

29 GA ROOFING + SIDING

2x4 RAFTERS
2' OC

PRE-ENGINEERED TRUSS 5' O.C.

2.5 HURRICANE TIE

6x6 POST
5' O.C.

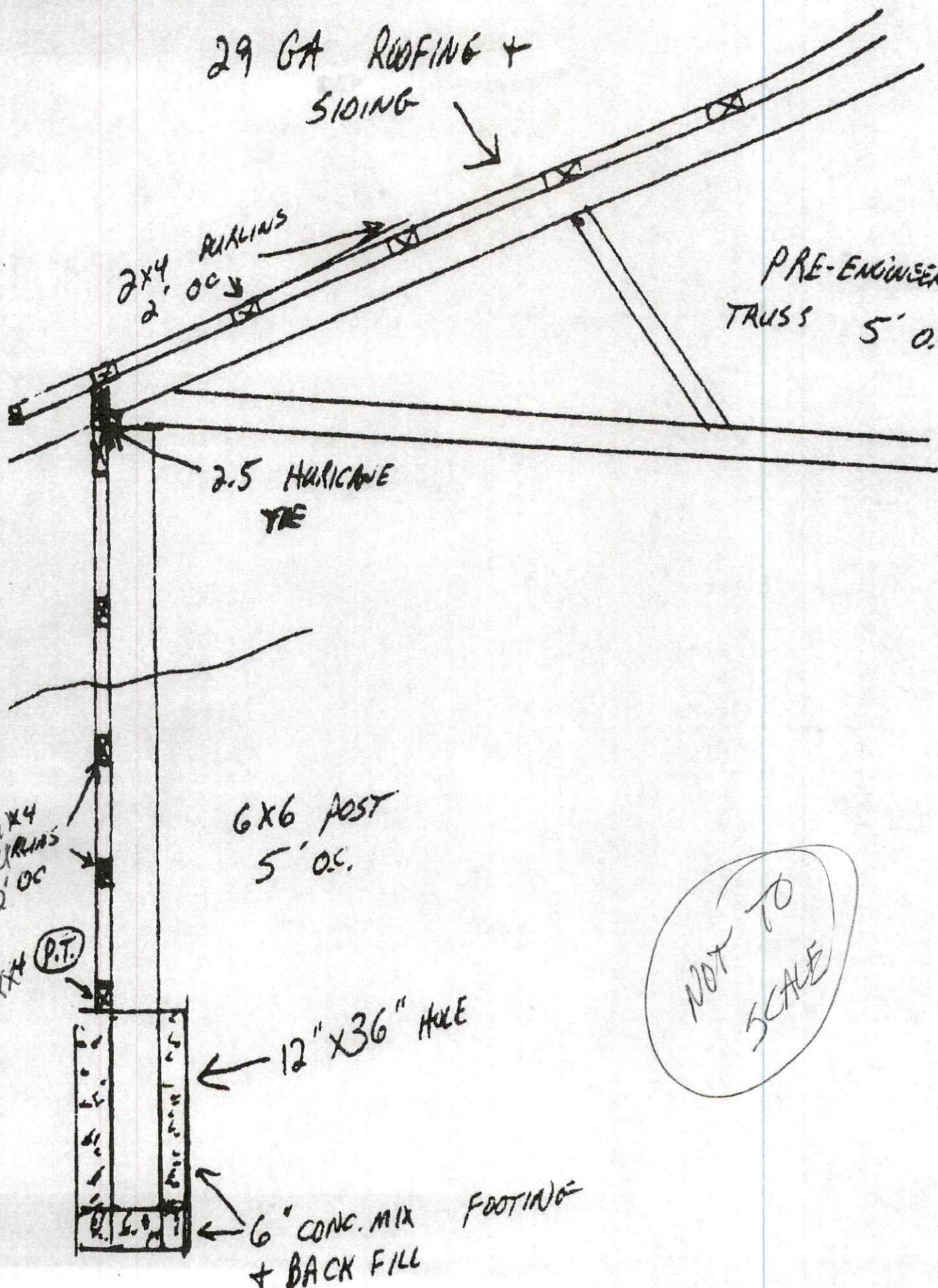
2x4 PURNALS
2' OC

2x4 (P.T.)

12" x 36" HOLE

6" CONC. MIX + BACK FILL FOOTING

NOT TO SCALE



Q0498 - Pope

Qty: 9

Truss: T01

Customer: David & Shelly Johnson

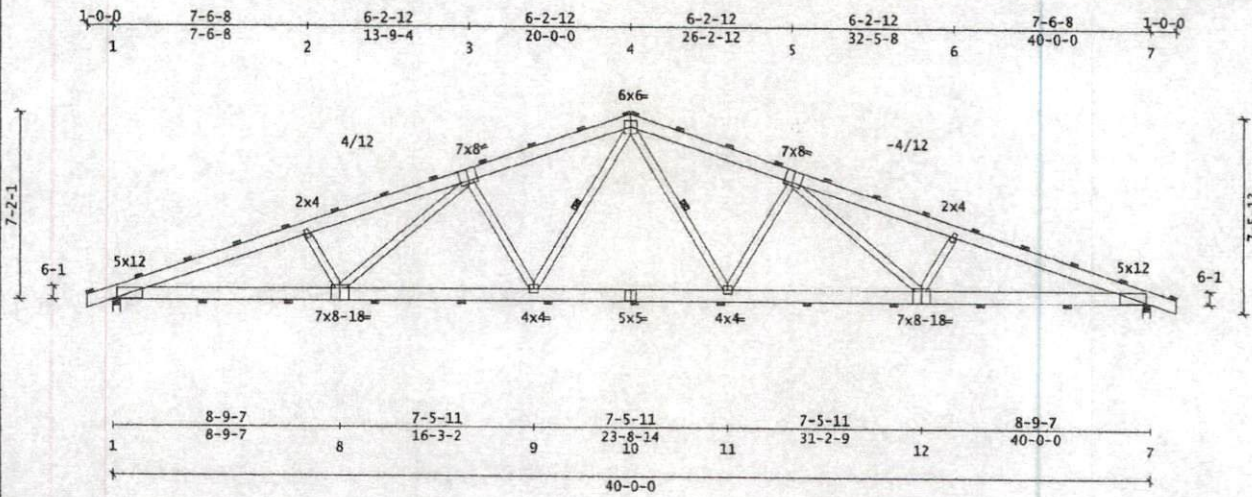
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TID: 104902

Date: 02/19/20

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Truss Mfr. Contact: Terrance Kirby



Code/Design: IBC-2015/TPI-2014
 CSF Live Dead Dur Factors
 TC 19.8 5.0 Live Wind Snow
 BC 0.0 5.0 Lum 1.25 1.60 N/A
 Total 29.8 Plt 1.25 1.60 N/A
 Spacing: 5-00-00 o.c. Plies: 1
 Repetitive Member Increase: No
 Green Lumber: No Wet Service: No
 Fab Tolerance: 204 Creep (Kcr) = 2.0
 OH Soffit Load: 2.0 psf

-----Snow Load Specs-----
 ASCE7-10 Ground Snow (Pg) = N/A
 Risk Cat: I Terrain Cat: C
 Roof Exposure: Partially Exposed
 Thermal Condition: Unheated(1.2)
 Unobstructed Slippery Roof: Yes
 Low-Slope Minimums (Pmin): No
 Unbalanced Snow Loads: No
 Rain Surcharge: No Ice Dam Chk: Yes

-----Wind Load Specs-----
 ASCE7-10 Wind Speed (V) = 120 mph
 Risk Cat: I Exposure Cat: C
 Bldg Dims: L = 0.0 ft B = 0.0 ft
 M.R.H(h) = 15.0 ft Kzt = 1.0
 Bldg Enclosure: Enclosed
 Wind DL(psf): TC = 4.0 BC = 1.0
 End Vertical Exposed: L = Yes R = Yes
 Wind Uplift Reporting: HybridMM

-----Additional Design Checks-----
 10 psf Non-Concurrent BCLL: No
 20 psf BC Limited Storage: No
 200 lb BC Accessible Ceiling: No
 300 lb TC Maintenance Load: No
 2000 lb TC Safe Load: No
 Unbalanced TCLL: Yes

Material Summary

TC 2x6 SP (ALSC6-2013) #1
 BC 2x6 SP (ALSC6-2013) #1
 Webs 2x4 SP (ALSC6-2013) #3/Stud

Member Forces Summary

Mem.	Ten	Comp	CSI
TC OH-1	47	0	0.06
1-2	4804	7504	0.92
2-3	4729	7133	0.73
3-4	3884	5557	0.67
4-5	3884	5557	0.67
5-6	4729	7133	0.73
6-7	4804	7504	0.92
7-OH	47	0	0.06
BC 1-8	7009	4290	0.98
11-12	5788	3505	0.78
Web 2-8	648	757	0.19
3-8	1253	774	0.68
3-9	1074	1343	0.70
4-9	1579	1058	0.60
4-11	1579	1058	0.60
5-11	1074	1343	0.70
5-12	1253	774	0.68
6-12	648	757	0.19

Reaction Summary

-----Reaction Summary (Lbs)-----
 Jnt --X-Loc- React -Up- --Width- --Reqd --Mat PSI
 1 01-12 3113 1055 03-08 04-07** SPF 470
 7 39-10-04 3114 1055 03-08 04-07** SPF 470
 Max Horiz = -262 / +262 at Joint: 1
 (** indicates Req'd Width > actual Width; enhancement may be required.
 See bearing block detail TD-BRG-0001A.

Loads Summary

This truss has been designed for the effects of an unbalanced top chord live load occurring at [20-00-00] using a 1.00 Full and 0.00 Reduced load factor.
 See Loadcase Report for loading combinations and additional details.
 Dead Loads may be slope adjusted: > 12.0/12

Notes

Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees
 Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints. Designed with hybrid plate values. See TD-SUB-0001 for info.
 Continuous Lateral Restraint (CLR) rows require diagonal bracing per D-MECLERBRACE. Alternatively, see D-WEBRETHORCE.
 Roof live load has been reduced in accordance Section 1607 of the IBC. Reduced live load has been computed based on an Unreduced live load of 20 psf.

Deflection Summary

TrussSpan	Limit	Actual (in)	Location
Vert LL	L/240	L/931(-0.51)	9-11
Vert DL	L/120	L/999(-0.27)	9-11
Vert CR	L/180	L/605(-0.78)	9-11
Horz LL	0.75in	(0.15)	#Jt 7
Horz CR	1.25in	(0.23)	#Jt 7
Chng CR	2L/90	L/999(0.00)	1-1
Chng CR	2L/90	L/999(0.00)	7-7

Vert CR and Horz CR are the vertical and horizontal deflections due to live load plus the creep component of deflection due to dead load, computed as Defl LL + (Kcr - 1) x Defl DL in accordance with ANST/TPI 1.

Bracing Data Summary

-----Bracing Data-----
 Chords; Sheathing required or bracing indicated:
 -----Purlins-----
 --OC-- --From-- --To-- #Bays
 TC 2-00-00 -1-00-00 41-00-00 23
 BC 3-04-00 0 40-00-00 12
 Web Bracing -- CLR -----
 Single: 9-4 4-11
 Continuous Restraint Bracing Req'd
 See BCSI-B3 3.0

Plate offsets (X, Y):

(None unless indicated below)
 Jnt3(0-00-10,01-14), Jnt5(00-10,01-14),
 Jnt8(0,-02-00), Jnt12(0,-02-00)



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 COA License Number 3449
 Date: 02/19/2020
 DE Job# 72504-W1

NOTICE A copy of this design shall be furnished to the erection contractor. The design of this individual truss is based on design criteria and requirements supplied by the Truss Manufacturer and relies upon the accuracy and completeness of the information set forth by the Building Designer. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. See the cover page and the "Important Information & General Notes" page for additional information. All connector plates shall be manufactured by Simpson Strong-Tie Company, Inc in accordance with ESR-2762. All connector plates are 20 gauge, unless the specified plate size is followed by a "-18" which indicates an 18 gauge plate, or "# 18", which indicates a high tension 18 gauge plate.



Component Solutions
 Truss Studio V
 2019.3.0.218
 Helpdesk: 1-866-252-8606
 CSHelp@strongtie.com

