

Graphical Illustration - Not To Scale
 Member Cut Length - 30'
 Member Pitch - 0/12

Design Information:

Building Code: IRC2015	Floor Dead Load: 10.0 lb/ft²	Roof Dead Load: 10.0 lb/ft²	Ground Snow Load: 20.0 lb/ft²
Design Methodology: ASD	Floor Live Load: 40.0 lb/ft²	Roof Live Load: 20.0 lb/ft²	
	Unbraced Length Top: 1'- 10 1/2"	Bottom: 30'	

Design Results:

	Location	Design	Control	Result	LDF	Load Combination
Critical Moment (Pos)	4'- 3/4"	4305.04 lb ft	15236.13 lb ft	Passed - 28%	1.15	D + Lr
Critical Moment (Neg)	20'	-5177.36 lb ft	9967.83 lb ft	Passed - 52%	1.15	D + Lr
Critical Shear	9'	2534.72 lb	7198.04 lb	Passed - 35%	1.15	D + Lr
Live Load Deflection	5'	0'- 1/16"	0'- 3/4" (L/360)	Passed - L/999	-	Lr
Total Load Deflection	25'- 1 13/16"	0'- 1/8"	0'- 1" (L/240)	Passed - L/812	-	D + Lr
Max. Reaction			Supported Mt Supporting Mt			
	0'- 4 1/2"	2439.47 lb	14437.37 lb 13956.13 lb	Passed - 17%	1.15	D + Lr
	0'- 4 1/2"	-310.78 lb	20086.78 lb -	Passed - 2%	1.60	0.6D + 0.6W
	10'	5567.76 lb	15421.77 lb 13956.15 lb	Passed - 40%	1.15	D + Lr
	10'	-769.78 lb	21456.38 lb -	Passed - 6%	1.60	0.6D + 0.6W
	20'	5799.66 lb	15421.71 lb 13956.09 lb	Passed - 42%	1.15	D + Lr
	20'	-767.97 lb	21456.30 lb -	Passed - 6%	1.60	0.6D + 0.6W
	29'- 7 1/2"	2446.38 lb	14437.40 lb 13956.15 lb	Passed - 18%	1.15	D + Lr
	29'- 7 1/2"	-305.83 lb	20086.82 lb -	Passed - 2%	1.60	0.6D + 0.6W

Design Notes:

* Member design assumed proper ply to ply connection. Verify connection between plies according to code specification

Loading:

Type	Start	End	Source	Maximum Load Magnitudes			
				Dead	Floor Live	Roof Live	Snow
Self Weight	0'	30'	Self Weight	9 lb/ft	-	-	-
Uniform	1'- 3/4"	29'- 3/4"	Smoothed Load	248 lb/ft	-	260 lb/ft	142 lb/ft
Point	0'- 3/4"	0'- 3/4"	CP1SE(c02)	371.00 lb	-	269.00 lb	147.00 lb
Point	2'- 3/4"	2'- 3/4"	CP1(c01)	-	-	-	-
Point	4'- 3/4"	4'- 3/4"	CP1(c02)	-	-	-	-
Point	6'- 3/4"	6'- 3/4"	CP1(c03)	-	-	-	-
Point	8'- 3/4"	8'- 3/4"	CP1(c04)	-	-	-	-
Point	10'- 3/4"	10'- 3/4"	CP1(c05)	-	-	-	-
Point	12'- 3/4"	12'- 3/4"	CP1(c06)	-	-	-	-
Point	14'- 3/4"	14'- 3/4"	CP1(c07)	-	-	-	-
Point	16'- 3/4"	16'- 3/4"	CP1(c08)	-	-	-	-
Point	18'- 3/4"	18'- 3/4"	CP1(c09)	-	-	-	-
Point	20'- 3/4"	20'- 3/4"	CP1(c10)	-	-	-	-
Point	22'- 3/4"	22'- 3/4"	CP1(c11)	-	-	-	-
Point	24'- 3/4"	24'- 3/4"	CP1(c12)	-	-	-	-
Point	26'- 3/4"	26'- 3/4"	CP1(c13)	-	-	-	-
Point	28'- 3/4"	28'- 3/4"	CP1(c14)	-	-	-	-
Point	29'- 11 1/4"	29'- 11 1/4"	CP1SE(c01)	362.00 lb	-	250.00 lb	137.00 lb

Support Information:

Support	Start	End	Source	Maximum Analysis Reactions			
				Dead	Floor Live	Roof Live	Snow
1	0'	0'- 5 1/2"	PBO9(i86)	1208.00 lb	-	1237.00/-131.00 lb	605.00 lb
2	9'- 9 1/4"	10'- 2 3/4"	PBO8(i84)	2763.00 lb	-	3034.00 lb	1535.00 lb
3	19'- 9 1/4"	20'- 2 3/4"	PBO7(i89)	2759.00 lb	-	3030.00 lb	1531.00 lb
4	29'- 6 1/2"	30'	PBO6(i91)	1218.00 lb	-	1231.00/-132.00 lb	601.00 lb

- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.

- This report is based on modeled conditions input by the user. Actual field conditions may differ from those shown. These results should be reviewed by a qualified design professional.

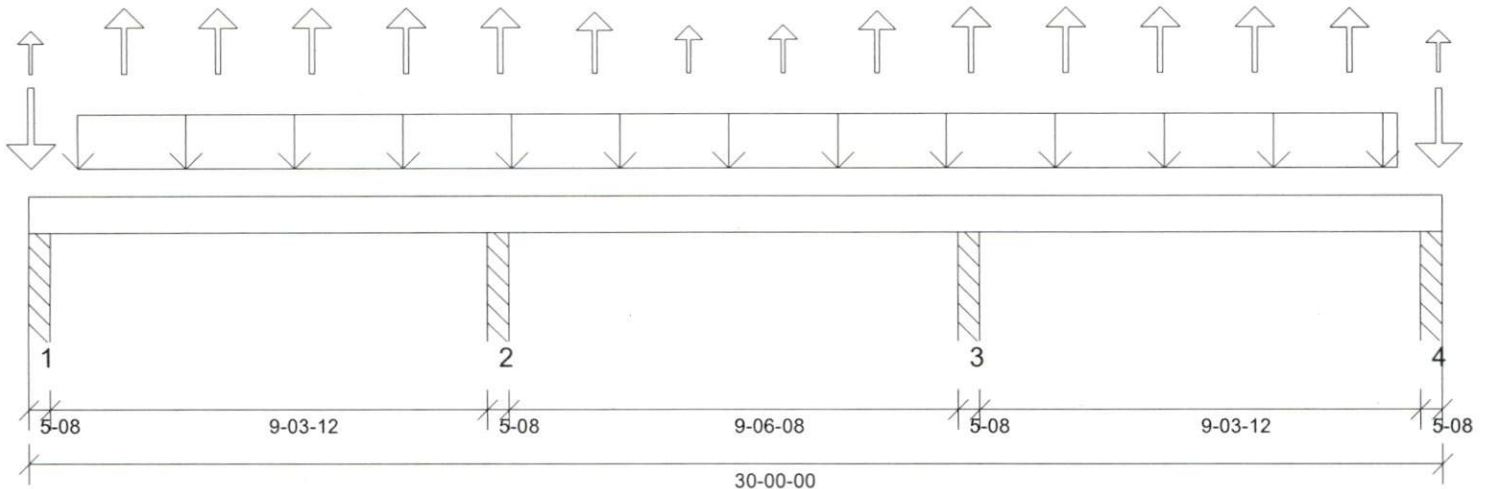


Job: 19100105
Member Type: Beam | Level: Carport
MiTek SAPPHIRE™ Structure Version 8.3.1.215.Update3
Designed by Single Member Design Engine
Member: 2 - 2.0 RigidLam LVL 1-3/4 x 9-1/4

Label: BM3-i87
Page: 4 of 4
Date: 11/11/2019 08:02:01
Status: Design Passed

Errors, Warnings & Notes:

- * CAUTION: The maximum net analysis reaction exceeds the user-defined maximum uplift value at one or more supports.
- * The dead loads used in the design of this member were applied to the structure as projected dead loads.
- * The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
- * Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.



Graphical Illustration - Not To Scale
 Member Cut Length - 30'
 Member Pitch - 0/12

Design Information:

Building Code: IRC2015	Floor Dead Load: 10.0 lb/ft ²	Roof Dead Load: 10.0 lb/ft ²	Ground Snow Load: 20.0 lb/ft ²
Design Methodology: ASD	Floor Live Load: 40.0 lb/ft ²	Roof Live Load: 20.0 lb/ft ²	
	Unbraced Length Top: 1'- 10 1/2"	Bottom: 30'	

Design Results:

	Location	Design	Control	Result	LDF	Load Combination
Critical Moment (Pos)	4'- 3/4"	4274.65 lb ft	15236.13 lb ft	Passed - 28%	1.15	D + Lr
Critical Moment (Neg)	20'	-5142.49 lb ft	9967.83 lb ft	Passed - 52%	1.15	D + Lr
Critical Shear	9'	2517.46 lb	7198.04 lb	Passed - 35%	1.15	D + Lr
Live Load Deflection	5'	0'- 1/16"	0'- 3/4" (L/360)	Passed - L/999	-	Lr
Total Load Deflection	25'- 1 13/16"	0'- 1/8"	0'- 1" (L/240)	Passed - L/817	-	D + Lr
Max. Reaction	0'- 4 1/2"	2423.09 lb	Supported Mt: 14437.37 lb Supporting Mt: 13956.13 lb	Passed - 17%	1.15	D + Lr
	0'- 4 1/2"	-307.79 lb	20086.78 lb	Passed - 2%	1.60	0.6D + 0.6W
	10'	5529.93 lb	15421.77 lb	Passed - 40%	1.15	D + Lr
	10'	-762.47 lb	21456.38 lb	Passed - 5%	1.60	0.6D + 0.6W
	20'	5760.57 lb	15421.71 lb	Passed - 41%	1.15	D + Lr
	20'	-761.28 lb	21456.30 lb	Passed - 5%	1.60	0.6D + 0.6W
	29'- 7 1/2"	2436.92 lb	14437.40 lb	Passed - 17%	1.15	D + Lr
	29'- 7 1/2"	-298.92 lb	20086.82 lb	Passed - 2%	1.60	0.6D + 0.6W

Design Notes:

* Member design assumed proper ply to ply connection. Verify connection between plies according to code specification

Loading:

Type	Start	End	Source	Maximum Load Magnitudes			
				Dead	Floor Live	Roof Live	Snow
Self Weight	0'	30'	Self Weight	9 lb/ft	-	-	-
Uniform	1'- 3/4"	29'- 3/4"	Smoothed Load	248 lb/ft	-	258 lb/ft	141 lb/ft
Point	0'- 3/4"	0'- 3/4"	CP1SE(c02)	370.00 lb	-	266.00 lb	145.00 lb
Point	2'- 3/4"	2'- 3/4"	CP1(c01)	-	-	-	-
Point	4'- 3/4"	4'- 3/4"	CP1(c02)	-	-	-	-
Point	6'- 3/4"	6'- 3/4"	CP1(c03)	-	-	-	-
Point	8'- 3/4"	8'- 3/4"	CP1(c04)	-	-	-	-
Point	10'- 3/4"	10'- 3/4"	CP1(c05)	-	-	-	-
Point	12'- 3/4"	12'- 3/4"	CP1(c06)	-	-	-	-
Point	14'- 3/4"	14'- 3/4"	CP1(c07)	-	-	-	-
Point	16'- 3/4"	16'- 3/4"	CP1(c08)	-	-	-	-
Point	18'- 3/4"	18'- 3/4"	CP1(c09)	-	-	-	-
Point	20'- 3/4"	20'- 3/4"	CP1(c10)	-	-	-	-
Point	22'- 3/4"	22'- 3/4"	CP1(c11)	-	-	-	-
Point	24'- 3/4"	24'- 3/4"	CP1(c12)	-	-	-	-
Point	26'- 3/4"	26'- 3/4"	CP1(c13)	-	-	-	-
Point	28'- 3/4"	28'- 3/4"	CP1(c14)	-	-	-	-
Point	29'- 11 1/4"	29'- 11 1/4"	CP1SE(c01)	362.00 lb	-	250.00 lb	137.00 lb

Support Information:

Support	Start	End	Source	Maximum Analysis Reactions			
				Dead	Floor Live	Roof Live	Snow
1	0'	0'- 5 1/2"	PBO2(i93)	1204.00 lb	-	1225.00/-130.00 lb	598.00 lb
2	9'- 9 1/4"	10'- 2 3/4"	PBO3(i90)	2752.00 lb	-	3005.00 lb	1519.00 lb
3	19'- 9 1/4"	20'- 2 3/4"	PBO4(i85)	2748.00 lb	-	3001.00 lb	1515.00 lb
4	29'- 6 1/2"	30'	PBO5(i88)	1215.00 lb	-	1224.00/-131.00 lb	597.00 lb

- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.

- This report is based on modeled conditions input by the user. Actual field conditions may differ from those shown. These results should be reviewed by a qualified design professional.



Job: 19100105
Member Type: Beam | Level: Carport
MiTek SAPPHIRE™ Structure Version 8.3.1.215.Update3
Designed by Single Member Design Engine

Member: 2 - 2.0 RigidLam LVL 1-3/4 x 9-1/4

Label: BM2-i92

Page: 2 of 4
Date: 11/11/2019 08:02:00

Status: Design Passed

Errors, Warnings & Notes:

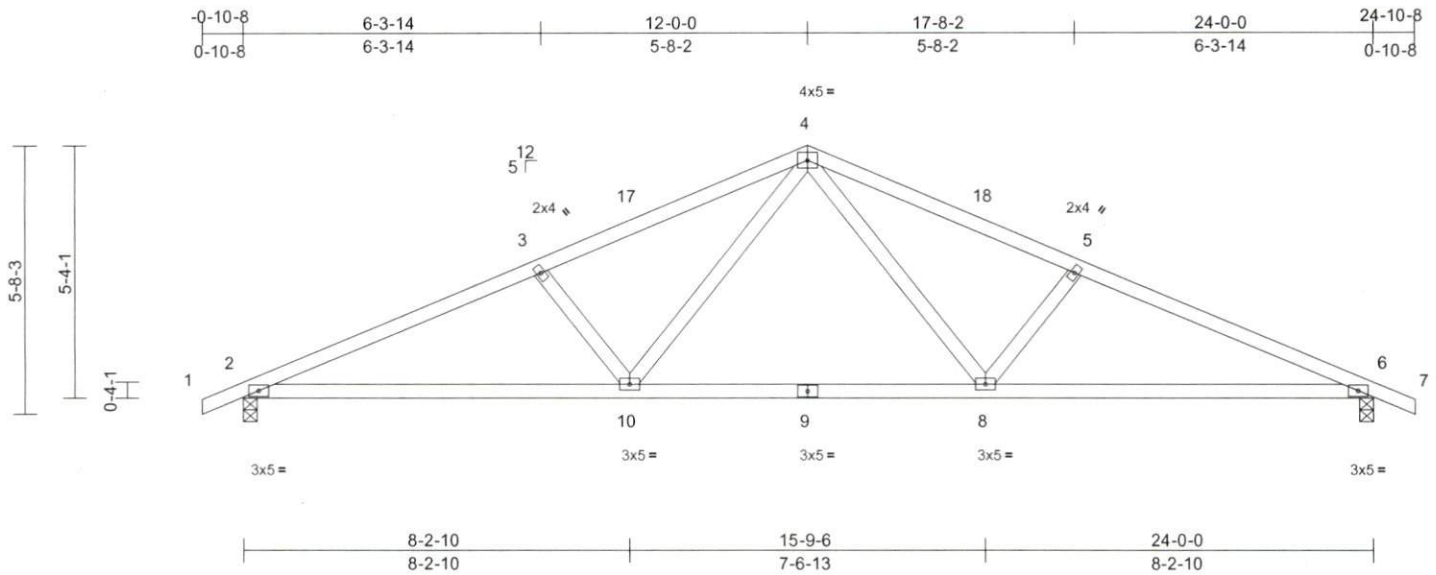
- * CAUTION: The maximum net analysis reaction exceeds the user-defined maximum uplift value at one or more supports.
- * The dead loads used in the design of this member were applied to the structure as projected dead loads.
- * The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
- * Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.

Job 19100105-B	Truss CP1	Truss Type Common	Qty 14	Ply 1	Job Reference (optional)	E13745809
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Carter Components (Sanford), Sanford, NC - 27332,

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Page: 1



Scale = 1:47

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	Vert(LL)	-0.09	8-16	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	Vert(CT)	-0.24	8-16	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	Horz(CT)	0.06	6	n/a	n/a		
BCLL	0.0*	Code	IRC2015/TPI2014	Matrix-MSH							
BCDL	10.0									Weight: 105 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2 *Except* 8-5,10-3:2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-8-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-7-3 oc bracing.

REACTIONS

(lb/size) 2=854/0-3-8, 6=854/0-3-8
Max Horiz 2=-53 (LC 16)
Max Uplift 2=-8 (LC 15), 6=-8 (LC 16)
Max Grav 2=1013 (LC 2), 6=1013 (LC 2)

FORCES

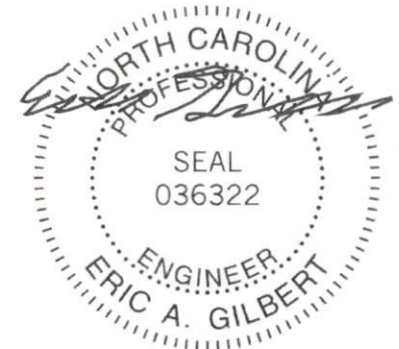
(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/25, 2-3=-1923/487, 3-17=-1710/438, 4-17=-1635/457, 4-18=-1635/457, 5-18=-1710/438, 5-6=-1923/487, 6-7=0/25
BOT CHORD 2-10=-363/1742, 9-10=-161/1150, 8-9=-161/1150, 6-8=-366/1742
WEBS 4-8=-115/612, 5-8=-395/226, 4-10=-115/612, 3-10=-395/226

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior (2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=13.9 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Fully Exp.; Ct=1.10

- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 8) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 6. This connection is for uplift only and does not consider lateral forces.
- 9) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



November 12, 2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 216 N. Lee Street, Suite 312, Alexandria, VA 22314.

ENGINEERING BY
TRENCO
A MiTek Affiliate

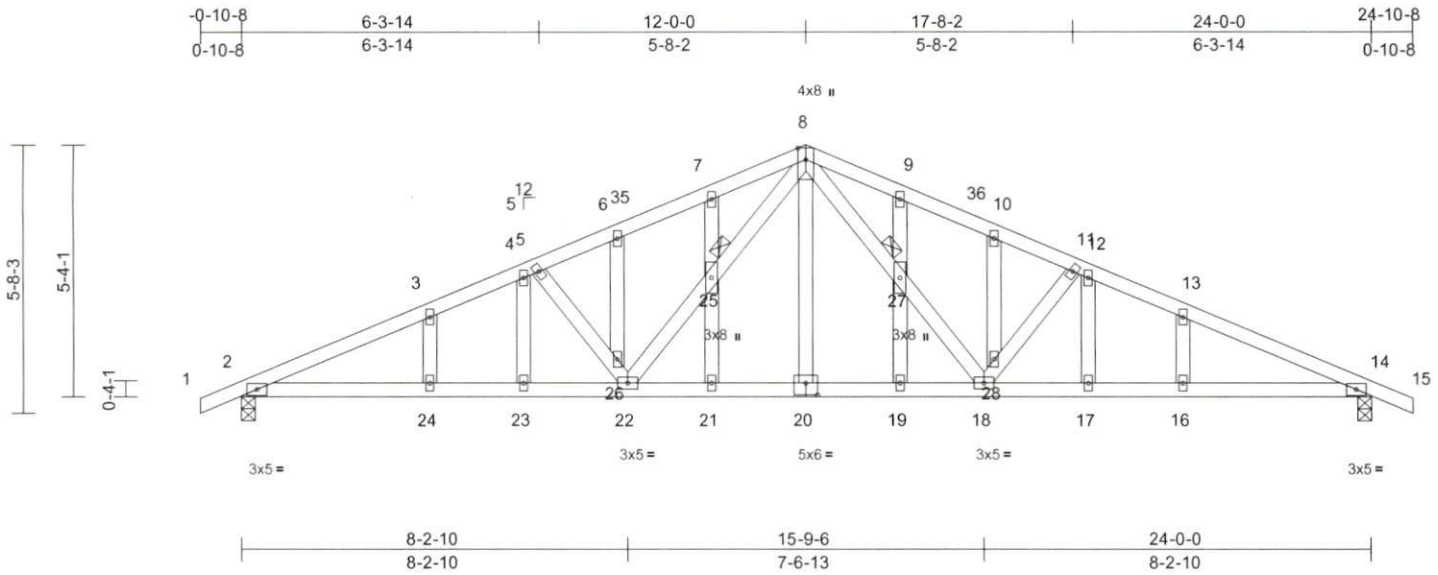
818 Soundside Road
Edenton, NC 27932

Job 19100105-B	Truss CP1SE	Truss Type Common	Qty 2	Ply 1	Job Reference (optional) E13745810
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Carter Components (Sanford), Sanford, NC - 27332,

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Page: 1



Scale = 1:47

Plate Offsets (X, Y): [20:0-3-0,0-3-0]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	Vert(LL)	-0.09	16	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	Vert(CT)	-0.18	16-34	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	Horz(CT)	0.06	14	n/a	n/a		
BCLL	0.0*	Code	IRC2015/TPI2014	Matrix-MSH							
BCDL	10.0										
											Weight: 142 lb FT = 20%

LUMBER	WEBS
TOP CHORD 2x4 SP No.2	8-27=-169/548, 18-27=-168/543,
BOT CHORD 2x4 SP No.2	18-28=-481/189, 11-28=-443/148,
WEBS 2x4 SP No.2 *Except* 18-11,22-5:2x4 SP No.3	22-25=-168/542, 8-25=-178/574,
OTHERS 2x4 SP No.3	5-26=-444/148, 22-26=-483/190,
BRACING	8-20=-15/145, 7-25=-84/61, 21-25=-45/50,
TOP CHORD Structural wood sheathing directly applied or 3-9-4 oc purlins.	6-26=-60/52, 4-23=-31/195, 3-24=-76/76,
BOT CHORD Rigid ceiling directly applied or 9-10-1 oc bracing.	9-27=-57/52, 19-27=-48/51, 10-28=-54/51,
JOINTS 1 Brace at Jt(s): 25, 27	12-17=-31/194, 13-16=-76/76
REACTIONS (lb/size) 2=854/0-3-8, 14=854/0-3-8	
Max Horiz 2=-53 (LC 16)	
Max Uplift 2=-8 (LC 15), 14=-9 (LC 16)	
Max Grav 2=1013 (LC 2), 14=1013 (LC 2)	
FORCES (lb) - Maximum Compression/Maximum Tension	
TOP CHORD 1-2=0/25, 2-3=-1944/446, 3-4=-1903/486,	
4-5=-1782/475, 5-6=-1664/443,	
6-35=-1638/464, 7-35=-1615/471,	
7-8=-1620/504, 8-9=-1606/500,	
9-36=-1615/471, 10-36=-1636/465,	
10-11=-1664/444, 11-12=-1782/476,	
12-13=-1903/486, 13-14=-1944/446,	
14-15=0/25	
BOT CHORD 2-24=-344/1759, 23-24=-344/1759,	
22-23=-344/1759, 21-22=-154/1167,	
20-21=-154/1167, 19-20=-154/1170,	
18-19=-154/1170, 17-18=-349/1759,	
16-17=-349/1759, 14-16=-349/1759	

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior (2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=13.9 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Fully Exp.; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable studs spaced at 2-0-0 oc.

- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
 - 10) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - 11) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 14. This connection is for uplift only and does not consider lateral forces.
 - 12) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- LOAD CASE(S)** Standard



November 12, 2019

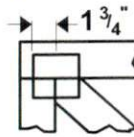
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

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TRENCO
A MiTek Affiliate

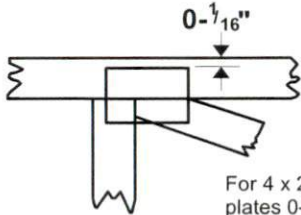
818 Soundside Road
Edenton, NC 27932

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

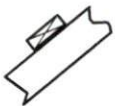
* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING

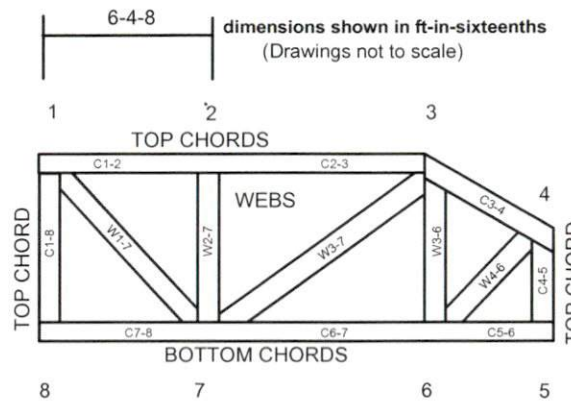


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

- ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
 DSB-89: Design Standard for Bracing.
 BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988
 ESR-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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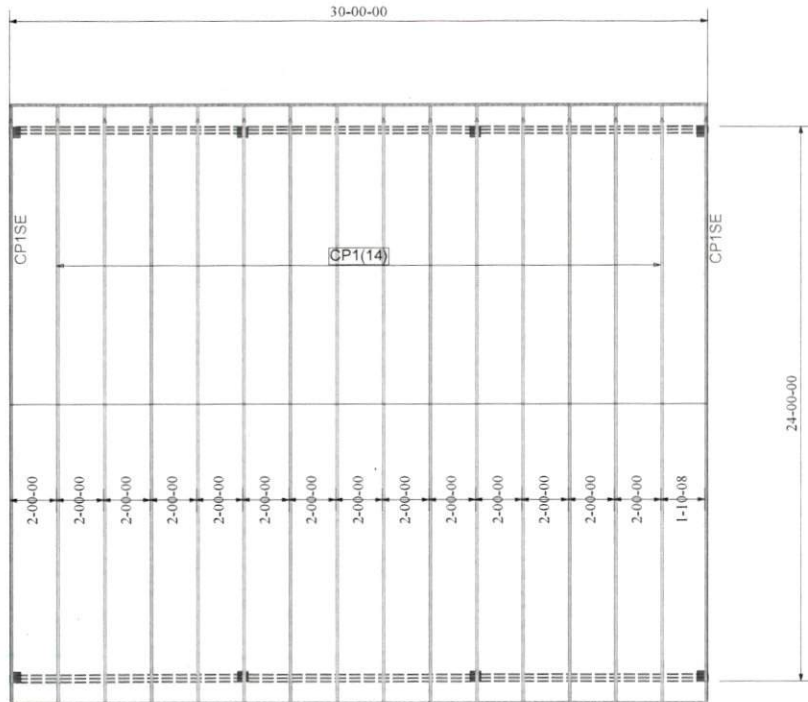
MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.



Products					
PlotID	Length	Product	Plies	Net Qty	Fab Type
BM2	30-00-00	2.0 RigidLam LVL 1-3/4 x 9-1/4	2	2	FF
BM3	30-00-00	2.0 RigidLam LVL 1-3/4 x 9-1/4	2	2	FF
PBO2	10-00-00	5-08 x 5-08 Generic Material	1	1	FF
PBO3	10-00-00	5-08 x 5-08 Generic Material	1	1	FF
PBO4	10-00-00	5-08 x 5-08 Generic Material	1	1	FF
PBO5	10-00-00	5-08 x 5-08 Generic Material	1	1	FF
PBO6	10-00-00	5-08 x 5-08 Generic Material	1	1	FF
PBO7	10-00-00	5-08 x 5-08 Generic Material	1	1	FF
PBO8	10-00-00	5-08 x 5-08 Generic Material	1	1	FF
PBO9	10-00-00	5-08 x 5-08 Generic Material	1	1	FF

Truss Connector Total List		
Manuf	Product	Qty
USP	One RT7A	32

ROOF TRUSS FRAMING

DRAWING SCALE : NTS



KMB Building LLC

Baptist Grove - Carport

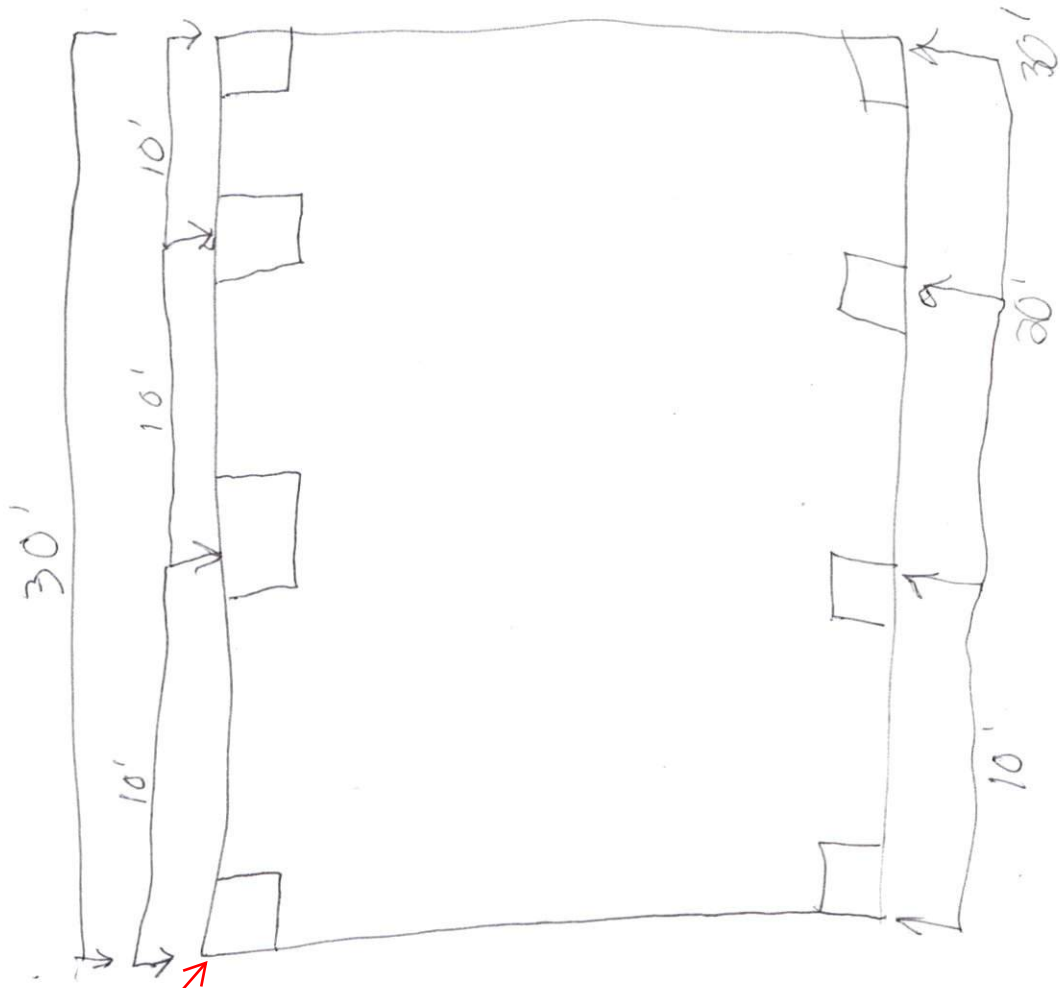
ROOF TRUSS PLACEMENT PLAN


REVISIONS	
DATE	BY

PROJECT NUMBER

SHEET NUMBER

1 / 1



 → ~~20"~~ 20" x 20" x 12" Depth
 Piers

Must use post bases
 if not embedding
 posts.