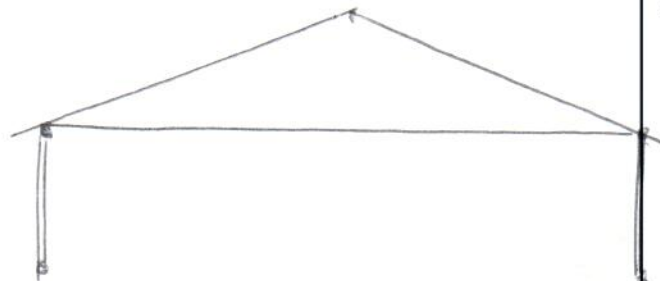


40x40x10' H OPEN FARM SHELTER



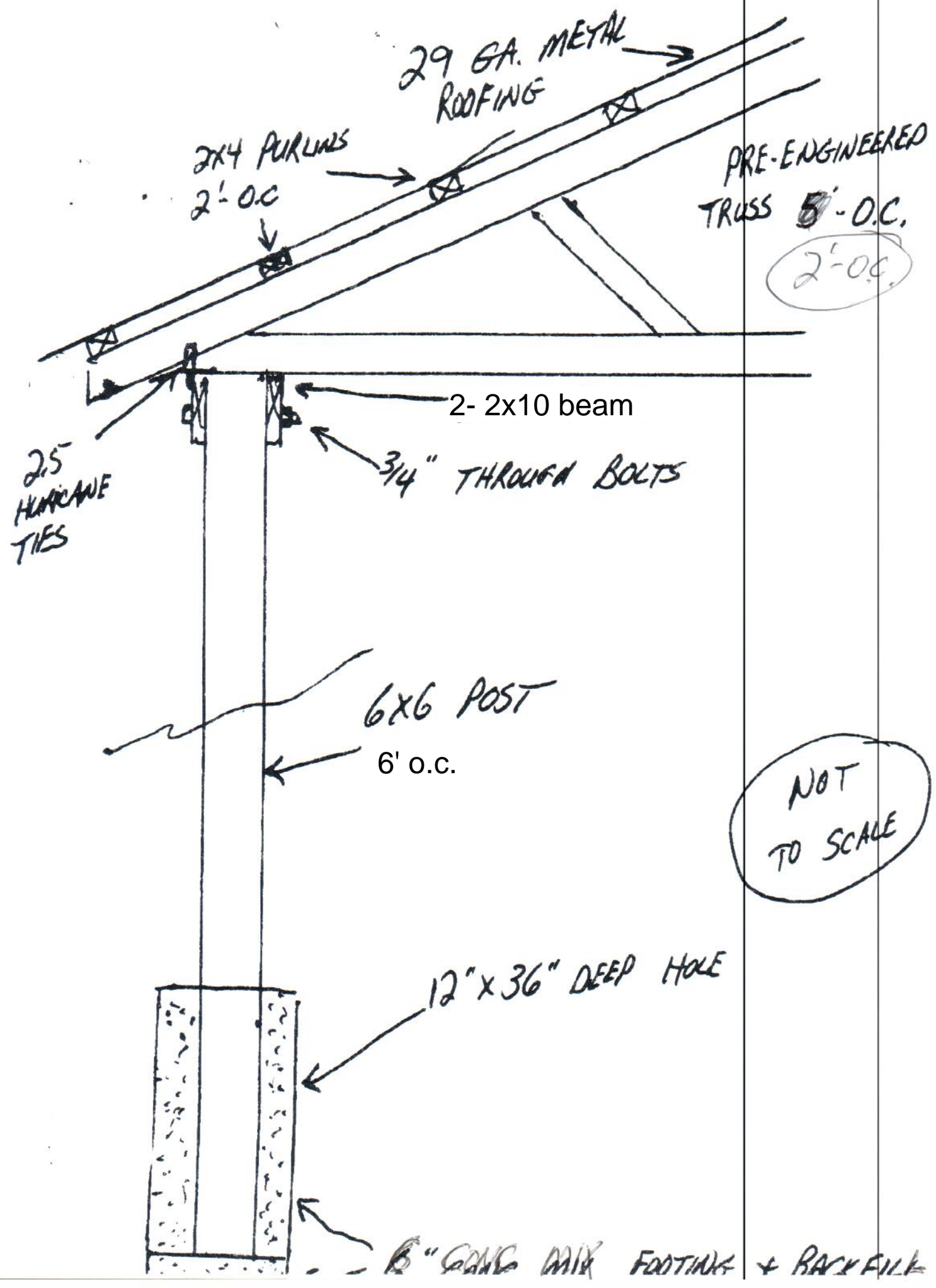
METAL
ROOF &
GABLE

NOTICE TO CONTRACTOR
All construction shall comply with current NC Building Codes and is subject to field inspection and verification.

APPROVED
Landed Building only online
Professional responsibility for
full compliance with this code.

05/01/2020





29 GA. METAL ROOFING

2x4 PURLINS
2'-0.C

PRE-ENGINEERED TRUSS 5'-0.C,
2'-0.C.

2- 2x10 beam

3/4" THROUGH BOLTS

2.5 HURRICANE TIES

6x6 POST

6' o.c.

12" x 36" DEEP HOLE

6" SAND MIX FOOTING + BACKFILL

NOT TO SCALE

Q0628 - Major

Qty: 21

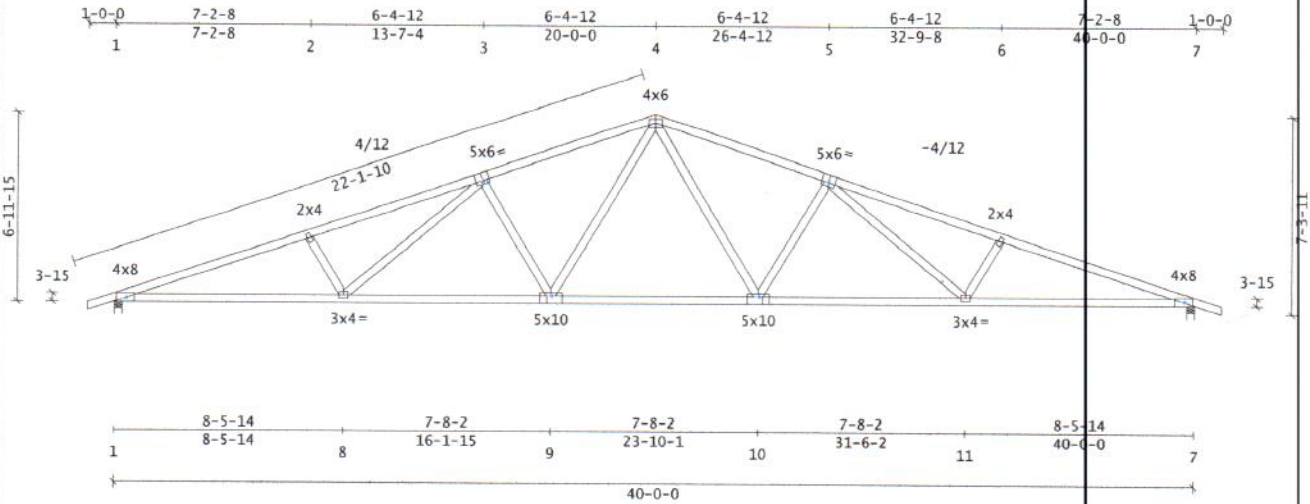
Truss: M01

Customer: David & Shelly Johnson

Agruss, Inc.
agrussterrance@yahoo.com
(803) 428-2122

SID:
TID:
Date: 04 / 02 / 20
Page: 1 of 1

Truss Mfr. Contact: Greg Jackson



Code/Design: IRC-2018/TPI-2014
PSF Live Dead Dur Factors
TC 20.0 10.0 Live Wind Snow
BC 0.0 10.0 Lum 1.25 1.60 1.15
Total 40.0 Plt 1.25 1.60 1.15
Spacing: 2-00-00 o.c. Plies: 1
Repetitive Member Increase: Yes
Green Lumber: No Wet Service: No
Fab Tolerance: 20% Creep (Kcr) = 2.0
OH Soffit Load: 2.0 psf

-----Snow Load Specs-----
ASCE7-16 Ground Snow(Sg) = 10.0 psf
Risk Cat: II Terrain Cat: B
Roof Exposure: Partially Exposed
Thermal Condition: Cold
Ventilated(1.1)
Unobstructed Slippery Roof: No
Low-Slope Minimums(Pfmin): No
Unbalanced Snow Loads: Yes
Rain Surcharge: No Ice Dam Chk: Yes

-----Wind Load Specs-----
ASCE7-16 Wind Speed(V) = 125 mph
Risk Cat: II Exposure Cat: B
Bldg Dims: L = 0.0 ft B = 0.0 ft
M.R.H(h) = 20.0 ft Kzt = 1.0
Bldg Enclosure: Enclosed
Wind DL(psf): TC = 3.0 BC = 3.0
End Vertical Exposed: L = Yes R = Yes
Wind Uplift Reporting: ASCE7 MWERS
C&C End Zone: 4-00-00

-----Additional Design Checks-----
10 psf Non-Concurrent BCLL: Yes
200 psf Limited Storage: Yes
200 lb Accessible Ceiling: No
300 lb TC Maintenance Load: No
2000 lb TC Safe Load: No
Unbalanced TCLL: Yes

Material Summary

TC 2x4 SP (ALSC6-2013) #1
BC 2x4 SP (ALSC6-2013) #1
Webs 2x4 SP (ALSC6-2013) #3/Stud

Reaction Summary

-----Reaction Summary(Lbs)-----
Jnt --X- Loc- React -Up- --Width- -Reqd -Mat PSI
1 01-12 1664 256 03-08 02-06 SPF 470
7 39-10-04 1664 256 03-08 02-06 SPF 470
Max Horiz = -69 / +69 at Joint 1

Deflection Summary

TrussSpan Limit Actual(in) Location
Vert LL L/240 L/999(-0.37) 9-10
Vert DL L/120 L/999(-0.38) 9-10
Vert CR L/180 L/635(-0.74) 9-10
Horz LL 0.75in (0.10) @Jt 7
Horz CR 1.25in (0.19) @Jt 7
Ohng CR 2L/180 L/999(-0.00) 1- 1
Ohng CR 2L/180 L/999(-0.00) 7- 7

Vert CR and Horz CR are the vertical and horizontal deflections due to live load plus the creep component of deflection due to dead load, computed as Defl_LL + (Kcr - 1) * Defl_DL in accordance with ANSI/TPI 1.

Member Forces Summary

Mem...	Ten	Comp	.CSI.
TC OH- 1	22	0	0.06
1- 2	1268	4108	0.68
2- 3	1245	3916	0.49
3- 4	1049	3027	0.43
4- 5	1048	3027	0.43
5- 6	1245	3916	0.49
6- 7	1268	4108	0.68
7-OH	22	0	0.06
BC 1- 8	3842	1167	0.86
7-11	3842	1145	0.83
8- 9	3131	949	0.71
9-10	2348	662	0.67
10-11	3131	927	0.71
Web 2- 8	194	377	0.08
3- 8	726	178	0.27
3- 9	310	687	0.37
4- 9	894	268	0.34
4-10	894	268	0.34
5-10	310	687	0.37
5-11	726	178	0.27
6-11	194	377	0.08

Loads Summary

This truss has been designed for the effects of an unbalanced top chord live load occurring at [20-00-00] using a 1.00 Full and 0.00 Reduced load factor.

See Loadcase Report for loading combinations and additional details.
Dead Loads may be slope adjusted: > 12.0/12

Notes

Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees
Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.
Designed with hybrid plate values. See TD-SUB-0001 for info.

Bracing Data Summary

-----Bracing Data-----
Chords; Sheathing required or bracing indicated:
-----Furlins-----
--oC-- --From-- --To-- #Bays
BC 5-08-00 0 40-00-00
Web Bracing -- None

Plate offsets (X, Y):

(None unless indicated below)
Jnt1(0,00-00), Jnt3(-00-05,00-15),
Jnt5(00-05,00-15), Jnt7(0,00-02),
Jnt9(0,-01-00), Jnt10(0,-01-00)

NOTICE A copy of this design shall be furnished to the erection contractor. The design of this individual truss is based on design criteria and requirements supplied by the Truss Manufacturer and relies upon the accuracy and completeness of the information set forth by the Building Designer. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. See the cover page and the "Important Information & General Notes" page for additional information. All connector plates shall be manufactured by Simpson Strong-Tie Company, Inc in accordance with ESR-2762. All connector plates are 20 gauge, unless the specified plate size is followed by a "-18" which indicates an 18 gauge plate, or "S18", which indicates a high tension 18 gauge plate.

C:\SSTClient\RG Files... Component Solutions
Truss Studio V
2019.10.1.11
Helpdesk: 1-866-252-8606
CSHelp@strongtie.com