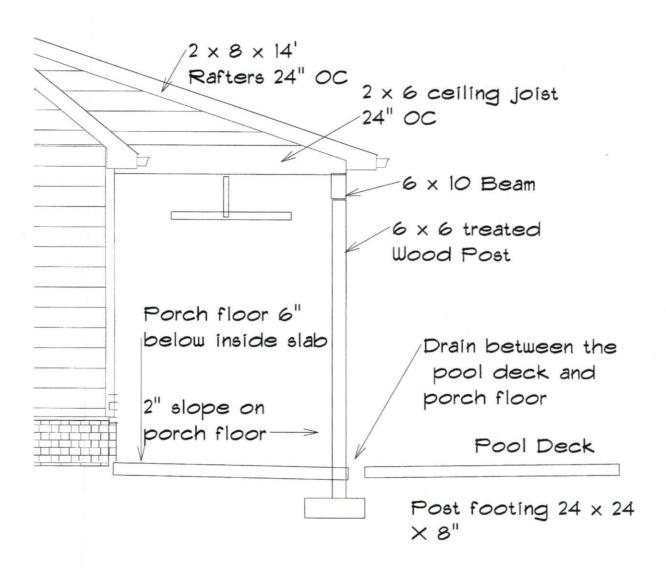
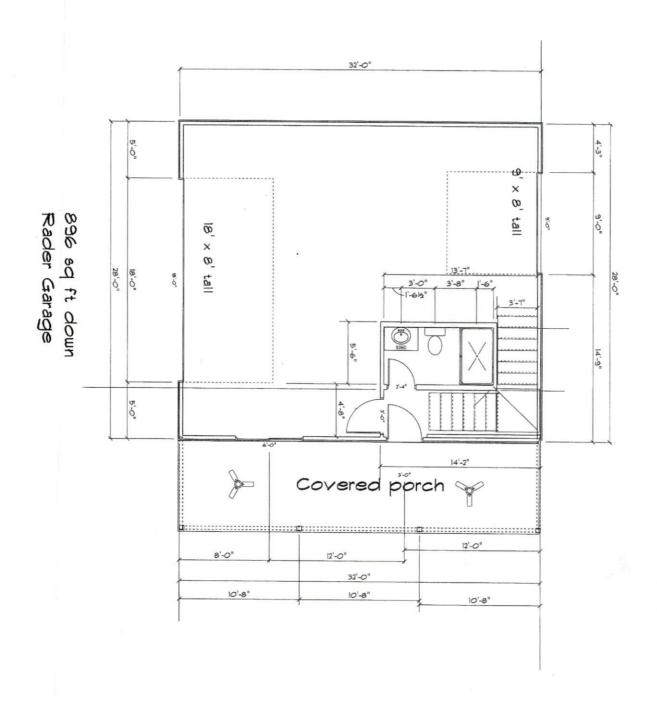


²itch







Trenco

818 Soundside Rd Edenton, NC 27932

Re: 19070077

CUSTOM CONTRACTING / RADAR GARAGE

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Carter Components (Sanford, NC)).

Pages or sheets covered by this seal: E13317692 thru E13317698

My license renewal date for the state of North Carolina is December 31, 2019.

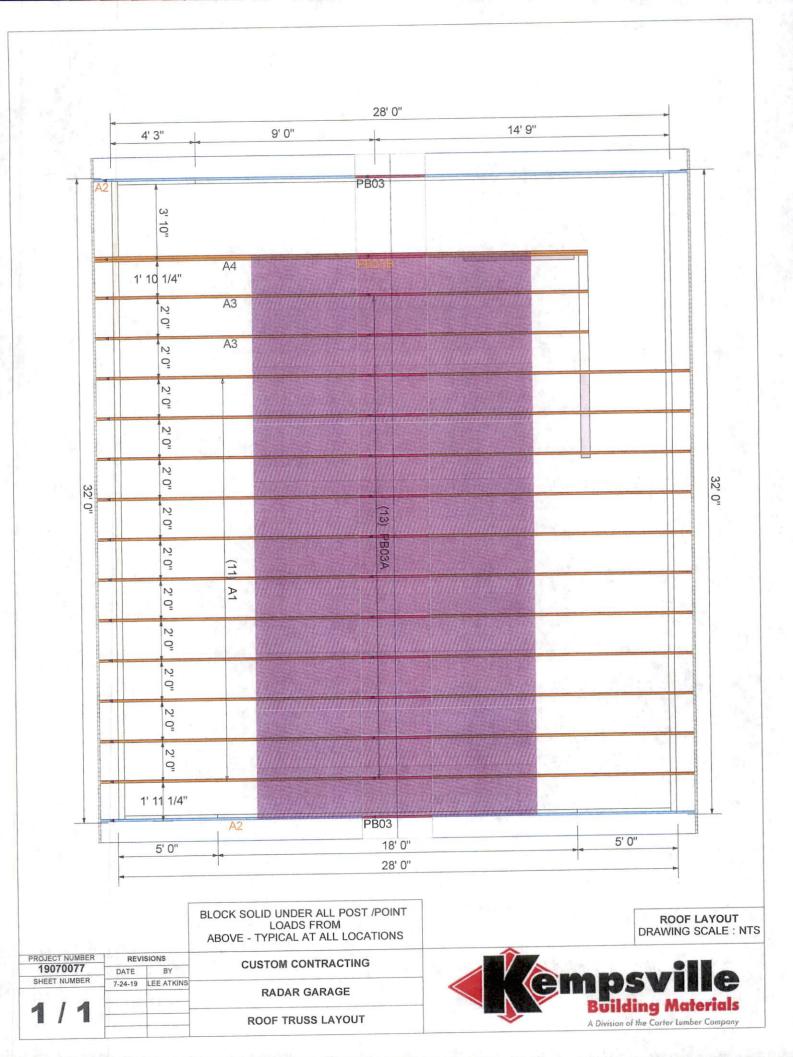
North Carolina COA: C-0844



July 24,2019

Gilbert, Eric

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



CUSTOM CONTRACTING / RADAR GARAGE Truss Type Truss E13317692 Job 11 Attic 19070077 Job Reference (optional) 8.310 s Jun 11 2019 MiTek Industries, Inc. Wed Jul 24 14:38:22 2019 Page 1 Sanford, NC - 27332. Carter Components (Sanford), ID:2_Fok_oW2orZ7dt8pE?V90zLuh6-CWMnb5NsyLW9_cV5CM9dCnj_HN70bND2LGRbrYyuq7F 18-7-15 9-3-1 9-11-13 12-2-7 2-5-5 0-8-12 2-2-11 2-2-11 0-8-12 2-5-5 3-4-0 Scale = 1:66.0 5x6 5x6 = 9.75 12 4x8 // 4x8 1 4x6 4x6 📏 7-4-10 4x8 4x8 3x5 || 10 5x8 5x8 < 11 4x5 4x5 / 12 3 5-6-0 10x12 = 14-0-0 1-7-0 17 16 18 10x12 8x10 = 8x10 = 8x10 = 27-11-0 21-1-4 6-9-12 6-9-12 14-3-8 [2:Edge,0-4-8], [2:0-1-12,0-1-7], [7:0-3-0,0-2-12], [8:0-3-0,0-2-12], [13:0-1-12,0-1-7], [15:Edge,0-4-8], [16:0-3-8,0-4-12], [18:0-3-8,0-4-12] Plate Offsets (X,Y)--GRIP LOADING (psf) L/d **PLATES** I/defl DEFL. in (loc) SPACING-2-0-0 MT20 244/190 20.0 240 TCLL (roof) Vert(LL) -0.33 16-18 >999 TC 0.72 Plate Grip DOL 1.15 18 9/20 0 180 Snow (Pf/Pg) Vert(CT) -0.47 16-18 >712 BC 0.37 Lumber DOL 1.15 TCDL 10.0 WB 0.79 Horz(CT) 0.02 15 n/a n/a Rep Stress Incr YES FT = 20% Weight: 275 lb BCLL 0.0 -0.20 16-18 856 360 Attic Code IRC2015/TPI2014 Matrix-MSH 10.0 BCDL BRACING-LUMBER-Sheathed or 3-9-3 oc purlins, except end verticals, and 2-0-0 oc TOP CHORD 2x6 SP No.2 *Except* TOP CHORD purlins (6-0-0 max.): 7-8. 4-7.8-11: 2x6 SP 2400F 2.0E, 5-6,9-10: 2x4 SP No.2 Rigid ceiling directly applied or 10-0-0 oc bracing **BOT CHORD** 2x10 SP 2400F 2.0E **BOT CHORD** JOINTS 1 Brace at Jt(s): 20 2x4 SP No.3 *Except* WEBS 5-18,6-9,10-16: 2x4 SP No.2 (lb/size) 15=1132/0-3-8, 19=1132/0-3-8 REACTIONS. Max Horz 19=-239(LC 11) Max Grav 15=1598(LC 27), 19=1598(LC 26) FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown 2-3=-276/109, 3-5=-1976/107, 5-6=-1282/211, 6-7=-206/253, 7-8=-44/361, TOP CHORD 8-9=-207/253, 9-10=-1282/211, 10-12=-1976/107, 12-13=-276/109, 2-19=-265/126, 13-15=-265/126 18-19=-5/1506, 16-18=0/1362, 15-16=0/1412 BOT CHORD 5-18=0/985, 6-20=-1614/186, 9-20=-1614/186, 10-16=0/985, 12-15=-1883/0, WEBS NOTES- Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=18.9 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Fully Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs

non-concurrent with other live loads. Provide adequate drainage to prevent water ponding.

* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

Ceiling dead load (5.0 psf) on member(s). 5-6, 9-10, 6-20, 9-20; Wall dead load (5.0 psf) on member(s). 5-18, 10-16 8) Bottom chord live load (40.0 psf) and additional bottom chord dead load (0.0 psf) applied only to room. 16-18

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

10) Attic room checked for L/360 deflection.

THE CONTRACTOR OF STREET SEAL 036322 GIL "Immini July 24,2019

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing bialding design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



CUSTOM CONTRACTING / RADAR GARAGE Qty PIV Truss Type Job Truss F13317693 GABLE A2 19070077 Job Reference (optional) 8.310 s Jun 11 2019 MiTek Industries, Inc. Wed Jul 24 14:38:24 2019 Page 1 Carter Components (Sanford), Sanford, NC - 27332, ID:2_Fok_oW2orZ7dt8pE?V90zLuh6-8vUX0nO6UymtDvfUJnB5HCoL9Apg3l6LoawivRyuq7D 18-7-15 17-11-3 21-1-4 2-2-11 0-8-12 2-5-5 15-8-9 3-6-1 Scale = 1:69.6 5x6 = 5x6 = 8 9.75 12 4x8 / 4x8 \ 4x6 / 9 4x6 \ 7-4-90 4x8 / 4x8 \ 10 3x5 || 5x8 / 5 5x8 < 3x5 = 11 4x5 🚿 4x5 12 5-6-0 10x12 = 14-0-0 0-4-0 1-7-0 17 163x6 || 3x6 || 3x6 || 3x6 || 18 10x12 = 8x10 = 8x10 = 8x10 = 3x6 || 27-11-0 6-9-12 21-1-4 14-3-8 6-9-12 6-9-12 [2:Edge,0-4-8], [2:0-1-12,0-1-7], [7:0-3-0,0-2-12], [8:0-3-0,0-2-12], [13:0-1-12,0-1-7], [15:Edge,0-4-8], [16:0-3-8,0-4-12], [18:0-3-8,0-4-12], [20:0-2-8,0-0-0] Plate Offsets (X,Y)--[23:0-1-12,0-1-0], [32:0-1-12,0-1-0] LOADING (psf) GRIP **PLATES** L/d CSI. DEFL in (loc) I/defl SPACING-1-11-4 20.0 TCLL (roof) MT20 244/190 240 Plate Grip DOL 1.15 TC 0.70 Vert(LL) -0.32 16-18 >999 Snow (Pf/Pg) 18.9/20.0 0.36 Vert(CT) -0.45 16-18 >735 180 Lumber DOL 1.15 BC TCDL 10.0 0.76 0.02 15 n/a n/a Rep Stress Incr YES WB Horz(CT) BCLL 0.0 FT = 20% -0.19 16-18 360 Weight: 311 lb Code IRC2015/TPI2014 Matrix-MSH 883 10.0 BCDL BRACING-LUMBER-Sheathed or 3-11-6 oc purlins, except end verticals, and 2-0-0 oc TOP CHORD TOP CHORD 2x6 SP No.2 *Except* purlins (6-0-0 max.): 7-8. 4-7,8-11: 2x6 SP 2400F 2.0E, 5-6,9-10: 2x4 SP No.2 Rigid ceiling directly applied or 10-0-0 oc bracing. BOT CHORD 2x10 SP 2400F 2.0E BOT CHORD 1 Brace at Jt(s): 20 2x4 SP No.3 *Except* JOINTS WEBS 5-18,6-9,10-16: 2x4 SP No.2 2x4 SP No.3 OTHERS (lb/size) 15=1097/0-3-8, 19=1097/0-3-8 REACTIONS. Max Horz 19=-232(LC 11) Max Grav 15=1548(LC 27), 19=1548(LC 26) FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-268/106, 3-5=-1914/103, 5-6=-1242/205, 7-8=-42/350, 9-10=-1242/205, 10-12=-1914/103, 12-13=-268/106, 2-19=-257/122, 13-15=-257/122 18-19=-5/1459, 16-18=0/1320, 15-16=0/1368 BOT CHORD 5-18=0/954, 6-20=-1564/180, 9-20=-1563/180, 10-16=0/954, 12-15=-1824/0, WEBS 3-19=-1824/0 NOTES- Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members NORTH and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1 4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=18.9 psf (flat

- roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Fully Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable studs spaced at 2-0-0 oc.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Ceiling dead load (5.0 psf) on member(s). 5-6, 9-10, 6-20, 9-20; Wall dead load (5.0 psf) on member(s).5-18, 10-16
- 11) Bottom chord live load (40.0 psf) and additional bottom chord dead load (0.0 psf) applied only to room. 16-18
- 12) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 15 and 19. This connection is for uplift only and does not consider lateral forces.

13))(Greeningsbauding representation does not depict the size or the orientation of the purlin along the top and/or bottom chord

SEAL

MA. GIL GII

July 24,2019

Edenton, NC 27932

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance reparading the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANS/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

CUSTOM CONTRACTING / RADAR GARAGE Qty Truss Type E13317693 Job Truss Job Reference (optional)

8.310 s Jun 11 2019 MiTek Industries, Inc. Wed Jul 24 14:38:24 2019 Page 2
ID:2_Fok_oW2orZ7dt8pE?V90zLuh6-8vUX0nO6UymtDvfUJnB5HCoL9Apg3l6LoawivRyuq7D GABLE A2 19070077

Carter Components (Sanford),

Sanford, NC - 27332,

NOTES-

14) Attic room checked for L/360 deflection.

CUSTOM CONTRACTING / RADAR GARAGE Truss Type Truss E13317694 Job Attic 19070077 Job Reference (optional) 8.310 s Jun 11 2019 MiTek Industries, Inc. Wed Jul 24 14:38:26 2019 Page 1 Carter Components (Sanford), Sanford NC - 27332 ID:2_Fok_oW2orZ7dt8pE?V90zLuh6-4IcHRTQM0a0bTDpsRBDZMdtdO_U9XBfeGuPo_Kyuq7B 18-7-15 17-11-3 21-1-4 2-2-11 0-8-12 2-5-5 9-3-1 9-11-13 12-2-7 Scale = 1:69.9 5x6 = 5x6 3x6 / 3x10 > 4x5 9 4x5 < 7-480 4x6 3x5 \ 10 4x8 5x6 < 11 9.75 12 4x5 / 14-0-0 12 17 15 16 14 3x6 6x8 = 10x12 = 10x12 = 5x6 = 23-9-8 6-9-12 [2:0-1-12,0-1-7], [7:0-3-0,0-2-12], [8:0-3-0,0-2-12], [14:0-3-8,0-7-12], [17:Edge,0-4-12] Plate Offsets (X,Y)--GRIP LOADING (psf L/d PLATES I/defl DEFL. in (loc) SPACING-2-0-0 MT20 244/190 20.0 TCLL (roof) Vert(LL) -0.27 14-16 >999 240 TC 0.91 Plate Grip DOL 1.15 18.9/20.0 180 Snow (Pf/Pg) -0.40 14-16 >707 BC 0.42 Vert(CT) Lumber DOL 1.15 TCDL 10.0 WB 0.76 Horz(CT) 0.01 13 n/a n/a Rep Stress Incr YES FT = 20% Weight: 243 lb BCLL 0.0 -0.18 14-16 946 360 Attic Matrix-MSH Code IRC2015/TPI2014 10.0 BCDL BRACING-LUMBER-Sheathed or 2-7-8 oc purlins, except end verticals, and 2-0-0 oc TOP CHORD 2x6 SP No 2 *Except TOP CHORD purlins (6-0-0 max.): 7-8. 4-7: 2x6 SP 2400F 2.0E, 5-6,9-10: 2x4 SP No.2 Rigid ceiling directly applied or 10-0-0 oc bracing **BOT CHORD** 2x10 SP 2400F 2.0E **BOT CHORD** 11-13 **WEBS** 1 Row at midpt 2x4 SP No.3 *Except* WEBS JOINTS 1 Brace at Jt(s): 18 5-16,6-9,10-14: 2x4 SP No.2 (lb/size) 17=963/0-3-8, 13=978/0-5-8 REACTIONS. Max Horz 17=202(LC 10) Max Grav 17=1355(LC 26), 13=1490(LC 26) FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown 2-3=-262/105, 3-5=-1404/55, 5-6=-918/174, 6-7=-336/114, 8-9=-329/112, TOP CHORD 9-10=-988/183, 10-11=-1279/43, 2-17=-258/130, 11-13=-2538/42 16-17=-114/1108, 14-16=-6/916 BOT CHORD 5-16=0/703, 6-18=-912/117, 9-18=-1005/127, 10-14=-283/495, 11-14=-12/1873, WEBS 3-16=-274/155, 3-17=-1314/0 NOTES-

1) Unbalanced roof live loads have been considered for this design

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=18.9 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Fully Exp., Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.

5) Provide adequate drainage to prevent water ponding.

- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Ceiling dead load (5.0 psf) on member(s). 5-6, 9-10, 6-18, 9-18; Wall dead load (5.0 psf) on member(s).5-16, 10-14
- 8) Bottom chord live load (40.0 psf) and additional bottom chord dead load (0.0 psf) applied only to room. 14-16
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

10) Attic room checked for L/360 deflection.



MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing building design end for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



CUSTOM CONTRACTING / RADAR GARAGE Qty Ply Truss Type Job Truss F13317695 ATTIC 2 Job Reference (optional) 19070077 A4 8.310 s Jun 11 2019 MiTek Industries, Inc. Wed Jul 24 14:38:27 2019 Page 1 Sanford, NC - 27332 Carter Components (Sanford), ID:2_Fok_oW2orZ7dt8pE?V90zLuh6-YUAgepR_nt8S4NO2_vkovqQn?OqUGhsnUY9MWmyuq7A 18-7-15 17-11-3 21-1-4 2-2-10 0-8-12 2-5-5 Scale = 1:69.9 5x6 = R 3x6 / 3x8 \ 4x5 3x5 // 7-489 3x8 3x5 N 10 3x5 || 4x6 4x5 11 9.75 12 4x5 / 14-0-0 12 15 16 14 3x6 || 6x8 = 8x10 = 10x12 = 5x6 = 23-9-8 6-9-12 14-3-8 [2:0-1-12,0-1-7], [7:0-3-0,0-2-12], [8:0-3-0,0-2-12], [14:0-3-8,0-7-8], [17:Edge,0-3-0]

TCLL (roof) Snow (Pf/Pg	20.0	SPACING- Plate Grip DOL Lumber DOL	3-0-0 1.15 1.15	TC BC	0.98 0.35	Vert(LL) Vert(CT)	in -0.22 -0.31	(loc) 14-16 14-16	l/defl >999 >892	L/d 240 180	MT20	244/190
TCDL BCLL	10.0	Rep Stress Incr	NO	WB	0.57	Horz(CT)	0.01	13	n/a	n/a	Weight: 486 lb	FT = 20%
BCLL	10.0	Code IRC2015/TP	12014	Matr	ix-MSH	Attic	-0.14	14-16	1222	360	Weight, 400 ib	11-2070

BRACING-

TOP CHORD

BOT CHORD

JOINTS

2-0-0 oc purlins (6-0-0 max.), except end verticals

Rigid ceiling directly applied or 10-0-0 oc bracing.

(Switched from sheeted: Spacing > 2-8-0).

1 Brace at Jt(s): 7, 8, 2, 11, 18

LUMBER-

REACTIONS.

Plate Offsets (X,Y)--

TOP CHORD 2x6 SP No.2 *Except*

5-6,9-10: 2x4 SP No.2

2x10 SP 2400F 2.0E BOT CHORD 2x4 SP No.3 *Except* WEBS

5-16,6-9,10-14: 2x4 SP No.2

(lb/size) 17=1444/0-3-8, 13=1466/0-5-8

Max Horz 17=407(LC 10)

Max Grav 17=2029(LC 26), 13=2238(LC 26)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-407/163, 3-5=-2114/127, 5-6=-1399/296, 6-7=-498/175, 7-8=-310/247, TOP CHORD

8-9=-482/171, 9-10=-1474/298, 10-11=-1923/141, 2-17=-388/187, 11-13=-3841/97

16-17=-281/1678, 14-16=-114/1404 BOT CHORD

5-16=-31/1015, 6-18=-1412/235, 9-18=-1517/238, 10-14=-414/771, 11-14=-106/2837, WEBS

3-16=-390/238, 3-17=-1983/0

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc, 2x4 - 1 row at 0-9-0 oc.

Bottom chords connected as follows: 2x10 - 2 rows staggered at 0-9-0 oc.

Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

5) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=18.9 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Fully Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.

6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.

Provide adequate drainage to prevent water ponding.

* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Ceiling dead load (5.0 psf) on member(s). 5-6, 9-10, 6-18, 9-18; Wall dead load (5.0 psf) on member(s).5-16, 10-14

10) Bottom chord live load (40.0 psf) and additional bottom chord dead load (0.0 psf) applied only to room. 14-16 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

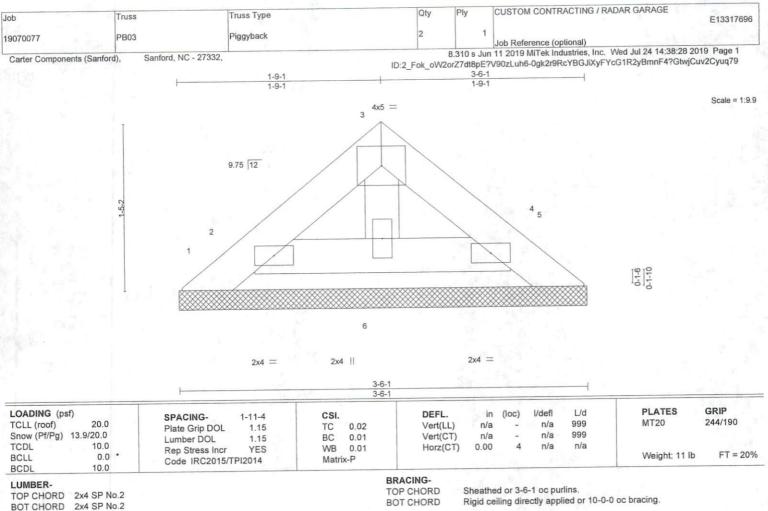
12) Attic room checked for L/360 deflection.

JORTH GI July 24,2019



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BOT CHORD 2x4 SP No.2 **OTHERS**

2x4 SP No.3

REACTIONS. All bearings 3-6-1.

(lb) - Max Horz 1=24(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5, 2, 4

Max Grav All reactions 250 lb or less at joint(s) 1, 5, 2, 4, 6

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=13.9 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Fully Exp.; Ct=1.10

Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 2-0-0 oc.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5.

9) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 4. This connection is for uplift only and does not consider lateral forces.

10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the ANS/ITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



CUSTOM CONTRACTING / RADAR GARAGE Qty Truss Type Job Truss E13317697 13 PB03A Piggyback 19070077 Job Reference (optional) 8.310 s Jun 11 2019 MiTek Industries, Inc. Wed Jul 24 14:38:29 2019 Page 1 Sanford, NC - 27332, Carter Components (Sanford). ID:2_Fok_oW2orZ7dt8pE?V90zLuh6-UtlQ3VSFJVOAKhXR6KnG_FVMVBbqkjE4yseTafyuq78 1-9-1 Scale = 1:9.2 3x5 = 3 9.75 12 0-4-12 0-1-10 2x4 = 2x4 = [3:0-2-8,Edge] Plate Offsets (X,Y)--LOADING (psf) **PLATES** GRIP L/d 2-0-0 CSI DEFL. in (loc) I/defl SPACING-TCLL (roof) 20.0 244/190 120 MT20 1.15 0.00 n/r TC 0.02 Vert(LL) 4 Plate Grip DOL Snow (Pf/Pg) 13.9/20.0 120 Lumber DOL 1.15 BC 0.04 Vert(CT) 0.00 n/r TCDL 10.0 WB 0.00 Horz(CT) 0.00 n/a n/a YES Rep Stress Incr BCLL 0.0 * Weight: 10 lb FT = 20% Code IRC2015/TPI2014 Matrix-P 10.0 BCDL BRACING-LUMBER-TOP CHORD Sheathed or 3-6-1 oc purlins. TOP CHORD 2x4 SP No.2 Rigid ceiling directly applied or 10-0-0 oc bracing **BOT CHORD** BOT CHORD 2x4 SP No.2 REACTIONS.

(lb/size) 2=94/2-2-6, 4=94/2-2-6

Max Horz 2=24(LC 12) Max Uplift 2=-3(LC 13), 4=-3(LC 14)

Max Grav 2=113(LC 2), 4=113(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=13.9 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Fully Exp.; Ct=1.10
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.

5) Gable requires continuous bottom chord bearing.

- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 4. This connection is for uplift only and does not consider lateral forces.
- 8) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



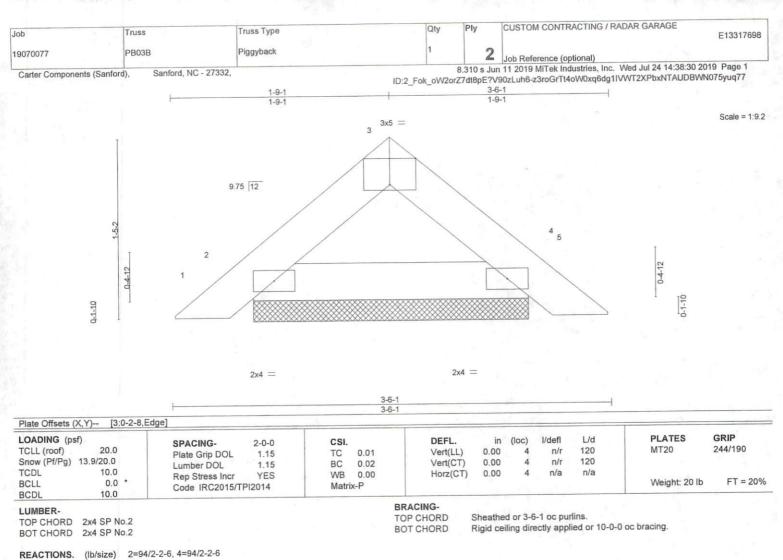
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(lb/size) 2=94/2-2-6, 4=94/2-2-6 Max Horz 2=24(LC 12)

Max Uplift 2=-3(LC 13), 4=-3(LC 14) Max Grav 2=113(LC 2), 4=113(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) 2-ply truss to be connected together as follows:

Top chords connected with 10d (0.131"x3") nails as follows: 2x4 - 1 row at 0-9-0 oc. Bottom chords connected with 10d (0.131"x3") nails as follows: 2x4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

5) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=13.9 psf (flat

roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Fully Exp.; Ct=1.10 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs

non-concurrent with other live loads. 7) Gable requires continuous bottom chord bearing.

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 4. This connection is for uplift only and does not consider lateral forces.

10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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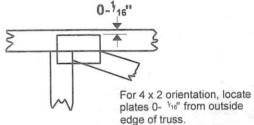


Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated.
Dimensions are in ft-in-sixteenths.
Apply plates to both sides of truss and fully embed teeth.



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal

Plate Connected Wood Truss Construction.

DSB-89: Design Standard for Bracing.

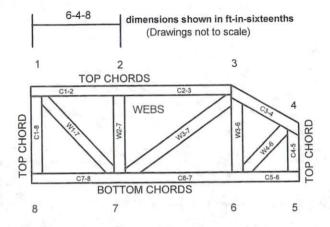
BCSI:

Building Component Safety Information,

Guide to Good Practice for Handling, Installing & Bracing of Metal Plate

Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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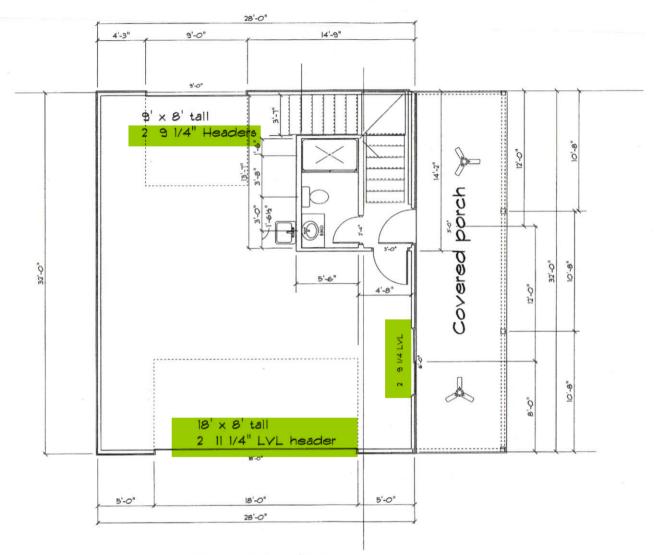
MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015



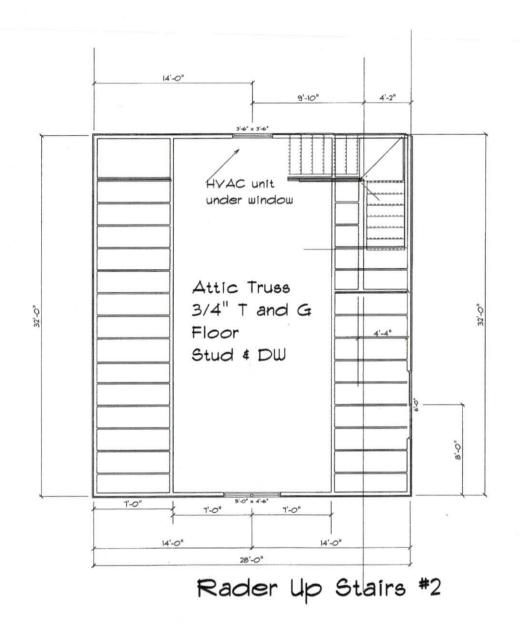
General Safety Notes

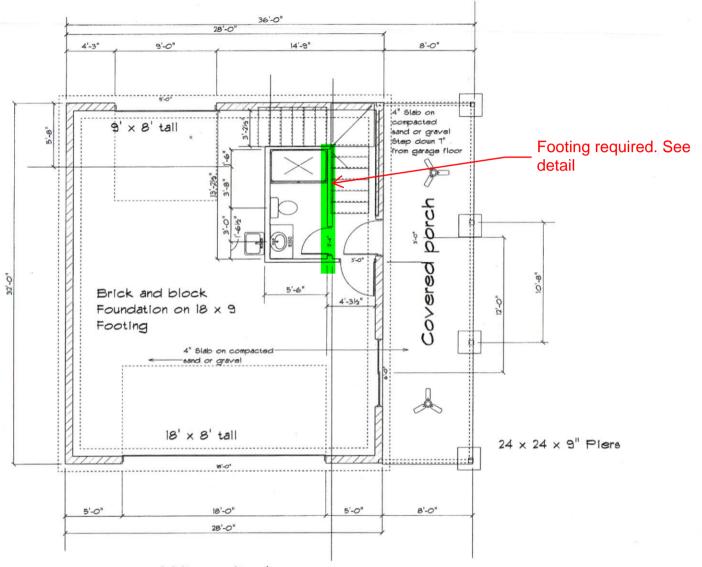
Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- 5. Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- 15. Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.



896 sq ft down Rader Garage





896 sq ft down Rader Garage Foundation

