/size) 4=14/Mechanical, 5=142/Mechanical, 7=309/0-3-8 Max Horz 7=119(LC 6) Max Uplift4=-6(LC 4), 5=-63(LC 6)

FORCES. (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-170/58, 2-3=-82/49, 3-4=-1/1

BOT CHORD 1-7=-8/163, 6-7=-5/4, 5-6=0/0

WEBS 2-7=-215/134, 3-6=-120/112

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Interior(1)
- zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 6 lb uplift at joint 4 and 63 lb uplift at joint 5.

8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



Structural wood sheathing directly applied or 5-10-8 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing.

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			1-3-8		4-7-0						
LOADIN	G (psf)	SPACING- 2-0	0-0 CSI .		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL 1.	15 TC	0.47	Vert(LL)	0.00	2	n/r	120	MT20	244/190
TCDL	10.0	Lumber DOL 1.	15 BC	0.60	Vert(TL)	0.02	3	n/r	120		
BCLL	0.0 *	Rep Stress Incr YE	ES WB	0.00	Horz(TL)	0.00		n/a	n/a		
BCDL	10.0	Code IRC2009/TPI20	07 (Mat	rix)						Weight: 31 lb	FT = 20%

TOP CHORD

BOT CHORD

end verticals.

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 2x4 SP No.3 WEBS OTHERS 2x4 SP No 3 WEDGE Left: 2x4 SP No.3

REACTIONS. (lb/size) 5=246/4-7-0, 6=-230/4-7-0, 7=443/4-7-0 Max Horz 7=132(LC 6) Max Uplift 5=-99(LC 6), 6=-230(LC 1)

Max Grav 5=246(LC 1), 6=21(LC 6), 7=443(LC 1)

FORCES. (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-120/78, 2-3=-3/0, 2-5=-174/144

BOT CHORD 1-7=-7/170, 6-7=0/0, 5-6=0/0, 4-5=0/0

NOTES-

1) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Interior(1) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) Gable studs spaced at 2-0-0 oc.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 99 lb uplift at joint 5 and 230 lb uplift at joint 6

7) Non Standard bearing condition. Review required.

8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



Structural wood sheathing directly applied or 5-10-8 oc purlins, except

Rigid ceiling directly applied or 10-0-0 oc bracing.

February 29,2016



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FORCES. (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-231/76, 2-3=-231/76 BOT CHORD 1-6=-21/127, 5-6=-20/127, 4-5=-20/127, 3-4=-20/127

WEBS 2-5=-80/88

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry

Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Gable studs spaced at 2-0-0 oc.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 85 lb uplift at joint 6 and 85 lb uplift at joint 4

8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



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LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x6 SP No.1 2x4 SP No.3

WEBS REACTIONS. (lb/size) 6=350/0-3-0, 4=350/0-3-0

Max Horz 6=-85(LC 4) Max Uplift6=-27(LC 6), 4=-27(LC 7)

FORCES. (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-231/59, 2-3=-231/59

BOT CHORD 1-6=-10/127, 5-6=-10/127, 4-5=-10/127, 3-4=-10/127 WEBS 2-5=-80/74

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Interior(1)

zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 6 and 27 lb uplift at joint

6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



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818 Soundside Roa Edenton, NC 27932

BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.





February 29,2016

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2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Interior(1)

- zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 30 lb uplift at joint 6 and 28 lb uplift at joint 4.

6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



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Edenton, NC 27932



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- Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Interior(1) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 24 lb uplift at joint 1, 31 lb uplift at joint 3 and 3 lb uplift at joint 4.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



February 29,2016

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FORCES. (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-69/37, 2-3=-69/37

BOT CHORD 1-4=-10/27, 3-4=-10/27 WEBS 2-4=-133/52

NOTES-

 Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Interior(1) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 21 lb uplift at joint 1 and 25 lb uplift at joint 3.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



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2x4 💋

2x4 📎

Rigid ceiling directly applied or 10-0-0 oc bracing.

	<u> </u>		<u>3-7-1</u> 3-7-1		
Plate Offsets (X,Y)	[2:0-2-0,Edge]				-
LOADING (psf) TCLL 20.0	SPACING- 2-0-0 Plate Grip DOL 1.15	CSI. TC 0.02	DEFL. in (Vert(LL) n/a	(loc) l/defl L/d - n/a 999	PLATES GRIP MT20 244/190
TCDL 10.0 BCLL 0.0 * BCDL 10.0	Lumber DOL 1.15 Rep Stress Incr YES Code IRC2009/TPI2007	BC 0.06 WB 0.00 (Matrix)	Vert(TL) n/a Horz(TL) 0.00	- n/a 999 3 n/a n/a	Weight: 10 lb FT = 20%
LUMBER- TOP CHORD 2x4 S	P No.1		BRACING- TOP CHORD S	Structural wood sheathing d	irectly applied or 3-7-1 oc purlins.

BOT CHORD

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

REACTIONS. (lb/size) 1=105/3-7-1, 3=105/3-7-1 Max Horz 1=-24(LC 4) Max Uplift 1=-8(LC 6), 3=-8(LC 7)

FORCES. (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-88/41, 2-3=-88/41

BOT CHORD 1-3=-12/59

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Interior(1)

zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between

the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 8 lb uplift at joint 1 and 8 lb uplift at joint 3.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



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Edenton, NC 27932



4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between

the bottom chord and any other members. 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 1, 35 lb uplift at joint 5, 5 lb uplift at joint 6 and 97 lb uplift at joint 7.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



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FORCES. (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-63/70, 2-3=-35/50, 3-4=-78/62

BOT CHORD1-5=0/0, 4-5=0/0WEBS2-5=-214/74

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Interior(1)

zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 18 lb uplift at joint 1, 39 lb uplift at joint 4 and 6 lb uplift at joint 5.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



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7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



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2x4 ⁄

2x4 📎

Structural wood sheathing directly applied or 4-0-1 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

		L				4-0-1						
						4-0-1					1	
Plate Of	fsets (X,Y)	[2:0-2-0,Edge]										
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.03	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.08	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	10.0	Code IRC2009/T	PI2007	(Matr	ix)						Weight: 12 lb	FT = 20%
	5					BRACING						

TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

REACTIONS. (lb/size) 1=122/4-0-1, 3=122/4-0-1 Max Horz 1=-28(LC 4) Max Uplift 1=-9(LC 6), 3=-9(LC 7)

FORCES. (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-103/47, 2-3=-102/47 BOT CHORD 1-3=-14/68

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Interior(1)

- zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 9 lb uplift at joint 1 and 9 lb uplift at joint 3.

6) Non Standard bearing condition. Review required.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



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TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

2x4 SP No.3 OTHERS

REACTIONS. (lb/size) 1=123/6-7-1, 3=123/6-7-1, 4=204/6-7-1 Max Horz 1=-52(LC 4) Max Uplift 1=-21(LC 6), 3=-25(LC 7)

FORCES. (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-69/38, 2-3=-69/38

BOT CHORD 1-4=-10/27, 3-4=-10/27 WEBS 2-4=-133/52

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Interior(1)

zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 21 lb uplift at joint 1 and 25 lb uplift at joint 3.

6) Non Standard bearing condition. Review required.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



818 Soundside Road Edenton, NC 27932

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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LUMBER-



2x4 💋

2x4 📎

Rigid ceiling directly applied or 10-0-0 oc bracing.

			<u>3-7-1</u> 3-7-1		
Plate Offsets (X,Y)-	[2:0-2-0,Edge]			1	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.02 BC 0.06 WB 0.00	DEFL. in (loc) I/defl Vert(LL) n/a - n/a Vert(TL) n/a - n/a Horz(TL) 0.00 3 n/a	L/d PLATES GRIP 999 MT20 244/190 999 p/a	
BCDL 10.0	Code IRC2009/TPI2007	(Matrix)		Weight: 10 lb FT = 2	20%
LUMBER- TOP CHORD 2x4 \$	SP No.1		BRACING- TOP CHORD Structural woo	od sheathing directly applied or 3-7-1 oc purling	S.

BOT CHORD

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

REACTIONS. (lb/size) 1=105/3-7-1, 3=105/3-7-1 Max Horz 1=-24(LC 4) Max Uplift 1=-8(LC 6), 3=-8(LC 7)

FORCES. (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-88/41, 2-3=-88/41

BOT CHORD 1-3=-12/59

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Interior(1)

zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between

the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 8 lb uplift at joint 1 and 8 lb uplift at joint 3.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



818 Soundside Road Edenton, NC 27932

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REACTIONS. (lb/size) 1=127/6-9-1, 3=127/6-9-1, 4=209/6-9-1 Max Horz 1=54(LC 5) Max Uplift 1=-22(LC 6), 3=-26(LC 7)

FORCES. (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-71/38, 2-3=-71/38

BOT CHORD 1-4=-11/27, 3-4=-11/27 WEBS 2-4=-137/53

NOTES-

 Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Interior(1) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 22 lb uplift at joint 1 and 26 lb uplift at joint 3.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



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2x4 💋

2x4 ||

2x4 📎

<u>3-9-1</u> <u>3-9-1</u>												
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL TCDL	20.0 10.0	Plate Grip DOL Lumber DOL	1.15 1.15	TC BC	0.02 0.01	Vert(LL) Vert(TL)	n/a n/a	-	n/a n/a	999 999	MT20	244/190
BCLL BCDL	0.0 * 10.0	Rep Stress Incr Code IRC2009/TPI	YES 2007	WB (Matr	0.01 ix)	Horz(TL)	0.00	3	n/a	n/a	Weight: 12 lb	FT = 20%
		-										

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 2x4 SP No.3 OTHERS

BRACING TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 3-9-1 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 1=61/3-9-1, 3=61/3-9-1, 4=101/3-9-1 Max Horz 1=-26(LC 4) Max Uplift 1=-10(LC 6), 3=-12(LC 7)

FORCES. (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-34/19, 2-3=-34/19

BOT CHORD 1-4=-5/13, 3-4=-5/13 WEBS 2-4=-66/27

NOTES-

 Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Interior(1) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 10 lb uplift at joint 1 and 12 lb uplift at joint 3.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



