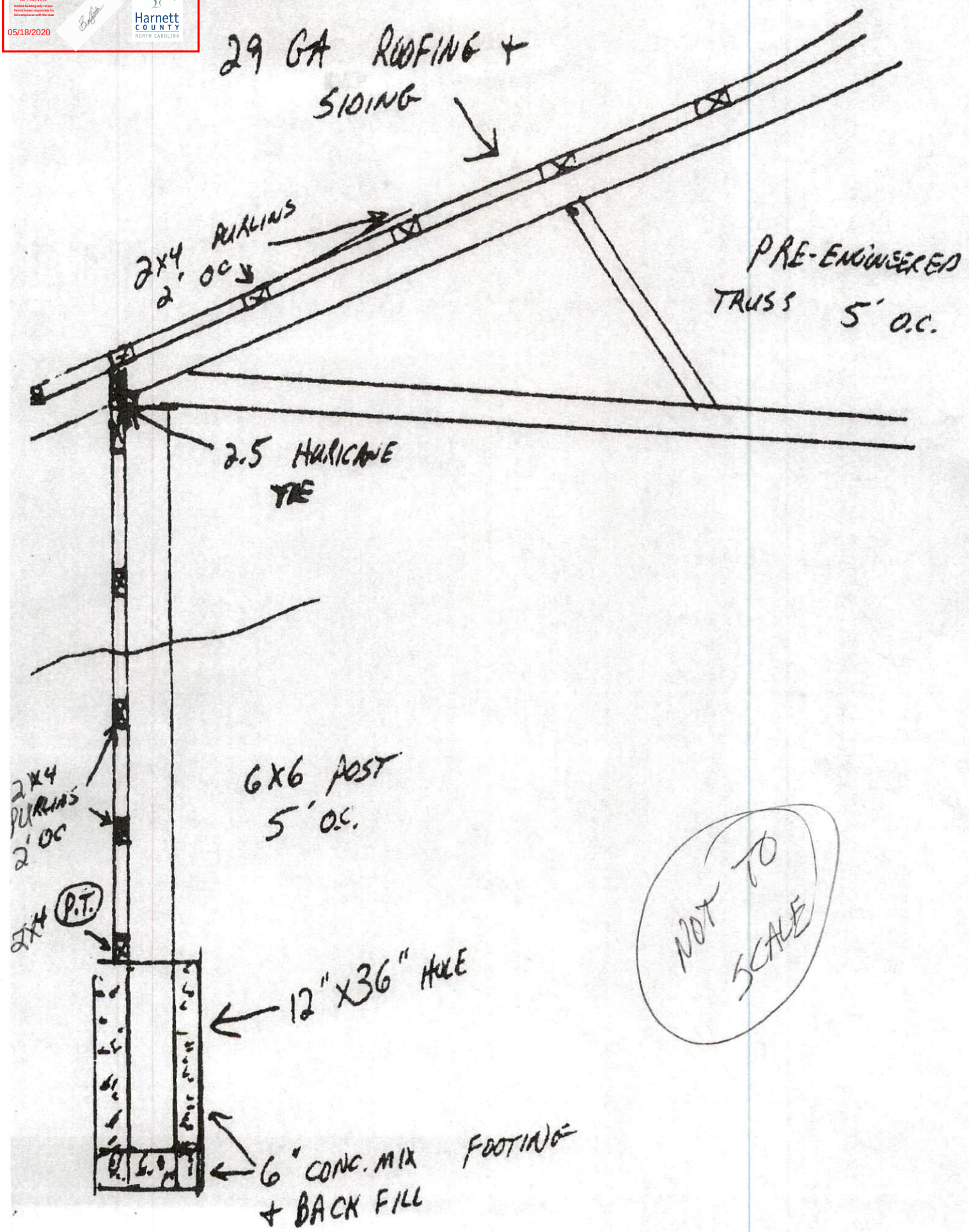
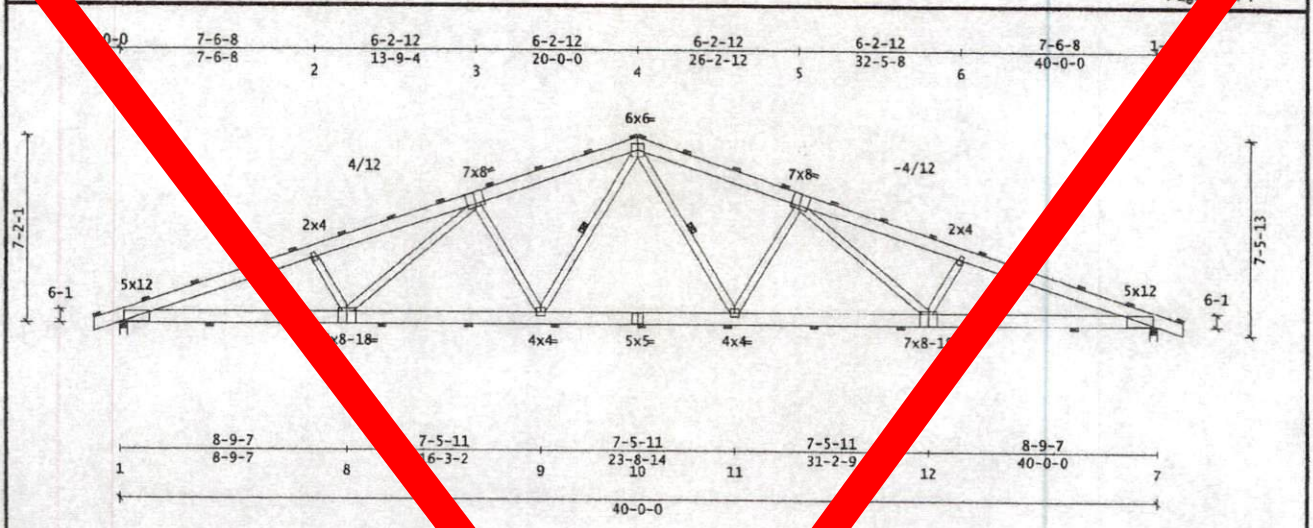


Q0498 - Pope | **Qty: 9** | **Truss: T01**

Customer: David & Shelly Johnson

SID: 0000[illegible]961
TID: 10[illegible]
Date: [illegible]9 / 20
Page: [illegible]1

Tr[illegible]ffr. Contact: Terrance Kirby



Code/Design: IBC-2015/TPI-2014

PSF	Live	Dead		Dur Factors		
TC	19.8	5.0	Live	Wind	Snow	
BC	0.0	5.0	Lum 1.25	1.60	N/A	
Total	29.8	Plt 1.25	1.60	N/A		

Spacing: 5-00-00 o.c. Plies: 1
Repetitive Member Increase: No
Green Lumber: No Wet Service: No
Fab Tolerance: 20% Creep (Kcr) = 2.0
OH Soffit Load: 2.0 psf

----------Snow L[illegible]pecs----------
ASCE7-10 Ground Sno[illegible] = N/A
Risk Cat: I Terrain[illegible] C
Roof Exposure: Partia[illegible]xposed
Thermal Condition: Unhe[illegible](1.2)
Unobstructed Slippery Ro[illegible]es
Low-Slope Minimums(Pfmin)
Unbalanced Snow Loads:
Rain Surcharge: No Ice Dam C[illegible]es

----------Win[illegible]d Specs----------
ASCE7-10 W[illegible]eed(V) = 120 mph
Risk Cat: [illegible] posure Cat: C
Bldg Dims[illegible] 0.0 ft B = 0.0 ft
M.R.H(h)[illegible]5.0 ft Kzt = 1.0
Bldg Fr[illegible]ure: Enclosed
Wind [illegible]): TC = 4.0 BC = 1.0
End [illegible]cal Exposed: L = Yes R = Yes
Win[illegible]ift Reporting: HybridMW

-------Additional Design Checks------

10 psf Non-Concurrent BCLL:	No
20 psf BC Limited Storage:	No
200 lb BC Accessible Ceiling:	No
300 lb TC Maintenance Load:	No
2000 lb TC Safe Load:	No
Unbalenced TCLL:	Yes

Material Summary

TC	2x6	SP	(ALSC6-2013)	#1
BC	2x6	SP	(ALSC6-2013)	#1
Webs	2x4	SP	(ALSC6-2013)	#3/Stud

Member Forces Summary

...Mem...		Ten	Comp	.CSI.
TC	OH- 1	47	0	0.06
	1- 2	4804	7504	0.92
	2- 3	4729	7133	0.73
	3- 4	3884	5557	0.67
	4- 5	3884	5557	0.67
	5- 6	4729	7133	0.73
	6- 7	4804	7504	0.92
	7-OH	47	0	0.06
BC	1- 8	7009	4290	0.98
	7-12	7009	4282	0.98
	8- 9	5788	3480	0.78
	9-10	4353	2451	0.57
	10-11	4353	2451	0.57
	11-12	5788	3505	0.78
Web	2- 8	648	757	0.19
	3- 8	1253	774	0.68
	3- 9	1074	1343	0.70
	4- 9	1579	1058	0.60
	4-11	1579	1058	0.60
	5-11	1074	1343	0.70
	5-12	1253	774	0.68
	6-12	648	757	0.19

Reaction Summary

----------Reaction Summary(L[illegible]----------

Jnt	--X-Loc-	React	-Up-	--Width-	[illegible]	-Mat	PSI
1	01-12	3113	1056	03-0[illegible]	*	SPF	470
7	39-10-04	3114	1056	03-[illegible]		SPF	470

Max Horiz = -262 / +262 a[illegible]nt
(**) indicates Reqd Width > a[illegible] Width[illegible]ancement may be required.

See bearing block detail T[illegible]0001A.

Loads Summary

This truss has been de[illegible]d for the effects o[illegible]unbalanced top chord live load occurring a[illegible]-00-00) using a 1.00 [illegible]nd 0.00 Reduced load factor.

See Loadcase Repo[illegible]t loading combinations and ad[illegible]nal details.
Dead Loads may b[illegible]pe adjusted: > 12.0/12

Notes

Plates desi[illegible]for Cq at 0.80 and Rotational Tolerance [illegible]0 degrees
Plates loc[illegible]at TC pitch breaks meet the prescriptive mi[illegible] size requirem[illegible] transfer unblocked diaphragm loads across th[illegible]oints.
Designe[illegible]n hybrid plate values. See TD-SUB-0001 for info.
Contin[illegible]Lateral Restraint (CLR) rows require diagonal brac[illegible]r
D-WE[illegible]ACE. Alternatively, see D-WEBRETNFORCE.
Roo[illegible] load has been reduced in accordance Section 1607 of th[illegible]
Re[illegible] live load has been computed based on an Unrreduced live l[illegible]

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/240	L/931(-0.51)	9-11
Vert DL	L/120	L/998(-0.27)	9-11
Vert CR	L/180	L/605(-0.78)	9-11
Horz LL	0.75in	(0.15)	@Jt 7
Horz CR	1.25in	(0.23)	@Jt 7
Ohng CR	2L/90	L/999(0.00)	1- 1
Ohng CR	2L/90	L/999(0.00)	7- 7

Vert CR and Horz CR are the vertical and horizontal deflections due to live load plus the creep component of deflection due to dead load, computed as Defl_LL + (Kcr - 1) x Defl_DL in accordance with ANSI/TPI 1.

Bracing Data Summary

----------Bracing Data----------
Chords; Sheathing required or bracing indicated:

----------Purlins----------

	---oc---	--From--	---To---	#Bays
TC	2-00-00	-1-00-00	41-00-00	23
BC	3-04-00	0	40-00-00	12

------- Web Bracing -- CLR ----------
Single: 9- 4 4-11
Continuous Restraint Bracing Req'd
See BCSI-B3 3.0

[illegible]ate offsets (X, Y):
[illegible]e unless indicated below)
[illegible](-00-10,01-14), Jnt5(00-10,01-14),
J[illegible],-02-00), Jnt12(0,-02-00)

DANSCO ENGINEERING, LLC
P.O. Box 3400
Apollo Beach, FL 33572
COA License Number 3449
Date: 02/19/2020
DE Job# 72504-W1

NOTICE A co[illegible]is design shall be furnished to the erection contractor. The design of this individual truss is based on design criteria and requirements supplied by the Truss M[illegible]turer and relies upon the accuracy and completeness of the information set forth by the Building Designer. A seal on this drawing indicates acceptan[illegible]ofessional engineering responsibility solely for the truss component design shown. See the cover page and the "Important Information & General Notes" [illegible]r additional information. All connector plates shall be manufactured by Simpson Strong-Tie Company, Inc in accordance with ESR-2762. All connector plates [illegible] gauge, unless the specified plate size is followed by a "-18" which indicates an 18 gauge plate, or "S# 18", which indicates a high tension 18 gauge plate.

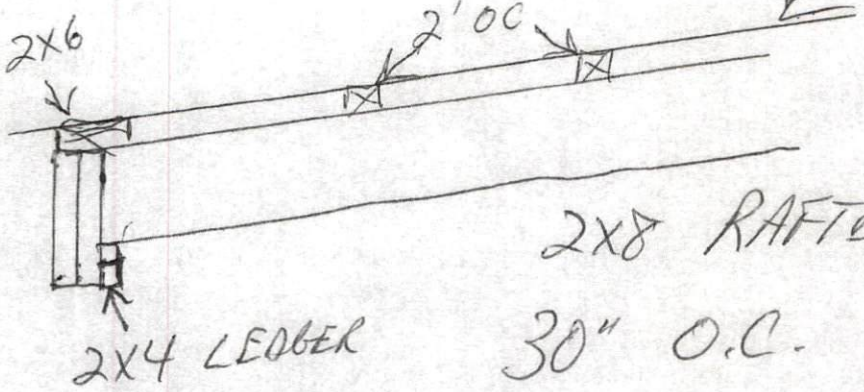
SIMPSON Strong-Tie

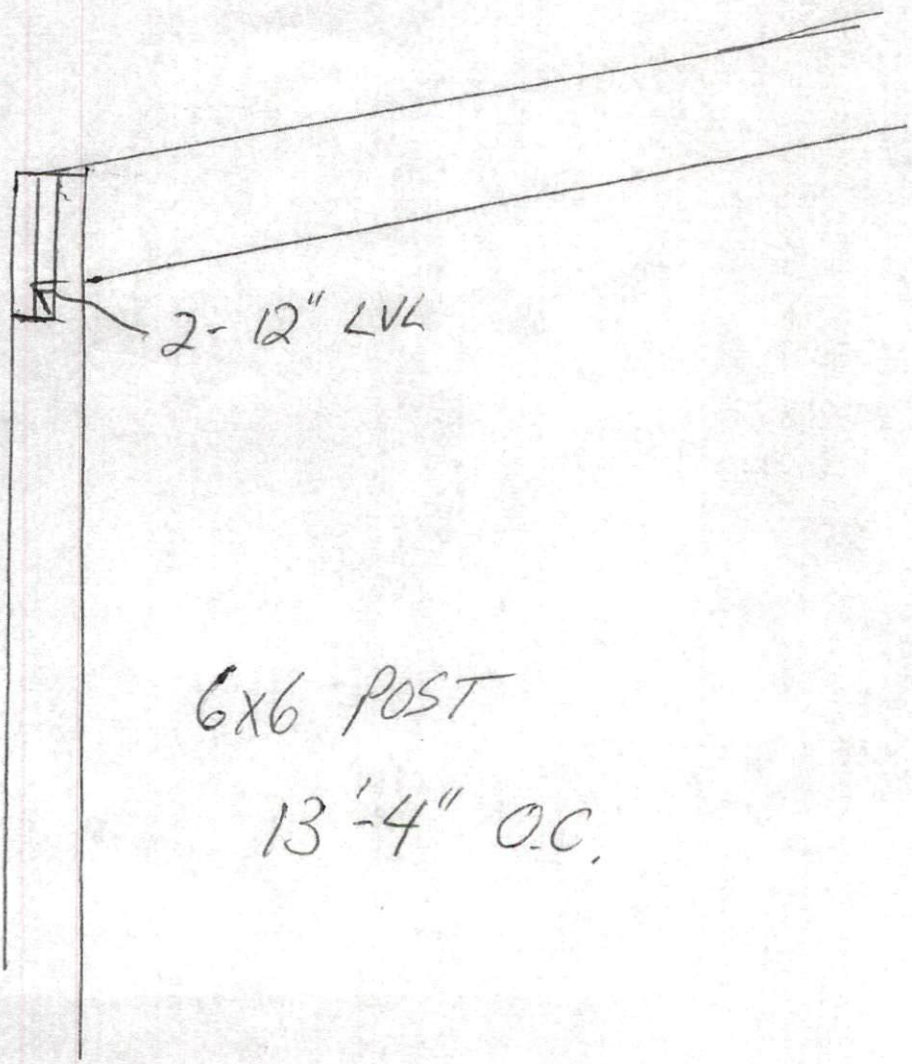
Component S[illegible]ons
Truss Studio
2019.3.0.218
Helpdesk: 1-86[illegible]-8606
CSHelp@strongtie.

EngDrwg: 2019r3RGT_L[illegible]

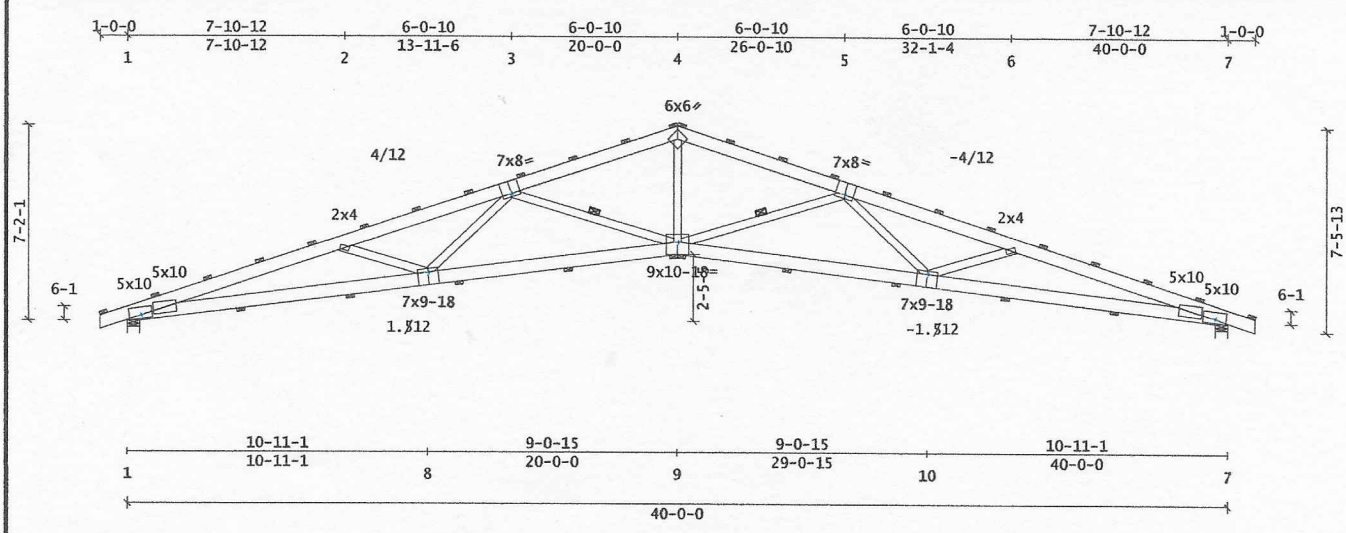
POPE

29 GA METAL ROOF → 2 LB. / SQ FT





Q0556 - cook q0556	Qty: 7	Truss: co2
Customer: David & Shelly Johnson		SID: 0000727290 TID: 107510 Date: 04 / 30 / 20
Truss Mfr. Contact: Greg Jackson		Page: 1 of 1



Code/Design: IBC-2015/TPI-2014

PSF	Live	Dead		Dur Factors		
TC	20.0*	5.0	Live	Wind	Snow	
BC	0.0	5.0	Lum 1.25	1.60	N/A	
Total		30.0	Plt 1.25	1.60	N/A	

Spacing: 5-00-00 o.c. Plies: 1
Repetitive Member Increase: No
Green Lumber: No Wet Service: No
Fab Tolerance: 20% Creep (Kcr) = 2.0
OH Soffit Load: 2.0 psf

----------Snow Load Specs----------
ASCE7-10 Ground Snow(Pg) = N/A
Risk Cat: I Terrain Cat: B
Roof Exposure: Partially Exposed
Thermal Condition: Unheated(1.2)
Unobstructed Slippery Roof: Yes
Low-Slope Minimums(Pfmin): No
Unbalanced Snow Loads: No
Rain Surcharge: No Ice Dam Chk: Yes

----------Wind Load Specs----------
ASCE7-10 Wind Speed(V) = 120 mph
Risk Cat: I Exposure Cat: B
Bldg Dims: L = 0.0 ft B = 0.0 ft
M.R.H(h) = 15.0 ft Kzt = 1.0
Bldg Enclosure: Enclosed
Wind DL(psf): TC = 4.0 BC = 1.0
End Vertical Exposed: L = Yes R = Yes
Wind Uplift Reporting: ASCE7 MWFRS
C&C End Zone: 4-00-00

-------Additional Design Checks------

10 psf Non-Concurrent BCLL:	No
20 psf BC Limited Storage:	Yes
200 lb BC Accessible Ceiling:	No
300 lb TC Maintenance Load:	No
2000 lb TC Safe Load:	No
Unbalanced TCLL:	Yes

Material Summary

TC	2x6	SP	2400/2.0	
	2x6	SP	(ALSC6-2013)	#1 3-4 4-5
BC	2x6	SP	2400/2.0	
Webs	2x4	SP	(ALSC6-2013)	#3/Stud
	2x4	SP	(ALSC6-2013)	#1 9-4

Member Forces Summary

...Mem...		Ten	Comp	.CSI.
TC	OH- 1	42	0	0.03
	1- 2	5587	11386	0.82
	2- 3	5060	10372	0.72
	3- 4	3808	7506	0.90
	4- 5	3798	7506	0.90
	5- 6	5050	10372	0.72
	6- 7	5577	11386	0.82
	7-OH	42	0	0.03
BC	1- 8	10769	5085	0.76
	7-10	10769	5066	0.75
	8- 9	9172	4232	0.62
	9-10	9172	4259	0.62
Web	2- 8	719	1069	0.29
	3- 8	1092	388	0.41
	3- 9	1336	2281	0.61
	4- 9	4034	1906	0.61
	5- 9	1336	2281	0.61
	5-10	1092	388	0.41
	6-10	720	1069	0.29

Reaction Summary

----------Reaction Summary(Lbs)----------

Jnt	--X-Loc-	React	-Up-	--Width-	-Reqd	-Mat	PSI
1	0	3113	539	05-08	04-09	SPF	453
7	40-00-00	3114	539	05-08	04-09	SPF	453

Max Horiz = -188 / +188 at Joint 1

Loads Summary

This truss has been designed for the effects of an unbalanced top chord live load occurring at [20-00-00] using a 1.00 Full and 0.00 Reduced load factor.
*20 psf Live Load has been reduced for pitch and/or area reductions allowed by the selected code to 19.8 psf.

See Loadcase Report for loading combinations and additional details.
Dead Loads may be slope adjusted: > 12.0/12

Notes

Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees
Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.
Continuous Lateral Restraint (CLR) rows require diagonal bracing per D-WEBCLRBRACE. Alternatively, see D-WEBREINFORCE.

Dansco Engineering, PA
License number C-3462
Date: 05/01/2020
DE Job# 72953-W2

Deflection Summary

Truss	Span	Limit	Actual(in)	Location
Vert LL		L/240	L/472(-0.99)	8- 9
Vert DL		L/120	L/864(-0.54)	8- 9
Vert CR		L/180	L/305(-1.53)	8- 9
Horz LL		0.75in	(0.44)	@Jt 7
Horz CR		1.25in	(0.67)	@Jt 7
Ohng CR		2L/180	L/999(-0.00)	1- 1
Ohng CR		2L/180	L/999(-0.00)	7- 7

Vert CR and Horz CR are the vertical and horizontal deflections due to live load plus the creep component of deflection due to dead load, computed as Defl_LL + (Kcr - 1) x Defl_DL in accordance with ANSI/TPI 1.

Bracing Data Summary

----------Bracing Data----------
Chords; Sheathing required or bracing indicated:

----------Purlins----------

	---oc---	--From--	---To---	#Bays
TC	2-00-00	-1-00-00	41-00-00	23
BC	4-00-00	0	40-00-00	10

------- Web Bracing -- CLR ----------
Single: 3- 9 9- 5
Continuous Restraint Bracing Req'd
See BCSI-B3 3.0

Plate offsets (X, Y):

(None unless indicated below)
Jnt1(-00-02,00-15), Jnt3(-00-10,01-14), Jnt5(00-10,01-14), Jnt7(00-02,00-15), Jnt8(00-04,-02-00), Jnt9(0,-00-13), Jnt10(-00-04,-02-00)

SIMPSON Strong-Tie

Component Solutions
Truss Studio V
2019.10.1.11
Helpdesk: 1-866-252-8606
CSHelp@strongtie.com

EngDrwg: 2019r3RGT_Eng