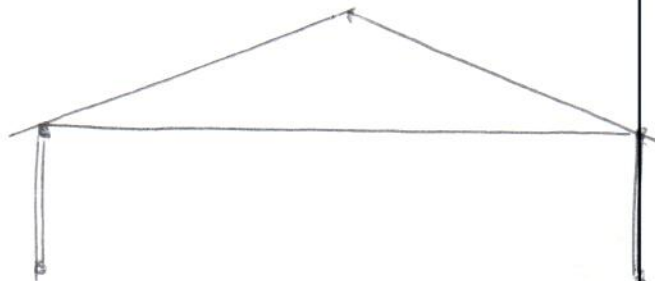
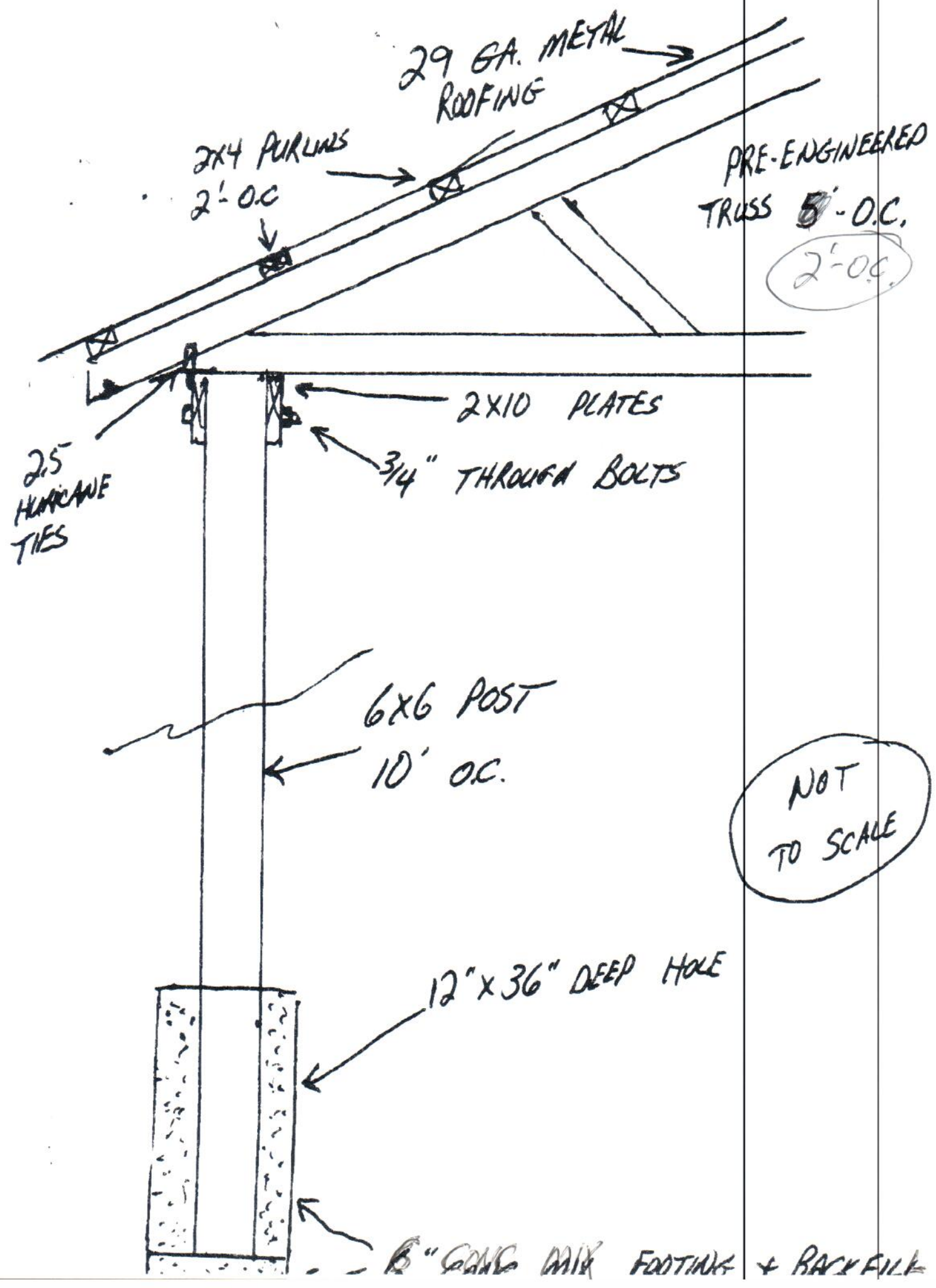


40x40x10' H OPEN FARM SHELTER



METAL  
ROOF &  
GABLE



29 GA. METAL ROOFING

2x4 PURLINS  
2'-0C

PRE-ENGINEERED TRUSS 5'-0C,  
2'-0C.

2.5 HURRICANE TIES

2x10 PLATES

3/4" THROUGH BOLTS

6x6 POST  
10' OC.

NOT TO SCALE

12" x 36" DEEP HOLE

8" SAND MIX FOOTING + BACKFILL

Q0628 - Major

Qty: 21

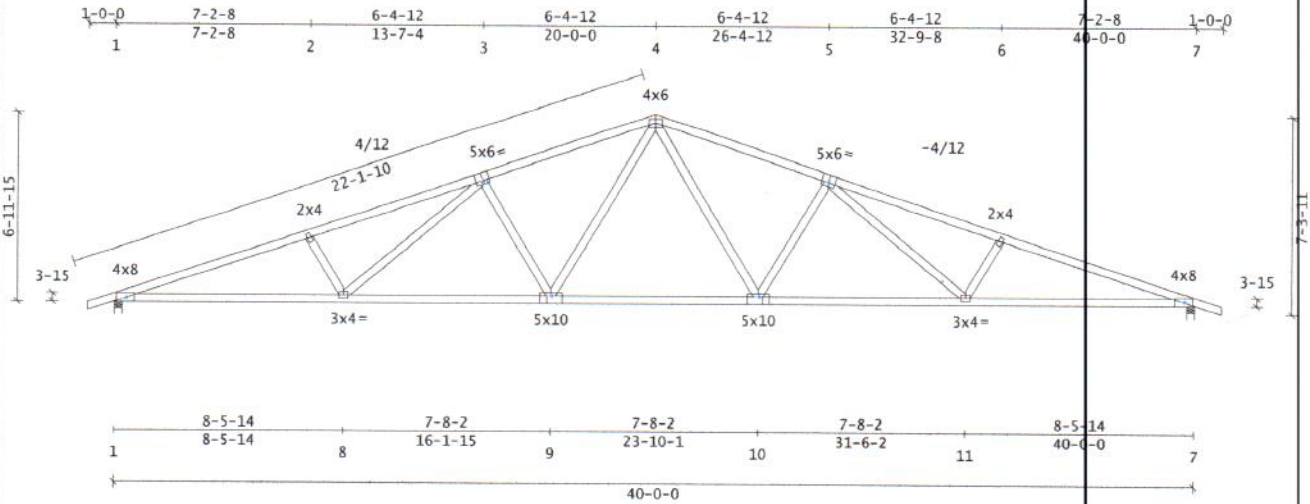
Truss: M01

Customer: David & Shelly Johnson

Agruss, Inc.  
agrussterrance@yahoo.com  
(803) 428-2122

SID:  
TID:  
Date: 04 / 02 / 20  
Page: 1 of 1

Truss Mfr. Contact: Greg Jackson



Code/Design: IRC-2018/TPI-2014  
PSF Live Dead Dur Factors  
TC 20.0 10.0 Live Wind Snow  
BC 0.0 10.0 Lum 1.25 1.60 1.15  
Total 40.0 Plt 1.25 1.60 1.15  
Spacing: 2-00-00 o.c. Plies: 1  
Repetitive Member Increase: Yes  
Green Lumber: No Wet Service: No  
Fab Tolerance: 20% Creep (Kcr) = 2.0  
OH Soffit Load: 2.0 psf

-----Snow Load Specs-----  
ASCE7-16 Ground Snow(Sg) = 10.0 psf  
Risk Cat: II Terrain Cat: B  
Roof Exposure: Partially Exposed  
Thermal Condition: Cold  
Ventilated(1.1)  
Unobstructed Slippery Roof: No  
Low-Slope Minimums(Pfmin): No  
Unbalanced Snow Loads: Yes  
Rain Surcharge: No Ice Dam Chk: Yes

-----Wind Load Specs-----  
ASCE7-16 Wind Speed(V) = 125 mph  
Risk Cat: II Exposure Cat: B  
Bldg Dims: L = 0.0 ft B = 0.0 ft  
M.R.H(h) = 20.0 ft Kzt = 1.0  
Bldg Enclosure: Enclosed  
Wind DL(psf): TC = 3.0 BC = 3.0  
End Vertical Exposed: L = Yes R = Yes  
Wind Uplift Reporting: ASCE7 MWERS  
C&C End Zone: 4-00-00

-----Additional Design Checks-----  
10 psf Non-Concurrent BCLL: Yes  
20 psf BC Limited Storage: Yes  
200 lb BC Accessible Ceiling: No  
300 lb BC Maintenance Load: No  
2000 lb BC Safe Load: No  
Unbalanced TCCLL: Yes

**Material Summary**

TC 2x4 SP (ALSC6-2013) #1  
BC 2x4 SP (ALSC6-2013) #1  
Webs 2x4 SP (ALSC6-2013) #3/Stud

**Reaction Summary**

-----Reaction Summary(Lbs)-----  
Jnt --X- Loc- React -Up- --Width- -Reqd -Mat PSI  
1 01-12 1664 256 03-08 02-06 SPF 470  
7 39-10-04 1664 256 03-08 02-06 SPF 470  
Max Horiz = -69 / +69 at Joint 1

**Deflection Summary**

Truss	Span	Limit	Actual (in)	Location
Vert LL	L/240	L/999	(-0.37)	9-10
Vert DL	L/120	L/999	(-0.38)	9-10
Vert CR	L/180	L/635	(-0.74)	9-10
Horz LL	0.75in	(0.10)	@Jt 7	
Horz CR	1.25in	(0.19)	@Jt 7	
Ohng CR	2L/180	L/999	(-0.00)	1- 1
Ohng CR	2L/180	L/999	(-0.00)	7- 7

Vert CR and Horz CR are the vertical and horizontal deflections due to live load plus the creep component of deflection due to dead load, computed as Defl\_LL + (Kcr - 1) \* Defl\_DL in accordance with ANSI/TPI 1.

**Member Forces Summary**

Mem	Ten	Comp	.CSI.
TC OH- 1	22	0	0.06
1- 2	1268	4108	0.68
2- 3	1245	3916	0.49
3- 4	1049	3027	0.43
4- 5	1048	3027	0.43
5- 6	1245	3916	0.49
6- 7	1268	4108	0.68
7-OH	22	0	0.06
BC 1- 8	3842	1167	0.86
7-11	3842	1145	0.83
8- 9	3131	949	0.71
9-10	2348	662	0.67
10-11	3131	927	0.71
Web 2- 8	194	377	0.08
3- 8	726	178	0.27
3- 9	310	687	0.37
4- 9	894	268	0.34
4-10	894	268	0.34
5-10	310	687	0.37
5-11	726	178	0.27
6-11	194	377	0.08

**Loads Summary**

This truss has been designed for the effects of an unbalanced top chord live load occurring at [20-00-00] using a 1.00 Full and 0.00 Reduced load factor.

See Loadcase Report for loading combinations and additional details.  
Dead Loads may be slope adjusted: > 12.0/12

**Notes**

Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees  
Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.  
Designed with hybrid plate values. See TD-SUB-0001 for info.

**Bracing Data Summary**

-----Bracing Data-----  
Chords; Sheathing required or bracing indicated:  
-----Furlins-----  
-----From-----To-----#Bays  
BC 5-08-00 0 40-00-00  
Web Bracing -- None

**Plate offsets (X, Y):**

(None unless indicated below)  
Jnt1(0,00-00), Jnt3(-00-05,00-15),  
Jnt5(00-05,00-15), Jnt7(0,00-02),  
Jnt9(0,-01-00), Jnt10(0,-01-00)

NOTICE A copy of this design shall be furnished to the erection contractor. The design of this individual truss is based on design criteria and requirements supplied by the Truss Manufacturer and relies upon the accuracy and completeness of the information set forth by the Building Designer. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. See the cover page and the "Important Information & General Notes" page for additional information. All connector plates shall be manufactured by Simpson Strong-Tie Company, Inc in accordance with ESR-2762. All connector plates are 20 gauge, unless the specified plate size is followed by a "-18" which indicates an 18 gauge plate, or "S18", which indicates a high tension 18 gauge plate.

C:\SST\Client\RG Files...  
Component Solutions  
Truss Studio V  
2019.10.1.11  
Helpdesk: 1-866-252-8606  
CSHelp@strongtie.com