BUILDER/CONTRACTOR RESPONSIBILITIES

Drawing Validity – These drawings, supporting structural calculations and design certification are based on the order documents as of the date of these drawings. These documents describe the material supplied by the manufacturer as of the date of these drawings. Any changes to the order documents after the date on these drawings may void these drawings, supporting structural calculations and design certification. The Builder/Contractor is responsible for notifying the building authority of all changes to the order documents which result in changes to the drawings, supporting structural calculations and design certification.

Builder Acceptance of Drawings – Approval of the manufacturer's drawings and design data affirms that the manufacturer has correctly interpreted and applied the requirements of the order documents and constitutes Builder/Contractor acceptance of the manufacturer's interpretations of the order documents and standard product specifications, including its design, fabrication and quality criteria standards and tolerances. (AISC code of standard practice Sept 86 Section 4.2.1) (Mar 05 Section 4.4.1)

Code Official Approval – It is the responsibility of the Builder/Contractor to ensure that all project plans and specifications comply with the applicable requirements of any governing building authority. The Builder/Contractor is responsible for securing all required approvals and permits from the appropriate agency as required.

Builder is responsible for State, Federal and OSHA safety compliance – The Builder/Contractor is responsible for applying and observing all pertinent safety rules and regulations and OSHA standards as applicable.

Building Erection – The Builder/Contractor is responsible for all erection of the steel and associated work in compliance with the Metal Building Manufacturers drawings. Temporary supports, such as temporary guys, braces, false work or other elements required for erection will be determined, furnished and installed by the erector. (AISC Code of Standard Practice Sept 86 Section 7.9.1) (Mar 05 Section 7.10.3)

Discrepancies – Where discrepancies exist between the Metal Building plans and plans for other trades, the Metal Building plans will govern. (AISC Code of Standard Practice Sept 86 Section 3.3) (Mar 05 Section 3.3)

Materials by Others – All interface and compatibility of any materials not furnished by the manufacturer are the responsibility of and to be coordinated by the Builder/Contractor or A/E firm. Unless specific design criteria concerning any interface between materials if furnished as a part of the order documents, the manufacturers assumptions will govern.

Modification of the Metal Building from Plans – The Metal Building supplied by the manufacturer has been designed according to the Building Code and specifications and the loads shown on this drawing. Modification of the building configuration, such as removing wall panels or braces, from that shown on these plans could affect the structural integrity of the building. The Metal Building Manufacturer or a Licensed Structural Engineer should be consulted prior to making any changes to the building configuration shown on these drawings. The Metal Building Manufacturer will assume no responsibility for any loads applied to the building not indicated on these drawings.

Foundation Design – The Metal Building Manufacturer is not responsible for the design, materials and workmanship of the foundation. Anchor rod plans prepared by the manufacturer are intended to show only location, diameter and projection of the anchor rods required to attach the Metal Building System to the foundation. It is the responsibility of the end customer to ensure that adequate provisions are made for specifying rod embedment, bearing values, tie rods and or other associated items embedded in the concrete foundation, as well as foundation design for the loads imposed by the Metal Building System, other imposed loads, and the bearing capacity of the soil and other conditions of the building site. (MBMA 06 Sections 3.2.2 and A3)

PROJECT NOTES

Material properties of steel bar, plate, and sheet used in the fabrication of built-up structural framing members conform to ASTM A529, ASTM A572, ASTM A1011 SS, or ASTM A1011 HSLAS with a minimum yield point of 50 ksi. Material properties of hot rolled structural shapes conform to ASTM A992, ASTM A529, or ASTM A572 with a minimum specified yield point of 50 ksi. Hot rolled angles, or other than flange braces, conform to ASTM 36 minimum. Hollow structural shaped conform to ASTM A500 grade b, minimum yield point is 42 ksi for round HSS and 46 ksi for rectangular HSS. Material properties of cold form light gage steel members conform to the requirements of ASTM A1011 SS Grade 55 or ASTM A1011 HSLAS Class 1 Grade 55, with a minimum yield point of 55 ksi.

The manufacturer does not assume any responsibility for the erection nor field supervision of the structure and or any special inspections that may be required by the local building authority during erection (including inspection of the high strength bolts or field welds) as required during erection. The coordination and the costs associated for setting up and Special Inspections are the responsibility of the Erector, Owner, Architect, or Engineer of Record.

Design is based upon the more severe loading of either the roof snow load or the roof live load.

Loads, as noted, are given within order documents and are applied in general accordance with the applicable provisions of the model code and/or specification indicated. Neither the manufacture nor the certifying engineer declares or attests that the loads as designated are proper for the local provisions that may apply or for site specific parameters. The manufacturer's Engineer's certification is limited to design loads supplied by an Architect and/or engineer of record for the overall construction project.

This project is designed using manufacture's standard serviceability standards. Generally this means that all stresses and deflections are within typical performance limits for normal occupancy and standard metal building products. If special requirements for deflections and vibrations must be adhered to, then they must be clearly stated in the contract documents.

This metal building system is designed as enclosed. All exterior components (i.e. doors, windows, vents, etc.) must be designed to withstand the specified wind loading for the design of components and cladding in accordance with the specified building code. Doors are to be closed when a maximum of 50% of design wind velocity is reached.

DESIGN LOADING

THIS STRUCTURE IS DESIGNED UTILIZING THE LOADS INDICATED AND APPLIED AS REQUIRED BY:

NCBC 18

THE BUILDER IS TO CONFIRM THAT THESE LOADS COMPLY WITH THE REQUIREMENTS OF THE LOCAL BUILDING DEPARTMENT.

FRAME / ROOF DEAD LOAD
- SUPERIMPOSED: 1.870 PSF
- COLLATERAL (LIGHTS): 0.5 PSF

FRAME / ROOF LIVE LOAD: 12 /20.00 PSF
- RISK CATEGORY: II – Normal

SNOW LOAD
- GROUND SNOW LOAD (Pg): 15.00 PSF
- SNOW LOAD IMPORTANCE FACTOR (Is): 1.0000
- FLAT ROOF SNOW LOAD (Pf): 10.5 PSF
- SNOW EXPOSURE FACTOR (Ce): 1.0
- THERMAL FACTOR (Ct): 1.00

WIND LOAD
- ULTIMATE WIND SPEED: 120 MPH
- NOMINAL WIND SPEED (VASD): 93 MPH (IBC Section 1609.3.1)
- WIND EXPOSURE CATEGORY: B
- TOPOGRAPHICAL FACTOR: 1.0
- INTERNAL PRESSURE COEFFICIENT (GCpi): 0.18 /–0.18
- ZONE 4, COMPONENT WIND LOAD ≤ 10FT²: 25.882 PSF PRESSURE –28.076 PSF SUCTION
- ZONE 5, COMPONENT WIND LOAD < 10FT²: 25.882 PSF PRESSURE –34.590 PSF SUCTION
- ZONES PER ASCE 7-10; FIG. 30.4-1
- ZONES PRESSURES SHOWN ARE UN-FACTORED

RAIN INTENSITY
- 5-MINUTE DURATION, 5-YEAR RECURRENCE (I1): 7.1000 IN/HOUR

SEISMIC LOAD
- SEISMIC IMPORTANCE FACTOR (Ie): 1.00
- S_S 0.2471 S_{DS} 0.2635
- S1 0.0907 S_{D1} 0.1440
- SITE CLASS: D
- SEISMIC DESIGN CATEGORY: C

ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE

	TRANSVERSE	LONGITUDINAL FRONT	LONGITUDINAL BACK
BASIC FORCE RESISTING SYSTEM*	H	H	H
RESPONSE MODIFICATION COEFFICIENT(R)	3	3	3
SYSTEM OVER-STRENGTH FACTOR(Ω_0)	2.5000	2.5000	2.5000
SEISMIC RESPONSE COEFFICIENT(C_s)	0.088	0.088	0.088
BLDG DESIGN BASE SHEAR (V)	1.66 (k)	1.67 (k)	

THE TRANSVERSE DIRECTION IS PARALLEL TO THE RIGID FRAMES
THE LONGITUDINAL DIRECTION IS PERPENDICULAR TO THE RIGID FRAMES

BASIC FORCE RESISTING SYSTEM*
C4. STEEL ORDINARY MOMENT FRAME
B3. STEEL ORDINARY CONCENTRIC BRACED FRAMES
H. STRUCTURAL STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE
G2. INVERTED PENDULUM SYSTEMS CANTILEVERED COLUMN SYSTEMS

DRAWING INDEX

DRAWING STATUS

☐ FOR APPROVAL
THESE DRAWINGS, BEING FOR APPROVAL, ARE BY DEFINITION NOT FINAL, AND ARE FOR CONCEPTUAL REPRESENTATION ONLY. THEIR PURPOSE IS TO CONFIRM PROPER INTERPRETATION OF THE PROJECT DOCUMENTS. ONLY DRAWINGS ISSUED "FOR ERECTOR INSTALLATION" CAN BE CONSIDERED AS COMPLETE.

☒ FOR CONSTRUCTION PERMIT
THESE DRAWINGS, BEING FOR PERMIT, ARE BY DEFINITION NOT FINAL. ONLY DRAWINGS ISSUED "FOR ERECTOR INSTALLATION" CAN BE CONSIDERED AS COMPLETE.

☐ FOR ERECTOR INSTALLATION
FINAL DRAWINGS FOR CONSTRUCTION.

FOR QUESTIONS OR ASSISTANCE CONCERNING ERECTION CALL:
817–462–4029
MONDAY – FRIDAY 7:30AM TO 5:00PM

ENGINEERING SEAL

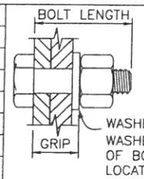
THIS CERTIFICATION COVERS PARTS MANUFACTURED AND DELIVERED BY THE MANUFACTURER ONLY, AND EXCLUDES PARTS SUCH AS DOORS, WINDOWS, FOUNDATION DESIGN AND ERECTION OF THE BUILDING.

THESE DRAWINGS AND THE METAL BUILDING SYSTEM THEY REPRESENT ARE THE PRODUCT OF AN AFFILIATE OF NCI GROUP, INC. – 10943 N. SAM HOUSTON PARKWAY W., HOUSTON, TX 77064. THE PROFESSIONAL ENGINEER WHOSE SEAL APPEARS HEREON IS EMPLOYED BY AN AFFILIATE OF NCI GROUP, INC. AND IS NOT THE ENGINEER-OF-RECORD FOR THE OVERALL PROJECT.

NORTH CAROLINA PROFESSIONAL SEAL 023748 ENGINEER MICHAEL W. CUSTER
1-17-20

THIS DOCUMENT WAS PRODUCED BY AND/OR UNDER MY DIRECT SUPERVISION

1/2"ø A325 BOLT GRIP TABLE	
GRIP	LENGTH
0 TO 9/16"	1 1/4" F.T.
Over 9/16" TO 1 1/16"	1 3/4" F.T.
Over 1 1/16" TO 1 5/16"	2"
Over 1 5/16" TO 1 9/16"	2 1/4"
Over 1 9/16" TO 1 13/16"	2 1/2"
Over 1 13/16" TO 2 1/16"	2 3/4"
LOCATIONS OF BOLTS LONGER THAN 2 3/4" NOTED ON ERECTION DRAWINGS	
F.T. DENOTES FULLY THREADED	



NOTE:
FULL THREAD ENGAGEMENT IS DEEMED TO HAVE BEEN MET WHEN THE END OF THE BOLT IS FLUSH WITH THE FACE OF THE NUT.

WASHER REQUIRED ONLY WHEN SPECIFIED. WASHER MAY BE LOCATED UNDER HEAD OF BOLT, UNDER NUT, OR AT BOTH AT LOCATIONS NOTED ON ERECTION DRAWINGS.

ADD 5/32" FOR EACH WASHER TO MATERIAL THICKNESS TO DETERMINE GRIP.

Rev. 4/4/2017

BUILDING SIZE: 40'–0" x 75'–0" x 16'–0" 4.0:12

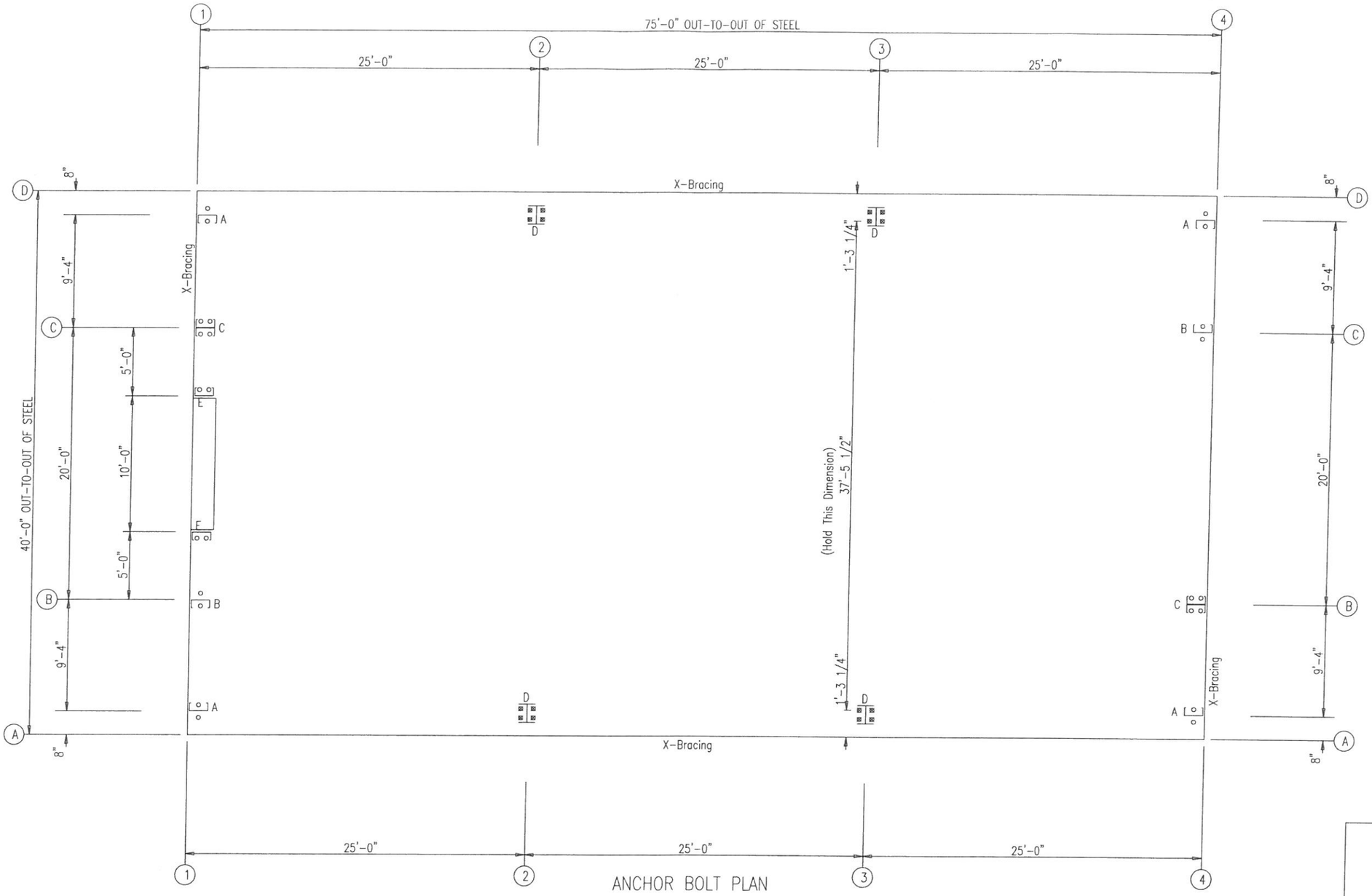
ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
A	1/13/20	FOR CONSTRUCTION PERMIT	FXD	HPD	CM

BUILDING IS MANUFACTURED BY SW STEEL FACTORY

MICHAEL W. CUSTER, P.E.
642 OAKBEND DRIVE
COPPEL TX. 75019
PH. 972–571–7082

PROJECT: THOMAS SANDERS
CUSTOMER: THOMAS SANDERS OWNER: THOMAS SANDERS
LOCATION: LILLINGTON, NC 27546

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	1/13/20	N.T.S.	1	A	17-B-48481	C1	A



ANCHOR BOLT PLAN

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	1/13/20	FOR ERECTOR INSTALLATION	FXD	HPD	CM

BUILDING IS MANUFACTURED BY SW STEEL FACTORY

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CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	1/13/20	N.T.S.	1	A	17-B-48481	F1	0

NORTH CAROLINA PROFESSIONAL SEAL 023748 ENGINEER MICHAEL W. CUSTER

THIS DOCUMENT WAS PRODUCED BY AND/OR UNDER MY DIRECT SUPERVISION

GENERAL NOTES

1. THE REACTIONS PROVIDED ARE BASED ON THE ORDER DOCUMENTS AT THE TIME OF MAILING. ANY CHANGES TO BUILDING LOADS OR DIMENSIONS MAY CHANGE THE REACTIONS. THE REACTIONS WILL BE SUPERSEDED AND VOIDED BY ANY FUTURE MAILING.
2. REACTIONS ARE PROVIDED AS UN-FACTORED FOR EACH LOAD GROUP APPLIED TO THE COLUMN. THE FOUNDATION ENGINEER WILL APPLY THE APPROPRIATE LOAD FACTORS AND COMBINE THE REACTIONS IN ACCORDANCE WITH THE BUILDING CODE AND DESIGN SPECIFICATIONS TO DETERMINE BEARING PRESSURES AND CONCRETE DESIGN. THE FACTORS APPLIED TO LOAD GROUPS FOR THE STEEL COLUMN DESIGN MAY BE DIFFERENT THAN THE FACTORS USED IN THE FOUNDATION DESIGN.
3. THE MANUFACTURER DOES NOT PROVIDE "MAXIMUM" LOAD COMBINATION REACTIONS. HOWEVER, THE INDIVIDUAL LOAD REACTIONS PROVIDED MAY BE USED BY THE FOUNDATION ENGINEER TO DETERMINE THE APPLICABLE LOAD COMBINATIONS FOR HIS/HER DESIGN PROCEDURES AND ALLOW FOR AN ECONOMICAL FOUNDATION DESIGN
4. THE METAL BUILDING MANUFACTURER IS RESPONSIBLE FOR THE DESIGN OF THE ANCHOR BOLT DIAMETER ONLY TO PERMIT THE TRANSFER OF FORCES BETWEEN THE BASE PLATE AND THE ANCHOR BOLT IN SHEAR, BEARING AND TENSION, BUT IS NOT RESPONSIBLE FOR THE ANCHOR BOLT EMBEDMENT FOR TRANSFER OF FORCES TO THE FOUNDATION. THE METAL BUILDING MANUFACTURER DOES NOT DESIGN AND IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL AND CONSTRUCTION OF THE FOUNDATION EMBEDMENTS. THE END USE CUSTOMER SHOULD ASSURE HIMSELF THAT ADEQUATE PROVISIONS ARE MADE IN THE FOUNDATION DESIGN FOR LOADS IMPOSED BY COLUMN REACTIONS OF THE BUILDING, OTHER IMPOSED LOADS, AND BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE. IT IS RECOMMENDED THAT THE ANCHORAGE AND FOUNDATION OF THE BUILDING BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER EXPERIENCED IN THE DESIGN OF SUCH STRUCTURES, (SECTION A3 MBMA 2006 METAL BUILDING SYSTEMS MANUAL)
5. BOTTOM OF ALL BASE PLATES ARE AT THE SAME ELEVATION. (UNLESS NOTED)
6. ANCHOR RODS ARE ASTM F1554 GRADE 36 MATERIAL UNLESS NOTED OTHERWISE.

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Snow Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert	Wind_Left2 Horz	Wind_Left2 Vert	Wind_Right2 Horz	Wind_Right2 Vert	Wind Press Horz
1	D	0.1	0.0	0.5	0.3	2.3	−5.2	0.0	4.1	2.3	−4.6	0.0	4.9	0.0
1	C	0.8	0.1	4.5	2.4	0.0	−0.5	2.3	−8.5	0.0	0.8	2.3	−7.2	−2.7
1	B	0.6	0.1	4.5	2.4	0.0	−3.0	0.0	−5.2	0.0	−1.7	0.0	−3.9	−2.7
1	A	0.1	0.0	0.5	0.3	0.0	−1.4	0.0	−0.5	0.0	−0.7	0.0	0.2	0.0

Frm Line	Col Line	Wind Suct Horz	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert	Seis_Left Horz	Seis_Left Vert	Seis_Right Horz	Seis_Right Vert	E1UNB_SL_L- Horz	E1UNB_SL_L- Vert	E1UNB_SL_R- Horz	E1UNB_SL_R- Vert
1	D	0.0	0.0	0.8	0.8	−2.1	0.4	−0.7	0.0	0.8	0.0	0.2	0.0	−0.1
1	C	3.0	0.8	−6.5	0.0	−1.8	0.0	0.7	0.3	−0.8	0.0	2.7	0.0	1.3
1	B	3.0	0.0	−3.4	0.0	−4.6	0.0	0.0	0.0	0.0	0.0	1.3	0.0	2.7
1	A	0.0	0.0	−0.4	0.0	−1.1	0.0	0.0	0.0	0.0	0.0	−0.1	0.0	0.2

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Snow Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert	Wind_Left2 Horz	Wind_Left2 Vert	Wind_Right2 Horz	Wind_Right2 Vert	Wind Press Horz
4	A	0.1	0.0	0.5	0.3	2.3	−5.2	0.0	4.1	2.3	−4.6	0.0	4.9	0.0
4	B	0.8	0.1	4.5	2.4	0.0	−0.5	2.3	−8.5	0.0	0.8	2.3	−7.2	−2.7
4	C	0.6	0.1	4.5	2.4	0.0	−3.0	0.0	−5.2	0.0	−1.7	0.0	−3.9	−2.7
4	D	0.1	0.0	0.5	0.3	0.0	−1.4	0.0	−0.5	0.0	−0.7	0.0	0.2	0.0

Frm Line	Col Line	Wind Suct Horz	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert	Seis_Left Horz	Seis_Left Vert	Seis_Right Horz	Seis_Right Vert	E2UNB_SL_L- Horz	E2UNB_SL_L- Vert	E2UNB_SL_R- Horz	E2UNB_SL_R- Vert
4	A	0.0	0.0	0.8	0.8	−2.1	0.4	−0.7	0.0	0.8	0.0	0.2	0.0	−0.1
4	B	3.0	0.8	−6.5	0.0	−1.8	0.0	0.7	0.3	−0.8	0.0	2.7	0.0	1.3
4	C	3.0	0.0	−3.4	0.0	−4.6	0.0	0.0	0.0	0.0	0.0	1.3	0.0	2.7
4	D	0.0	0.0	−0.4	0.0	−1.1	0.0	0.0	0.0	0.0	0.0	−0.1	0.0	0.2

ENDWALL COLUMN: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc._Bolt Qty	Anc._Bolt Dia	Base_Plate (in) Width	Base_Plate (in) Length	Base_Plate (in) Thick	Grout (in)
1	D	2	0.625	7.000	8.000	0.250	0.0
1	C	4	0.625	7.000	8.000	0.250	0.0
1	B	2	0.625	7.000	8.000	0.250	0.0
1	A	2	0.625	7.000	8.000	0.250	0.0
4	A	2	0.625	7.000	8.000	0.250	0.0
4	B	4	0.625	7.000	8.000	0.250	0.0
4	C	2	0.625	7.000	8.000	0.250	0.0
4	D	2	0.625	7.000	8.000	0.250	0.0

NOTES FOR REACTIONS

BUILDING REACTIONS ARE BASED ON THE FOLLOWING BUILDING DATA:

WIDTH (FT)	= 40
LENGTH (FT)	= 75
EAVE HEIGHT (FT)	= 16 / 16
ROOF SLOPE (rise/12)	= 4.0:12 / 4.0:12
DEAD LOAD (psf)	= 1.870
COLLATERAL LOAD (psf)	= 0.5
ROOF LIVE LOAD (psf)	= 20.00
FRAME LIVE LOAD (psf)	= 12
ROOF SNOW LOAD (psf)	= 10.5
GROUND SNOW LOAD (psf)	= 15.0000
WIND SPEED (MPH)	= 120
WIND CODE	= IBC 15
EXPOSURE	= B
CLOSED/OPEN	= Closed
IMPORTANCE – WIND	= 1.00
IMPORTANCE – SEISMIC	= 1.00
SEISMIC ZONE	= C

REACTION KEY:

WIND Left/Right 1 = (with +GCpi Internal Pressure)
WIND Left/Right 2 = (with -GCpi Internal Pressure)
Wind_Long 1 = Wind Load Case B at Left EW
Wind_Long 2 = Wind Load Case B at Right EW
MIN_SNOW = Minimum Snow (Pm) per code
E1UNB_SL_L = Endwall Unbalanced Snow Left
E1UNB_SL_R = Endwall Unbalanced Snow Right
F1UNB_SL_L = Rigid Frame Unbalanced Snow Left
F1UNB_SL_R = Rigid Frame Unbalanced Snow Right

ANCHOR BOLT SUMMARY

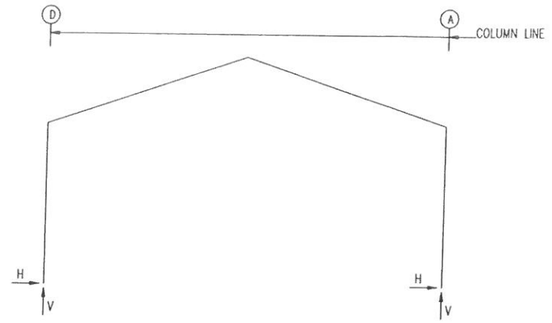
	Qty	Locate	Dia (in)	Type	Proj (in)
○	4	Jamb	5/8"	F1554	2.00
○	20	Endwall	5/8"	F1554	2.00
☒	16	Frame	3/4"	F1554	2.50

BUILDING BRACING REACTIONS

Wall Loc	Wall Line	Col Line	Reactions in plane of wall ± Reactions(k) Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Panel_Shear (lb/ft) Wind	Panel_Shear (lb/ft) Seis
L_EW	1	D,C	Bracing, see EW reactions					
F_SW	A	2,3	3.2	*	0.8	*		
R_EW	4	A,B	Bracing, see EW reactions					
B_SW	D	3,2	3.2	*	0.8	*		

*See RF reactions table for vertical and horizontal reactions in plane of the rigid frame.

FRAME LINES: 2 3



RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc._Bolt Qty	Anc._Bolt Dia	Base_Plate (in) Width	Base_Plate (in) Length	Base_Plate (in) Thick	Grout (in)
2*	D	4	0.750	6.000	9.500	0.375	0.0
2*	A	4	0.750	6.000	9.500	0.375	0.0

2* Frame lines: 2 3

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Snow Horiz	Snow Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert
2*	D	0.4	1.4	0.1	0.3	1.8	6.0	1.6	5.3	−5.5	−10.3	2.2	−6.2
2*	A	−0.4	1.4	−0.1	0.3	−1.8	6.0	−1.6	5.2	−2.2	−6.2	5.5	−10.3

Frame Line	Column Line	Wind_Left2 Horiz	Wind_Left2 Vert	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	Seismic_Left Horiz	Seismic_Left Vert	Seismic_Right Horiz	Seismic_Right Vert
2*	D	−6.2	−6.4	1.6	−2.3	1.4	−9.9	0.2	−9.3	−0.2	−0.2	0.2	0.2
2*	A	−1.6	−2.3	6.2	−6.4	−0.2	−9.3	−1.4	−9.9	−0.2	0.2	0.2	−0.2

Frame Line	Column Line	Seismic_Long Horiz	Seismic_Long Vert	F1UNB_SL_L- Horiz	F1UNB_SL_L- Vert	F1UNB_SL_R- Horiz	F1UNB_SL_R- Vert
2*	D	0.0	−0.5	1.3	5.0	1.3	3.0
2*	A	0.0	−0.5	−1.3	3.0	−1.3	5.0

2* Frame lines: 2 3

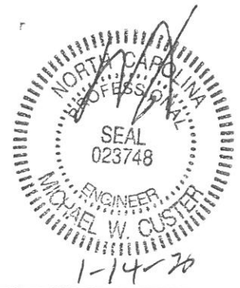
ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	1/13/20	FOR ERECTOR INSTALLATION	FXD	HPD	CM

BUILDING IS MANUFACTURED BY SW STEEL FACTORY

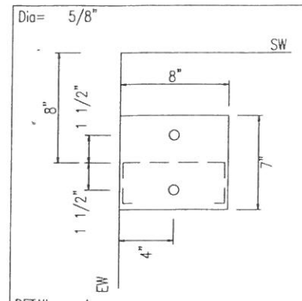
MICHAEL W. CUSTER, P.E.
642 OAKBEND DRIVE
COPPEL TX. 75019
PH. 972-571-7082

PROJECT: THOMAS SANDERS
CUSTOMER: THOMAS SANDERS
OWNER: THOMAS SANDERS
LOCATION: LILLINGTON, NC 27546

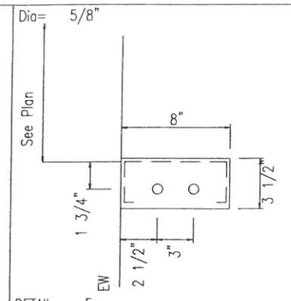
CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	1/13/20	N.T.S.	1	A	17-B-48481	F2	0



THIS DOCUMENT WAS PRODUCED BY AND/OR UNDER MY DIRECT SUPERVISION



DETAIL A



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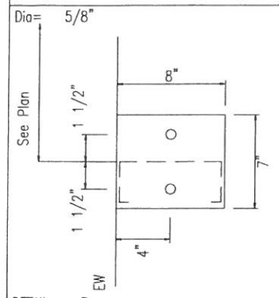

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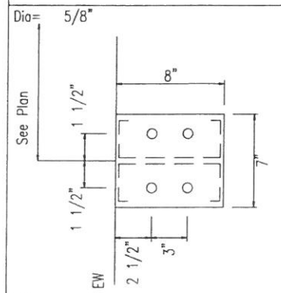

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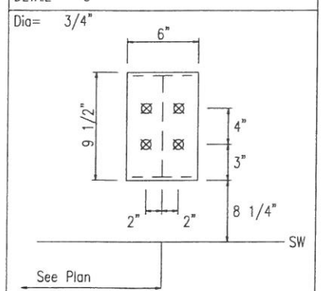

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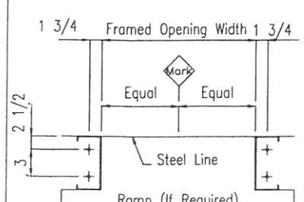
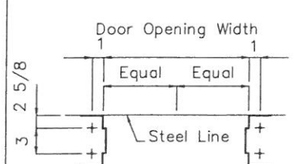


ARDia	Framed Opening AR Layout
5/8"	



AR Dia	Walk Door AR Layout
1/2"	

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	1/13/20	FOR ERECTOR INSTALLATION	FXD	HPD	CM

BUILDING IS MANUFACTURED BY SW STEEL FACTORY

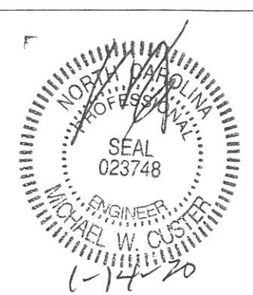
MICHAEL W. CUSTER, P.E.
642 OAKBEND DRIVE
COPPEL TX. 75019
PH. 972-571-7082

PROJECT: THOMAS SANDERS

CUSTOMER: THOMAS SANDERS | OWNER: THOMAS SANDERS

LOCATION: LILLINGTON, NC 27546

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	1/13/20	N.T.S.	1	A	17-B-48481	F3	0



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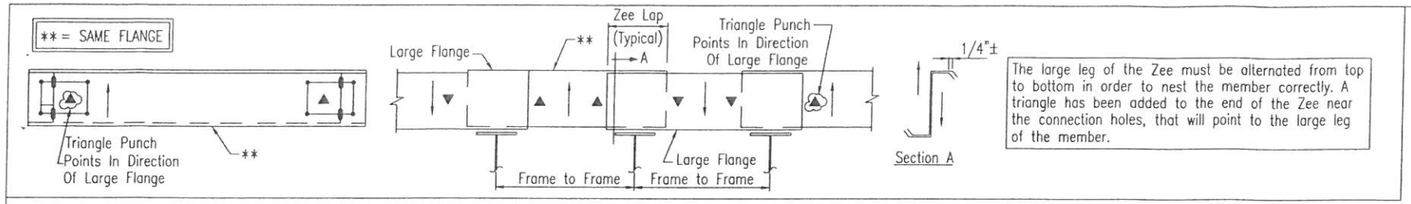
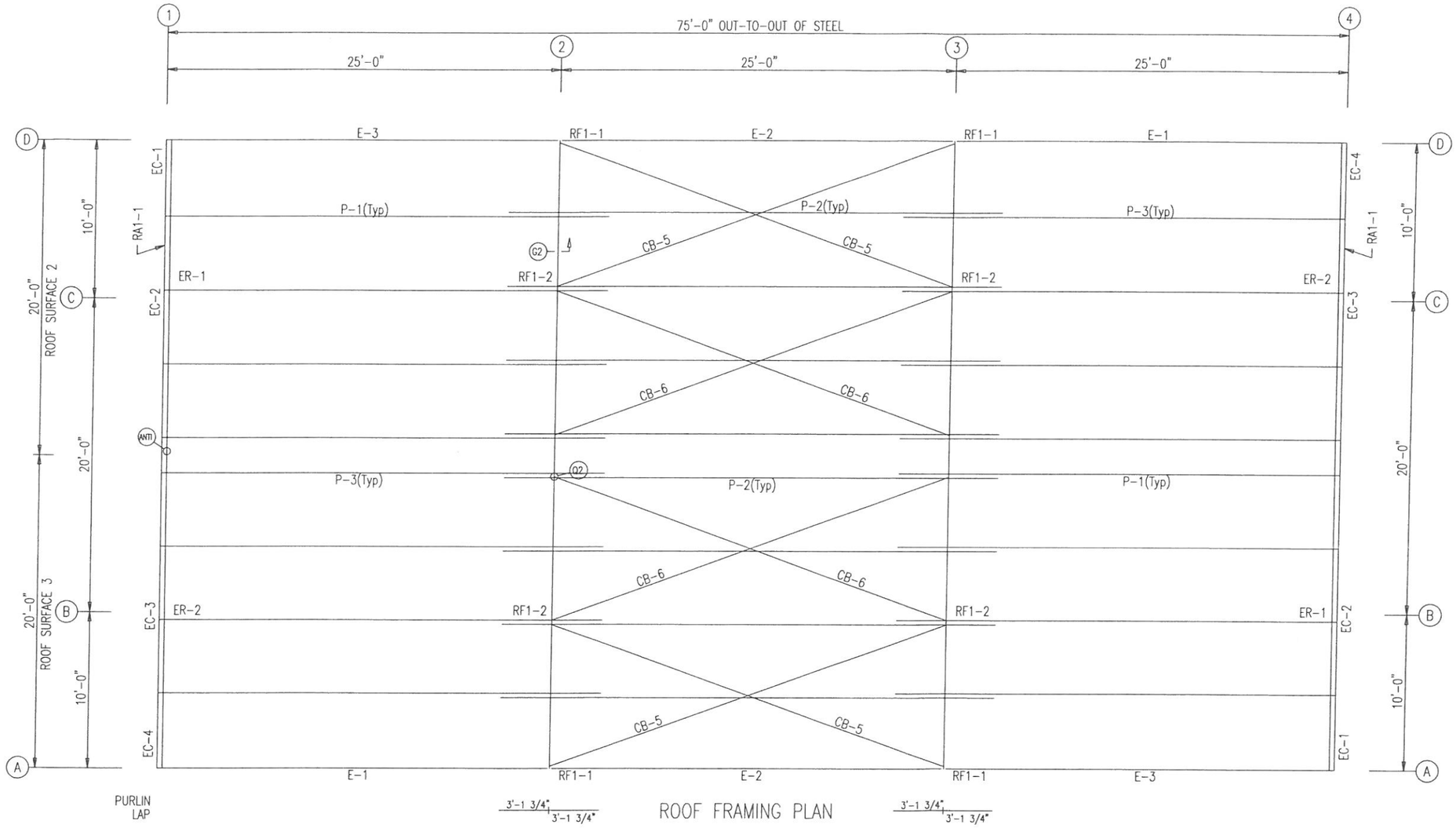


The large leg of the Zee must be alternated from top to bottom in order to nest the member correctly. A triangle has been added to the end of the Zee near the connection holes, that will point to the large leg of the member.

MEMBER TABLE
ROOF PLAN

MARK	PART	LENGTH
P-1	8X25Z14	28'-1 1/2"
P-2	8X25Z16	31'-3 1/2"
P-3	8X25Z14	28'-1 1/2"
E-1	8ES4L14	24'-11 1/2"
E-2	8ES4L14	24'-11 1/2"
E-3	8ES4L14	24'-11 1/2"
CB-5	1/4" CABLE	26'-10"
CB-6	1/4" CABLE	27'-4"



PURLIN LAP

ROOF FRAMING PLAN

GENERAL NOTES:

1. INSTALL ALL PURLIN AND FLANGE BRACES (FB) AS SHOWN.
2. ROOF PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.
3. STRUT PURLINS, IF PROVIDED, MUST BE INSTALLED AND FASTENED TO ROOF SHEETING PER "PBR" PANEL ROOF DETAIL.
4. DO NOT ADD ANY ADDITIONAL ROOF OPENINGS WITHOUT BUILDING MANUFACTURER APPROVAL OR PROFESSIONAL ENGINEER APPROVAL.
5. DO NOT STACK SHEET BUNDLES ON ROOF. ONLY RAISE INDIVIDUAL SHEETS AS NEEDED.
6. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHAVINGS CAUSED BY DRILLING.

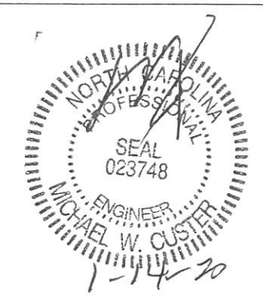
ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
A	1/13/20	FOR CONSTRUCTION PERMIT	FXD	HPD	CM

BUILDING IS MANUFACTURED BY SW STEEL FACTORY

MICHAEL W. CUSTER, P.E.
642 OAKBEND DRIVE
COPPEL TX. 75019
PH. 972-571-7082

PROJECT: THOMAS SANDERS
CUSTOMER: THOMAS SANDERS
OWNER: THOMAS SANDERS
LOCATION: LILLINGTON, NC 27546

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	1/13/20	N.T.S.	1	A	17-B-48481	E1	A



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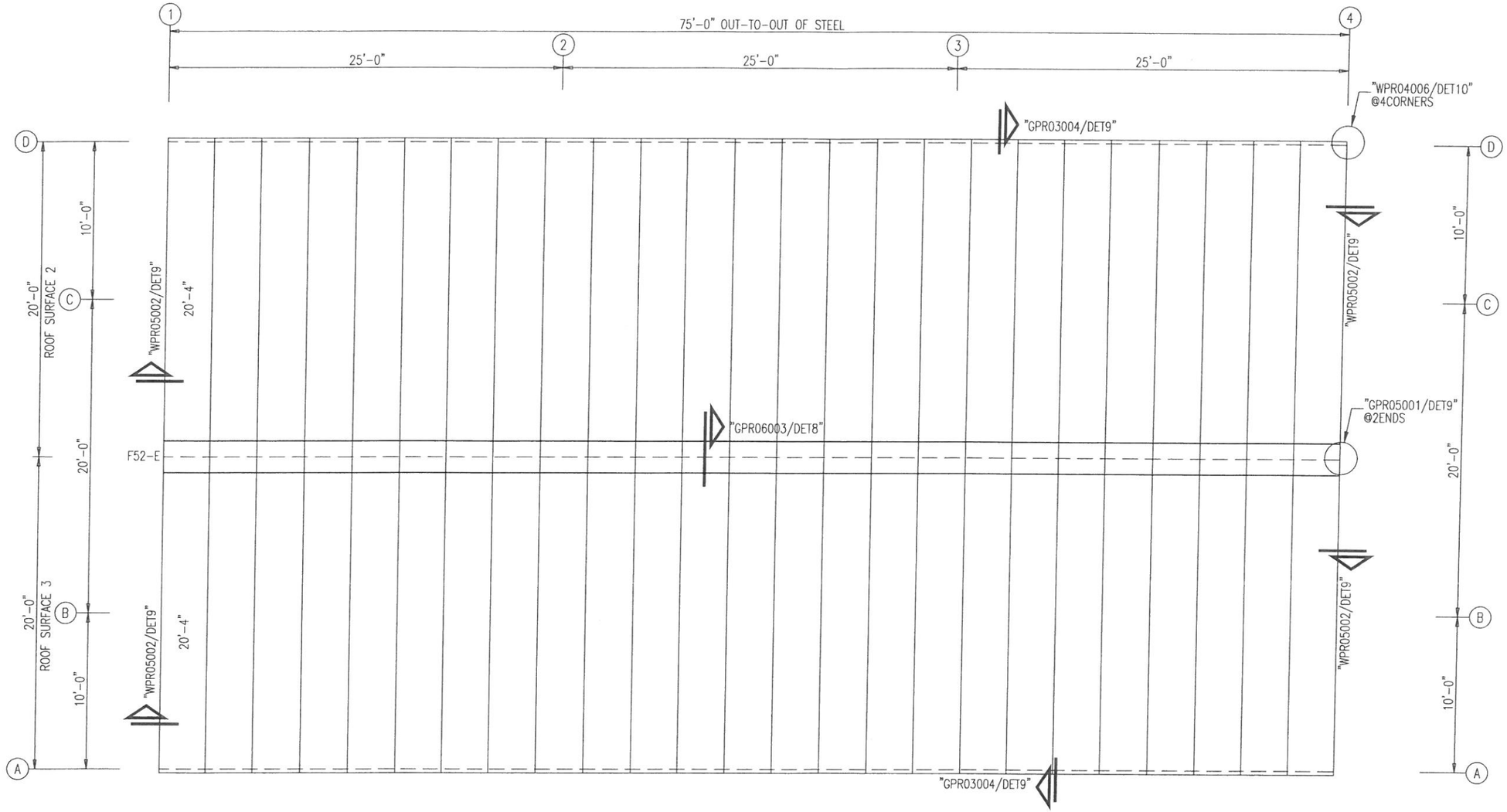
PBR ROOF SHEETING NOTE:
PBR ROOF PANELS ARE TO BE FIELD CUT IF THE PANELS EXTEND OUTSIDE OF THE ROOF PLANE, PANELS ARE NOT TO BE BACK LAPPED.



ROOF SHEETING PLAN

PANELS: 26 Gauge PBR – Polar White

GENERAL NOTES:

1. INSTALL ALL PURLIN AND FLANGE BRACES (FB) AS SHOWN.
2. ROOF PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.
3. STRUT PURLINS, IF PROVIDED, MUST BE INSTALLED AND FASTENED TO ROOF SHEETING PER "PBR" PANEL ROOF DETAIL.
4. DO NOT ADD ANY ADDITIONAL ROOF OPENINGS WITHOUT BUILDING MANUFACTURER APPROVAL OR PROFESSIONAL ENGINEER APPROVAL.
5. DO NOT STACK SHEET BUNDLES ON ROOF. ONLY RAISE INDIVIDUAL SHEETS AS NEEDED.
6. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHAVINGS CAUSED BY DRILLING.

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
A	1/13/20	FOR CONSTRUCTION PERMIT	FXD	HPD	CM

BUILDING IS MANUFACTURED BY SW STEEL FACTORY

MICHAEL W. CUSTER, P.E.
642 OAKBEND DRIVE
COPPEL TX. 75019
PH. 972-571-7082

PROJECT: THOMAS SANDERS
CUSTOMER: THOMAS SANDERS
OWNER: THOMAS SANDERS
LOCATION: LILLINGTON, NC 27546

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	1/13/20	N.T.S.	1	A	17-B-48481	E2	A

NORTH CAROLINA PROFESSIONAL SEAL 023748 ENGINEER MICHAEL W. CUSTER
1-14-20

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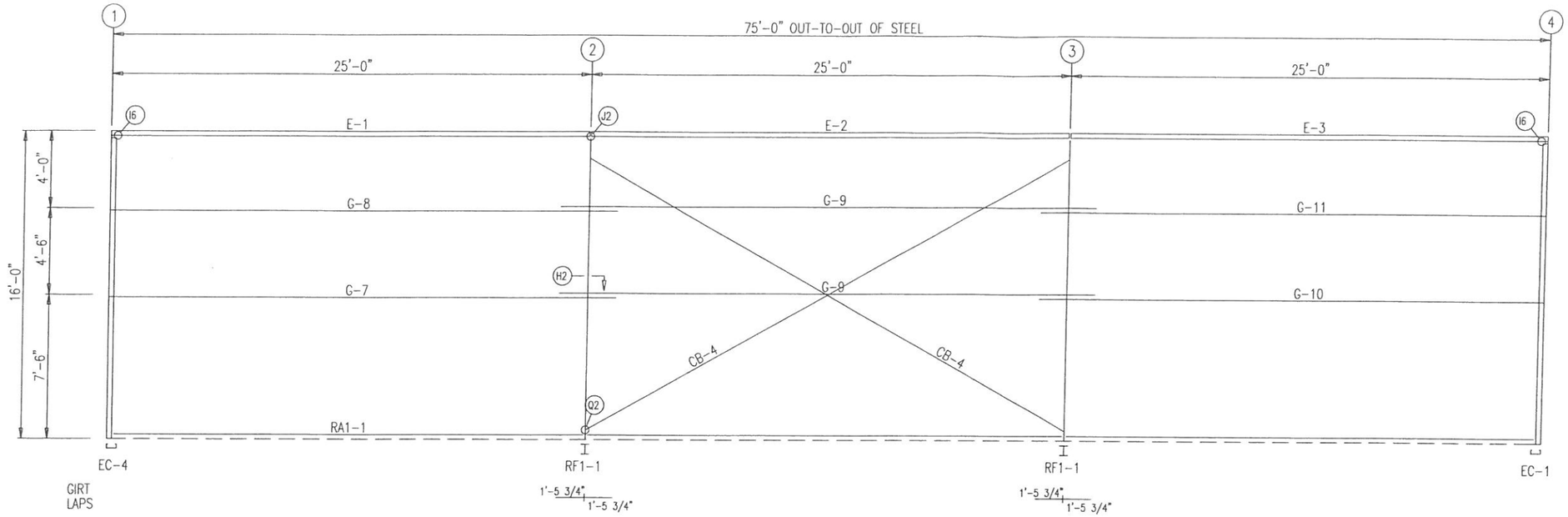
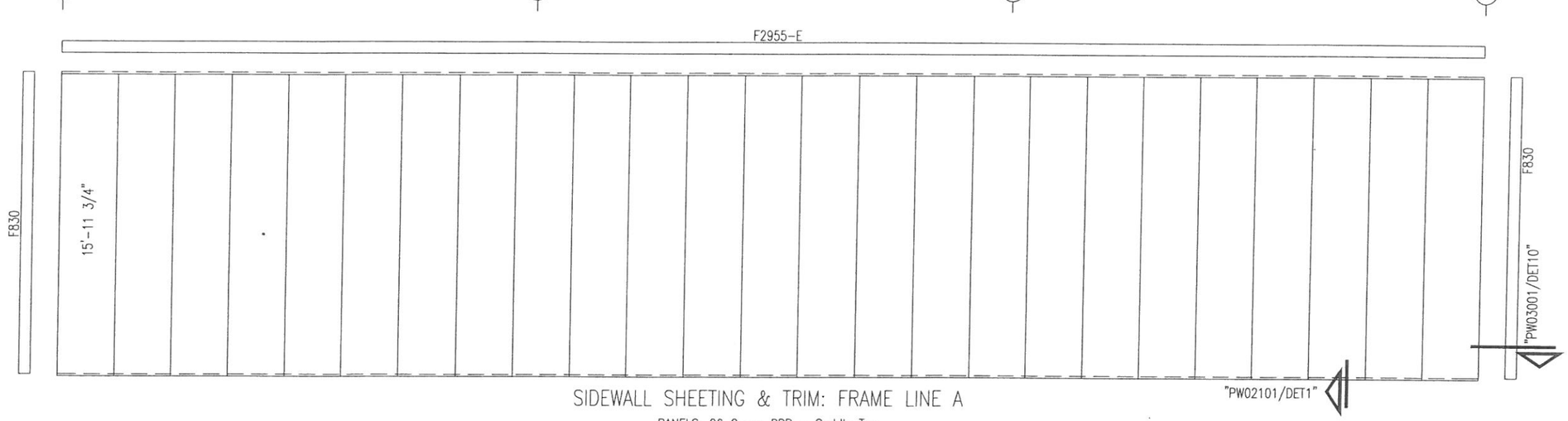
MEMBER TABLE FRAME LINE A		
MARK	PART	LENGTH
E-1	8ES4L14	24'-11 1/2"
E-2	8ES4L14	24'-11 1/2"
E-3	8ES4L14	24'-11 1/2"
G-7	8X25Z14	26'-5 1/2"
G-8	8X25Z16	26'-5 1/2"
G-9	8X25Z16	27'-11 1/2"
G-10	8X25Z14	26'-5 1/2"
G-11	8X25Z16	26'-5 1/2"
CB-4	1/4" CABLE	29'-3"



SIDEWALL FRAMING: FRAME LINE A



SIDEWALL SHEETING & TRIM: FRAME LINE A

PANELS: 26 Gauge PBR – Saddle Tan

GENERAL NOTES:

1. INSTALL ALL GIRTS AND FLANGE BRACES (FB) AS SHOWN.
2. WALL PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.
3. OTHER THAN FOR WALK DOORS AND WINDOWS SHOWN ON THE CONTRACT, DO NOT ADD ADDITIONAL WALL OPENINGS WITHOUT APPROVAL OF BUILDING MANUFACTURER OR PROFESSIONAL ENGINEER.
4. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHAVINGS CAUSED BY DRILLING.

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
A	1/13/20	FOR CONSTRUCTION PERMIT	FXD	HPD	CM

BUILDING IS MANUFACTURED BY SW STEEL FACTORY

MICHAEL W. CUSTER, P.E.
642 OAKBEND DRIVE
COPPEL TX. 75019
PH. 972-571-7082

PROJECT: THOMAS SANDERS

CUSTOMER: THOMAS SANDERS | OWNER: THOMAS SANDERS

LOCATION: LILLINGTON, NC 27546

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	1/13/20	N.T.S.	1	A	17-B-48481	E3	A

NORTH CAROLINA PROFESSIONAL SEAL 023748 ENGINEER MICHAEL W. CUSTER
1-14-26

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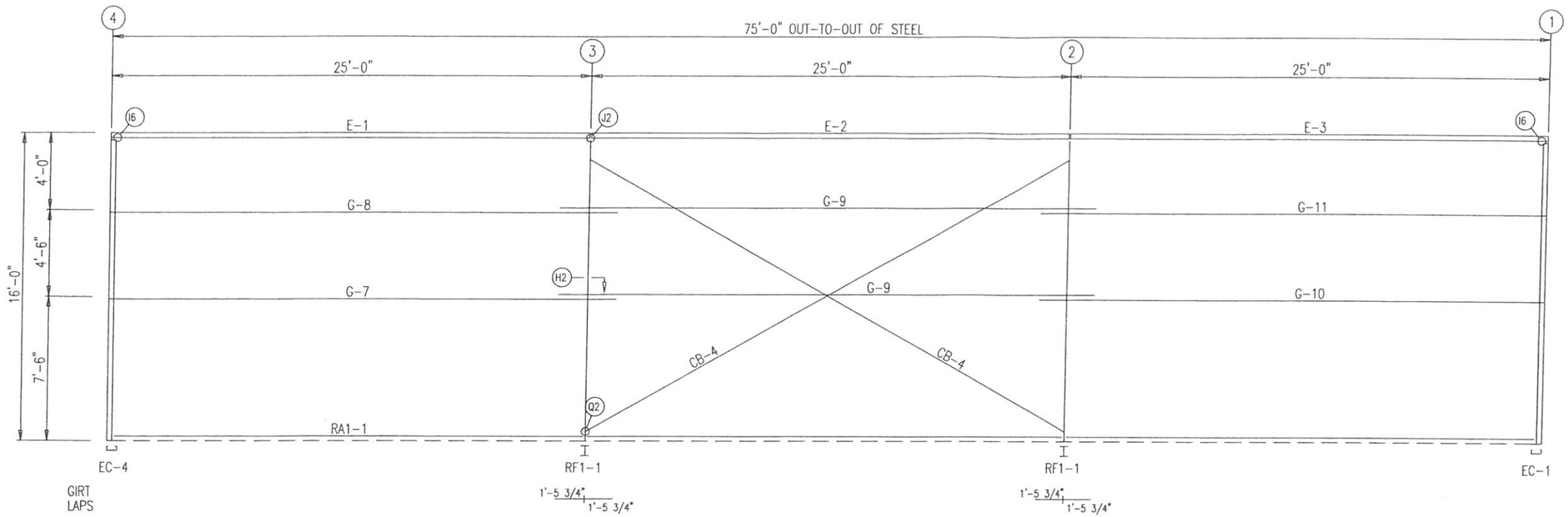
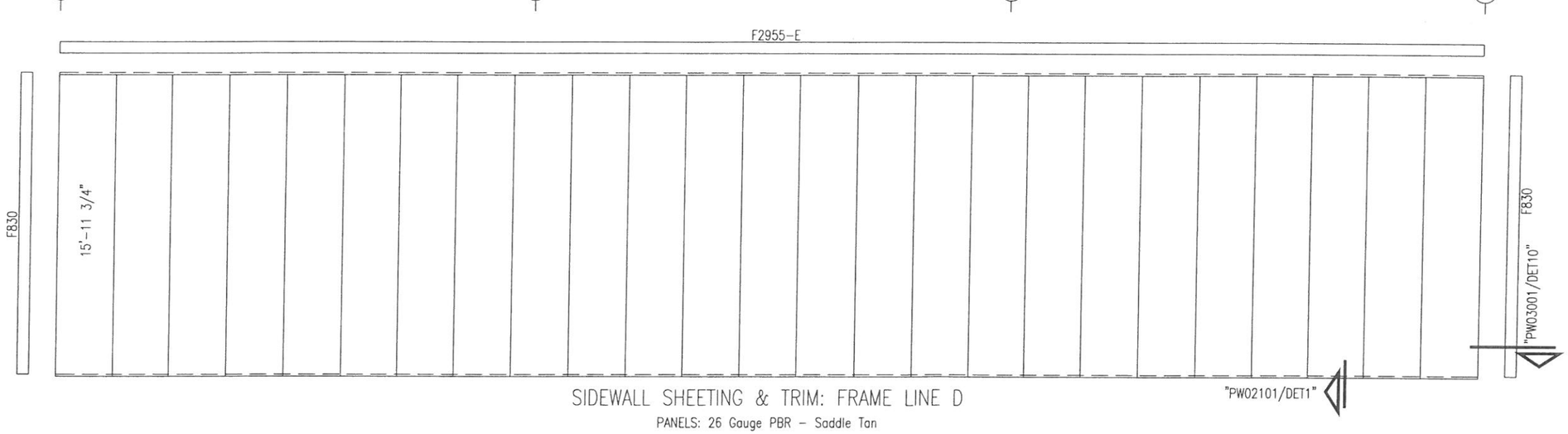
MEMBER TABLE FRAME LINE D		
MARK	PART	LENGTH
E-1	8ES4L14	24'-11 1/2"
E-2	8ES4L14	24'-11 1/2"
E-3	8ES4L14	24'-11 1/2"
G-7	8X25Z14	26'-5 1/2"
G-8	8X25Z16	26'-5 1/2"
G-9	8X25Z16	27'-11 1/2"
G-10	8X25Z14	26'-5 1/2"
G-11	8X25Z16	26'-5 1/2"
CB-4	1/4" CABLE	29'-3"



SIDEWALL FRAMING: FRAME LINE D



SIDEWALL SHEETING & TRIM: FRAME LINE D

PANELS: 26 Gauge PBR – Saddle Tan

GENERAL NOTES:

1. INSTALL ALL GIRTS AND FLANGE BRACES (FB) AS SHOWN.
2. WALL PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.
3. OTHER THAN FOR WALK DOORS AND WINDOWS SHOWN ON THE CONTRACT, DO NOT ADD ADDITIONAL WALL OPENINGS WITHOUT APPROVAL OF BUILDING MANUFACTURER OR PROFESSIONAL ENGINEER.
4. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHAVINGS CAUSED BY DRILLING.

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
A	1/13/20	FOR CONSTRUCTION PERMIT	FXD	HPD	CM

BUILDING IS MANUFACTURED BY SW STEEL FACTORY

MICHAEL W. CUSTER, P.E.
642 OAKBEND DRIVE
COPPEL TX. 75019
PH. 972-571-7082

PROJECT: THOMAS SANDERS

CUSTOMER: THOMAS SANDERS | OWNER: THOMAS SANDERS

LOCATION: LILLINGTON, NC 27546

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	1/13/20	N.T.S.	1	A	17-B-48481	E4	A

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BEARING FRAME ONLY!

WASHER TO BE USED AT ENDWALL COLUMN TO ENDWALL RAFTER CONNECTION. USE ONE WASHER ON COLUMN SIDE. WASHER NOT NEEDED ON CLIP SIDE.

BOLT TABLE
FRAME LINE 1

LOCATION	QUAN	TYPE	DIA	LENGTH
ER-1/ER-2	4	A325	5/8"	1 3/4"
Columns/Raf	4	A325	1/2"	1 1/4"

MEMBER TABLE
FRAME LINE 1

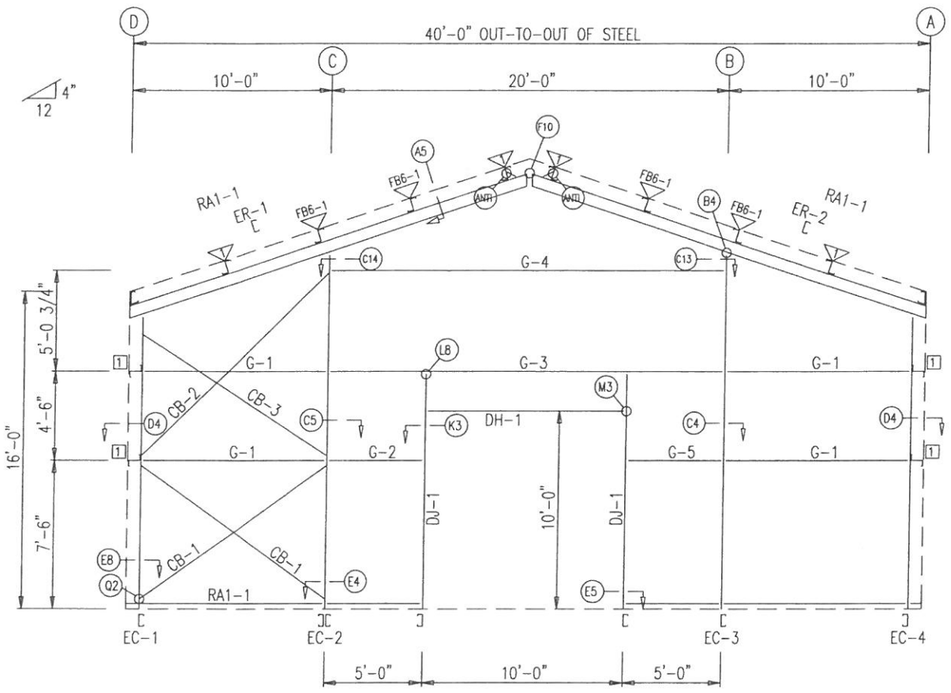
MARK	PART	LENGTH
EC-1	8F35C14	14'-6 7/16"
EC-2	8F70D12	17'-7 3/4"
EC-3	8F25C12	17'-7 3/4"
EC-4	8F25C14	14'-6 7/16"
ER-1	10F35C13	21'-0 3/4"
ER-2	10F35C13	21'-0 3/4"
DJ-1	8F35C14	12'-0"
DH-1	8F35C14	10'-0"
G-1	8X25Z16	8'-8"
G-2	8X25Z16	4'-4 1/4"
G-3	8X35Z13	19'-7 3/4"
G-4	8X25Z14	19'-7 3/4"
G-5	8X25Z16	4'-8"
CB-1	1/4" CABLE	12'-1"
CB-2	1/4" CABLE	13'-5"
CB-3	1/4" CABLE	11'-6"

FLANGE BRACE TABLE
FRAME LINE 1

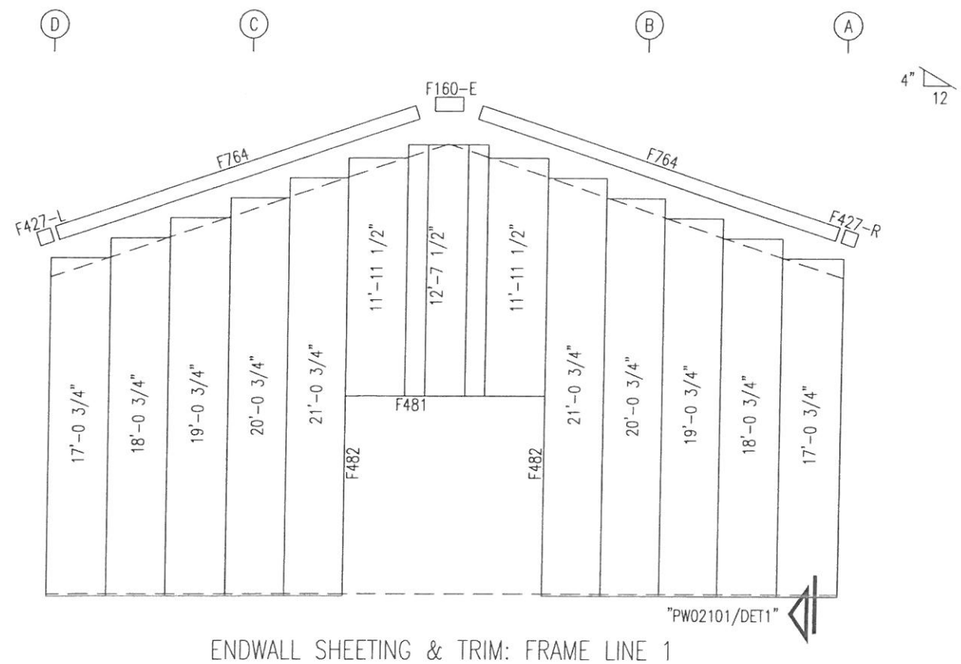
▽ ID	PART	LENGTH
FB30	L2X2X14G	2'-6"
FB6-1	L2X2X1/8	2'-6"

CONNECTION PLATES
FRAME LINE 1

□ ID	MARK/PART
1	SC-5



ENDWALL FRAMING: FRAME LINE 1



ENDWALL SHEETING & TRIM: FRAME LINE 1

PANELS: 26 Gauge PBR – Saddle Tan

GENERAL NOTES:

1. INSTALL ALL GIRTS AND FLANGE BRACES (FB) AS SHOWN.
2. WALL PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.
3. OTHER THAN FOR WALK DOORS AND WINDOWS SHOWN ON THE CONTRACT, DO NOT ADD ADDITIONAL WALL OPENINGS WITHOUT APPROVAL OF BUILDING MANUFACTURER OR PROFESSIONAL ENGINEER.
4. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHAVINGS CAUSED BY DRILLING.

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
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BUILDING IS MANUFACTURED BY SW STEEL FACTORY

MICHAEL W. CUSTER, P.E.
642 OAKBEND DRIVE
COPPEL TX. 75019
PH. 972-571-7082

PROJECT: THOMAS SANDERS

CUSTOMER: THOMAS SANDERS — OWNER: THOMAS SANDERS

LOCATION: LILLINGTON, NC 27546

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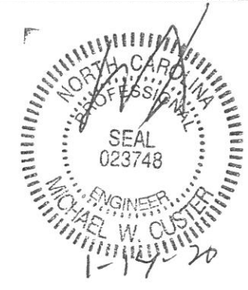
	1/13/20	N.T.S.	1	A	17-B-48481	E5	A



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BEARING FRAME ONLY!

WASHER TO BE USED AT ENDWALL COLUMN TO ENDWALL RAFTER CONNECTION. USE ONE WASHER ON COLUMN SIDE. WASHER NOT NEEDED ON CLIP SIDE.

BOLT TABLE
FRAME LINE 4

LOCATION	QUAN	TYPE	DIA	LENGTH
ER-1/ER-2	4	A325	5/8"	1 3/4"
Columns/Raf	4	A325	1/2"	1 1/4"

MEMBER TABLE
FRAME LINE 4

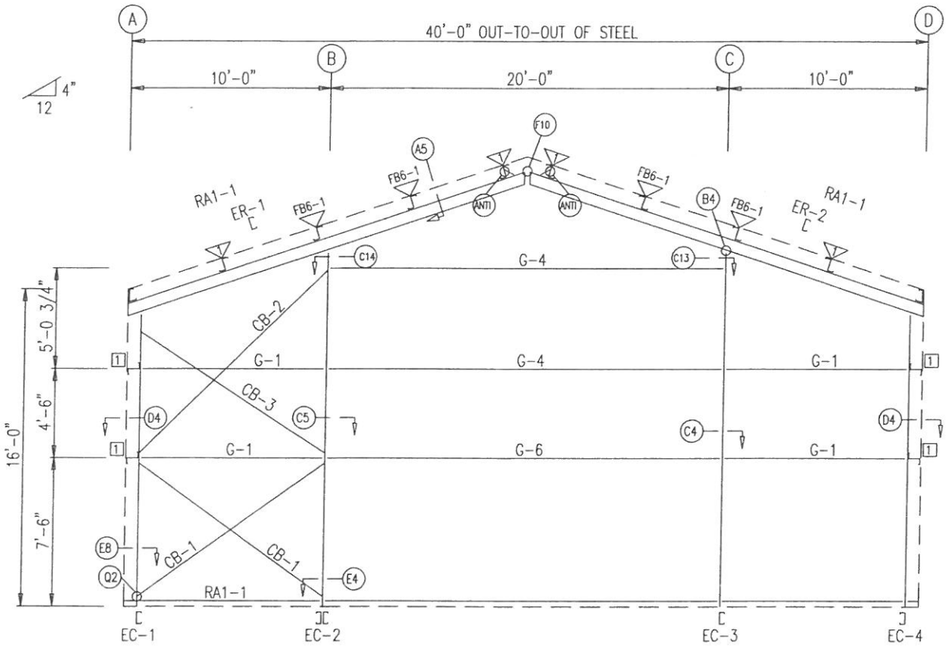
MARK	PART	LENGTH
EC-1	8F35C14	14'-6 7/16"
EC-2	8F70D12	17'-7 3/4"
EC-3	8F25C12	17'-7 3/4"
EC-4	8F25C14	14'-6 7/16"
ER-1	10F35C13	21'-0 3/4"
ER-2	10F35C13	21'-0 3/4"
G-1	8X25Z16	8'-8"
G-4	8X25Z14	19'-7 3/4"
G-6	8X35Z14	19'-7 3/4"
CB-1	1/4" CABLE	12'-1"
CB-2	1/4" CABLE	13'-5"
CB-3	1/4" CABLE	11'-6"

FLANGE BRACE TABLE
FRAME LINE 4

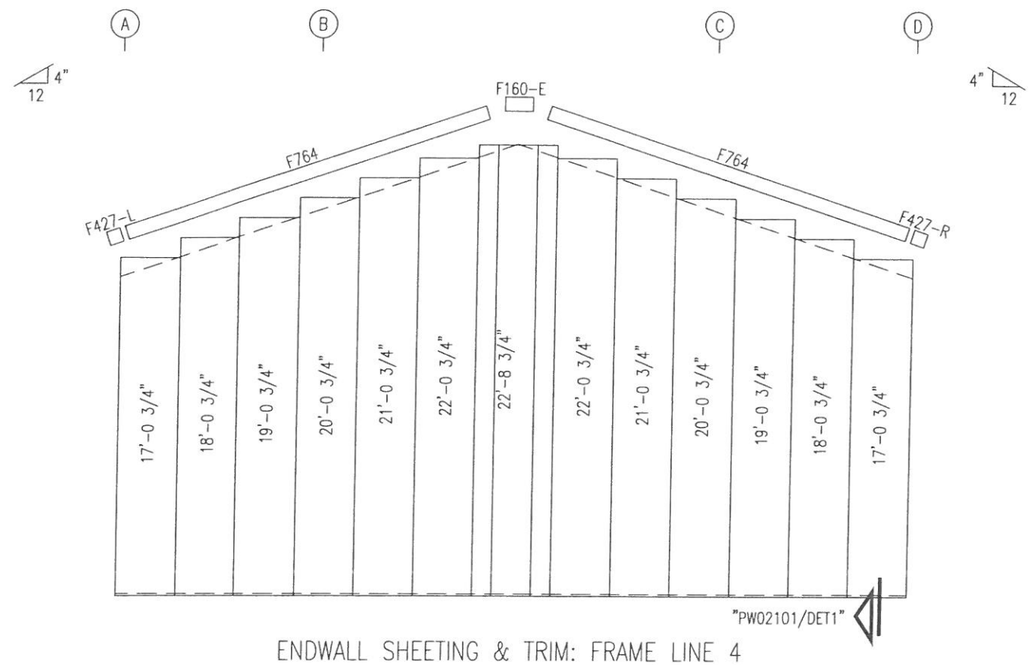
▽ ID	PART	LENGTH
FB30	L2X2X14G	2'-6"
FB6-1	L2X2X1/8	2'-6"

CONNECTION PLATES
FRAME LINE 4

□ID	MARK/PART
1	SC-5



ENDWALL FRAMING: FRAME LINE 4



ENDWALL SHEETING & TRIM: FRAME LINE 4

PANELS: 26 Gauge PBR – Saddle Tan

GENERAL NOTES:

1. INSTALL ALL GIRTS AND FLANGE BRACES (FB) AS SHOWN.
2. WALL PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.
3. OTHER THAN FOR WALK DOORS AND WINDOWS SHOWN ON THE CONTRACT, DO NOT ADD ADDITIONAL WALL OPENINGS WITHOUT APPROVAL OF BUILDING MANUFACTURER OR PROFESSIONAL ENGINEER.
4. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHAVINGS CAUSED BY DRILLING.

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
A	1/13/20	FOR CONSTRUCTION PERMIT	FXD	HPD	CM

BUILDING IS MANUFACTURED BY SW STEEL FACTORY

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642 OAKBEND DRIVE
COPPEL TX. 75019
PH. 972-571-7082

PROJECT: THOMAS SANDERS

CUSTOMER: THOMAS SANDERS OWNER: THOMAS SANDERS

LOCATION: LILLINGTON, NC 27546

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	1/13/20	N.T.S.	1	A	17-B-48481	E6	A

NORTH CAROLINA PROFESSIONAL SEAL 023748 ENGINEER MICHAEL W. CUSTER

1-14-20

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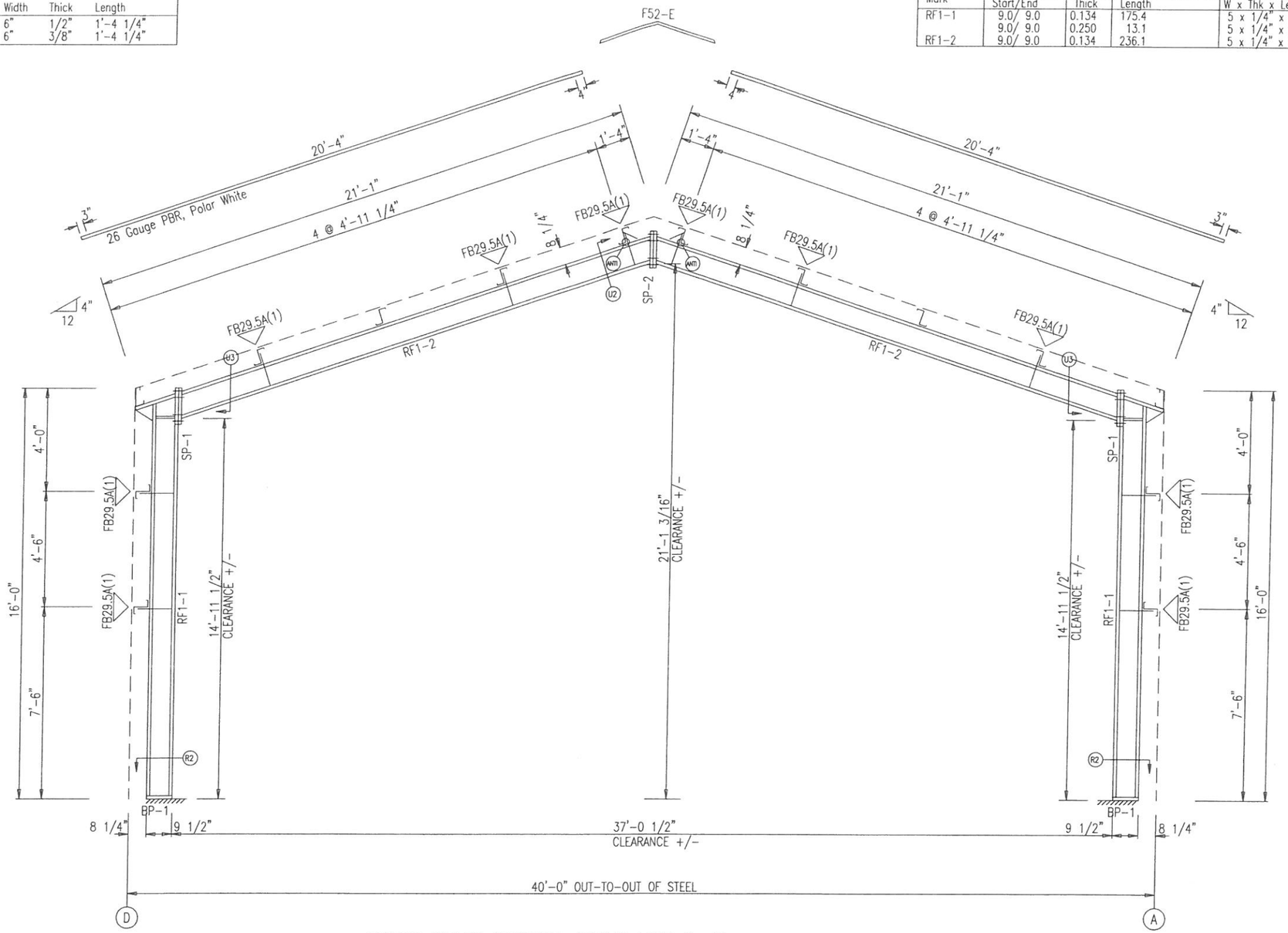
SPLICE PLATE & BOLT TABLE									
Mark	Qty Top	Bot	Int	Type	Dia	Length	Width	Thick	Length
SP-1	4	4	0	A325	3/4"	2"	6"	1/2"	1'-4 1/4"
SP-2	4	4	0	A325	3/4"	1 3/4"	6"	3/8"	1'-4 1/4"

STIFFENER TABLE				
Mark	Stiff Mark	Plate Size Width	Thick	Length
RF1-1	St- 1	2 1/2	1/4"	9"

BASE PLATE TABLE			
Col Mark	Plate Size Width	Thick	Length
BP-1	6"	3/8"	9 1/2"

FLANGE BRACES: BOTH SIDES(UNLESS NOTED)
FBxxA(1): xx=length(in)
A - L2X2X14G

MEMBER TABLE					
Mark	Web Depth Start/End	Web Plate Thick	Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RF1-1	9.0/ 9.0	0.134	175.4	5 x 1/4" x 185.4	5 x 1/4" x 175.4
	9.0/ 9.0	0.250	13.1	5 x 1/4" x 18.4	
RF1-2	9.0/ 9.0	0.134	236.1	5 x 1/4" x 233.0	5 x 1/4" x 233.0



FRAME CROSS SECTION: FRAME LINE 2 3

GENERAL NOTES:

1. SNUG TIGHT - ALL BOLTED JOINTS WITH A325 TYPE 1 BOLTS ARE SPECIFIED AS SNUG-TIGHTENED JOINTS IN ACCORDANCE WITH THE SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS, DECEMBER 31, 2009. PRE-TENSIONING METHODS, INCLUDING TURN-OF-NUT, CALIBRATED WRENCH, TWIST-OFF-TYPE TENSION-CONTROL BOLTS OR DIRECT TENSION INDICATOR ARE NOT REQUIRED. INSTALLATION INSPECTION REQUIREMENTS FOR SNUG TIGHT BOLTS (SPECIFICATION FOR STRUCTURAL JOINTS SECTION 9.1) IS SUGGESTED.
2. ALL FIELD WELDED CONNECTIONS OF SECONDARY FRAMING SHALL BE BOLTED WITH A325 MACHINE BOLTS
3. INSTALL ALL FLANGE BRACES ON COLUMN AND RAFTER AS SHOWN

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
A	1/13/20	FOR CONSTRUCTION PERMIT	FXD	HPD	CM

BUILDING IS MANUFACTURED BY SW STEEL FACTORY

MICHAEL W. CUSTER, P.E.
642 OAKBEND DRIVE
COPPEL TX. 75019
PH. 972-571-7082

PROJECT: THOMAS SANDERS
CUSTOMER: THOMAS SANDERS
OWNER: THOMAS SANDERS
LOCATION: LILLINGTON, NC 27546

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	1/13/20	N.T.S.	1	A	17-B-48481	E7	A

NORTH CAROLINA PROFESSIONAL SEAL 023748 ENGINEER MICHAEL W. CUSTER
1-14-20

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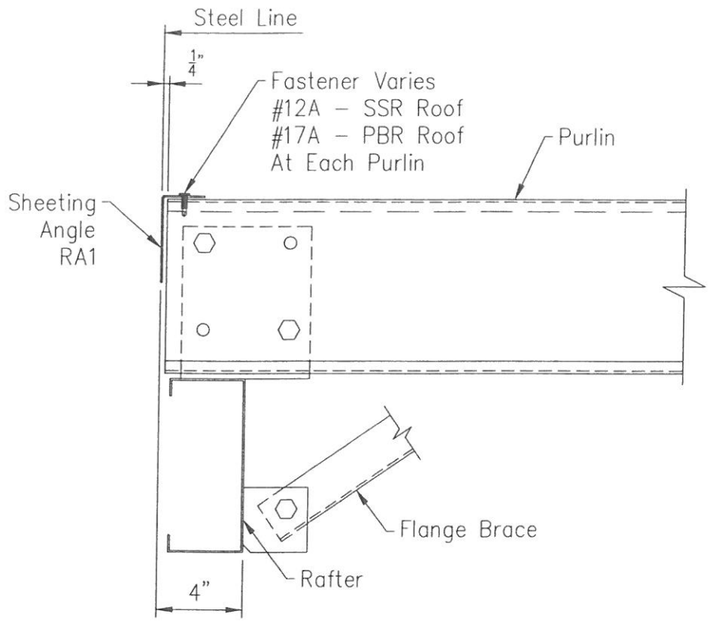
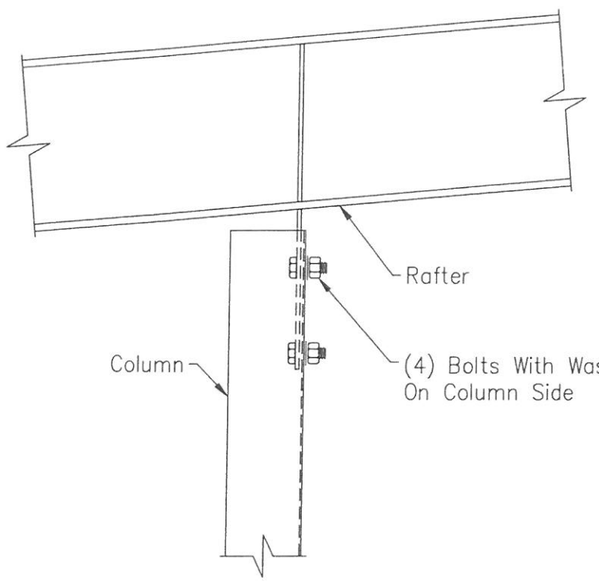


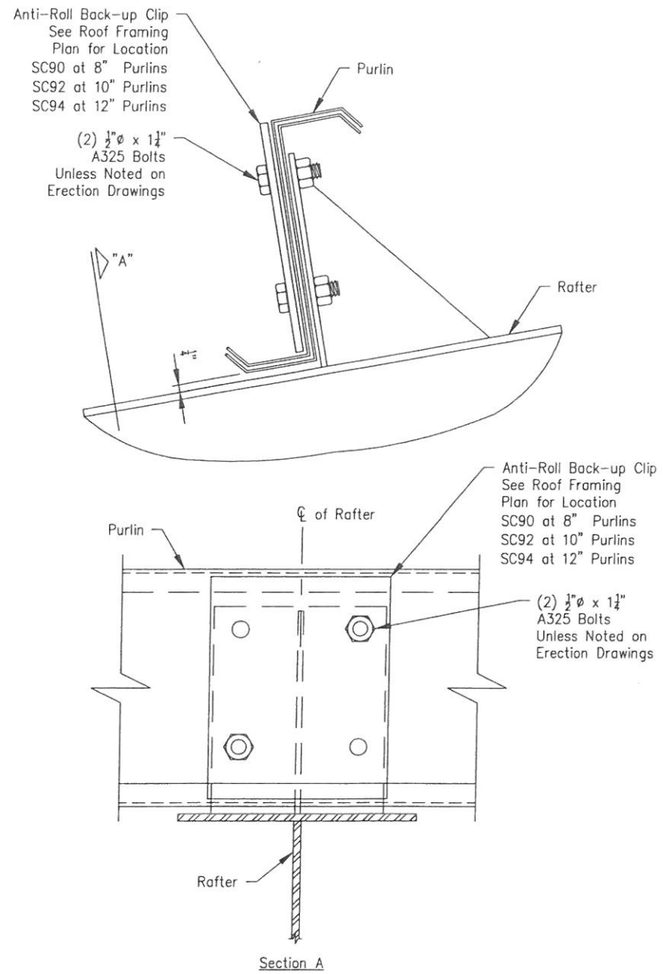
A5	Purlin To Bearing Frame Single Cold Form Rafter	Date Nov '19
Page MB–A5		Rev 01



B4	Cold Form Endwall Column To Rafter	Date Jun '17
Page MB–B4		Rev 00

Purlin with Anti-Roll Back-up Clip at Rafter

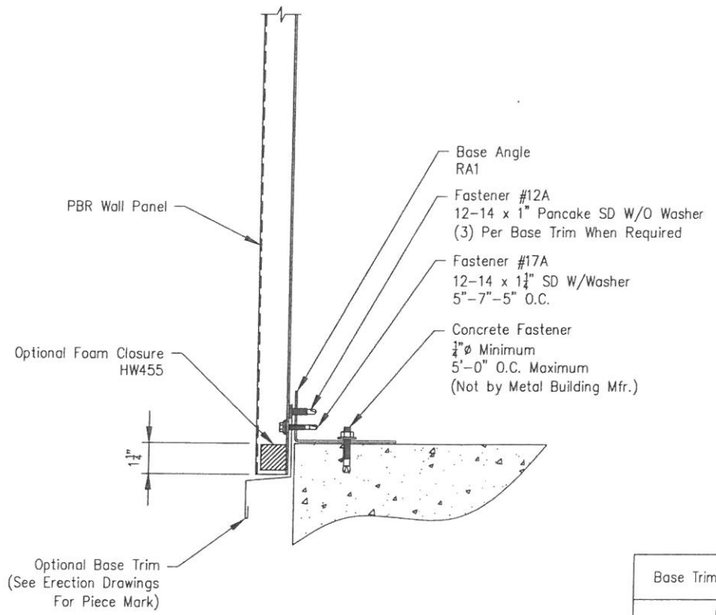
Page CF02035	
Date Dec '14	Rev 04



Section A

PBR Wall Panel
Base Angle Without Panel Recess

Page PW02101	
Date Apr '18	Rev 02



Base Trim
F406
F407
F408

Wall panel must be held off of base trim a minimum of 1/4" to prevent bottom of wall panel from rusting.

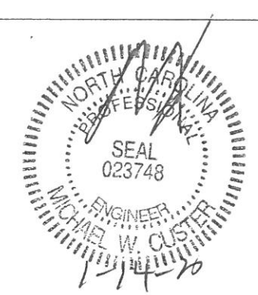
ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
A	1/13/20	FOR CONSTRUCTION PERMIT	FXD	HPD	CM

BUILDING IS MANUFACTURED BY SW STEEL FACTORY

MICHAEL W. CUSTER, P.E.
642 OAKBEND DRIVE
COPPEL TX. 75019
PH. 972–571–7082

PROJECT: THOMAS SANDERS
CUSTOMER: THOMAS SANDERS
OWNER: THOMAS SANDERS
LOCATION: LILLINGTON, NC 27546

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	1/13/20	N.T.S.	1	A	17–B–48481	DET1	A



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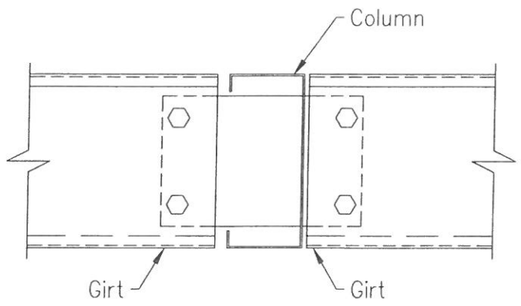
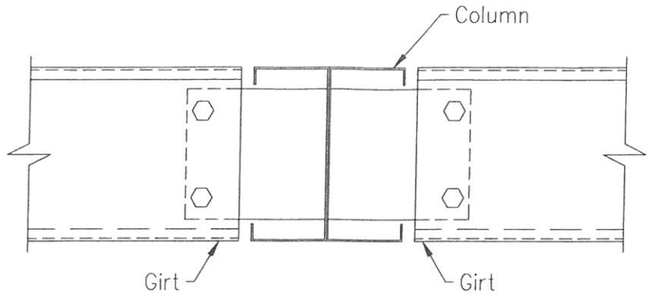
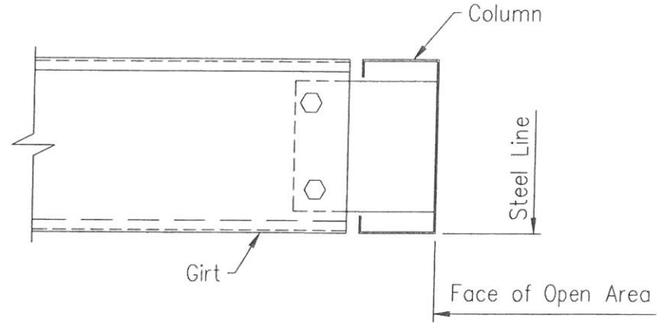


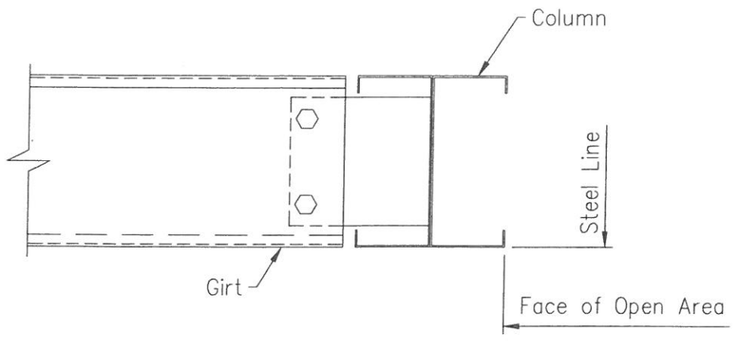
C4	Girt To Cold Form Column	Date Jun '17
Page MB-C4		Rev 00



C5	Girt To Double Cee Cold Form Endwall Column	Date Dec '17
Page MB-C5		Rev 00



C13	Girt To Cold Form Endwall Column – Partially Open	Date Jun '17
Page MB-C13		Rev 00



C14	Girt To Double Cee Cold Form Endwall Column – Partially Open	Date Jun '17
Page MB-C14		Rev 00

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
A	1/13/20	FOR CONSTRUCTION PERMIT	FXD	HPD	CM

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642 OAKBEND DRIVE
COPPEL TX. 75019
PH. 972-571-7082

PROJECT: THOMAS SANDERS

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LOCATION: LILLINGTON, NC 27546

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	1/13/20	N.T.S.	1	A	17-B-48481	DET2	A

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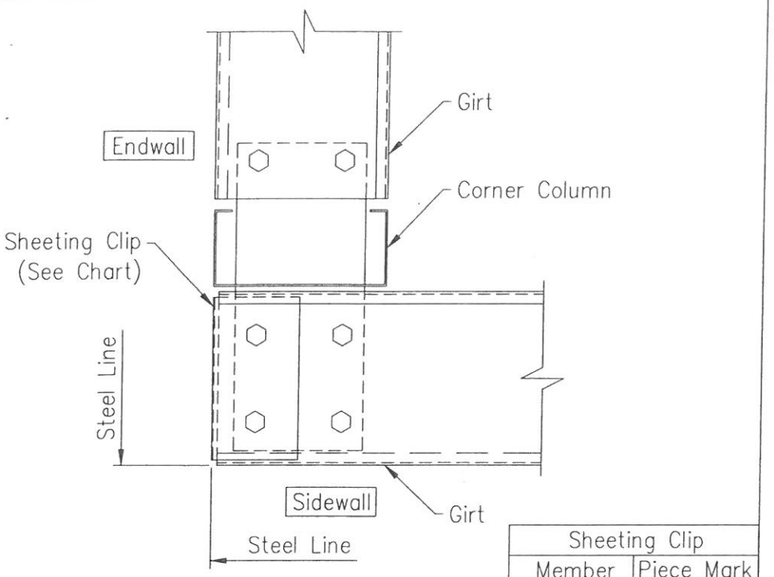
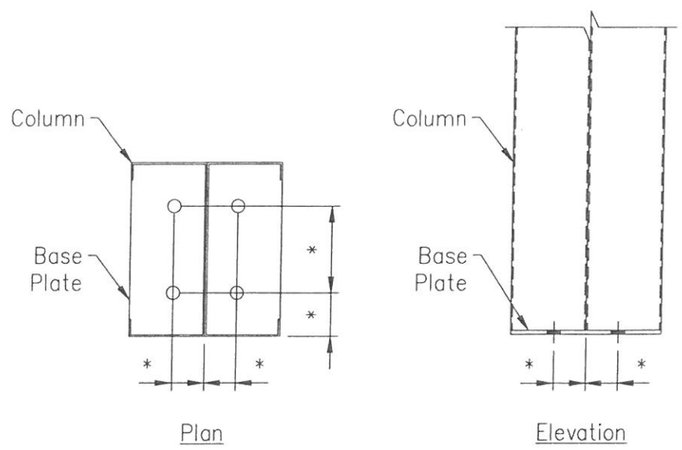


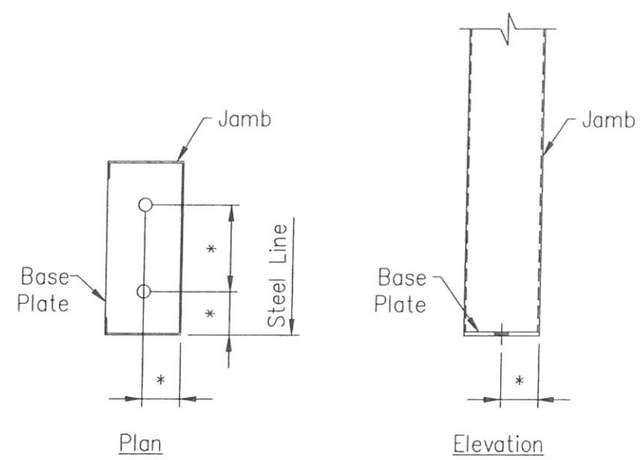
Sheeting Clip	
Member	Piece Mark
8"	SC5
10"	SC54
12"	SC55

D4	Girt To Cold Form Corner Column	Date Oct '19
Page MB-D4		Rev 01



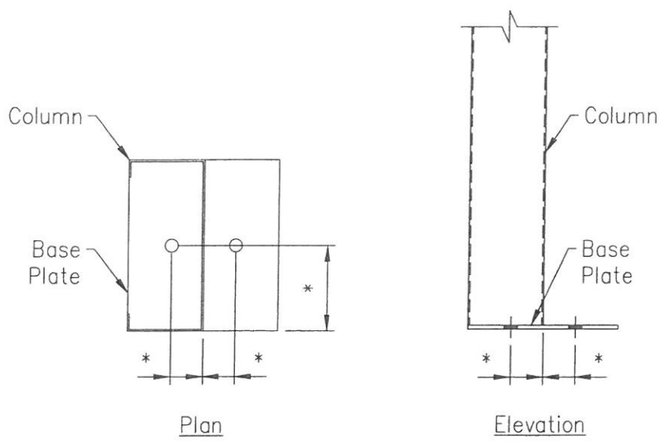
* – Refer To Anchor Rod Setting Plan For Dimension

E4	Double Cee Cold Form Endwall Column Or Door Jamb Base Plate	Date Dec '18
Page MB-E4		Rev 01



* – Refer To Anchor Rod Setting Plan For Dimension

E5	Door Jamb Base Plate	Date Dec '18
Page MB-E5		Rev 01



* – Refer To Anchor Rod Setting Plan For Dimension

E8	Cold Form Endwall Column Base Plate	Date Dec '18
Page MB-E8		Rev 01

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
A	1/13/20	FOR CONSTRUCTION PERMIT	FXD	HPD	CM

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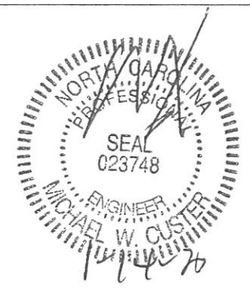
	1/13/20	N.T.S.	1	A	17-B-48481	DET3	A



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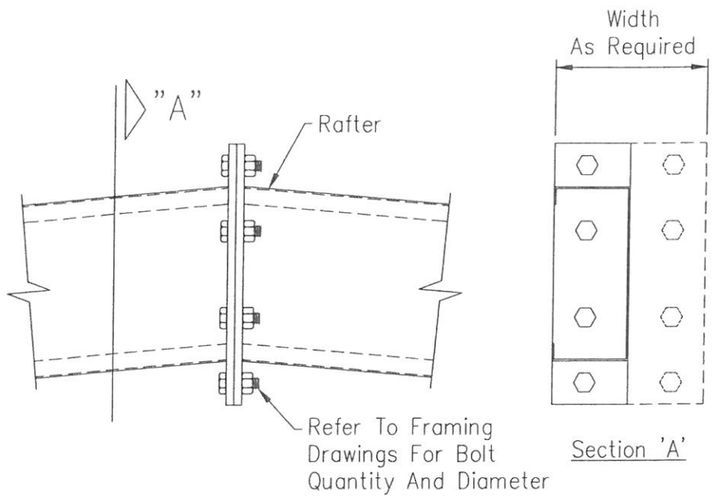
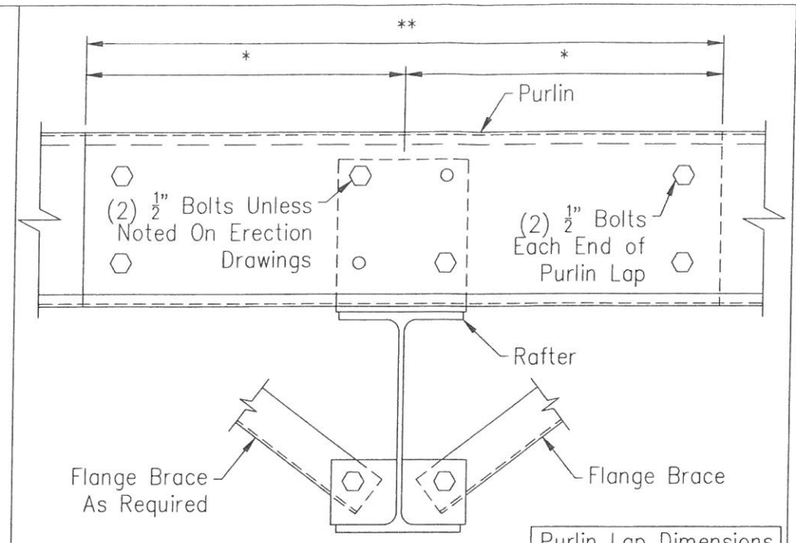
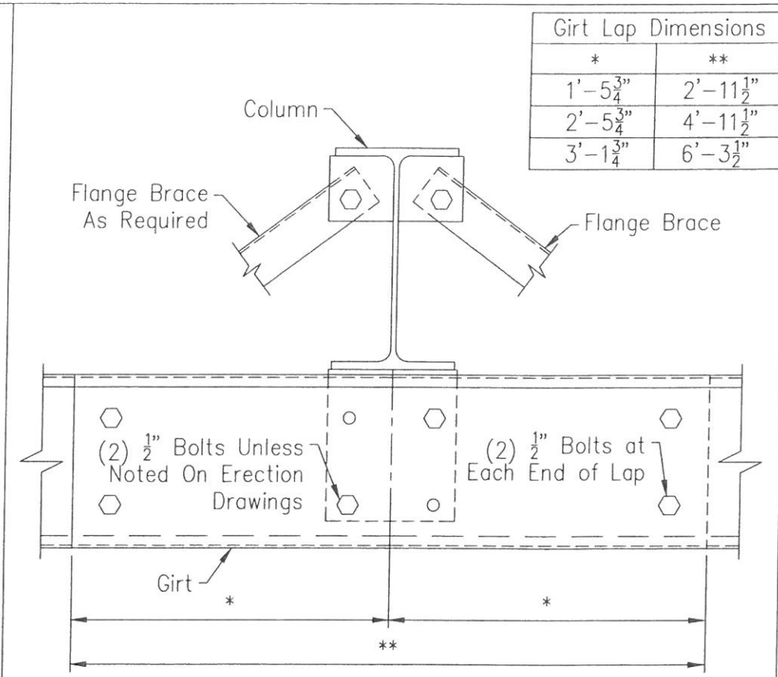


F10	Endwall Bearing Frame – Cold Form Rafter Splice At Ridge	Date Jun '17
Page MB-F10		Rev 00



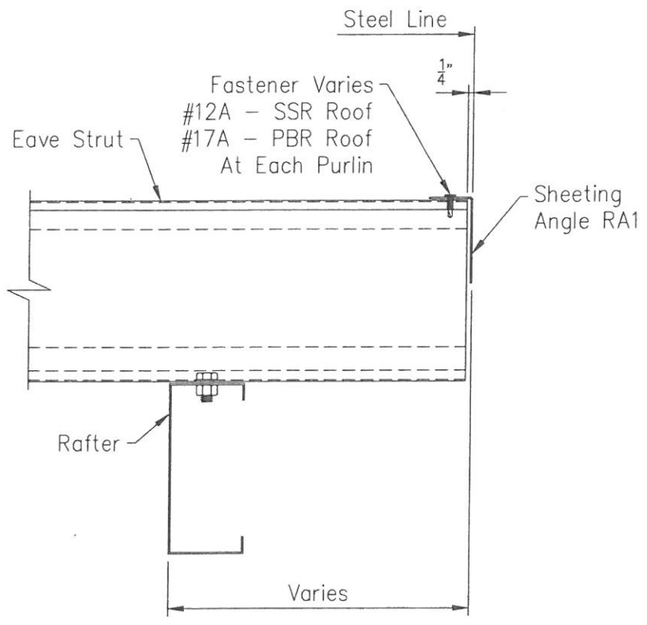
Purlin Lap Dimensions	
*	**
1'–5¾"	2'–11½"
2'–5¾"	4'–11½"
3'–1¾"	6'–3½"

G2	Purlin To Rigid Frame	Date Sep '19
Page MB-G2		Rev 01



Girt Lap Dimensions	
*	**
1'–5¾"	2'–11½"
2'–5¾"	4'–11½"
3'–1¾"	6'–3½"

H2	Girt To Rigid Frame	Date Sep '19
Page MB-H2		Rev 01



16	Low Side Eave Strut To Bearing Frame – Cold Form	Date Jun '17
Page MB-16		Rev 00

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
A	1/13/20	FOR CONSTRUCTION PERMIT	FXD	HPD	CM

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642 OAKBEND DRIVE
COPPEL TX. 75019
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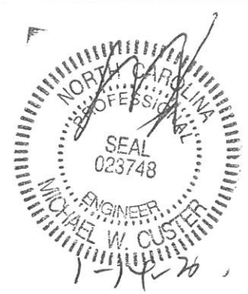
	1/13/20	N.T.S.	1	A	17–B–48481	DET4	A



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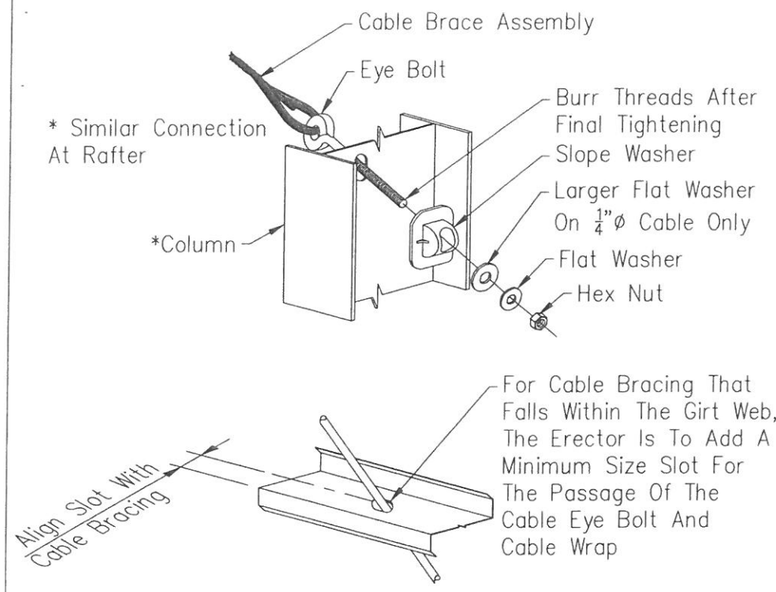
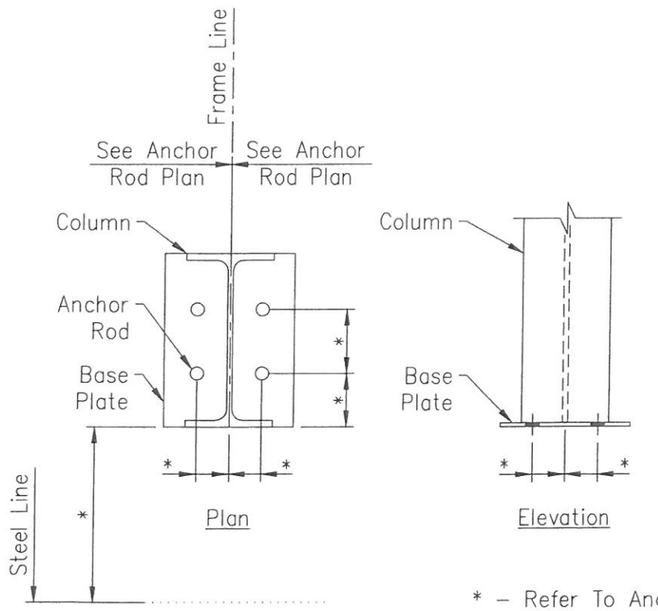


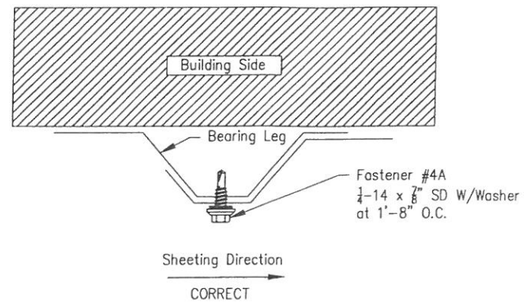
Q2	Cable Brace Attachment At Web	Date Mar '18
Page MB-Q2		Rev 01

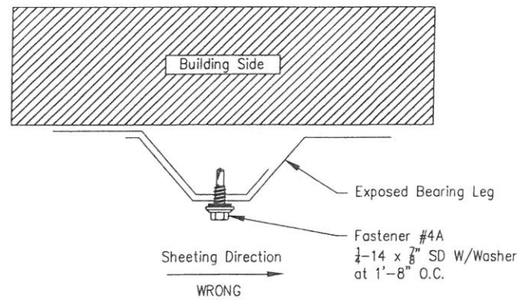


R2	Anchor Rods At Frame Column	Date Dec '17
Page MB-R2		Rev 00

PBR Wall Panel
Panel Side Lap

Page PW05002
Date Mar '19 | Rev 03





Screw Application

Page TH06006
Date May '19 | Rev 01

Standard Grade

Description	Fastener Number	Application
1/4"-14 x 7/8" Type 2	4A	Stitch & Trim Screw
12-14 x 1 1/4" Type 2	17A	Member Screw (Up To 4" Insulation)
12-14 x 1 1/2" Type 2	17B	Member Screw (Up To 6" Insulation)

Long Life

Description	Fastener Number	Application
1/4"-14 x 7/8" Type 1	4	Stitch & Trim Screw
12-14 x 1 1/4" Type 2	3	Member Screw (Up To 4" Insulation)
12-14 x 1 1/2" Type 2	3A	Member Screw (Up To 6" Insulation)

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
A	1/13/20	FOR CONSTRUCTION PERMIT	FXD	HPD	CM

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	1/13/20	N.T.S.	1	A	17-B-48481	DET6	A

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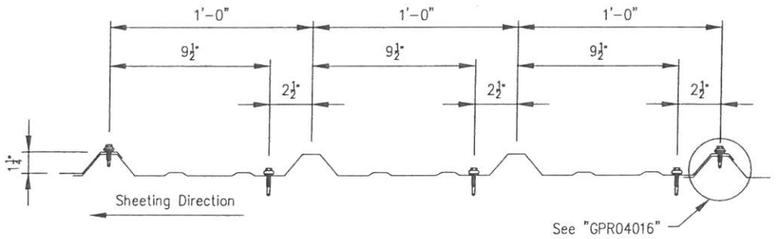
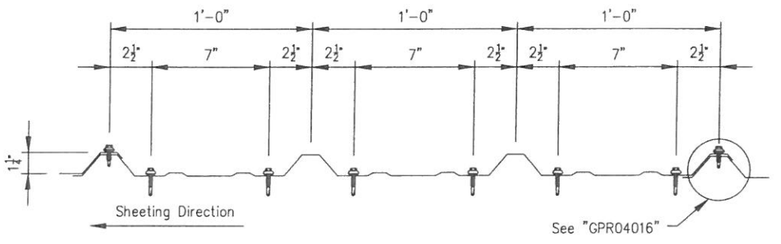
PBR Roof Panel
Fastener And Tape Sealant Location

Page GPR00011 | Date Apr '19 | Rev 01



All Roof Members Except As Noted Below



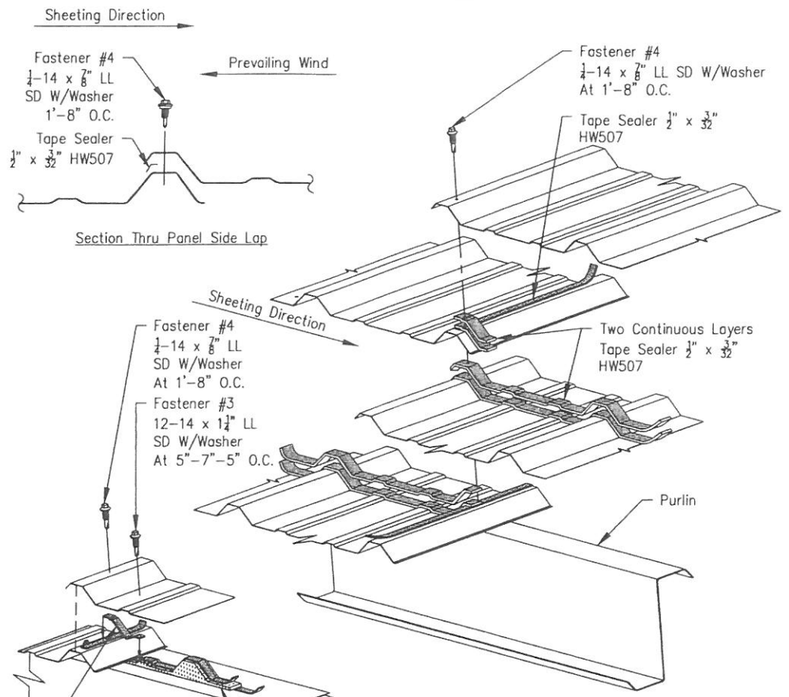
At Eave Strut, Panel End Lap And Peak Purlin

Note:
Screw patterns shown satisfy U.L. 90 requirements for roof.

PBR Roof Panel
Side Lap And End Lap Details

Page GPR04016 | Date Apr '19 | Rev 04

Section Thru Panel Side Lap

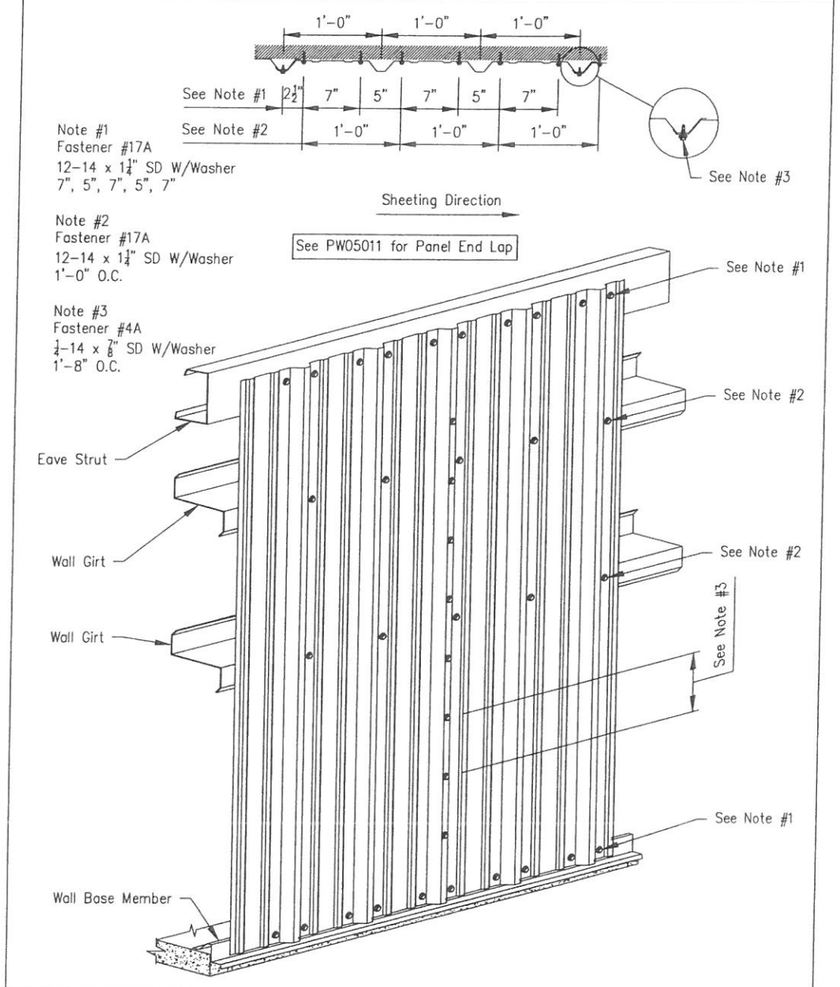


Panel Endlap Detail

Panel Sidelap Detail
At Low Eave

PBR Wall Panel
Fastener Location

Page PW05003 | Date Aug '15 | Rev 04



ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
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NORTH CAROLINA PROFESSIONAL SEAL 023748 ENGINEER MICHAEL W. CUSTER

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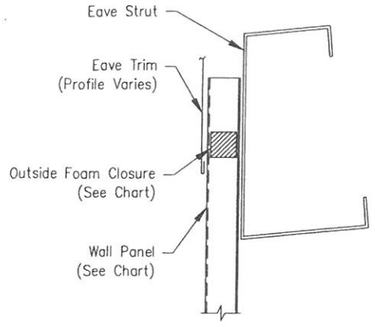
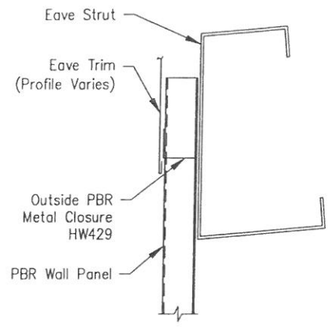
Single Skin Wall Panel Outside Closure Requirements at Eave

Page GD06002 | Date Feb '19 | Rev 02



Detail at Foam Closure
(Low Eave Shown High Eave Similar)



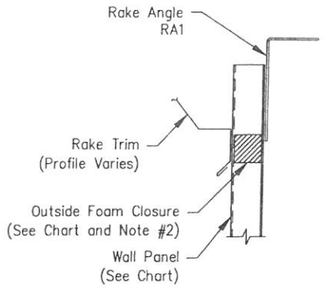
Detail at Optional Metal Closure
For PBR Panel Only
(Low Eave Shown High Eave Similar)

Note:
Foam Closures Are Required When Job Requires Air Infiltration
Or Sealed Wall Requirements, See GD16002.

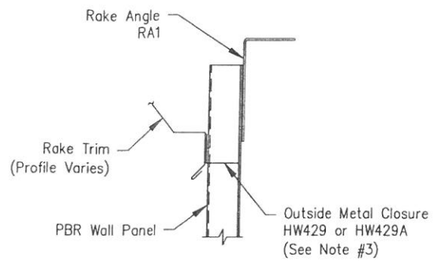
Wall Panel	Foam Closure
PBR	HW456
AVP	HW465
PBU	HW460
VistaShadow	HW465
NuWall	HW424
PBC	HW462
PBD	HW463
ShadowRib	HW412
Designer Series (Fluted Only)	HW4037
RBR (Reverse Rolled PBR)	HW455
RBU (Reverse Rolled PBU)	HW459
7.2	HW461

Single Skin Wall Panel Outside Closure Requirements at Rake

Page GD06003 | Date Feb '19 | Rev 02

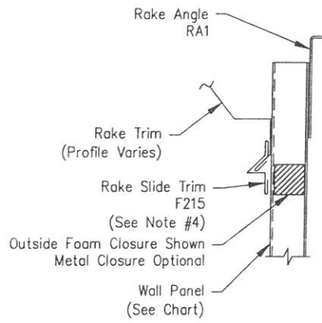


Detail at Foam Closure



Detail at Optional Metal Closure
For PBR Panel Only

Notes:
1. Outside Panel Closures are Required at all Sheeted Endwalls.
2. PBR Wall Panel Outside Foam Closures HW456 Required for Roof Slope 4:12 or Less and HW422 for Roof Slope Greater Than 4:12. Field Form/Notch HW422 to Panel Profile.
3. PBR Wall Panel Outside Metal Closure HW429 for Roof Slope 0 :12 Thru 1½ :12 and HW429A for Roof Slope Greater Than 3½ :12 Thru 4½ :12.
4. Rake Slide Trim Required for All Standing Seam Roofs and at Screw Down Roof Runs Greater Than 100'-0"
5. Foam Closures are Required when Job Requires Air Infiltration or Sealed Wall Requirements, See GD16002.

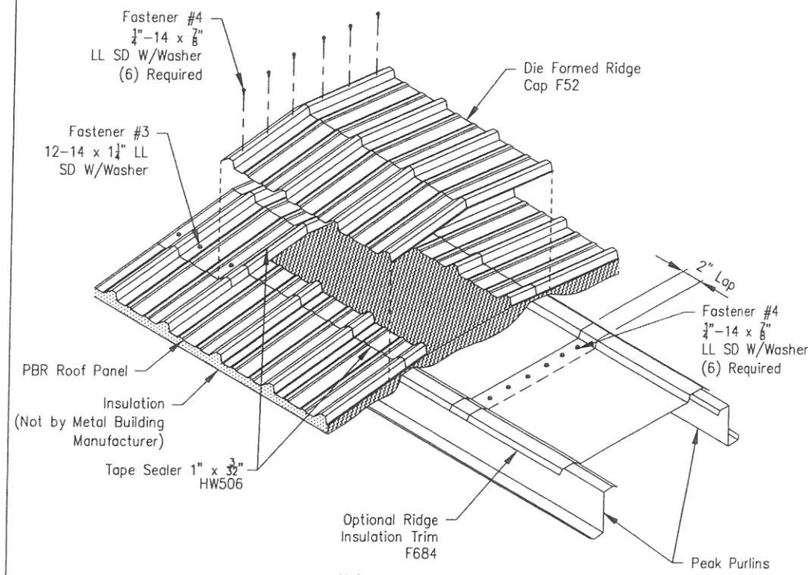


Detail at Rake Slide Trim

Wall Panel	Foam Closure
PBR	HW456/HW422
AVP	HW465
PBU	HW460
VistaShadow	HW465
NuWall	HW424
PBC	HW462
PBD	HW463
ShadowRib	HW412
Designer Series (Fluted Only)	HW4037
RBR (Reverse Rolled PBR)	HW455
RBU (Reverse Rolled PBU)	HW459
7.2	HW461

PBR Roof Panel
Fixed Ridge Detail

Page GPR06003 | Date Apr '19 | Rev 04



Note:
Install optional ridge insulation trim F684 as insulation and roof sheeting is being applied. Temporary attachment, if required, is not by metal building manufacturer.

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
A	1/13/20	FOR CONSTRUCTION PERMIT	FXD	HPD	CM

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NORTH CAROLINA PROFESSIONAL SEAL 023748 ENGINEER MICHAEL W. CUSTER
1/13/20

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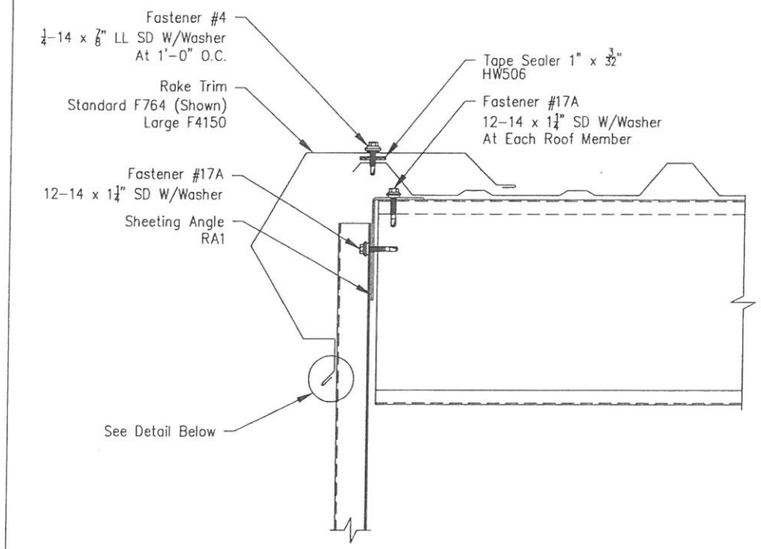
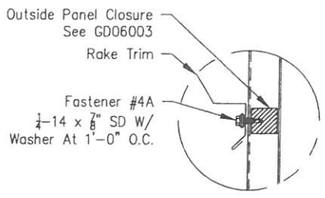
PBR Roof Panel - Southern Standard And Southern Large Classic Rake Trim - Sheeted Wall

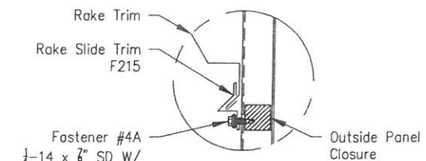
Page WPR05002 | Date May '19 | Rev 08





Standard

For Roof Runs 100'–0" Or Less

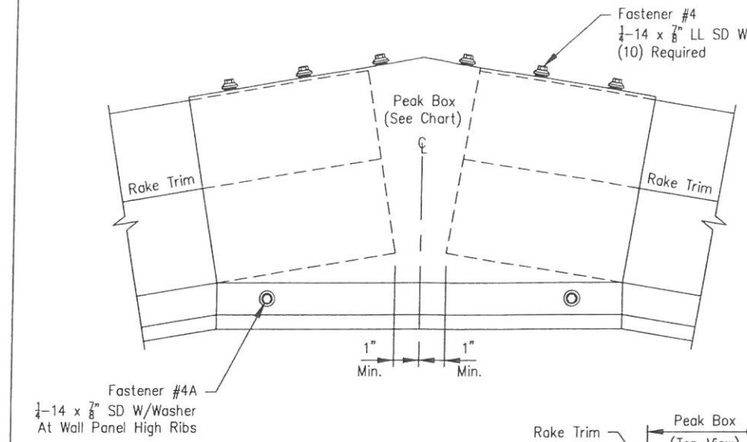


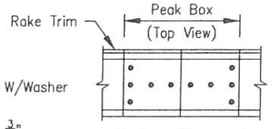
Alternate

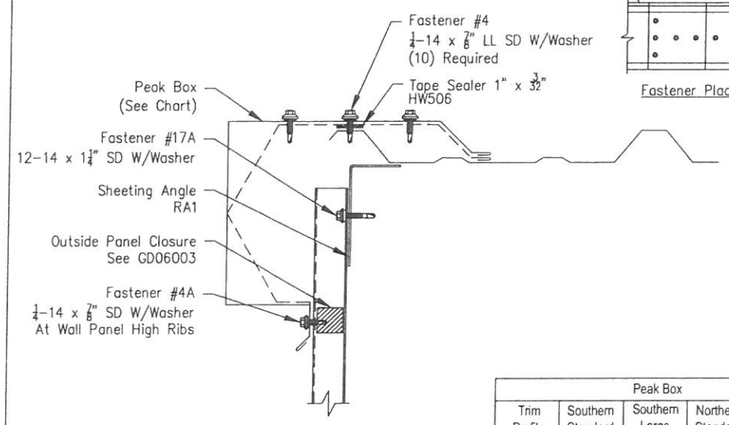
For Roof Runs Greater Than 100'–0"

PBR Roof Panel Peak Box At Fixed Ridge

Page GPR05001 | Date Apr '19 | Rev 06





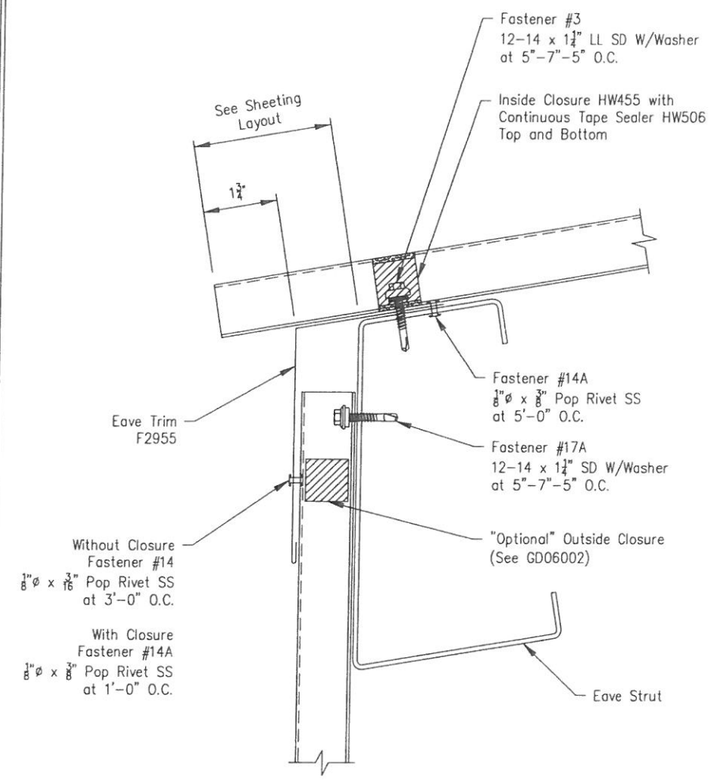


Note:
Flashing profile varies dependent on the building order. Attachment as illustrated is applicable for all profiles.

Peak Box				
Trim Profile	Southern Standard	Southern Large	Northern Standard	Northern Large
Cascading	F1519	F1585	F1672	F1760
Classic	F160	F4153	F381	F1024
Contoured	F2219	F2285	F2372	F2460
Signature	F916	F3853	F236	F1018

PBR Panel Flat Eave Trim - Sheeted Wall

Page GPR03004 | Date Feb '17 | Rev 05



ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
A	1/13/20	FOR CONSTRUCTION PERMIT	FXD	HPD	CM

BUILDING IS MANUFACTURED BY SW STEEL FACTORY

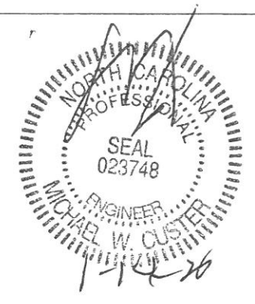
MICHAEL W. CUSTER, P.E.
642 OAKBEND DRIVE
COPPEL TX. 75019
PH. 972-571-7082

PROJECT: THOMAS SANDERS

CUSTOMER: THOMAS SANDERS | OWNER: THOMAS SANDERS

LOCATION: LILLINGTON, NC 27546

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	1/13/20	N.T.S.	1	A	17-B-48481	DET9	A



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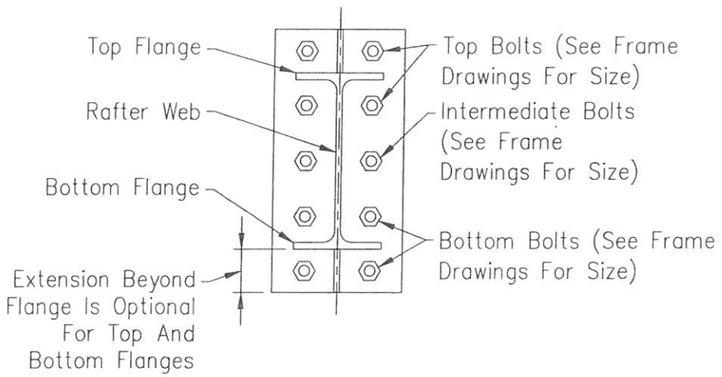
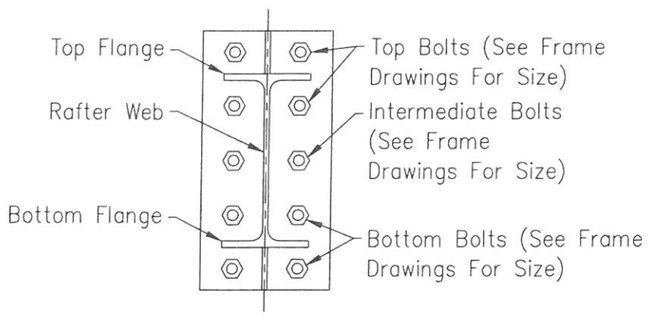


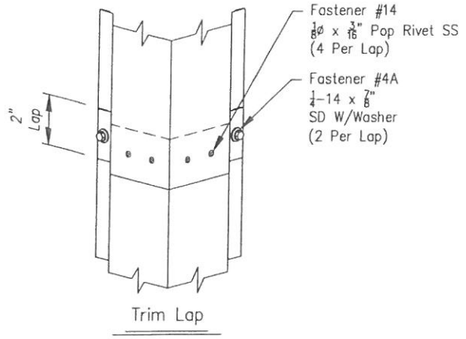
U2	Bolts At Rigid Frame Ridge Rafter Connection	Date Jun '17
Page MB–U2		Rev 00



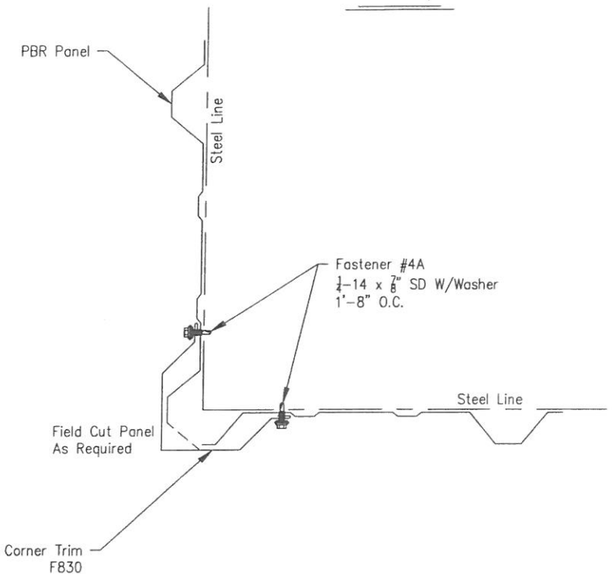
U3	Bolts At Rigid Frame Rafter To Column Connection	Date Jun '17
Page MB–U3		Rev 00

PBR Wall Panel
Outside Corner - On Module

Page PW03001 | Date Apr '19 | Rev 08

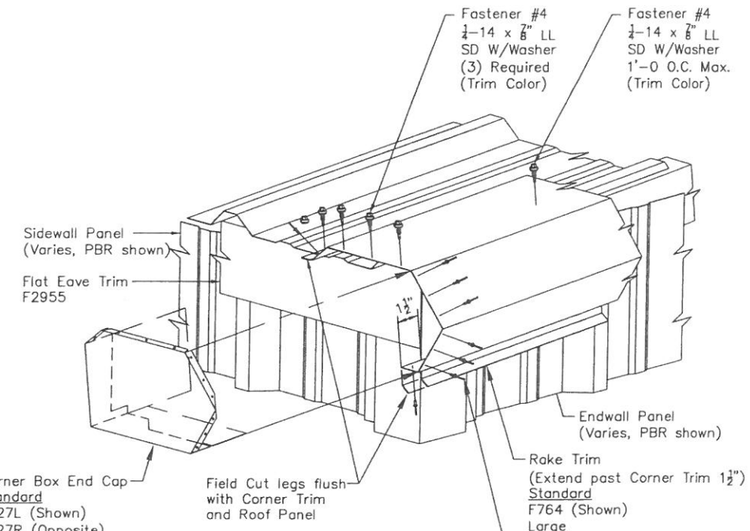


Trim Lap



PBR Roof Panel - Southern Standard and Southern Large
Low Eave Rake Corner with Flat Eave Trim - 1¼" Wall Panel

Page WPR04006 | Date Jul '17 | Rev 05



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	1/13/20	N.T.S.	1	A	17–B–48481	DET10	A

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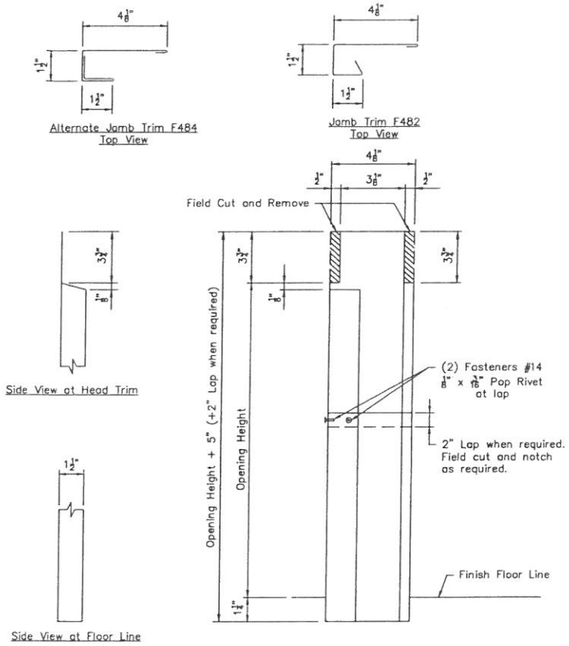
PBR Wall Panel - Three Sided Framed Opening
Jamb Trim Field Cut Details

Page PW07027 | Date May '19 | Rev 03

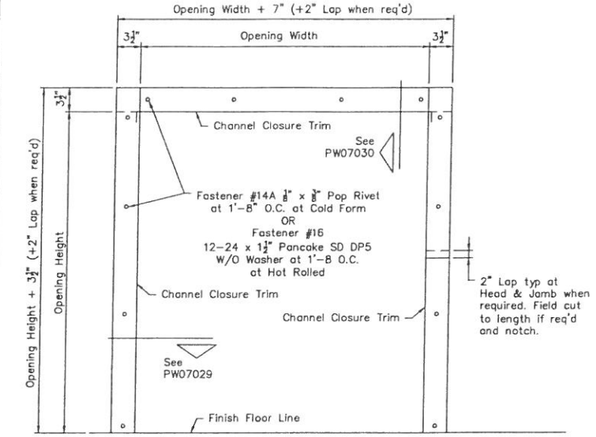


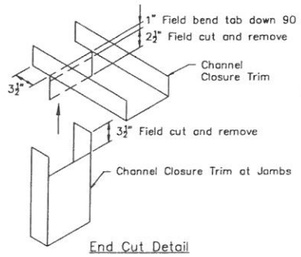
Jamb Trim F482 and
Alternate Jamb Trim F484
Front View
Right Jamb Trim as shown
Left Jamb Trim opposite hand

Note: Field measure Opening Height before making field cuts and adjust cut dimensions accordingly.

PBR Wall Panel - Three Sided Framed Opening
"Optional" Channel Closure Trim

Page PW07028 | Date May '19 | Rev 04





End Cut Detail

Optional
Channel Closure Trim Piece Mark

F981	– 8" Member	F2993	– 10½" Member
F2994	– 8¼" Member	F169	– 12" Member
F982	– 10" Member	F2995	– 12½" Member

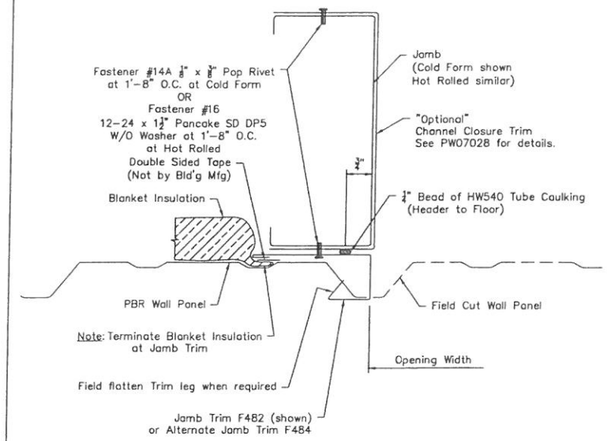
Note: All trim is to be installed BEFORE blanket insulation is applied to walls.

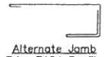
Note: Field measure Opening Width and Height before making field cuts and adjust cut dimensions accordingly.

Note: The interior leg of the Channel Closure Trim is to be oriented (±1/8") to match the interior leg of the Header or Jamb.

PBR Wall Panel - Three Sided Framed Opening - Jamb Trim Installation

Page PW07029 | Date Apr '19 | Rev 04





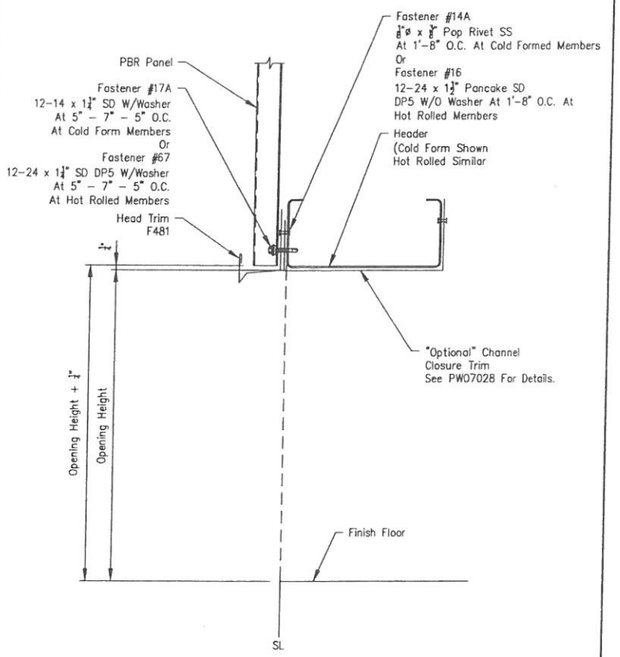
Alternate Jamb
Trim F484 Profile

Note: All trim is to be installed BEFORE blanket insulation is applied to walls.

Note: Panel position is shown with Panel Rib and Opening on 1'-0" module. Location of Rib may vary depending on the Opening Width and location. Field measure before cutting Panel and Trim.

PBR Wall Panel - Three Sided Framed Opening
Head Trim Installation

Page PW07030 | Date Oct '19 | Rev 03



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CUSTOMER: THOMAS SANDERS
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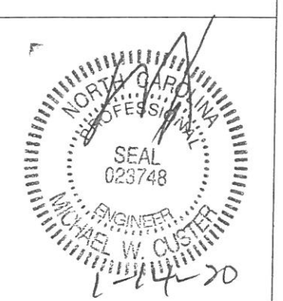

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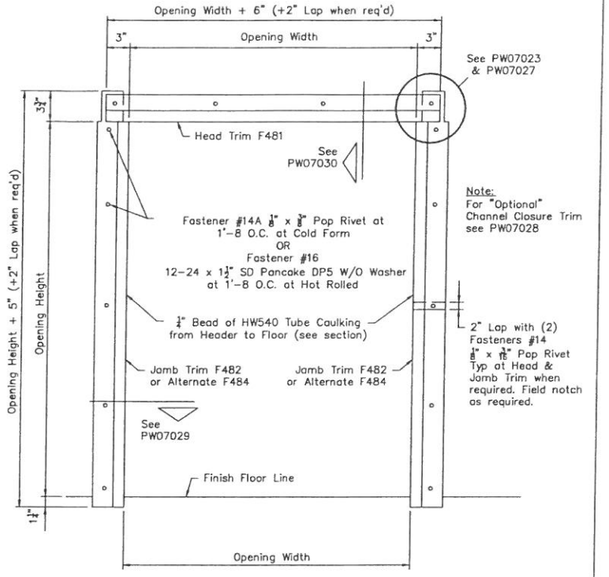
PBR Wall Panel - Three Sided Framed Opening - Trim Installation with Field Notch Panel at Head Trim

Page PW07022 | Date Apr '19 | Rev 04

Note: Trim Installation can be done by Field Notch Panel as shown on PW07022 & PW07023 OR with Field Notch and Bend Tabs at Head Trim as shown on PW07024 & PW07025.



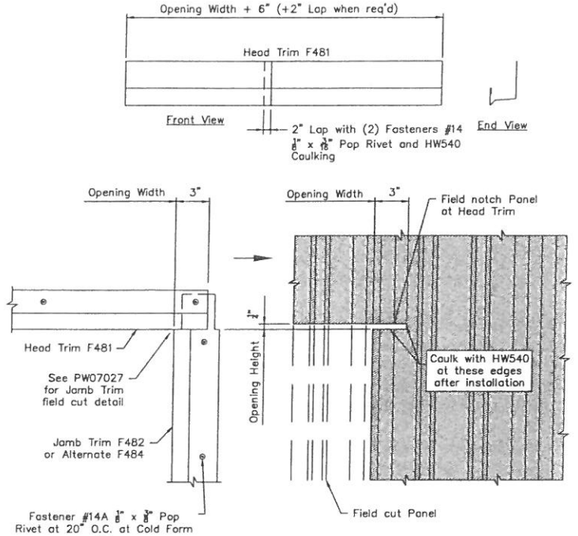
Note: All trim is to be installed BEFORE blanket insulation is applied to walls.

Note: Field measure Opening Width and Height before making field cuts and adjust cut dimensions accordingly.

PBR Wall Panel - Three Sided Framed Opening Field Notch Panel at Head Trim

Page PW07023 | Date Apr '19 | Rev 04

Note: Trim Installation can be done by Field Notch Panel as shown on PW07022 & PW07023 OR with Field Notch and Bend Tabs at Head Trim as shown on PW07024 & PW07025.



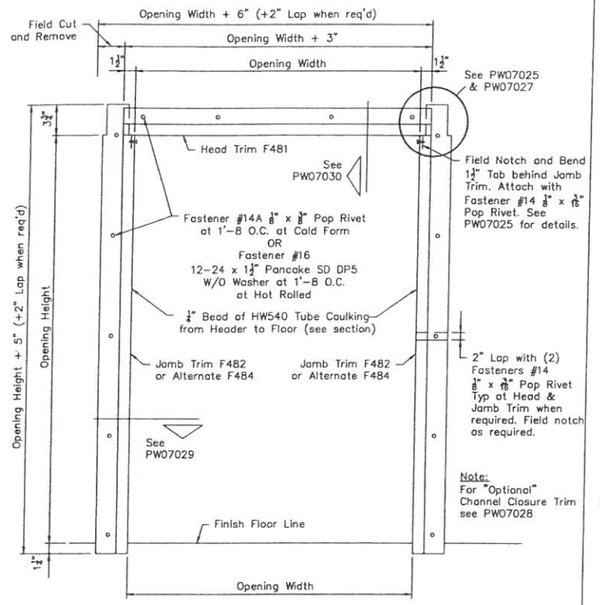
Note: All trim is to be installed BEFORE blanket insulation is applied to walls

Note: Panel position is shown with Panel Rib and Opening on 1'-0 module. Location of Rib may vary depending on the Opening Width and location. Field measure before cutting Panel and Trim.

PBR Wall Panel - Three Sided Framed Opening - Trim Installation with Field Notch and Bend Tabs at Head Trim

Page PW07024 | Date Apr '19 | Rev 04

Note: Trim Installation can be done by Field Notch Panel as shown on PW07022 & PW07023 OR with Field Notch and Bend Tabs at Head Trim as shown on PW07024 & PW07025.



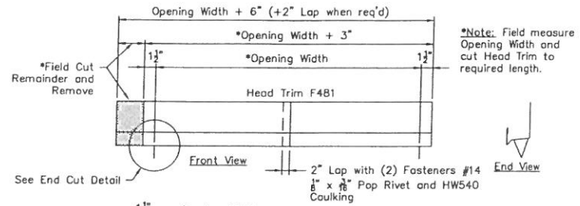
Note: All trim is to be installed BEFORE blanket insulation is applied to walls.

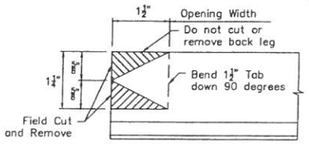
Note: Field measure Opening Width and Height before making field cuts and adjust cut dimensions accordingly.

PBR Wall Panel - Three Sided Framed Opening - Field Notch and Bend Tabs at Head Trim

Page PW07025 | Date Apr '19 | Rev 04

Note: Trim Installation can be done by Field Notch Panel as shown on PW07022 & PW07023 OR with Field Notch and Bend Tabs at Head Trim as shown on PW07024 & PW07025.

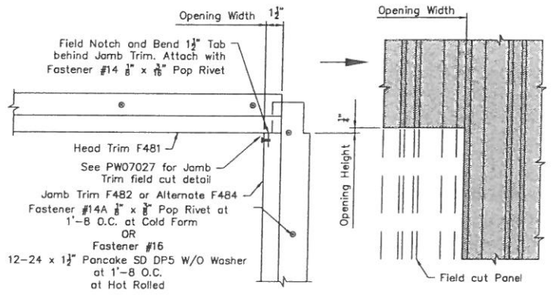




End Cut Detail
(Viewed from top of Head Trim)

Note: All trim is to be installed BEFORE blanket insulation is applied to walls

Note: Panel position is shown with Panel Rib and Opening on 1'-0 module. Location of Rib may vary depending on the Opening Width and location. Field measure before cutting Panel and Trim.



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	1/13/20	N.T.S.	1	A	17-B-48481	DET12	A

NORTH CAROLINA PROFESSIONAL SEAL 023748 ENGINEER MICHAEL W. CUSTER

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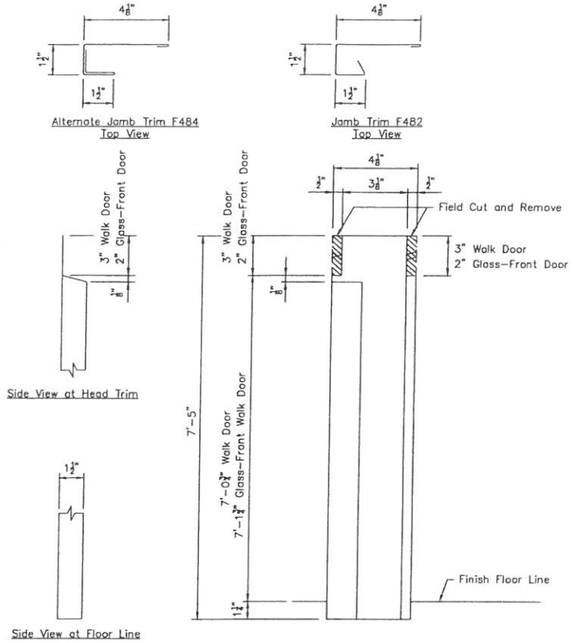
PBR Wall Panel - Walk Door And Glass-Front Walk Door Jamb Trim Field Cut Details

Page PW09027 | Date May '19 | Rev 02

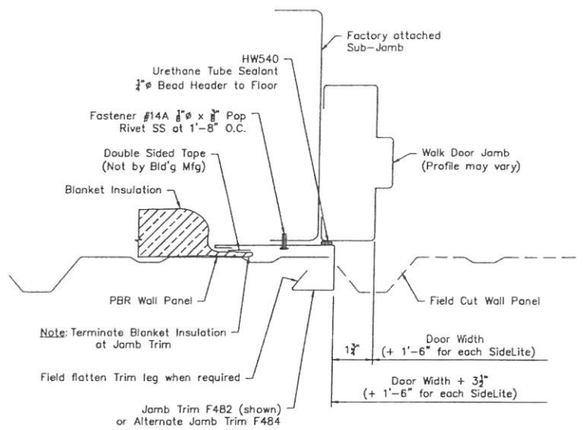


Jamb Trim F482 and Alternate Jamb Trim F484
Front View
Right Jamb Trim as shown
Left Jamb Trim opposite hand

Note: Field measure Door Height before making field cuts and adjust cut dimensions accordingly so that Jamb Trim fits to Head Trim & at 1¾" below Finish Floor Line.

PBR Wall Panel - Pre-Assembled Walk Door & Glass-Front Walk Door Jamb Trim Installation

Page PW09029 | Date Apr '19 | Rev 04



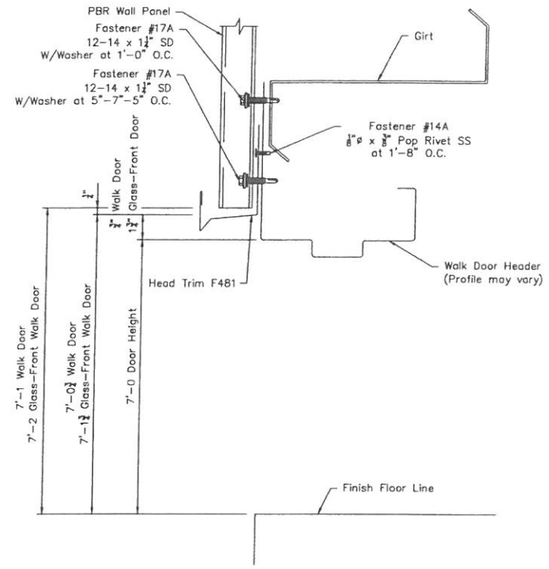
Alternate Jamb Trim F484 Profile

Note: All trim is to be installed BEFORE blanket insulation is applied to walls.

Note: Panel position is shown with Panel Rib and Door on 1'-0" module. Location of Rib may vary depending on the Door Width and location. Field measure before cutting Panel and Trim.

PBR Wall Panel - Pre-Assembled Walk Door And Glass-Front Walk Door Head Trim Installation

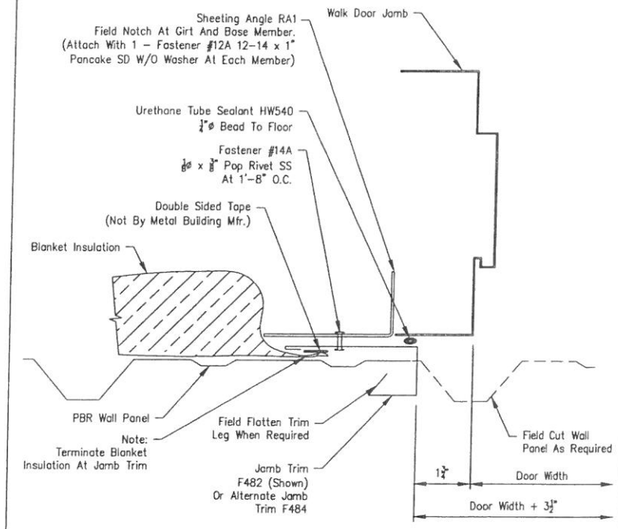
Page PW09030 | Date May '19 | Rev 03



Note: All trim is to be installed BEFORE blanket insulation is applied to walls.

PBR Wall Panel - Knock Down DoorJamb Trim Installation

Page PW09031 | Date Apr '19 | Rev 06



Alternate Jamb Trim F484

Note:
All Trim Is To Be Installed BEFORE Blanket Insulation Is Applied To Walls.

Location Of Panel Rib May Vary Depending On Door Width, Door Location And Panel Starting Location. Refer To Erection Drawings For Panel Layout. Field Measure Before Cutting Panel And Trim.

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NORTH CAROLINA PROFESSIONAL SEAL 023748 ENGINEER MICHAEL W. CUSTER
1-14-20

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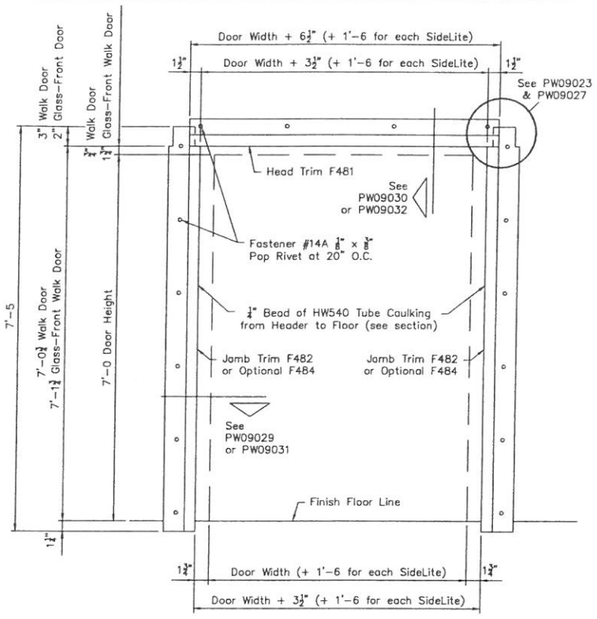
PBR Wall Panel - Walk Door & Glass-Front Walk Door - Trim Installation with Field Notch Panel at Head Trim

Page PW09022 | Date Apr '19 | Rev 02

Note: Trim Installation can be done by Field Notch Panel as shown on PW09022 & PW09023 OR with Field Notch and Bend Tabs at Head Trim as shown on PW09024 & PW09025.



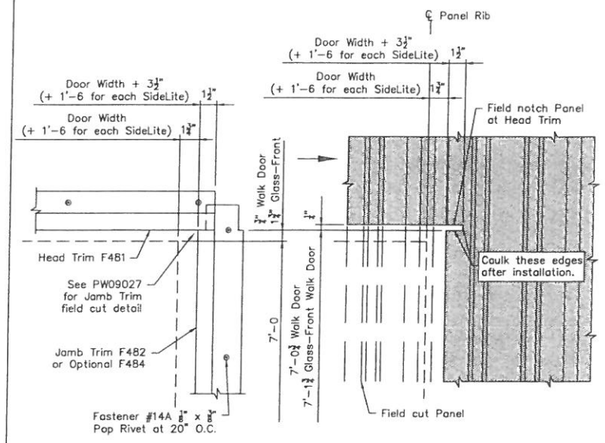
Note: All trim is to be installed BEFORE blanket insulation is applied to walls.

Note: Field measure Door Width and Height before making field cuts and adjust cut dimensions accordingly.

PBR Wall Panel - Walk Door And Glass-Front Walk Door Field Notch Panel at Head Trim

Page PW09023 | Date May '19 | Rev 01

Note: Trim Installation can be done by Field Notch Panel as shown on PW09022 & PW09023 OR with Field Notch and Bend Tabs at Head Trim as shown on PW09024 & PW09025.



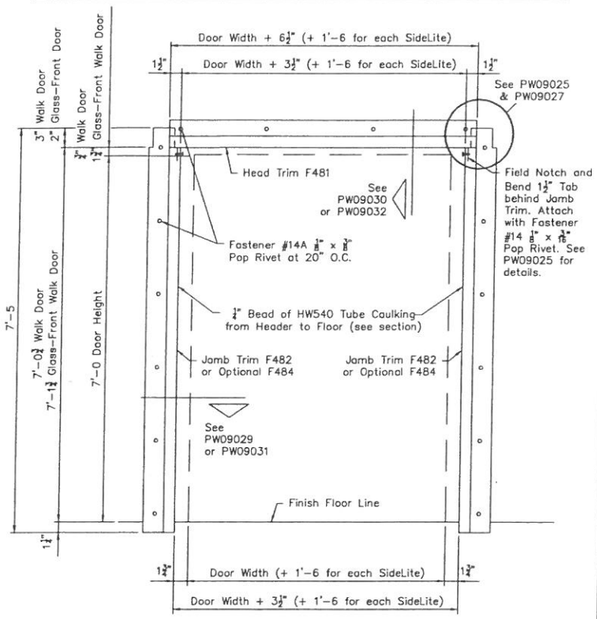
Note: All trim is to be installed BEFORE blanket insulation is applied to walls.

Note: Panel position is shown with Panel Rib and Door on 1'-0 module. Location of Rib may vary depending on the Door Width and location. Field measure before cutting Panel and Trim.

PBR Wall Panel - Walk Door & Glass-Front Walk Door - Trim Installation with Field Notch and Bend Tabs at Head Trim

Page PW09024 | Date Apr '19 | Rev 02

Note: Trim Installation can be done by Field Notch Panel as shown on PW09022 & PW09023 OR with Field Notch and Bend Tabs at Head Trim as shown on PW09024 & PW09025.



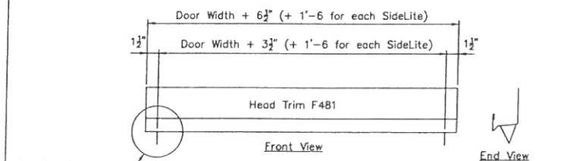
Note: All trim is to be installed BEFORE blanket insulation is applied to walls.

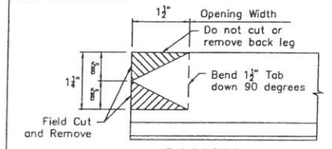
Note: Field measure Door Width and Height before making field cuts and adjust cut dimensions accordingly.

PBR Wall Panel - Walk Door And Glass-Front Walk Door - Field Notch and Bend Tabs at Head Trim

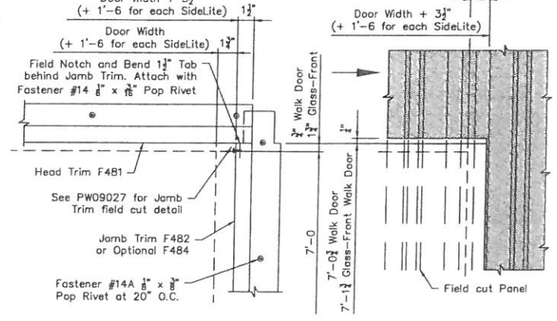
Page PW09025 | Date May '19 | Rev 01

Note: Trim Installation can be done by Field Notch Panel as shown on PW09022 & PW09023 OR with Field Notch and Bend Tabs at Head Trim as shown on PW09024 & PW09025.





End Cut Detail
(Viewed from top of Head Trim)



Note: All trim is to be installed BEFORE blanket insulation is applied to walls.

Note: Panel position is shown with Panel Rib and Door on 1'-0 module. Location of Rib may vary depending on the Door Width and location. Field measure before cutting Panel and Trim.

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	1/13/20	N.T.S.	1	A	17-B-48481	DET14	A

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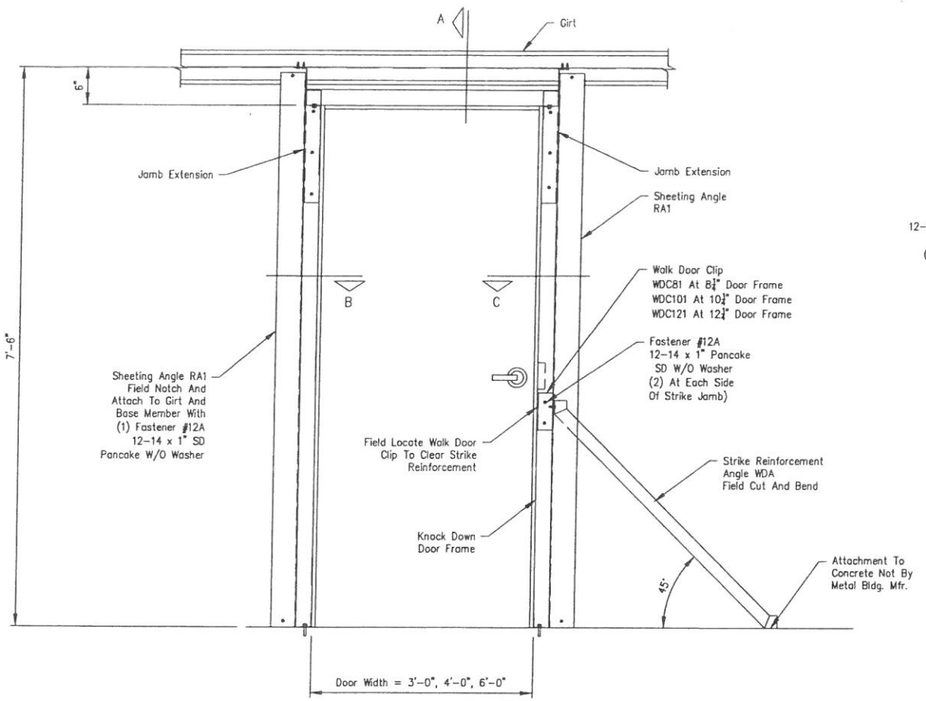
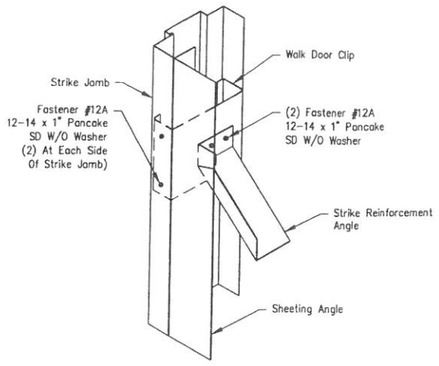
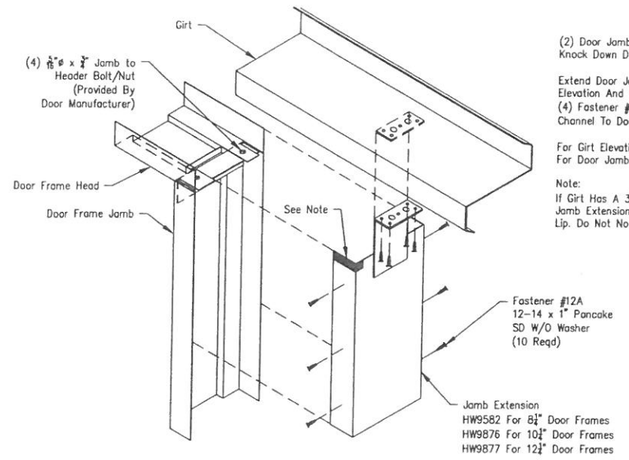


Door Elevation



Walk Door Clip/Strike Reinforcement Angle Isometric



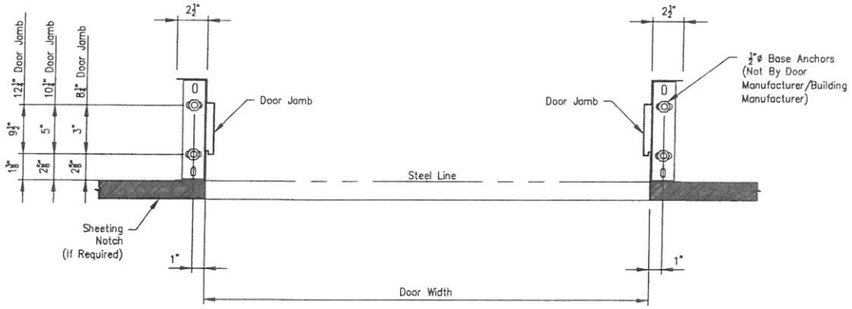
Door Jamb Extension Isometric

(2) Door Jamb Extensions Are Required For All Knock Down Doors.

Extend Door Jamb Extension To The 7'-6" Girt Elevation And Attach To The Web Of The Girt With (4) Fastener #12A, Attach Door Jamb Extension Channel To Door Jamb With (6) Fastener #12A.

For Girt Elevations Above 7'-6" Refer To AC05132 For Door Jamb Extension Requirements.

Note:
If Girt Has A 3½" Flange, Field Notch Jamb Extension Channel To Clear Girt Lip. Do Not Notch Girt Lip.

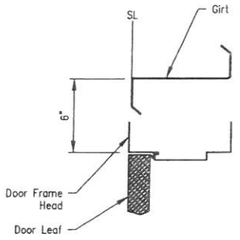


The Adequacy Of The ½"ø Base Anchor Is Not The Responsibility Of The Building Manufacturer. The Adequacy Of These Base Anchors Should Be Determined By A Qualified Foundation Engineer.

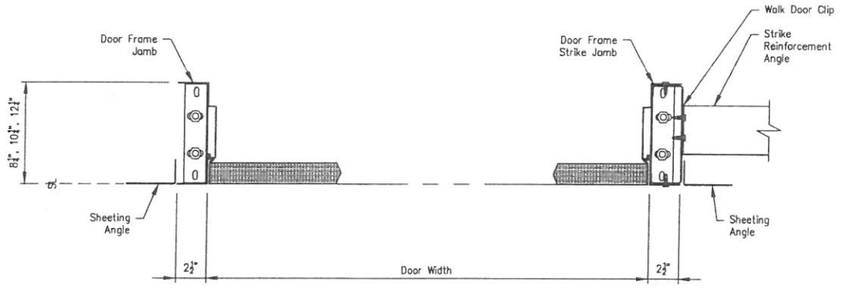
Verify Door Jamb Base Clip Dimensions With Patterns Shown Prior To Placement Of Door Anchors And Adjust Patterns If Needed.

Note: 12¼" Frames May Not Have Kerf Door Frame Feature Depending On Door Manufacturer.

Knock Down Door Anchor Placement



Section A



Section B

Section C

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
A	1/13/20	FOR CONSTRUCTION PERMIT	FXD	HPD	CM

BUILDING IS MANUFACTURED BY SW STEEL FACTORY

MICHAEL W. CUSTER, P.E.
642 OAKBEND DRIVE
COPPEL TX. 75019
PH. 972-571-7082

PROJECT: THOMAS SANDERS
CUSTOMER: THOMAS SANDERS
OWNER: THOMAS SANDERS
LOCATION: LILLINGTON, NC 27546

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	1/13/20	N.T.S.	1	A	17-B-48481	DET15	A

NORTH CAROLINA PROFESSIONAL SEAL 023748 ENGINEER MICHAEL W. CUSTER
1-14-20

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Knock Down Door - Girt At 7'-6" Without Low Girt

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Date	Nov '18	Rev 00