

REAR ELEVATION

SHAKES

SIDING

SCALE 1/4" = 1'-0"

FRONT ELEVATION

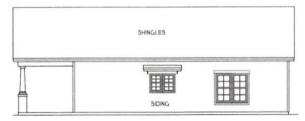
SCALE 1/4" = 1-0"

ATTIC VENTILATION:

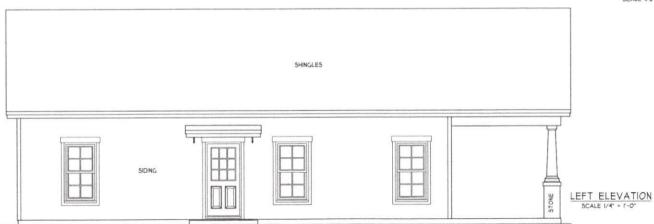
THE NET TREE VENTILATING AREA SHALL BE NOT LESS ENCEPT THE TREE VENTILATING AREA SHALL BE NOT LESS ENCEPT THE TREE AREA SHALL BE NOT SHOW FROWING AND AT LEAST SO PERCENT OF THE REQUIRED VENTILATING AREA IS PROVIDED BY VENTILATORS LOCATED IN THE ALL ALL SHALL BE NOT LEAST SHET ABOVE EAVE OF CORNICE VENTS WITH THE BALANCE OF THE REQUIRED VENTILATION TO BE PROVIDED BY EAVE OR CORNICE VENTS.

GROSS ATTIC AREA TO BE VENTILATED 1300 SQ.FT. 1300/150 = 8.44 SQ.FT. NET FREE AREA

ENERGY COMPLIANCE
ZONE 3 = MAX. GLAZING U-FACTOR 35
R-VALUE = CELING R38. WALLS RIS.
FLOORS RIP FOR JOHNSTON. WAYNE COUNTY
ZONE 4 = MAX. GLAZING L-FACTOR 35
R-VALUE = CELING R38. WALLS RIS.
FLOORS RIP FOR WAKE. GRANGE COUNTY



RIGHT ELEVATION



MANCAVE

THE

30 0 #

930

BRETT

FIRST FLOOR STORAGE AREA PORCH

HEATHER HALL 165 HEATHERSTONE CT BENSON NC 27504 (919) 207-1403

H SQUARED HOME DESIGN, INC.



THIS PLAN HAS BREN DRAWN IN ACCORDANCE WITH NORTH CAROLINA STATE RESIDENTIAL BUILDING CODES 20W EDITION

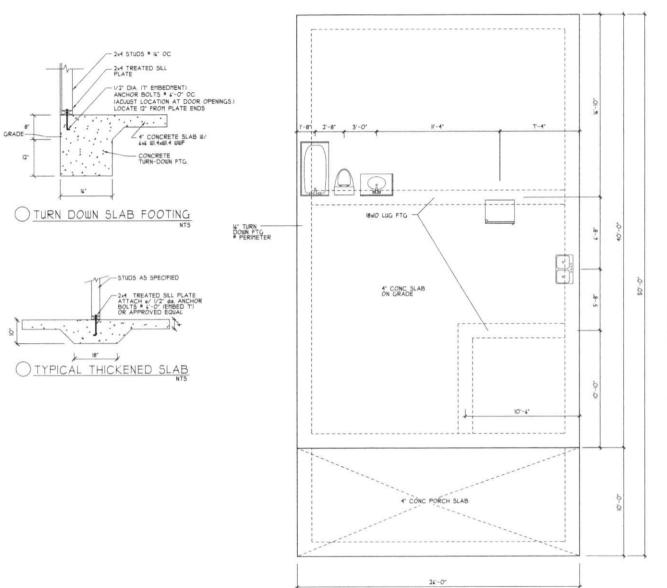
THIS PLAN IS TO ONLY BE BUILT BY THE ABOVE CITED BUILDER OR HOMEOMINER NOT FOR MULTIPLE BUILDS UNLESS APPROVED BY H SQUARED.

08/05/19

OUTBLDG

040618





DAMP PROOFING

FOR DAMP PROOFING (WATER PROOFING REFER TO SECTION 405 (406 IN 2018 EDITION NC RES. CODES

ANCHOR BOLTS

ANCHOR BOLTS TO BE PLACED WITHIN 12" OF EVERY CORNER AND FROM EVERY SPLICE AND AT 4"-0" O.C. WITH 1" MIN. IN CONC.

FOUNDATION PLAN

SCALE 1/4" = 1-0"

ANDERSON MANCAVE BRETT THE

930 #

= 930 = 110 = 260

FIRST FLOOR STORAGE AREA PORCH

HEATHER HALL 165 HEATHERSTONE CT BENSON NC 27504 (9)9) 207-1403

H SQUARED HOME DESIGN, INC.



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THIS PLAN IS TO ONLY BE BUILT BY THE ABOVE CITED BUILDER OR HOMEOWNER. NOT FOR MULTIPLE BUILDS UNLESS APPROVED BY H SQUARED.

08/05/19

OUTBLDG

FILE: 040618



ANDERSON MANCAVE BRETT THE

930 #

930 FIRST FLOOR STORAGE AREA PORCH

HEATHER HALL 165 HEATHERSTONE CT BENSON NC 27504 (919) 207-1403

H SQUARED HOME DESIGN, INC.

ANY DEVIATION OF THE SPECPIED HEASUREMENTS OR DITIENSIONS VOIDS H SQUARED HOME DESIGN. INC.'S LIABILITY.

THIS PLAN HAS BEEN DRAWN
N ACCORDANCE WITH NORTH
CARDUNA STATE RESIDENTIAL
BUILDING CODES 20W EDITION

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08/05/19

FILE:

OUTBLDG 040618

24'-0" 4'-0" 2'4" STORAGE FILES. HOBBY ROOF TRUSSES & SUPPORTS BY MANUF. OFFICE BATH ROOF OVER DOOR 12'-6" REF. ò KITCHENETTE 20 VAULTED CEILING VAULTED CEILING FLAT CLG . 13'4" LIVING ROOM ROOF TRUSSES & SUPPORTS BY MANUF STORAGE 9' CLG 10'-0" (2)2'6" 2'8"X5'2" 5'-2" 5'-2" FRONT PORCH 7'-10" 10'-4" T'-10"

26'-0"

HEADER SCHEDULE

SPAN SIZE LESS THAN 3'-0" (2) 2 X 4 3'-0" to 4'-0" (2) 2 X 4 4'-0" to 5'-0" (2) 2 X 8 5'-0" to 8'-0" (2) 2 X 10

ALL EXTERIOR OR LOAD BEARING HEADERS SHALL BE (2)2XIO UNLESS NOTED OTHERWISE

> FIRST FLOOR PLAN SCALE 1/4" = 1'-0"

STRUCTURAL NOTES

BICHERES BAL APPLES ONL'TO STRUTIRAL COPPONENTS INCLIDING
BOOT BANTERS, MEY ALET'S BOCKES FLOORS MALE BAHTS AND MACHES
COLINIS, CANTERNES, OFFETT LOAD BEARN, GALLS, PER I LEDIER
STSTEM AND FOOTNACS. BICHER'S SEAL DOES NOT CERTEY OFBEODHAL
STSTEM AND WINCHESTENSAL AFOUT MICLIONIC ROOF SYSTEM. ALL
REQUIREMENTS FOR PROPESSIONAL CERTIFICATION SHALL BE PROVIDED BY
THE APPROPRIATE BY PROPESSIONAL CERTIFICATION SHALL BE PROVIDED BY

DESIGN LOADS (R3OL4)

REQUIRED

24 PLATE OVER MINITUM (2) CJ.
ATTACH TO EACH CJ W (2) 16 MJL5

** PLATE SHALL BE CONTNUOUS TO
BEARNG SUPPORT UNLESS FLOOR
15 SHEATHED WITH PLYWOOD.

异

AS SPECIFIED

-4" LONG (MN) 2X4 PLATE ATTACHED TO 2X4 BOTTOM PLATE (2) ROWS OF ION NAILS AT 8" O.C.

24 TREATED SILL HASONRY CAP FLOOR JOIST SUB FLOOR

BLOCKING IN BETWEEN

SPACE TIES * 48" OC WHEN KNEE WALL IS NOT INSTALLED. II/ (4) IOU NALLS . EACH CONNECTION

RAFTER TIE DOWN (TYP)

DROPPED GIRDER

PASSENGER VEHICLE GARAGES FIRE ESCAPES SNOW UND LOAD (BASED ON 85/120 MPH RDRAILS AND HANDRAILS SENGER VEHICLE GARAGES ESCAPES RIOR BALCONIES WITH OUT PERMANE NG ROOMS # \$8\$\$\$0\$\$\$\$\$\$\$\$\$\$\$\$

BRACED WALL PANELS SHALL BE CONSTRUCTED ACCORDING TO VELOCITY

THE AROM YAD LOCATION OF BRACKING SHALL COPIETY BITH TABLE BACIDAD.
THE LENGTH OF BRACKING PARES SHALL BE DETERMED BY SECTION BACIDAD. A
LITERAL BRACKING SHALL BE SATISFED PER TETHOD 3 BY CONTRIBUDIES,
SHEATHING SHALL BITH STRECTIBAL SHEATHING PER SECTION BACIDAD 3.
NOTE THAT ANY SHEEPE BRACKED MALL DETAL SHALL BE MISTALLED AS SPECIFED
NOTE THAT ANY SHEEPE BRACKED MALL DETAL SHALL BE MISTALLED AS SPECIFED.

COUNTETE SHALL HAVE A PRISENT SO DAY STREIGHT OF SOOD BY AND AN CAMBRIDGE STREET OF SOOD BY AND A SOOR SHALL BE CAMBRIDGE STREET HANDLED CHARGEST RECEIVED THE STREET HANDLED SHALL BE CAMBRIDGE STREET HANDLED SHALL BE CAMBRIDGE SOO BY SHALL BE CAMBRIDGE SOOD BY SHALL BE CAMBRIDGE SOOD BY SHALL BE CAMBRIDGE SOOT BY SHALL BE SHALL BY SHAL

ALL PRAPING LUMBER SHALL BE SOP \$2 Fb = 875 PS) UNLESS NOTED OTHERWISE (UND). ALL TREATED LUMBER SHALL BE SYP \$ 2 Fb=475 PS). PLATE HATERIAL MAY BE SPF \$ 3 OR SYP \$3 FFc(shrp) = 425 PSI = MIN).

ALL WOODEN BEHRN AND HEADERS SHALL HAVE THE FOLLOWING, BRD SHPPORTS, IN THE YOUNG HEADERS STORE FOR BEHRN IN THE YOUNG HEADER THAN A TO WIND.

LY L. SHALL BE LAWRANTED YERERE WINDER FA-200 PS, FA-206 PS, B-400 PS FA-300 PS, B-400 PS FA-300 PS, B-400 PS FA-300 PS, SHALL BE LAMMATED VENEER UNIDER F1-2400 PS, F1-285 PS, E1-140 PS, SHALL BE PARALLE, STRAND LIMBER, F1-2500 PS, F1-250 PS, E1-2500 PS, SHALL BE LAMMATED STRAND LIMBER, F1-2500 PS, F1-450 PS, E1-1550 PS, F1-1500 PS, E1-1550 PS, F1-1500 PS, E1-1500 PS, E1-1500

OF ALL ROOP TRUIS AND 1-MOST LAYOUTS SHALL BE PREFARED IN ACCORDANCE
WITH ANY SEALED STRUCTURE DAMAGE. TRUISES NO 1-0015 SHALL BE
INSTALED ACCORDING TO THE FLAME ACCURES SPECIFICATIONS. ANY CHARGE IN
TRUIS OR 1-MOST LAYOUT SHALL BE COORDINATED WITH DESCAME OR BRANCHES
BALL STRUCTURAL STREL SHALL BE COORDINATED WITH DESCAME OR MINES
SEPTORTED AT EACH BEIOD WITH A PRIMED RESEARCH CARE MY SHALL BE
ALL STRUCTURAL STREL SHALL BE ACCURED TO REACH SHIPMORT WITH THO LAG
SEPTORTED AT EACH BE ON TAXABLE TO THE MALED TO THE MOST BEAT TO
ACCURED AT MATERIA SHALL BE ACCURED TO THE BOAT BALLE AND
ACCURED AT MATERIA SHALL BE ACCURED TO THE BOAT BALLE AND
ACCURED AT MATERIA SHALL BE ACCURED TO THE MALED TO THE MOST BHATE. AND
ACCURED AT MATERIA SHALL BE ACCURED TO THE BOAT BALLE A W O.C.

IN RESAN SHALL BE DEFORED STREE, ASTING GRADE LO

FLITCH BEATH SHALL BE BOLTED TOGETHER UBNG (2) ROBS OF 1/2" DUARTER BOLTS NSTIT AGOT WITH MASHESS PLACED WERE THE THERADED BND OF BOLT. BOLTS SHALL BE SPACED AT 2" P.C. ITAKK, AND STAGERED AT THE TOP ACID BOTTOM OF BEAT 1/2" EDGE DISTANCE), WITH 2 BOLTS LOCATED AT 4" PROY EACH BND.

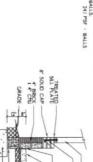
BRICK LINTELS SHALL BE 3 1/2"x3 1/2"x1/4" STEEL ANGLE FOR UP TO L'-O" SPAN AND L'XT'x5/4" STEEL ANGLE WITH L' LEG VERTICAL FOR SPANS UP TO 4"-O" ILINO).

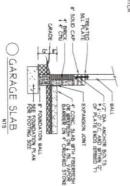
THE POSITIVE AND NEGATIVE DESIGN PRESSURE FOR DOORS AND WINDOWS FOR A HEAN ROOF HEIGHT OF 35 FEET OR LESS SHALL BE 25 PSP.

THE POSITIVE AND REGATIVE DESIGN PRESSURES REGIRRED FOR ANY ROOF OR MALL CLANDING APPLICATION NOT SPECIFICALLY ADDRESSED IN THE NORTH CAROLINA STATE RESIDENTIAL CODE - 2018 EDITION SHALL BE AS FOLLOWS:

TURN DOWN SLAB FOOTING

45.4 PSF - 2.25/2 PITCH OR LESS 34.8 PSF - 2.25/2 TO 1/2 PITCH 2) PSF - 1/2 TO 12/2 PITCH



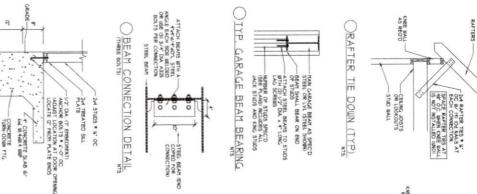


TYPICAL THICKENED SLAB

4" CONCRETE SLAB

PIGUPE REG2 10.3.3 PORTAL FRAME WITH HCL

- 4" STEP



of PLATE ENDS (EMBED 1)

OCRAWL SECTION

TURN DOWN SLAB 9 GARAGE

8.XM. CONC LLC

PL CHI

TREATED SILL

4" BRICK VENEE FINISH GRADE

FLOOR JOIST

BAND MATERIA SUBFLOOR 24 STUDS

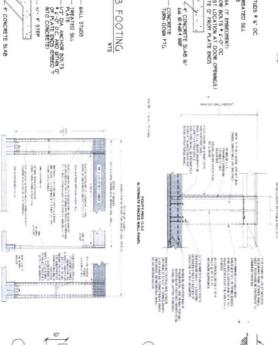
-TREATED SILL
PLATE

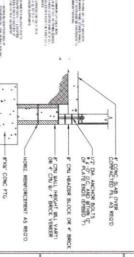
AND WITHIN 2" OF PLATE ENDS (EMBED 1" INTO CONCRETE)

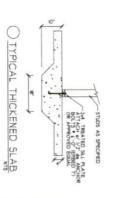
+/- 4" STEP

SLAB SLAB

FOOTING











H SQUARED HOME DESIGN, INC.

SLAB FND. W/ STEM WALL

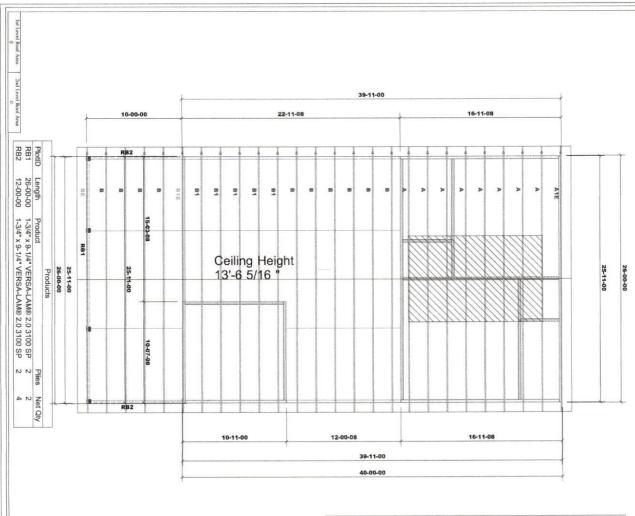
165 HEATHERSTONE CT BENSON NC 27504

*PLEASE NOTE THAT NOT ALL DETAILS APPLY TO EVERY PLAN.

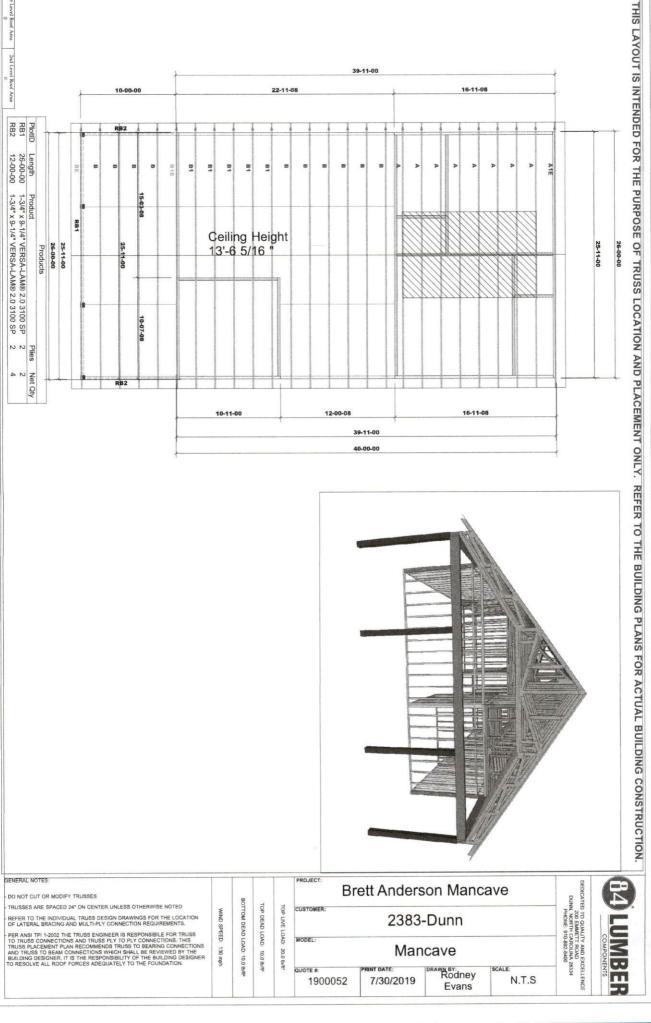
BASIC BUILDING

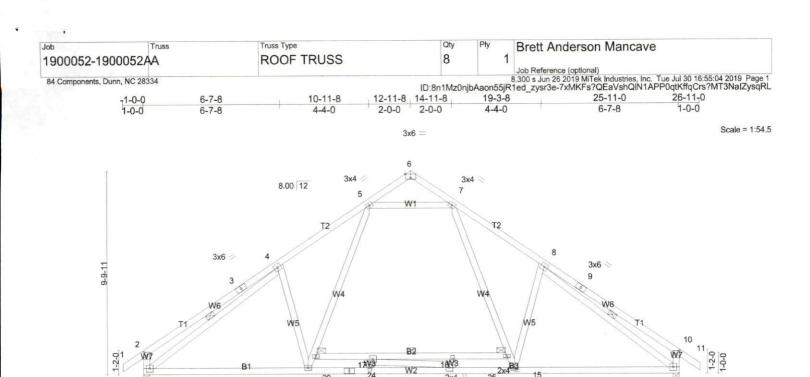
FOOTING SIZE 經 解 验

DETAIL SHEET









	7-11-8		14-11-8	17-11-8	25-11-0	
	7-11-8	3-0-0	4-0-0	3-0-0	7-11-8	
Plate Offsets (X,Y)- [2:0	0-2-0,0-1-12], [6:0-3-0,Edge], [10:0-2-0,0-1	-12]				
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (loc) I/defl	L/d PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15 Lumber DOL 1.15	TC 0.65 BC 0.88	Vert(LL) Vert(CT)	-0.51 14-18 >602 -0.78 14-18 >396	240 MT20 180	244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.52 Matrix-MS	Horz(CT)	0.04 12 n/a	n/a Weight: 18	2 lb FT = 20%

4 24

19

3x6

18

2x4 ||

20

21

3x4

2x4 =

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP DSS *Except* B2: 2x4 SP No.1

WEBS 2x4 SP No.3 BRACING-

WEBS

TOP CHORD BOT CHORD

2x4

14

25

23

Structural wood sheathing directly applied or 4-5-12 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing. Except: 6-0-0 oc bracing: 15-20

12

0-3-8(0-2-0)

1 Row at midpt

4-22, 8-12

15

13

3x4

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 22=1189/0-3-8 (min. 0-2-0), 12=1189/0-3-8 (min. 0-2-0)

0-3-8(0-2-0)

Max Horz 22=-208(LC 10) Max Uplift22=-96(LC 12), 12=-96(LC 12) Max Grav22=1279(LC 18), 12=1281(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-423/214, 3-4=-373/235, 4-5=-1568/237, 7-8=-1571/237, 8-9=-373/235, 9-10=-424/214, 2-22=-430/245, 10-12=-430/245

B1

BOT CHORD 21-22=-3/1386, 19-21=0/1181, 18-19=0/1181, 14-18=0/1181, 14-23=0/1074, 13-23=0/1074,

12-13=-4/1269

WEBS 7-15=-35/736, 13-15=-68/585, 8-13=-252/220, 20-21=-68/587, 5-20=-35/733, 4-21=-253/220,

4-22=-1427/0, 8-12=-1429/0, 5-7=-954/229

JOINT STRESS INDEX

2 = 0.86, 3 = 0.28, 4 = 0.79, 5 = 0.63, 6 = 0.13, 7 = 0.63, 8 = 0.79, 9 = 0.28, 10 = 0.86, 12 = 0.62, 13 = 0.43, 14 = 0.28, 15 = 0.27, 16 = 0.27, 17 = 0.28, 18 = 0.27, 19 = 0.71, 20 = 0.27, 21 = 0.43 and 22 = 0.28, 10

NOTES-

1) Unbalanced roof live loads have been considered for this design.

7) Wind: ASCE 7-10; Vult=130mph; (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=6ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) All plates are 4x4 MT20 unless otherwise indicated.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 22 and 12. This connection is for uplift only

and does not consider lateral forces. 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

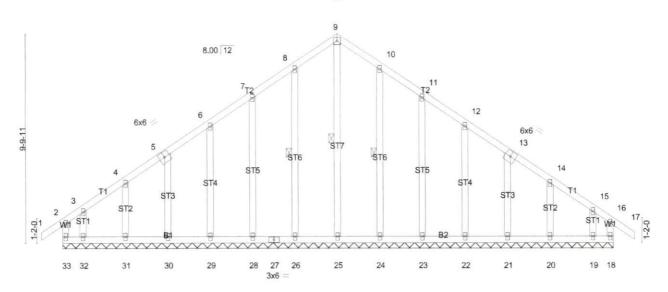
8) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.

Job Truss Type Qty Brett Anderson Mancave 1 1900052-1900052AA1E Common Supported Gable 1 Job Reference (optional)

8.300 s Jun 26 2019 MiTek Industries, Inc. Tue Jul 30 16:55:06 2019 Page 1
ID:8n1Mz0njbAaon55jR1ed_zysr3e-4KU4gY0hmBlawkum8bRt5FyoUSixJtkfwNsgNSysqRJ 84 Components, Dunn, NC 28334 1-0-0 26-11-0 12-11-8 25-11-0 12-11-8 12-11-8 1-0-0

4x4 =

Scale = 1:53.0



25-11-0 25-11-0 SPACING-DEFL PLATES GRIP LOADING (psf) 2-0-0 CSL (loc) I/defl L/d TC BC WB -0.00 120 244/190 Plate Grip DOL 0.18 Vert(LL) MT20 1.15 TCLL 20.0 n/r Lumber DOL TCDL 10.0 1.15 0.09 Vert(CT) -0.01 17 n/r 90 -0.00BCLL 0.0 Rep Stress Incr YES 0.12 Horz(CT) 18 n/a n/a BCDL 10.0 Code IRC2015/TPI2014 Matrix-R Weight: 187 lb FT = 20%

LUMBER-TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 **WEBS**

BRACING-TOP CHORD BOT CHORD WEBS

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 6-0-0 oc bracing. 1 Row at midpt 9-25, 8-26, 10-24

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

2x4 SP No.3 REACTIONS.

ONS. All bearings 25-11-0. (lb) - Max Horz 33=208(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) 26, 28, 29, 30, 31, 24, 23, 22, 21, 20, 19 except 33=-163(LC 10), 18=-109(LC 11),

32=-118(LC 11)

Max Grav All reactions 250 lb or less at joint(s) 33, 18, 26, 28, 29, 30, 31, 32, 24, 23, 22, 21, 20, 19 except 25=259(LC 12)

FORCES. (lb) - Max, Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD $\,$ 8-9=-236/292, 9-10=-236/292

9-25=-257/145

JOINT STRESS INDEX

2 = 0.55, 3 = 0.27, 4 = 0.27, 5 = 0.12, 6 = 0.27, 7 = 0.27, 8 = 0.27, 9 = 0.22, 10 = 0.27, 11 = 0.27, 12 = 0.27, 13 = 0.12, 14 = 0.27, 15 = 0.27, 16 = 0.55, 18 = 0.63, 19 = 0.27, 20 = 0.27, 21 = 0.27, 21 = 0.27, 22 = 0.2723 = 0.27, 24 = 0.27, 25 = 0.27, 26 = 0.27, 27 = 0.12, 28 = 0.27, 29 = 0.27, 30 = 0.27, 31 = 0.27, 32 = 0.27 and 33 = 0.63

OTHERS

1) Unbalanced roof live loads have been considered for this design.

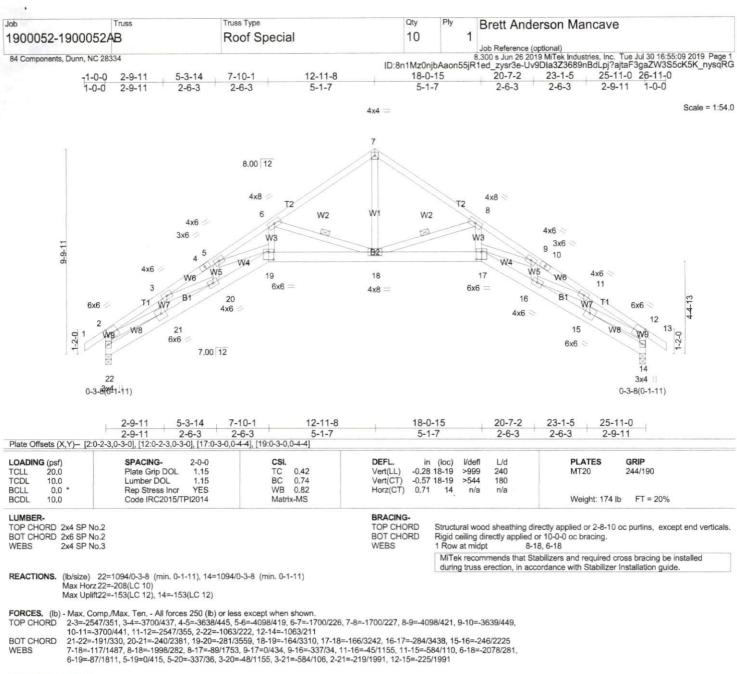
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 4) All plates are 2x4 MT20 unless otherwise indicated.

- 5) Gable requires continuous bottom chord bearing.
- 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
 7) Gable studs spaced at 2-0-0 oc.

- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

 10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 33, 18, 26, 28, 29, 30, 31, 32, 24, 23, 22,
- 21, 20, and 19. This connection is for uplift only and does not consider lateral forces.

 11) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



JOINT STRESS INDEX

2 = 0.75, 3 = 0.36, 4 = 0.43, 5 = 0.25, 6 = 0.65, 7 = 0.69, 8 = 0.65, 9 = 0.25, 10 = 0.43, 11 = 0.36, 12 = 0.75, 14 = 0.54, 15 = 0.60, 16 = 0.35, 17 = 0.77, 18 = 0.52, 19 = 0.77, 20 = 0.35, 21 = 0.60 and 22 = 0.75, 14 = 0.75, 150.54

NOTES-

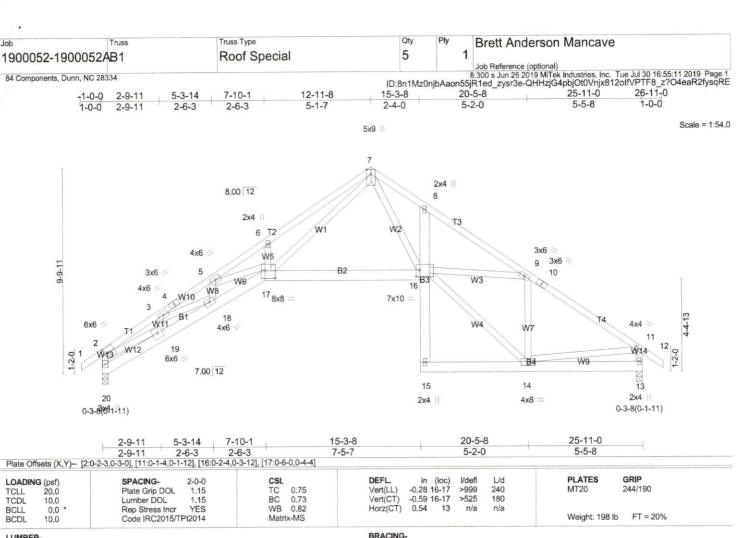
1) Unbalanced roof live loads have been considered for this design

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=6ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom
- chord and any other members.

 5) Bearing at joint(s) 22, 14 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 22 and 14. This connection is for uplift only and does not consider lateral forces.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



LUMBER-

TOP CHORD 2x4 SP No.2 *Except* T2: 2x4 SP No.1

BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3 *Except*

W1: 2x4 SP No.2

REACTIONS. (lb/size) 20=1094/0-3-8 (min. 0-1-11), 13=1094/0-3-8 (min. 0-1-11)

Max Horz 20=-208(LC 10) Max Uplift20=-153(LC 12), 13=-153(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2541/348, 3-4=-3718/441, 4-5=-3689/453, 5-6=-4077/395, 6-7=-4118/550, 7-8=-2318/343, 8-9=-2375/275, 9-10=-1138/230,

BOT CHORD

10-11=-1297/211, 2-20=-1064/221, 11-13=-1029/235 19-20=-191/332, 18-19=-238/2375, 17-18=-283/3582, 16-17=0/1298 3-19=-583/109, 3-18=-57/1181, 5-18=-348/23, 5-17=0/366, 6-17=-254/197, 7-17=-361/3013, 7-16=-151/1359, 14-16=-111/1293, WEBS

9-16=0/925, 9-14=-916/157, 2-19=-217/1982, 11-14=-24/823

JOINT STRESS INDEX

2 = 0.75, 3 = 0.37, 4 = 0.44, 5 = 0.25, 6 = 0.27, 7 = 0.90, 8 = 0.64, 9 = 0.38, 10 = 0.17, 11 = 0.75, 13 = 0.95, 14 = 0.72, 15 = 0.45, 16 = 0.79, 17 = 0.78, 18 = 0.35, 19 = 0.60 and 20 = 0.54

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

Rigid ceiling directly applied or 10-0-0 oc bracing.

1) Unbalanced roof live loads have been considered for this design.

1) Unid: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=6ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Bearing at joint(s) 20 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 20 and 13. This connection is for uplift only and does not consider lateral forces.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

Truss Type Job **Brett Anderson Mancave** 1 **GABLE** 1900052-1900052AB1E 84 Components, Dunn, NC 28334 1-0-0 7-10-1 12-11-8 2-9-11 5-3-14 5-1-7 2-9-11 2-6-3 2-6-3 1-0-0 Scale = 1:54.7 10x12 5x9 8 00 12 W2 ST4 6 T2 ST3 ST1 ST5 3x6 4x6 W5 6x6 9-9-11 9 B2 3x6 5 10 W3 WE 4x6 W8 ¹⁷8x8 4 W10 7x10 ST9 3 ST B1 18 T4 W11 6x6 W4 4x4 4x6 W ST6 ST10 11 12-0-1-W12 19 W14 1-2-0 6x6 **B**4 7.00 12 15 13 14 20 4x8 = 0-3-8(0-1-11) 0-3-8(841-11) 7-10-1 15-3-8 20-5-8 25-11-0 2-9-11 5-3-14 7-5-7 5-2-0 5-5-8 2-9-11 2-6-3 2-6-3 Plate Offsets (X,Y)- [2:0-2-3,0-3-0], [7:0-1-11,0-5-0], [11:0-1-4,0-1-12], [16:0-2-4,0-3-12], [17:0-6-0,0-4-4] DEFL PLATES GRIP SPACING-I/defl LOADING (psf) TC BC 0.75 244/190 Plate Grip DOL 1.15 Vert(LL) -0.28 16-17 >999 240 MT20 TCLL Vert(CT) -0.59 16-17 >525 180 TCDL 10.0 Lumber DOL 1.15 0.0 Rep Stress Incr WB 0.82 Horz(CT) 0.54 13 BCLL Weight: 235 lb FT = 20% BCDL 10.0 Code IRC2015/TPI2014 Matrix-MS

Qtv

LUMBER-

TOP CHORD 2x4 SP No.2 *Except* T2: 2x4 SP No.1

BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3 *Except*

W1: 2x4 SP No.2

OTHERS 2x4 SP No.3

(lb/size) 20=1094/0-3-8 (min. 0-1-11), 13=1094/0-3-8 (min. 0-1-11) REACTIONS.

Max Horz 20=-208(LC 10) Max Uplift20=-153(LC 12), 13=-153(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-2541/348, 3-4=-3718/441, 4-5=-3689/453, 5-6=-4077/395, 6-7=-4118/550, 7-8=-2318/343, 8-9=-2375/275, 9-10=-1138/230, 10-11=-1297/211, 2-20=-1064/221, 11-13=-1029/235 19-20=-191/332, 18-19=-238/2375, 17-18=-283/3582, 16-17=0/1298 TOP CHORD

BOT CHORD

3-19=-583/109, 3-18=-57/1181, 5-18=-348/23, 5-17=0/366, 6-17=-254/197, 7-17=-361/3013, 7-16=-151/1359, 14-16=-111/1293, 9-16=0/925, 9-14=-916/157, 2-19=-217/1982, 11-14=-24/823

JOINT STRESS INDEX

2 = 0.75, 3 = 0.37, 4 = 0.44, 5 = 0.25, 6 = 0.27, 7 = 0.90, 7 = 0.29, 8 = 0.64, 9 = 0.38, 10 = 0.16, 11 = 0.75, 13 = 0.95, 14 = 0.72, 15 = 0.45, 16 = 0.79, 17 = 0.78, 18 = 0.35, 19 = 0.60, 20 = 0.54, 21 = 0.27, 22 = 0.27, 23 = 0.27, 24 = 0.27, 25 = 0.27, 26 = 0.28, 27 = 0.27, 28 = 0.27, 29 = 0.27, 30 = 0.27, 31 = 0.27, 32 = 0.27, 33 = 0.27, 34 = 0.27, 34 = 0.27, 36 = 0.27, 36 = 0.27 and 37 = 0.27

BRACING-TOP CHORD

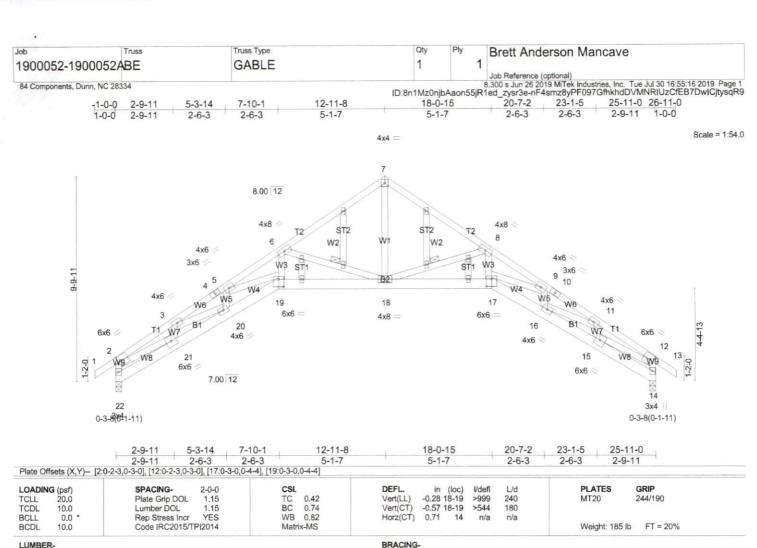
BOT CHORD

Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=6ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as
 applicable, or consult qualified building designer as per ANSI/TPI 1.
- All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Bearing at joint(s) 20 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 20 and 13. This connection is for uplift only
- and does not consider lateral forces.
- 10) This truss is designed in accordance with the 2015 international Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3

OTHERS 2x4 SP No.3 TOP CHORD

BOT CHORD WERS

Structural wood sheathing directly applied or 2-8-10 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Row at midpt

8-18, 6-18

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 22=1094/0-3-8 (min. 0-1-11), 14=1094/0-3-8 (min. 0-1-11)

Max Horz 22=-208(LC 10) Max Uplift22=-153(LC 12), 14=-153(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2547/351, 3-4=-3700/437, 4-5=-3638/445, 5-6=-4098/419, 6-7=-1700/226, 7-8=-1700/227, 8-9=-4098/421, 9-10=-3639/449

20-17-37(0)/441, 11-12=-2547/355, 2-22=-1063/222, 12-14=-1063/211 21-22=-191/330, 20-21=-240/2381, 19-20=-281/3559, 18-19=-164/3310, 17-18=-166/3242, 16-17=-284/3438, 15-16=-246/2225 7-18=-117/1487, 8-18=-1998/282, 8-17=-89/1753, 9-17=0/434, 9-16=-337/34, 11-16=-45/1155, 11-15=-584/110, 6-18=-2078/281, **BOT CHORD** WEBS 6-19=-87/1811, 5-19=0/415, 5-20=-337/36, 3-20=-48/1155, 3-21=-584/106, 2-21=-219/1991, 12-15=-225/1991

2 = 0.75, 3 = 0.36, 4 = 0.43, 5 = 0.25, 6 = 0.65, 7 = 0.69, 8 = 0.65, 9 = 0.25, 10 = 0.43, 11 = 0.36, 12 = 0.75, 14 = 0.54, 15 = 0.60, 16 = 0.35, 17 = 0.77, 18 = 0.52, 19 = 0.77, 20 = 0.35, 21 = 0.60, 22 = 0.54, 23 = 0.27, 24 = 0.27, 25 = 0.27, 26 = 0.27, 27 = 0.27, 28 = 0.27, 29 = 0.27 and 30 = 0.27

NOTES-

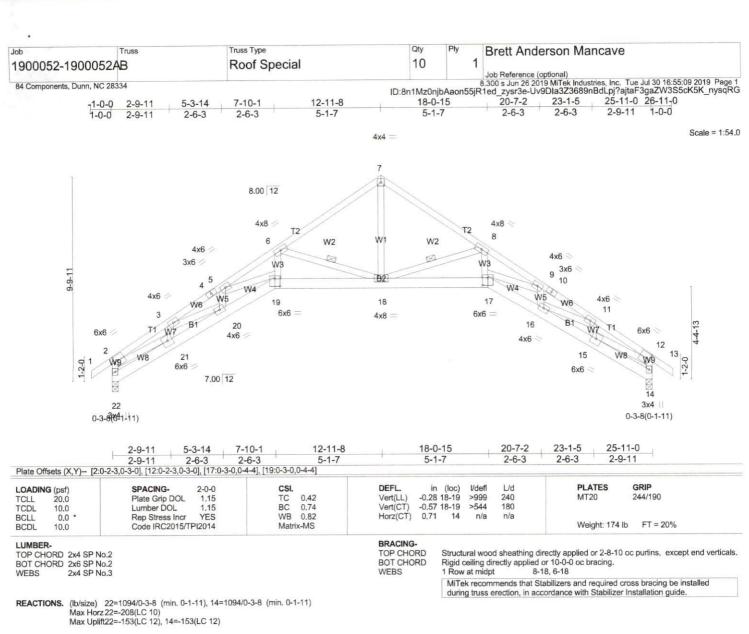
1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=6ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as
- applicable, or consult qualified building designer as per ANSI/TPI 1.
 4) All plates are 2x4 MT20 unless otherwise indicated.

- 5) Gable studs spaced at 2-0-0 oc.
 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- ship of the members.

 8) Bearing at joint(s) 22, 14 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 22 and 14. This connection is for uplift only and does not consider lateral forces.
- 10) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



FORCES, (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-2547/351, 3-4=-3700/437, 4-5=-3638/445, 5-6=-4098/419, 6-7=-1700/226, 7-8=-1700/227, 8-9=-4098/421, 9-10=-3639/449,

10-11=-3700/441, 11-12=-2547/355, 2-22=-1063/222, 12-14=-1063/211 21-22=-191/330, 20-21=-240/2381, 19-20=-281/3559, 18-19=-164/3310, 17-18=-166/3242, 16-17=-284/3438, 15-16=-246/2225 **BOT CHORD** 7-18=-117/1487, 9-18=-1998/282, 8-17=-89/1753, 9-17=0/434, 9-16=-337/34, 11-16=-45/1155, 11-15=-584/110, 6-18=-2078/281, WEBS 6-19=-87/1811, 5-19=0/415, 5-20=-337/36, 3-20=-48/1155, 3-21=-584/106, 2-21=-219/1991, 12-15=-225/1991

JOINT STRESS INDEX

2 = 0.75, 3 = 0.36, 4 = 0.43, 5 = 0.25, 6 = 0.65, 7 = 0.69, 8 = 0.65, 9 = 0.25, 10 = 0.43, 11 = 0.36, 12 = 0.75, 14 = 0.54, 15 = 0.60, 16 = 0.35, 17 = 0.77, 18 = 0.52, 19 = 0.77, 20 = 0.35, 21 = 0.60 and 22 = 0.75, 12 = 0.75, 13 = 0.75, 14 = 0.75, 150.54

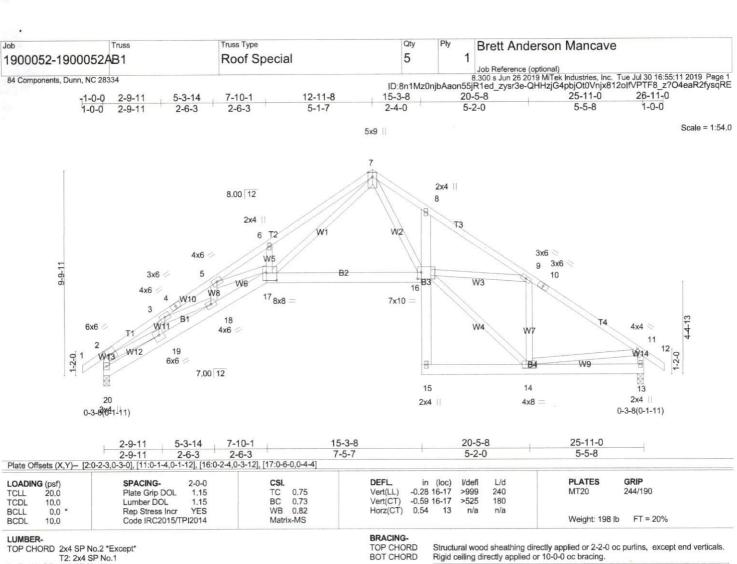
NOTES-

1) Unbalanced roof live loads have been considered for this design

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=6ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Bearing at joint(s) 22, 14 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 22 and 14. This connection is for uplift only
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3 *Except*

W1: 2x4 SP No.2

REACTIONS. (lb/size) 20=1094/0-3-8 (min. 0-1-11), 13=1094/0-3-8 (min. 0-1-11) Max Horz 20=-208(LC 10) Max Uplift20=-153(LC 12), 13=-153(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-2541/348, 3-4=-3718/441, 4-5=-3689/453, 5-6=-4077/395, 6-7=-4118/550, 7-8=-2318/343, 8-9=-2375/275, 9-10=-1138/230, 10-11=-1297/211, 2-20=-1064/221, 11-13=-1029/235
19-20=-191/332, 18-19=-238/2375, 17-18=-283/3582, 16-17=0/1298 TOP CHORD

BOT CHORD 3-19=-583/109, 3-18=-57/1181, 5-18=-348/23, 5-17=0/366, 6-17=-254/197, 7-17=-361/3013, 7-16=-151/1359, 14-16=-111/1293,

9-16=0/925, 9-14=-916/157, 2-19=-217/1982, 11-14=-24/823

JOINT STRESS INDEX 2 = 0.75, 3 = 0.37, 4 = 0.44, 5 = 0.25, 6 = 0.27, 7 = 0.90, 8 = 0.64, 9 = 0.38, 10 = 0.17, 11 = 0.75, 13 = 0.95, 14 = 0.72, 15 = 0.45, 16 = 0.79, 17 = 0.78, 18 = 0.35, 19 = 0.60 and 20 = 0.54

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=6ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Bearing at joint(s) 20 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 20 and 13. This connection is for uplift only and does not consider lateral forces.

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

Truss Type Otv **Brett Anderson Mancave** Job **GABLE** 1 1 1900052-1900052AB1E Job Reference (optional)
8.300 s Jun 26 2019 MiTek Industries, Inc. Tue Jul 30 16:55:13 2019 Page 1
ID:8n1Mz0njbAaon55jR1ed_zysr3e-MgPk8x646KeaGpx62Z3WtjlrvHxcStVhXy3Y7YysqRC 84 Components, Dunn, NC 28334 25-11-0 12-11-8 20-5-8 26-11-0 -1-0-0 2-9-11 5-3-14 7-10-1 15-3-8 5-5-8 1-0-0 2-9-11 2-6-3 2-6-3 5-1-7 2-4-0 5-2-0 1-0-0 Scale = 1:54.7 10x12 5x9 8.00 12 W2 ST4 6 T2 ST3 ST1 ST5 3x6 4x6 ST2 W5 6x6 9 B2 3x6 5 10 W3 WA 16 4x6 W8 17_{8x8} = 4 W10 7x10 ST9 3 ST B 18 T4 W11 6x6 W4 4×4 4x6 ST6 W7 ST10 12-0-1-11 ST8 W12 19 W14 W13 1-2-0 6x6 B4 W9 7.00 12 15 14 20 4x8 = 0-3-8(0-1-11) 0-3-87641-11) 20-5-8 5-3-14 7-10-1 15-3-8 25-11-0 2-9-11 2-9-11 2-6-3 2-6-3 7-5-7 5-2-0 5-5-8 Plate Offsets (X,Y)- [2:0-2-3,0-3-0], [7:0-1-11,0-5-0], [11:0-1-4,0-1-12], [16:0-2-4,0-3-12], [17:0-6-0,0-4-4] in (loc) DEFI I/defl L/d PLATES GRIP SPACING-LOADING (psf 244/190 TC BC 0.75 -0.28 16-17 TCLL 20.0 Plate Grip DOL 1.15 Vert(LL) >999 240 MT20 0.73 Vert(CT) -0.59 16-17 >525 180 1.15 TCDL 10.0 Lumber DOL 0.0 * Rep Stress Incr WB 0.82 0.54 13 n/a BCLL Weight: 235 lb FT = 20% BCDL 10.0 Code IRC2015/TPI2014 Matrix-MS

LUMBER

TOP CHORD 2x4 SP No.2 *Except* T2: 2x4 SP No.1

BOT CHORD 2x6 SP No.2

2x4 SP No.3 *Except* WEBS

W1: 2x4 SP No.2

OTHERS 2x4 SP No.3

REACTIONS. (lb/size) 20=1094/0-3-8 (min. 0-1-11), 13=1094/0-3-8 (min. 0-1-11)

Max Horz 20=-208(LC 10)

Max Uplift20=-153(LC 12), 13=-153(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2.3=.2541/348, 3.4=.3718/441, 4.5=.3689/453, 5.6=.4077/395, 6.7=.4118/550, 7.8=.2318/343, 8.9=.2375/275, 9.10=.1138/230, 10-.11=.1297/211, 2.20=.1064/221, 11-.13=.1029/235 19-20=.191/332, 18-19=.238/2375, 17-.18=.283/3582, 16-.17=0/1298 TOP CHORD

BOT CHORD

3-19=-583/109, 3-18=-57/1181, 5-18=-348/23, 5-17=0/366, 6-17=-254/197, 7-17=-361/3013, 7-16=-151/1359, 14-16=-111/1293, 9-16=0/925, 9-14=-916/157, 2-19=-217/1982, 11-14=-24/823

JOINT STRESS INDEX

2 = 0.75, 3 = 0.37, 4 = 0.44, 5 = 0.25, 6 = 0.27, 7 = 0.90, 7 = 0.29, 8 = 0.64, 9 = 0.38, 10 = 0.16, 11 = 0.75, 13 = 0.95, 14 = 0.72, 15 = 0.45, 16 = 0.79, 17 = 0.78, 18 = 0.35, 19 = 0.60, 20 = 0.54, 21 = 0.27, 18 = 0.27, 18 = 0.45, 18 = 0.22 = 0.27, 23 = 0.27, 24 = 0.27, 25 = 0.27, 26 = 0.28, 27 = 0.27, 26 = 0.27, 26 = 0.27, 26 = 0.27, 26 = 0.27, 26 = 0.27, 26 = 0.27, 26 = 0.27, 26 = 0.27, 26 = 0.27, 26 = 0.27, 26 = 0.27, 27 = 0.27, 27 = 0.27, 30 = 0.27, 31 = 0.27, 32 = 0.27, 33 = 0.27, 34 = 0.27, 34 = 0.27, 36 = 0.27

BRACING-

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals.

MiTek recommends that Stabilizers and required cross bracing be installed

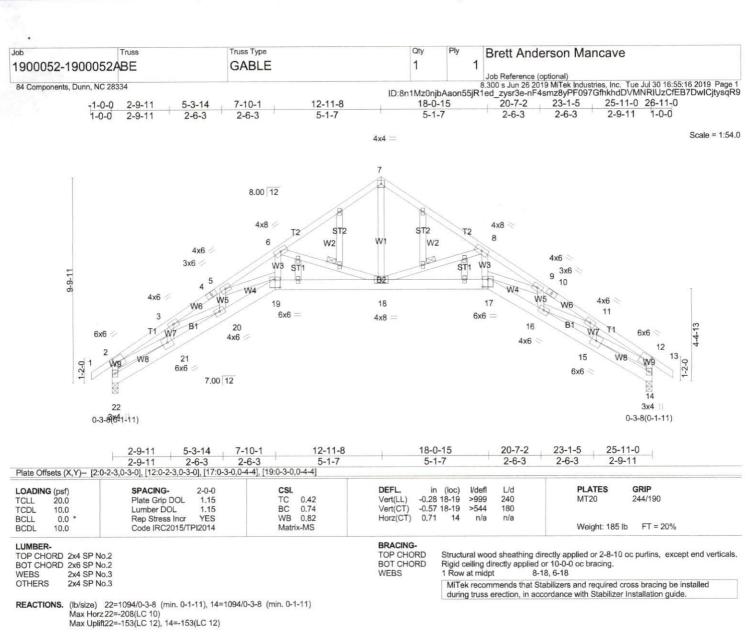
during truss erection, in accordance with Stabilizer Installation guide.

Rigid ceiling directly applied or 10-0-0 oc bracing.

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=6ft; Cat. II; Exp B; Enclosed;
- MWFRS (directional) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.6.1

 Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Bearing at joint(s) 20 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 20 and 13. This connection is for uplift only
- and does not consider lateral forces.
- 10) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2547/351, 3-4=-3700/437, 4-5=-3638/445, 5-6=-4098/419, 6-7=-1700/226, 7-8=-1700/227, 8-9=-4098/421, 9-10=-3639/449,

10-11=-3700/441, 11-12=-2547/355, 2-22=-1063/222, 12-14=-1063/211

21-22=-191/330, 20-21=-240/2381, 19-20=-281/3559, 18-19=-164/3310, 17-18=-166/3242, 16-17=-284/3438, 15-16=-246/2225 **BOT CHORD** 7-18=-117/1487, 8-18=-1998/282, 8-17=-89/1753, 9-17=0/434, 9-16=-337/34, 11-16=-45/1155, 11-15=-584/110, 6-18=-2078/281, **WEBS**

6-19=-87/1811, 5-19=0/415, 5-20=-337/36, 3-20=-48/1155, 3-21=-584/106, 2-21=-219/1991, 12-15=-225/1991

JOINT STRESS INDEX

2 = 0.75, 3 = 0.36, 4 = 0.43, 5 = 0.25, 6 = 0.65, 7 = 0.69, 8 = 0.65, 9 = 0.25, 10 = 0.43, 11 = 0.36, 12 = 0.75, 14 = 0.54, 15 = 0.60, 16 = 0.35, 17 = 0.77, 18 = 0.52, 19 = 0.77, 20 = 0.35, 21 = 0.60, 22 = 0.54, 23 = 0.27, 24 = 0.27, 25 = 0.27, 26 = 0.27, 27 = 0.27, 28 = 0.27, 29 = 0.27 and 30 = 0.27

NOTES.

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=6ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as
- applicable, or consult qualified building designer as per ANSI/TPI 1.
 4) All plates are 2x4 MT20 unless otherwise indicated.

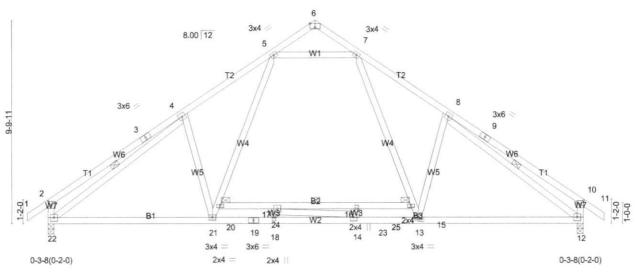
5) Gable studs spaced at 2-0-0 oc.

- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom
- chord and any other members.

 8) Bearing at joint(s) 22, 14 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 22 and 14. This connection is for uplift only and does not consider lateral forces.
- 10) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI1.

900052-190005	Truss 2AA		ROC	F TRUSS		8	У	Ply 1	Brett Anderson Mancave Job Reference (optional)
84 Components, Dunn, NC					10.11.0		1z0njb/	Aaon55jR	8.300 s Jun 26 2019 MiTek Industries, Inc. Tue Jul 30 16:55:04 2019 1ed_zysr3e-7xMKFs?QEaVshQIN1APP0qtKffqCrs?MT3NaI
-1-0		6-7-8		10-11-8	12-11-8		-	19-3-8	25-11-0 26-11-0
1-0-	0	6-7-8		4-4-0	2-0-0	2-0-0		4-4-0	6-7-8 1-0-0
					3x	x6 =			Scale
					6	:			



	7-11-8	10-11-8	14-11-8	17-11-8	25-11-0 7-11-8		
	7-11-8	3-0-0	4-0-0	3-0-0			
Plate Offsets (X,Y)- [2:	0-2-0,0-1-12], [6:0-3-0,Edge], [10:0-2-0,0-1	-12]					
OADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (loc) I/defl	L/d	PLATES	GRIP
TCLL 20.0 TCDL 10.0	Plate Grip DOL 1.15 Lumber DOL 1.15	TC 0.65 BC 0.88	Vert(LL) Vert(CT)	-0.51 14-18 >602 -0.78 14-18 >396	240 180	MT20	244/190
BCLL 0.0 *	Rep Stress Incr YES	WB 0.52	Horz(CT)	0.04 12 n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-MS				Weight: 182 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP DSS *Except*

B2: 2x4 SP No.1

WEBS 2x4 SP No.3

BRACING-TOP CHORD

WEBS

BOT CHORD

Structural wood sheathing directly applied or 4-5-12 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing. Except:

6-0-0 oc bracing: 15-20 4-22, 8-12

1 Row at midpt

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 22=1189/0-3-8 (min. 0-2-0), 12=1189/0-3-8 (min. 0-2-0)

Max Horz 22=-208(LC 10) Max Uplift22=-96(LC 12), 12=-96(LC 12) Max Grav 22=1279(LC 18), 12=1281(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-423/214, 3-4=-373/235, 4-5=-1568/237, 7-8=-1571/237, 8-9=-373/235, 9-10=-424/214, 2-22=-430/245, 10-12=-430/245
21-22=-3/1386, 19-21=0/1181, 18-19=0/1181, 14-18=0/1181, 14-23=0/1074, 13-23=0/ TOP CHORD

12-13=-4/1269 **WEBS**

7-15=-35/736, 13-15=-68/585, 8-13=-252/220, 20-21=-68/587, 5-20=-35/733, 4-21=-253/220,

4-22=-1427/0, 8-12=-1429/0, 5-7=-954/229

JOINT STRESS INDEX

2 = 0.86, 3 = 0.28, 4 = 0.79, 5 = 0.63, 6 = 0.13, 7 = 0.63, 8 = 0.79, 9 = 0.28, 10 = 0.86, 12 = 0.62, 13 = 0.43, 14 = 0.28, 15 = 0.27, 16 = 0.27, 17 = 0.28, 18 = 0.27, 19 = 0.71, 20 = 0.27, 21 = 0.43 and 22 = 0.28, 10

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=6ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) All plates are 4x4 MT20 unless otherwise indicated.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom

chord and any other members, with BCDL = 10.0psf.
6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 22 and 12. This connection is for uplift only and does not consider lateral forces.

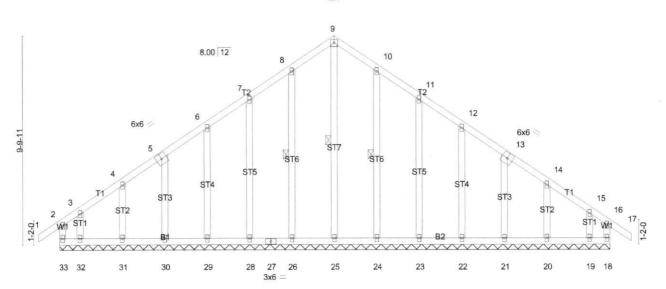
7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

8) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.

Job **Brett Anderson Mancave** 1900052-1900052AA1E Common Supported Gable 1 1 Job Reference (optional)
8.300 s Jun 26 2019 MiTek Industries, Inc. Tue Jul 30 16:55:06 2019 Page 1
ID:8n1Mz0njbAaon55jR1ed_zysr3e-4KU4gY0hmBlawkum8bRt5FyoUSixJtkfwNsgNSysqRJ 84 Components, Dunn, NC 28334 -1-0-0 1-0-0 25-11-0 26-11-0 12-11-8 1-0-0 12-11-8

4x4

Scale = 1:53.0



25-11-0 LOADING (psf) GRIP SPACING-2-0-0 CSL DEFI in (loc) I/defl 1/4 PLATES. TC BC -0.00 120 244/190 Plate Grip DOL 0.18 Vert(LL) TCLL 20.0 1.15 17 n/r MT20 10.0 Vert(CT) -0.01 17 90 TCDL Lumber DOL Rep Stress Incr YES Code IRC2015/TPI2014 BCLL 0.0 WB 0.12 Horz(CT) -0.00 18 n/a n/a Weight: 187 lb FT = 20% Matrix-R BCDL 10.0

25-11-0

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 WEBS

BRACING-TOP CHORD **BOT CHORD** WEBS

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 6-0-0 oc bracing. 1 Row at midpt 9-25, 8-26, 10-24

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS.

ONS. All bearings 25-11-0. (lb) - Max Horz 33=208(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) 26, 28, 29, 30, 31, 24, 23, 22, 21, 20, 19 except 33=-163(LC 10), 18=-109(LC 11),

32=-118(LC 11)

Max Grav All reactions 250 lb or less at joint(s) 33, 18, 26, 28, 29, 30, 31, 32, 24, 23, 22, 21, 20, 19 except 25=259(LC 12)

FORCES. (|b) - Max. Comp./Max. Ten. - All forces 250 (|b) or less except when shown. TOP CHORD 8-9=-236/292, 9-10=-236/292

9-25=-257/145

JOINT STRESS INDEX

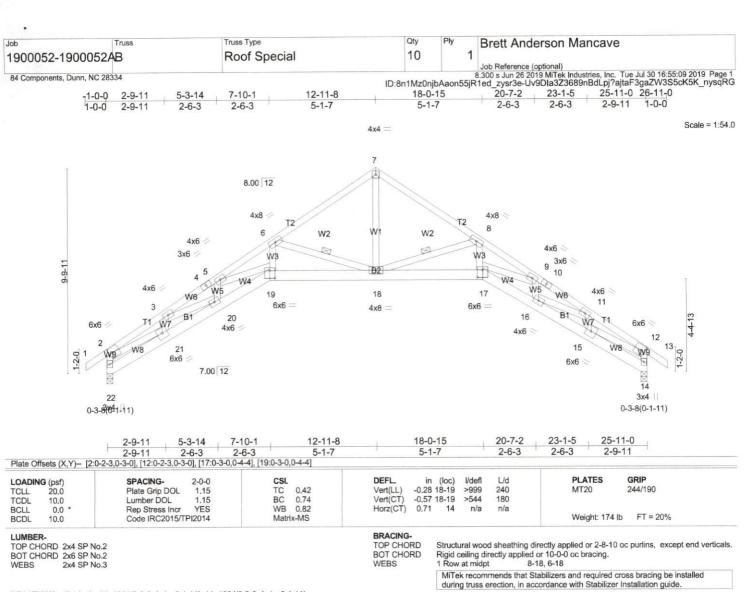
2 = 0.55, 3 = 0.27, 4 = 0.27, 5 = 0.12, 6 = 0.27, 7 = 0.27, 8 = 0.27, 9 = 0.22, 10 = 0.27, 11 = 0.27, 12 = 0.27, 13 = 0.12, 14 = 0.27, 15 = 0.27, 16 = 0.55, 18 = 0.63, 19 = 0.27, 20 = 0.27, 21 = 0.27, 21 = 0.27, 21 = 0.27, 22 = 0.27, 23 = 0.27, 24 = 0.27, 25 = 0.27, 25 = 0.27, 27 = 0.12, 28 = 0.27, 29 = 0.27, 30 = 0.27, 31 = 0.27,

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 3) Truss designed for wind loads in the plane of the truss only. For study exposed to wind (normal to the face), see Standard Industry Gable End Details as
- applicable, or consult qualified building designer as per ANSI/TPI 1.
 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
- 10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 33, 18, 26, 28, 29, 30, 31, 32, 24, 23, 22, 21, 20, and 19. This connection is for uplift only and does not consider lateral forces.

11) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



REACTIONS. (lb/size) 22=1094/0-3-8 (min. 0-1-11), 14=1094/0-3-8 (min. 0-1-11) Max Horz 22=-208(LC 10)

Max Uplift22=-153(LC 12), 14=-153(LC 12)

FORCES, (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD

2-3=-2547/351, 3-4=-3700/437, 4-5=-3638/445, 5-6=-4098/419, 6-7=-1700/226, 7-8=-1700/227, 8-9=-4098/421, 9-10=-3639/449, 10-11=-3700/441, 11-12=-2547/355, 2-22=-1063/222, 12-14=-1063/211

21-22=-191/330, 20-21=-240/2381, 19-20=-281/3559, 18-19=-164/3310, 17-18=-166/3242, 16-17=-284/3438, 15-16=-246/2225 7.18=.117/1487, 8-18=.1998/282, 8-17=.89/1753, 9-17=0/434, 9-16=.337/34, 11-16=.45/1155, 11-15=.584/110, 6-18=.2078/281, WEBS 6-19=-87/1811, 5-19=0/415, 5-20=-337/36, 3-20=-48/1155, 3-21=-584/106, 2-21=-219/1991, 12-15=-225/1991

JOINT STRESS INDEX

2 = 0.75, 3 = 0.36, 4 = 0.43, 5 = 0.25, 6 = 0.65, 7 = 0.69, 8 = 0.65, 9 = 0.25, 10 = 0.43, 11 = 0.36, 12 = 0.75, 14 = 0.54, 15 = 0.60, 16 = 0.35, 17 = 0.77, 18 = 0.52, 19 = 0.77, 20 = 0.35, 21 = 0.60 and 22 = 0.75, 14 = 0.54, 15 = 0.60, 16 = 0.35, 17 = 0.77, 18 = 0.52, 19 = 0.77, 20 = 0.35, 21 = 0.60 and 22 = 0.75, 14 = 0.54, 15 = 0.60, 16 = 0.35, 17 = 0.77, 18 = 0.52, 19 = 0.77, 20 = 0.35, 21 = 0.60 and 22 = 0.75, 20 = 0.70.54

NOTES-

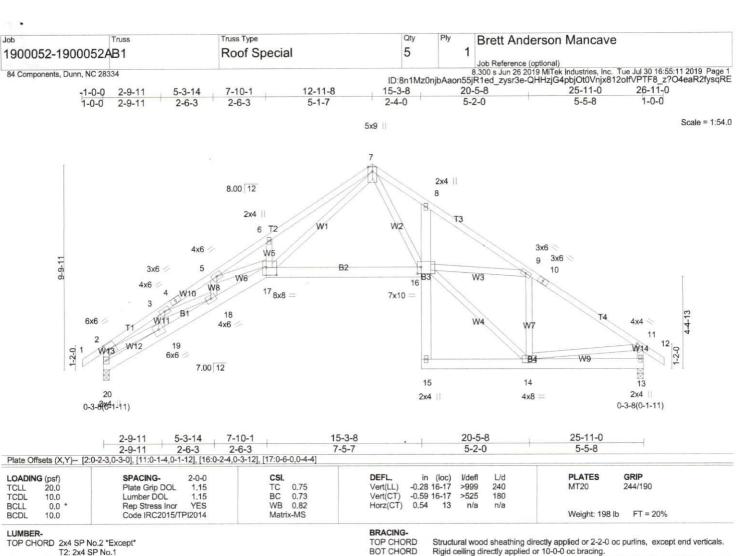
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=6ft; Cat. II; Exp B; Enclosed;

MWFRS (directional) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom
- chord and any other members.

 5) Bearing at joint(s) 22, 14 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 22 and 14. This connection is for uplift only and does not consider lateral forces.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



BOT CHORD

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

TOP CHORD 2x4 SP No.2 *Except* T2: 2x4 SP No.1 BOT CHORD 2x6 SP No.2

2x4 SP No.3 *Except* W1: 2x4 SP No.2 WEBS

REACTIONS. (lb/size) 20=1094/0-3-8 (min. 0-1-11), 13=1094/0-3-8 (min. 0-1-11) Max Horz 20=-208(LC 10)

Max Uplift20=-153(LC 12), 13=-153(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-2541/348, 3-4=-3718/441, 4-5=-3689/453, 5-6=-4077/395, 6-7=-4118/550, 7-8=-2318/343, 8-9=-2375/275, 9-10=-1138/230, 10-11=-1297/211, 2-20=-1064/221, 11-13=-1029/235
19-20=-191/332, 18-19=-238/2375, 17-18=-283/3582, 16-17=0/1298
3-19=-583/109, 3-18=-57/1181, 5-18=-348/23, 5-17=0/366, 6-17=-254/197, 7-17=-361/3013, 7-16=-151/1359, 14-16=-111/1293,

BOT CHORD

WEBS 9-16=0/925, 9-14=-916/157, 2-19=-217/1982, 11-14=-24/823

JOINT STRESS INDEX

2 = 0.75, 3 = 0.37, 4 = 0.44, 5 = 0.25, 6 = 0.27, 7 = 0.90, 8 = 0.64, 9 = 0.38, 10 = 0.17, 11 = 0.75, 13 = 0.95, 14 = 0.72, 15 = 0.45, 16 = 0.79, 17 = 0.78, 18 = 0.35, 19 = 0.60 and 20 = 0.54

NOTES-

Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=6ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom

- chord and any other members.
- 5) Bearing at joint(s) 20 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 20 and 13. This connection is for uplift only and does not consider lateral forces.

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

Joh Truss Type Qtv **Brett Anderson Mancave GABLE** 1 1900052-1900052AB1E 1 84 Components, Dunn, NC 28334 12-11-8 -1-0-0 2-9-11 5-3-14 7-10-1 5-5-8 1-0-0 2-9-11 2-6-3 2-6-3 5-1-7 2-4-0 5-2-0 Scale = 1:54.7 10x12 5x9 || 8.00 12 W2 ST4 6 T2 ST3 ST1 ST5 3x6 4x6 W5 6x6 9-9-11 9 3x6 5 B2 10 W3 WA **B**3 16 4x6 W8 17_{8x8} = 4 W10 7x10 ST9 3 STI B1 18 T4 W11 6x6 W4 4x4 4x6 ST6 ST10 12 0-7-1 11 ST8 W12 19 W14 W13 6x6 W9 B4 7.00 12 15 14 13 20 4x8 = 0-3-87841-11) 0-3-8(0-1-11) 7-10-1 15-3-8 20-5-8 25-11-0 2-9-11 5-3-14 2-9-11 2-6-3 2-6-3 5-2-0 5-5-8 7-5-7 Plate Offsets (X,Y)- [2:0-2-3,0-3-0], [7:0-1-11,0-5-0], [11:0-1-4,0-1-12], [16:0-2-4,0-3-12], [17:0-6-0,0-4-4] **PLATES** SPACING-DEFL in (loc) GRIP LOADING (psf. TCLL 20.0 Plate Grip DOL 1.15 TC BC 0.75 Vert(LL) -0.28 16-17 >999 240 MT20 244/190 -0.59 16-17 180 Vert(CT) >525 TCDL 10.0 Lumber DOL 1.15 BCLL 0.0 Rep Stress Incr WB Horz(CT) 0.54 n/a Weight: 235 lb FT = 20% BCDL 10.0 Code IRC2015/TPI2014 Matrix-MS

LUMBER

TOP CHORD 2x4 SP No.2 *Except* T2: 2x4 SP No.1

2x6 SP No.2 2x4 SP No.3 *Except* **BOT CHORD**

WEBS W1: 2x4 SP No.2

OTHERS 2x4 SP No.3

REACTIONS. (lb/size) 20=1094/0-3-8 (min. 0-1-11), 13=1094/0-3-8 (min. 0-1-11)

Max Horz 20=-208(LC 10)

Max Uplift20=-153(LC 12), 13=-153(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3--2541/348, 3-4--3718/441, 4-5--3689/453, 5-6-4077/395, 6-7-4118/550, 7-8--2318/343, 8-9--2375/275, 9-10-1138/230, 10-11-1297/211, 2-20-1064/221, 11-13--1029/235 19-20-191/332, 18-19--238/2375, 17-18--283/3582, 16-17-0/1298 TOP CHORD

BOT CHORD

3-19=-583/109, 3-18=-57/1181, 5-18=-348/23, 5-17=0/366, 6-17=-254/197, 7-17=-361/3013, 7-16=-151/1359, 14-16=-111/1293, 9-16=0/925, 9-14=-916/157, 2-19=-217/1982, 11-14=-24/823

JOINT STRESS INDEX

2 = 0.75, 3 = 0.37, 4 = 0.44, 5 = 0.25, 6 = 0.27, 7 = 0.90, 7 = 0.29, 8 = 0.64, 9 = 0.38, 10 = 0.16, 11 = 0.75, 13 = 0.95, 14 = 0.72, 15 = 0.45, 16 = 0.79, 17 = 0.78, 18 = 0.35, 19 = 0.60, 20 = 0.54, 21 = 0.27, 22 = 0.27, 23 = 0.27, 24 = 0.27, 25 = 0.27, 26 = 0.28, 27 = 0.27, 28 = 0.27, 29 = 0.27, 30 = 0.27, 31 = 0.27, 32 = 0.27, 33 = 0.27, 34 = 0.27, 34 = 0.27, 35 = 0.27, 36 = 0.27, 37 = 0.27

BRACING-TOP CHORD

BOT CHORD

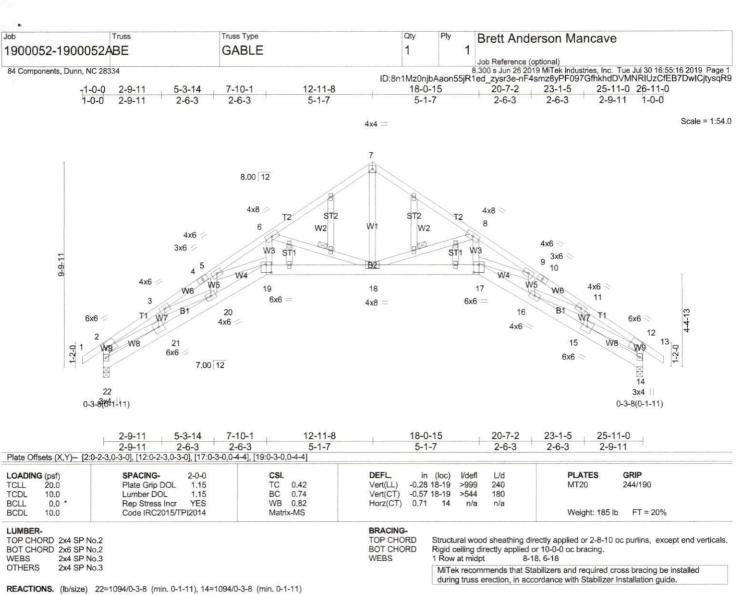
Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed

during truss erection, in accordance with Stabilizer Installation guide.

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=6ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Bearing at joint(s) 20 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 20 and 13. This connection is for uplift only
- and does not consider lateral forces. 10) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Max Horz 22=-208(LC 10)

Max Uplift22=-153(LC 12), 14=-153(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-2547/351, 3-4=-3700/437, 4-5=-3638/445, 5-6=-4098/419, 6-7=-1700/226, 7-8=-1700/227, 8-9=-4098/421, 9-10=-3639/449, 10-11=-3700/441, 11-12=-2547/355, 2-22=-1063/222, 12-14=-1063/211 TOP CHORD

BOT CHORD 21-22--191/330, 20-21--240/2381, 19-20--281/3559, 18-19--164/3310, 17-18--166/3242, 16-17--284/3438, 15-16--246/2225 7-18=-117/1487, 8-18=-1998/292, 8-17=-89/1753, 9-17=0/434, 9-16=-337/34, 11-16=-45/1155, 11-15=-584/110, 6-18=-2078/281, WERS 6-19=-87/1811, 5-19=0/415, 5-20=-337/36, 3-20=-48/1155, 3-21=-584/106, 2-21=-219/1991, 12-15=-225/1991

JOINT STRESS INDEX

2 = 0.75, 3 = 0.36, 4 = 0.43, 5 = 0.25, 6 = 0.65, 7 = 0.69, 8 = 0.65, 9 = 0.25, 10 = 0.43, 11 = 0.36, 12 = 0.75, 14 = 0.54, 15 = 0.60, 16 = 0.35, 17 = 0.77, 18 = 0.52, 19 = 0.77, 20 = 0.35, 21 = 0.60, 22 = 0.5423 = 0.27, 24 = 0.27, 25 = 0.27, 26 = 0.27, 27 = 0.27, 28 = 0.27, 29 = 0.27 and 30 = 0.27

1) Unbalanced roof live loads have been considered for this design

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=6ft; Cat. II; Exp B; Enclosed;
- MWFRS (directional) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

All plates are 2x4 MT20 unless otherwise indicated.
 Gable studs spaced at 2-0-0 oc.

- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Bearing at joint(s) 22, 14 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface. 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 22 and 14. This connection is for uplift only and does not consider lateral forces.
- 10) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.