

Design trusses less than or equal to 2000 lbs are limited to single ply LVL Kerto-S. The contractor shall refer to the attached truss layout for the principal truss configuration. To determine the principal truss configuration and number of steel studs required to support the principal truss, the contractor shall refer to the attached truss layout. A registered design professional shall be retained to verify these specifications and to provide a stamped design professional seal to be affixed to the truss layout. The contractor shall refer to the attached truss layout for all members not shown.

Signature: Lenny Norris
 Lenny Norris

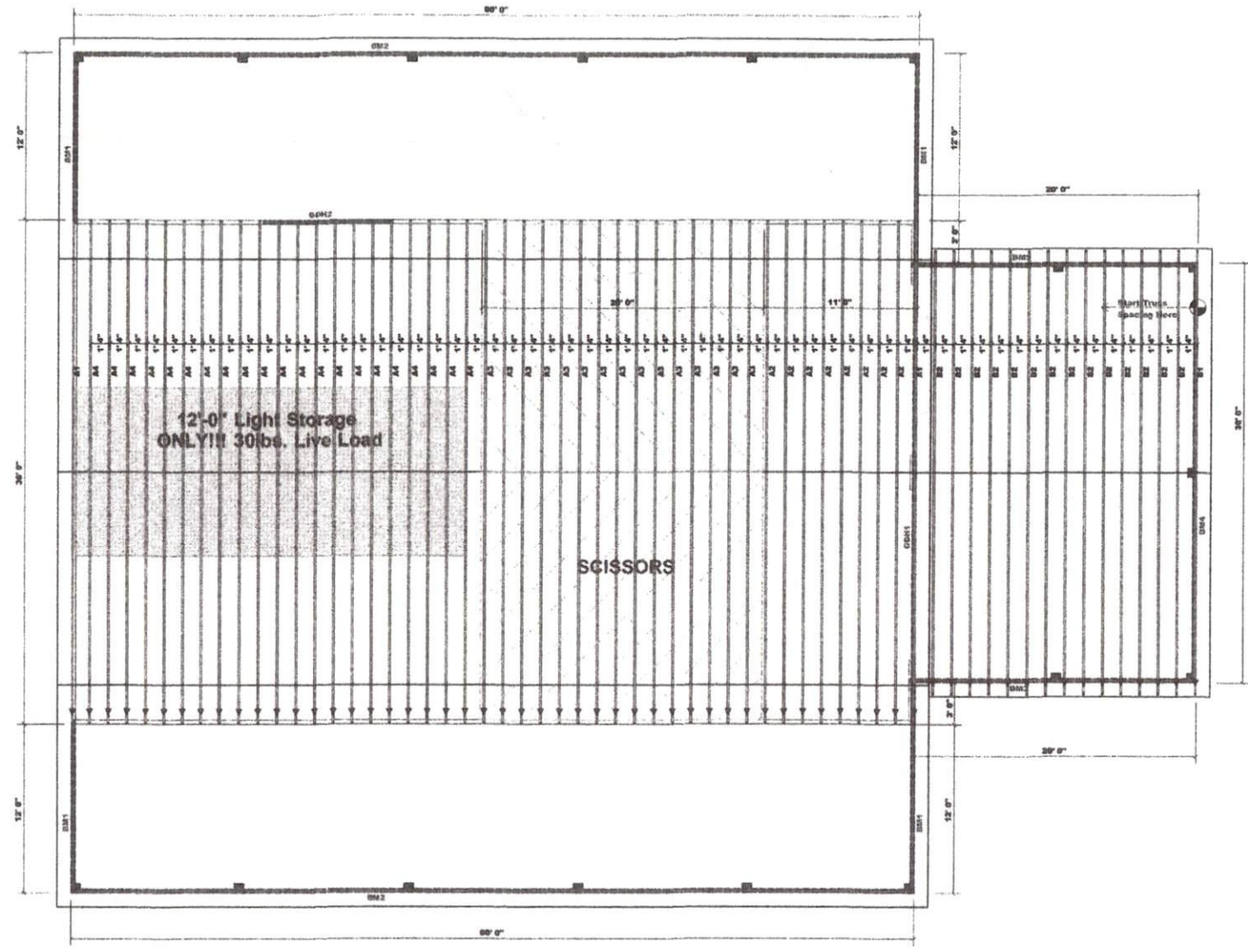
LOAD CHART FOR JACK STUDS
 BASED ON TABLE 10.1.1 OF
 NUMBER OF JACK STUDS REQUIRED @ 24" ON CENTER

TRUSS REACTION (LBS)	NO. OF JACK STUDS	TRUSS REACTION (LBS)	NO. OF JACK STUDS
1700	1	2500	1
3400	2	5100	2
5100	3	7650	3
6800	4	10200	4
8500	5	12750	5
10200	6	15300	6
11900	7		
13600	8		
15300	9		

CITY / CO	ADDRESS	MODEL	DATE REV.	DRAWN BY	SALES REP.
Dunn / Harnett	3943 Fairground Road	ROOF	/ /	Lenny Norris	Lenny Norris

BUILDER	JOB NAME	PLAN	SEAL DATE	QUOTE #	JOB #
BLM Builders, LLC	Stewarts Garage	CUSTOM GARAGE	Seal Date	Quote #	J0619-2765

NOTE: THIS IS A TRUSS LAYOUT DRAWING ONLY. THESE TRUSSES ARE DESIGNED AS INDICATED BUILDING COMPONENTS TO BE ASSEMBLED ON THE BUILDING CHANGING AT THE DISCRETION OF THE BUILDING OWNER. THE CONTRACTOR SHALL VERIFY THE TRUSS CONFIGURATION ON THE PLACEMENT DRAWING. THE BUILDING OWNER IS RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND TRUSS REACTION RECORDS. THE CONTRACTOR SHALL VERIFY THE TRUSS CONFIGURATION ON THE PLACEMENT DRAWING. THE BUILDING OWNER IS RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND TRUSS REACTION RECORDS. THE CONTRACTOR SHALL VERIFY THE TRUSS CONFIGURATION ON THE PLACEMENT DRAWING. THE BUILDING OWNER IS RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND TRUSS REACTION RECORDS.



Estimation

Name	Selection	Formula	Calculation
Roof Area	1st Floor	Roof Area	5099.67
Roof Decking	1st Floor	Roof Decking	175 sheets

BEAM LEGEND

PlotID	Length	Product	Plies	Net Qty
BM2	60' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	4
BM4	30' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2
BM3	21' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	4
BM1	13' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	8
GDH1	36' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2
GDH2	9' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2

Truss Placement Plan
SCALE: 3/16" = 1'-0"

▲ = Denotes Left End of Truss
(Reference Engineered Truss Drawing)

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

○ -- Denotes Reaction Greater than 3,000 lbs. Reaction / # of Studs

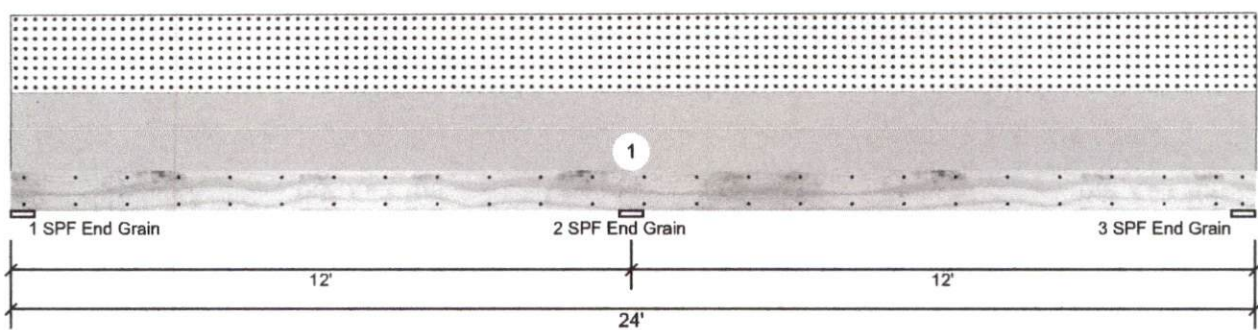


Client: BIM
 Project: HARNETT
 Address: HARNETT

Date: 6/18/2019
 Designer: Lenny Norris
 Job Name: STEWART
 Project #: J0619-2765

BM2 Kerto-S LVL 1.750" X 9.250" 2-Ply - PASSED

Level: Level



Member Information

Type: Girder	Application: Floor
Plies: 2	Design Method: ASD
Moisture Condition: Dry	Building Code: IBC/IRC 2015
Deflection LL: 480	Load Sharing: No
Deflection TL: 360	Deck: Not Checked
Importance: Normal	
Temperature: Temp <= 100°F	

Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind	Const
1	0	665	631	0	0
2	0	2011	1907	0	0
3	0	665	631	0	0

Bearings

Bearing	Length	Cap. React	D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	5.500"	8%	661 / 675	1335	L_	D+S
2 - SPF End Grain	5.500"	23%	2019 / 1915	3934	LL	D+S
3 - SPF End Grain	5.500"	8%	661 / 675	1335	_L	D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Neg Moment	-4565 ft-lb	12'	14423 ft-lb	0.316 (32%)	D+S	LL
Unbraced	-4565 ft-lb	12'	13568 ft-lb	0.336 (34%)	D+S	LL
Pos Moment	2780 ft-lb	4'11 1/16"	14423 ft-lb	0.193 (19%)	D+S	L_
Unbraced	2780 ft-lb	4'11 1/16"	8016 ft-lb	0.347 (35%)	D+S	L_
Shear	1758 lb	11'2 3/4"	7943 lb	0.221 (22%)	D+S	LL
LL Defl inch	0.074 (L/1884)	5'8 5/16"	0.290 (L/480)	0.250 (25%)	S	L_
TL Defl inch	0.135 (L/1035)	5'6 11/16"	0.387 (L/360)	0.350 (35%)	D+S	L_

Design Notes

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

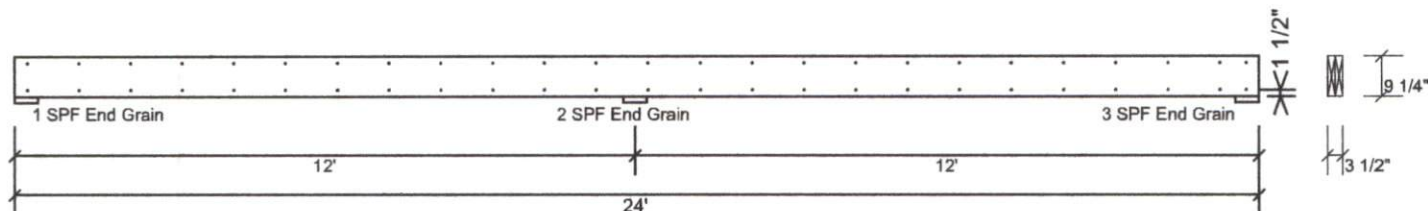
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	132 PLF	0 PLF	132 PLF	0 PLF	0 PLF	MONO
	Self Weight				7 PLF					

<p>Notes</p> <p>Calculated Structural Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.</p> <p>Lumber</p> <ol style="list-style-type: none"> 1. Dry service conditions, unless noted otherwise 2. LVL not to be treated with fire retardant or corrosive chemicals 	<p>Handling & Installation</p> <ol style="list-style-type: none"> 1. LVL beams must not be cut or drilled 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals 3. Damaged Beams must not be used 4. Design assumes top edge is laterally restrained 5. Provide lateral support at bearing points to avoid lateral displacement and rotation 	<p>6. For flat roofs provide proper drainage to prevent ponding</p>	<p>Manufacturer Info</p> <p>Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633</p>	<p>Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS</p>
				<p>This design is valid until 12/11/2021</p>



BM2 Kerto-S LVL 1.750" X 9.250" 2-Ply - PASSED

Level: Level


Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

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Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive

chemicals
Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
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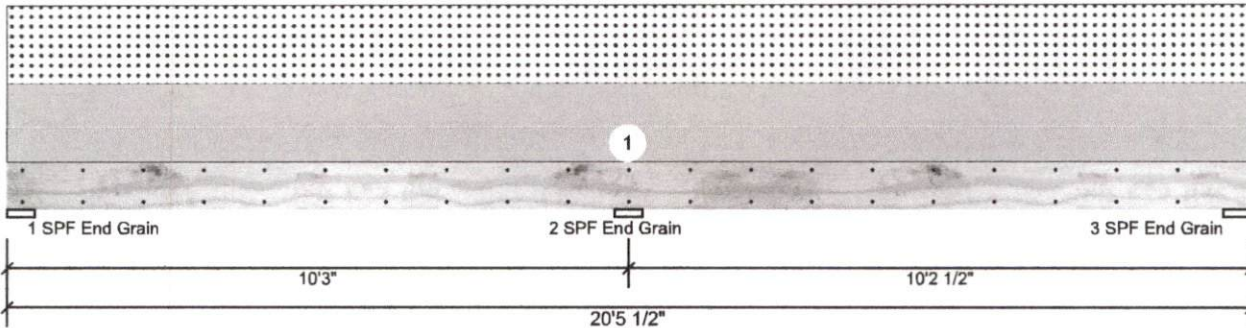
Manufacturer Info

 Metsä Wood
 301 Merritt 7 Building, 2nd Floor
 Norwalk, CT 06851
 (800) 622-5850
 www.metsawood.com/us
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BM3 Kerto-S LVL 1.750" X 9.250" 2-Ply - PASSED

Level: Level



Member Information

Reactions UNPATTERNED lb (Uplift)

Type: Girder	Application: Floor
Plies: 2	Design Method: ASD
Moisture Condition: Dry	Building Code: IBC/IRC 2015
Deflection LL: 480	Load Sharing: No
Deflection TL: 360	Deck: Not Checked
Importance: Normal	
Temperature: Temp <= 100°F	

Brg	Live	Dead	Snow	Wind	Const
1	0	1372	1343	0	0
2	0	4060	3972	0	0
3	0	1364	1334	0	0

Bearings

Bearing	Length	Cap.	React D/L lb	Total Ld.	Case	Ld. Comb.
1 - SPF End Grain	5.500"	17%	1361 / 1430	2791	L_	D+S
2 - SPF End Grain	5.500"	48%	4083 / 3995	8078	LL	D+S
3 - SPF End Grain	5.500"	17%	1352 / 1424	2776	_L	D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Neg Moment	-7943 ft-lb	10'3"	14423 ft-lb	0.551 (55%)	D+S	LL
Unbraced	-7943 ft-lb	10'3"	13784 ft-lb	0.576 (58%)	D+S	LL
Pos Moment	4874 ft-lb	4'2 15/16"	14423 ft-lb	0.338 (34%)	D+S	L_
Unbraced	4874 ft-lb	4'2 15/16"	9027 ft-lb	0.540 (54%)	D+S	L_
Shear	3538 lb	9'5 3/4"	7943 lb	0.445 (45%)	D+S	LL
LL Defl inch	0.099 (L/1193)	4'10 15/16"	0.246 (L/480)	0.400 (40%)	S	L_
TL Defl inch	0.179 (L/659)	4'9 11/16"	0.328 (L/360)	0.550 (55%)	D+S	L_

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- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	325 PLF	0 PLF	325 PLF	0 PLF	0 PLF	B2 TRUSS
	Self Weight				7 PLF					

Notes
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Lumber
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Handling & Installation
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 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
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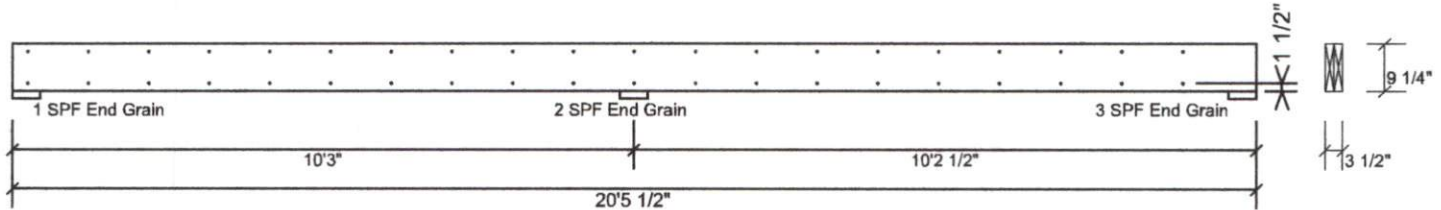
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BM3 Kerto-S LVL 1.750" X 9.250" 2-Ply - PASSED

Level: Level


Multi-Ply Analysis

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Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

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Trenco

818 Soundside Rd
Edenton, NC 27932

Re: J0619-2765

Bim Builders/Stewarts Garage/Harnett

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: E13181419 thru E13181424

My license renewal date for the state of North Carolina is December 31, 2019.

North Carolina COA: C-0844



June 19, 2019

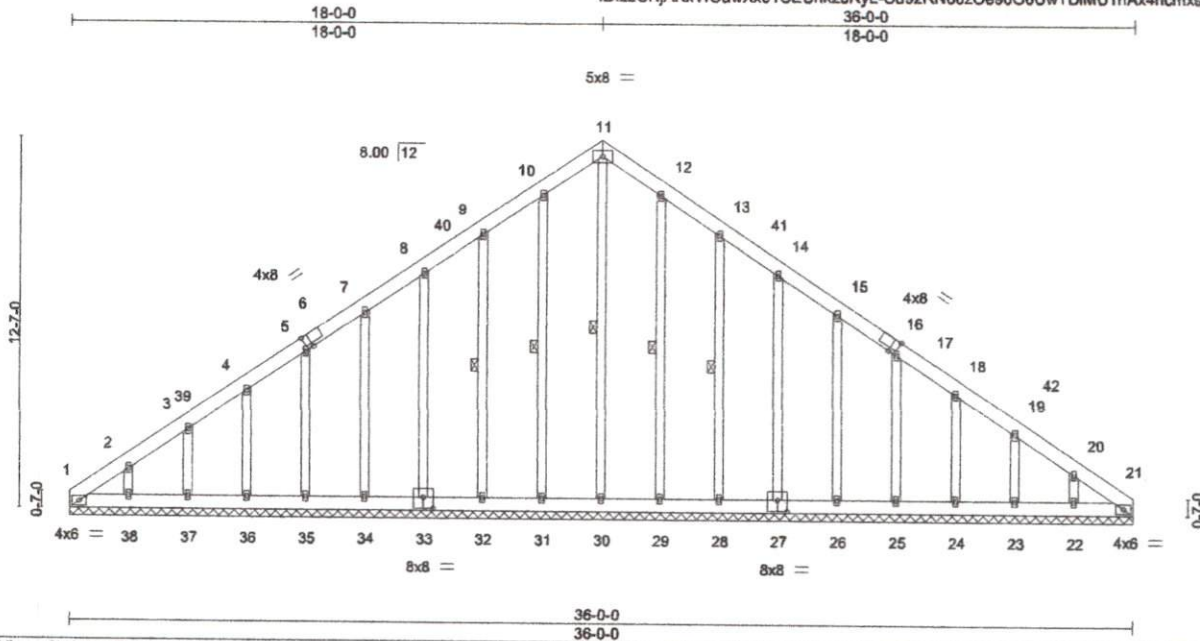
Strzyzewski, Marvin

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Job J0619-2765	Truss A1	Truss Type GABLE	Qty 2	Ply 1	Bim Builders/Stewarts Garage/Harnett	E13181419
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Comtech, Inc., Fayetteville, NC 28309

8.130 s Mar 11 2018 MITek Industries, Inc. Tue Jun 18 15:02:21 2019 Page 1
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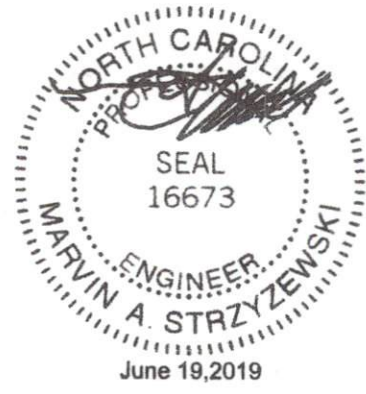
Plate Offsets (X,Y)- [6:0-2-9,Edge], [16:0-2-9,Edge], [27:0-4-0,0-4-8], [33:0-4-0,0-4-8]					
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.05	Vert(LL) n/a - n/a 999	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.03	Vert(CT) n/a - n/a 999		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.18	Horz(CT) 0.01 21 n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S			
				Weight: 339 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x6 SP No.1	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD 2x6 SP No.1	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
OTHERS 2x4 SP No.3 *Except*	WEBS 1 Row at midpt 11-30, 10-31, 9-32, 12-28, 13-28
11-30,10-31,12-29: 2x4 SP No.2	

REACTIONS. All bearings 36-0-0.
 (lb) - Max Horz 1=-362(LC 8)
 Max Uplift All uplift 100 lb or less at joint(s) 21, 31, 32, 33, 34, 35, 36, 37, 29, 27, 26, 25, 24, 23 except
 1=-112(LC 10), 38=-112(LC 12), 28=-103(LC 13), 22=-109(LC 13)
 Max Grav All reactions 250 lb or less at joint(s) 1, 21, 31, 32, 33, 34, 35, 36, 37, 38, 29, 28, 27, 26, 25,
 24, 23, 22 except 30=278(LC 22)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-2=-399/285, 2-3=-311/248, 3-4=-254/222, 9-10=-246/289, 10-11=-277/304,
 11-12=-277/290, 20-21=-316/207
 BOT CHORD 1-38=-181/283, 37-38=-181/283, 36-37=-181/283, 35-36=-181/283, 34-35=-181/283,
 33-34=-181/283, 32-33=-181/283, 31-32=-181/283, 30-31=-181/283, 29-30=-181/283,
 28-29=-181/283, 27-28=-181/283, 26-27=-181/283, 25-26=-181/283, 24-25=-181/283,
 23-24=-181/283, 22-23=-181/283, 21-22=-181/283

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-0-0 to 4-4-13, Interior(1) 4-4-13 to 18-0-0, Exterior(2) 18-0-0 to 22-4-13 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 - All plates are 2x4 MT20 unless otherwise indicated.
 - Gable requires continuous bottom chord bearing.
 - Gable studs spaced at 2-0-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 21, 31, 32, 33, 34, 35, 36, 37, 29, 27, 26, 25, 24, 23 except (j=lb) 1=112, 38=112, 28=103, 22=108.



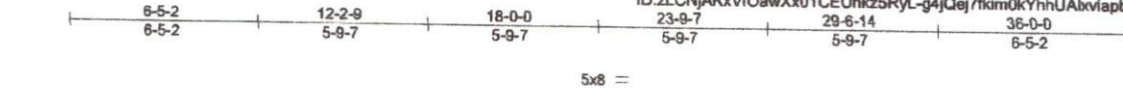
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
 Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPH Quality Criteria, DSB-88 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

ENGINEERING BY TRENCO
 818 Soundside Road
 Edenton, NC 27932

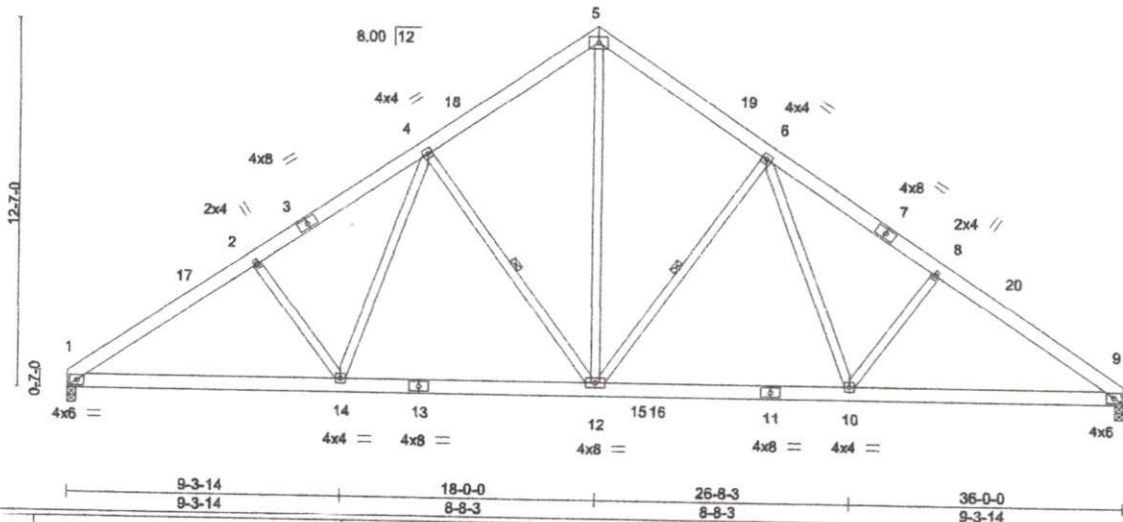
Job J0619-2765	Truss A2	Truss Type MOD. QUEEN	Qty 8	Ply 1	Bim Builders/Stewarts Garage/Hamett	E13181420
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Comtech, Inc., Fayetteville, NC 28309

8.130 s Mar 11 2018 MITEK Industries, Inc. Tue Jun 18 15:02:22 2019 Page 1
 ID:zLCNjAKxVfOawXx01CEUhcZ5RyL-g4jQej7fikm0kYhhUAxvIapbLKpl.3a0Rjxs6Tz50Ei



Scale = 1:76.6



LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.20	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.42	Vert(LL) -0.11 12-14 >999 360		
BCLL 0.0	Lumber DOL 1.15	WB 0.79	Vert(CT) -0.18 12-14 >999 240		
BCDL 10.0	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.06 9 n/a n/a		
	Code IRC2015/TPI2014		Wind(LL) 0.05 12 >999 240		
				Weight: 271 lb	FT = 20%

LUMBER-

TOP CHORD 2x6 SP No.1
 BOT CHORD 2x6 SP No.1
 WEBS 2x4 SP No.3 *Except*
 5-12: 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 4-10-10 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
 WEBS 1 Row at midpt 4-12, 6-12

REACTIONS.

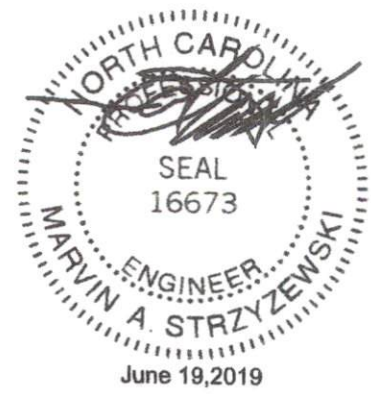
(lb/size) 1=1428/0-3-8, 9=1428/0-3-8
 Max Horz 1=290(LC 9)
 Max Uplift 1=-75(LC 12), 9=-75(LC 13)
 Max Grav 1=1579(LC 19), 9=1579(LC 20)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-2=-2427/446, 2-4=-2264/482, 4-5=-1629/449, 5-6=-1629/449, 6-8=-2264/482, 8-9=-2428/446
 BOT CHORD 1-14=-264/2143, 12-14=-109/1759, 10-12=-110/1637, 9-10=-263/1926
 WEBS 2-14=-368/233, 4-14=-81/609, 4-12=-690/258, 5-12=-315/1402, 6-12=-690/258, 6-10=-81/609, 8-10=-368/233

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCCL=6.0psf; BCCL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 18-0-0, Exterior(2) 18-0-0 to 22-4-13 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members, with BCCL = 10.0psf.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANS/TP1 Quality Criteria, DSB-86 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

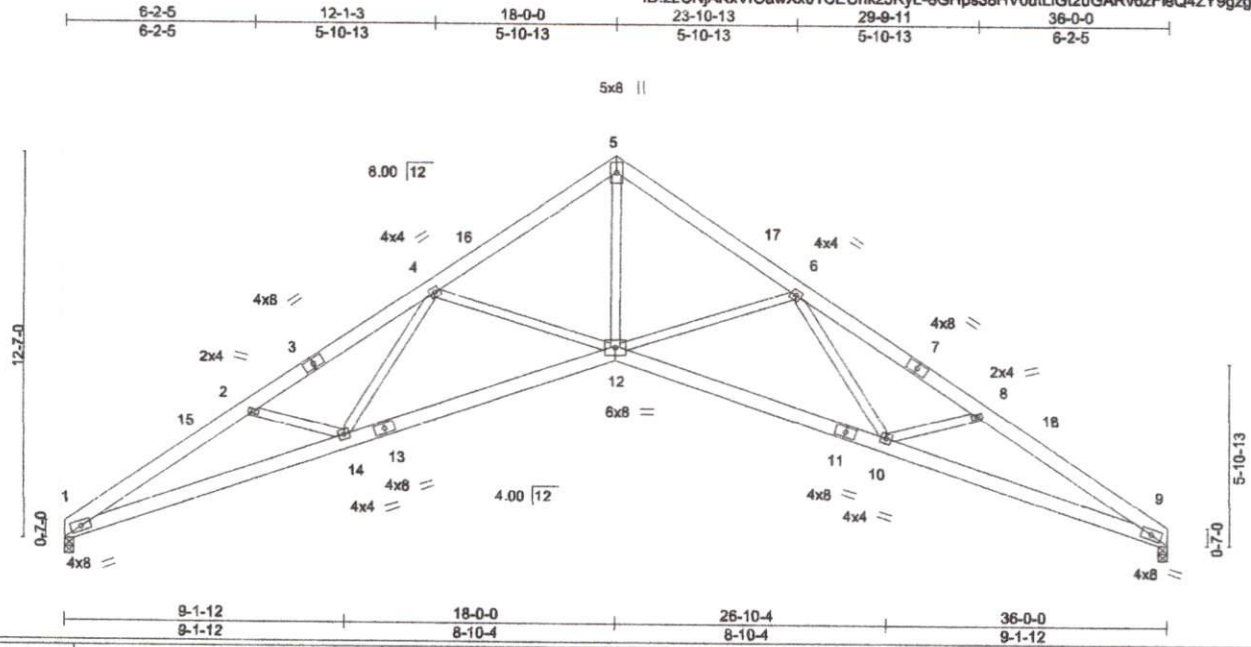


818 Soundside Road
 Edenton, NC 27932

Job J0619-2765	Truss A3	Truss Type SCISSORS	Qty 15	Ply 1	Bim Builders/Stewarts Garage/Harnett	E13181421
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Comtech, Inc., Fayetteville, NC 28309

8.130 s Mar 11 2018 MITek Industries, Inc. Tue Jun 18 15:02:23 2019 Page 1
 ID:zLcNjAKxVfOawXr01CEUhkz5RyL-8GHps38HV0utLIGt2uGARv6zFleQ4ZY9gzgPgvz50Ek



LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.27	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.53	Vert(LL) -0.21 12 >999 360		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.62	Vert(CT) -0.45 12-14 >956 240		
BCDL 10.0	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.40 9 n/a n/a		
	Code IRC2015/TPI2014		Wind(LL) 0.15 12-14 >999 240	Weight: 244 lb	FT = 20%

LUMBER-
 TOP CHORD 2x6 SP No.1
 BOT CHORD 2x6 SP No.1
 WEBS 2x4 SP No.3 *Except*
 5-12: 2x4 SP No.2

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 3-8-3 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 1=1428/0-3-8, 9=1428/0-3-8
 Max Horz 1=-290(LC 10)
 Max Uplift 1=-75(LC 12), 9=-75(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-2=-3999/733, 2-4=-3665/607, 4-5=-2624/408, 5-6=-2624/406, 6-8=-3665/606,
 8-9=-3999/732
 BOT CHORD 1-14=-544/3526, 12-14=-249/2942, 10-12=-251/2898, 9-10=-542/3418
 WEBS 5-12=-271/2462, 6-12=-805/316, 6-10=-71/596, 8-10=-357/274, 4-12=-805/316,
 4-14=-71/596, 2-14=-357/274

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 18-0-0, Exterior(2) 18-0-0 to 22-4-13 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
 - 5) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSITPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9.



Job J0619-2765	Truss A4	Truss Type MOD. QUEEN	Qty 21	Ply 1	Bim Builders/Stewarts Garage/Hamett	E13181422
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Comtech, Inc., Fayetteville, NC 28309

8.130 s Mar 11 2018 MITek Industries, Inc. Tue Jun 18 15:02:25 2019 Page 1
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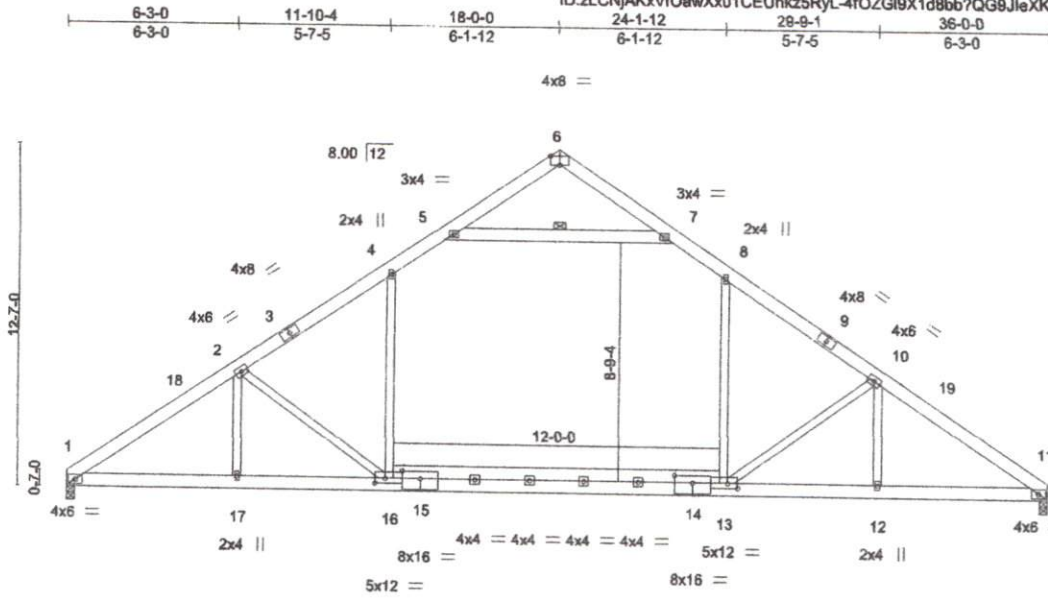


Plate Offsets (X,Y)--	[6:0-4-0,Edge], [13:0-4-8,0-2-4], [14:0-0-0,0-2-12], [14:0-8-0,0-3-4], [15:0-0-0,0-2-12], [15:0-8-0,0-3-4], [16:0-4-8,0-2-4]
-----------------------	--

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.71	in (loc) /defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.85	Vert(LL) -0.30 13-16 >999 360		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.60	Vert(CT) -0.46 13-16 >942 240		
BCDL 10.0	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.07 11 n/a n/a		
	Code IRC2015/TP12014		Wind(LL) 0.25 16-17 >999 240	Weight: 288 lb	FT = 20%

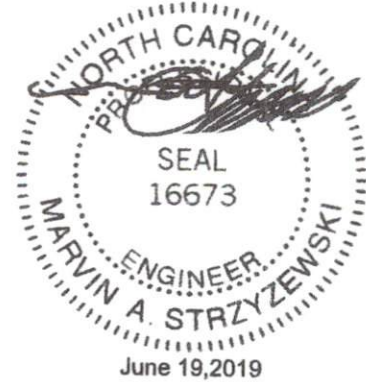
LUMBER-
 TOP CHORD 2x6 SP No.1
 BOT CHORD 2x6 SP No.1
 WEBS 2x4 SP No.3 *Except*
 5-7: 2x6 SP No.1

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 4-2-15 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
 WEBS 1 Row at midpt 5-7

REACTIONS. (lb/size) 1=1428/0-3-8, 11=1428/0-3-8
 Max Horz 1=-290(LC 8)
 Max Uplift 1=-75(LC 12), 11=-75(LC 13)
 Max Grav 1=1605(LC 19), 11=1605(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-2=-2530/435, 2-4=-2200/430, 4-5=-1581/432, 7-8=-1582/432, 8-10=-2203/430,
 10-11=-2533/435
 BOT CHORD 1-17=-260/2246, 16-17=-260/2246, 13-16=-92/1742, 12-13=-258/2031, 11-12=-258/2031
 WEBS 8-13=-26/838, 10-13=-661/238, 4-16=-26/836, 2-16=-661/234, 5-7=-1848/452

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 18-0-0, Exterior(2) 18-0-0 to 22-3-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members, with BCDL = 10.0psf.
 - 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 11.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
 Design valid for use only with MITek9 connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

ENGINEERING BY
TRENCO
 A MITEK COMPANY
 818 Soundside Road
 Edenton, NC 27832

Job	Truss	Truss Type	Qty	Ply	Bim Builders/Stewarts Garage/Harnett	E13181423
J0619-2765	B1	GABLE	1	1		

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8.130 s Mar 11 2018 MITek Industries, Inc. Tue Jun 18 15:02:27 2019 Page 1
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Job Reference (optional)

0-10-8 7-9-14 15-0-0 22-2-3 30-0-0 30-10-8
 0-10-8 7-9-14 7-2-3 7-2-2 7-9-14 0-10-8

Scale = 1:65.8

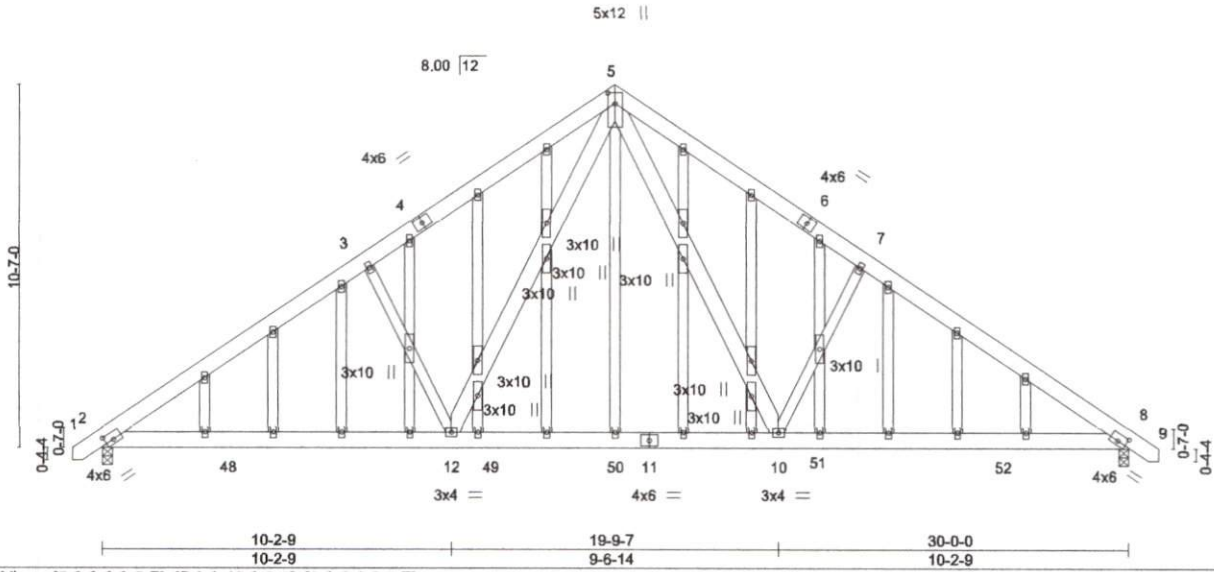


Plate Offsets (X,Y)- [2:0-3-0,0-2-7], [5:0-3-12,0-2-8], [8:0-3-0,0-2-7]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC 0.27	Vert(LL)	-0.13 10-12	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.38	Vert(CT)	-0.18 10-12	>999	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.88	Horz(CT)	0.03 8	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014		Matrix-S	Wind(LL)	0.15 8-10	>999	240		
								Weight: 326 lb	FT = 20%

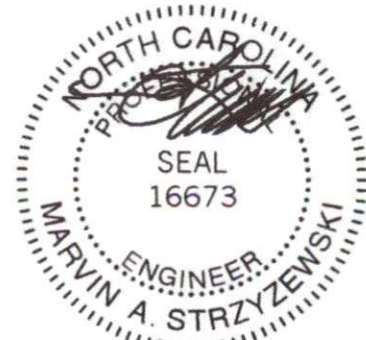
LUMBER-
 TOP CHORD 2x6 SP No.1
 BOT CHORD 2x6 SP No.1
 WEBS 2x6 SP No.1 *Except*
 3-12,7-10: 2x4 SP No.3
 OTHERS 2x4 SP No.3

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 5-7-9 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 7-6-8 oc bracing.

REACTIONS. (lb/size) 2=1241/0-3-8, 8=1241/0-3-8
 Max Horz 2=-314(LC 10)
 Max Uplift 2=-254(LC 12), 8=-254(LC 13)
 Max Grav 2=1241(LC 1), 8=1288(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-1757/1285, 3-5=-1591/1387, 5-7=-1663/1387, 7-8=-1808/1285
 BOT CHORD 2-12=-924/1371, 10-12=-453/916, 8-10=-919/1383
 WEBS 3-12=-504/392, 5-12=-735/741, 5-10=-735/871, 7-10=-504/392

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-8-12 to 3-8-1, Exterior(2) 3-8-1 to 15-0-0, Corner(3) 15-0-0 to 19-4-13 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 - 4) All plates are 2x4 MT20 unless otherwise indicated.
 - 5) Gable studs spaced at 2-0-0 oc.
 - 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members, with BCDL = 10.0psf.
 - 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=254, 8=254.



June 19, 2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
 Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



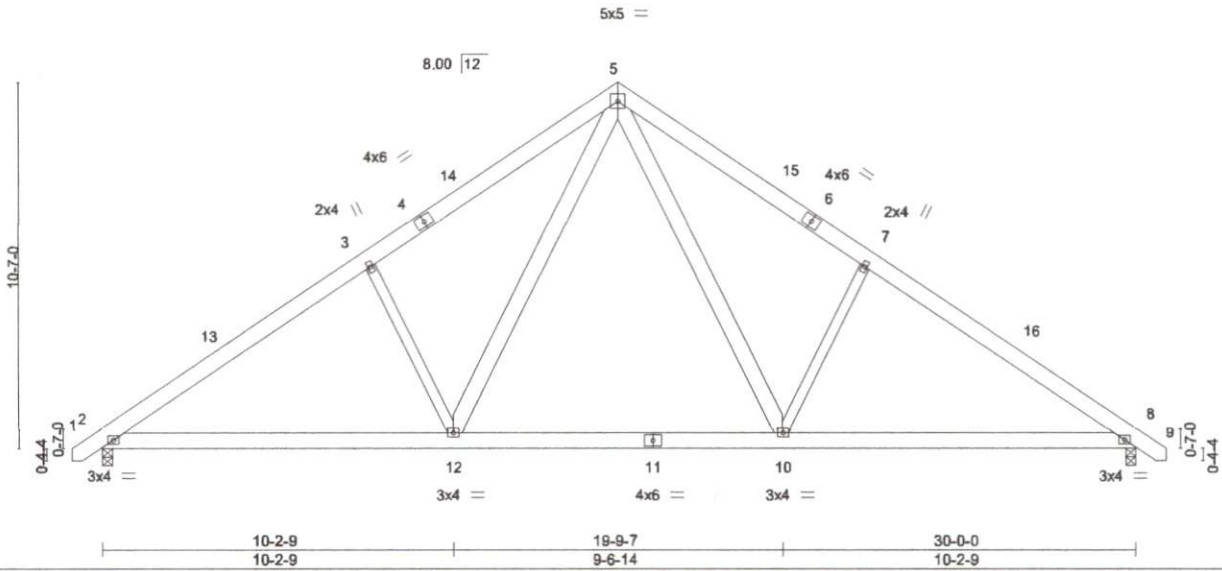
Job	Truss	Truss Type	Qty	Ply	Bim Builders/Stewarts Garage/Hamett	E13181424
J0619-2765	B2	GABLE	14	1		

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8.130 s Mar 11 2018 MiTek Industries, Inc. Tue Jun 18 15:02:28 2019 Page 1
 ID:zLCNjAKxVfOawXx01CEUhz5RyL-UE4ivnCPJYw9ST8qrRrL8zprHmOvNzupEOAL7z50Ef



Scale = 1:65.3



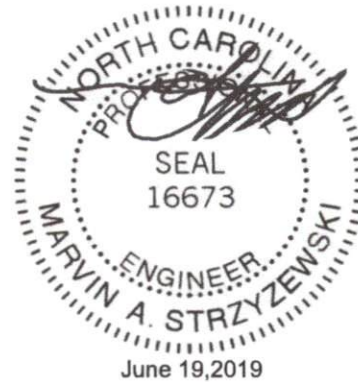
LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.25	Vert(LL) -0.14	10-12	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.38	Vert(CT) -0.18	10-12	>999	240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.31	Horz(CT) 0.03	8	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.03	2-12	>999	240	Weight: 226 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x6 SP No.1	TOP CHORD Structural wood sheathing directly applied or 5-7-3 oc purlins.
BOT CHORD 2x6 SP No.1	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x6 SP No.1 *Except*	
3-12,7-10: 2x4 SP No.3	

REACTIONS. (lb/size) 2=1241/0-3-8, 8=1241/0-3-8
 Max Horz 2=-251(LC 10)
 Max Uplift 2=-75(LC 12), 8=-75(LC 13)
 Max Grav 2=1298(LC 19), 8=1298(LC 20)

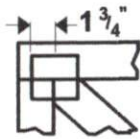
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-1836/362, 3-5=-1688/453, 5-7=-1688/453, 7-8=-1836/362
 BOT CHORD 2-12=-157/1598, 10-12=0/1036, 8-10=-165/1428
 WEBS 3-12=-505/297, 5-12=-166/863, 5-10=-166/864, 7-10=-505/297

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCCL=6.0psf; BCCL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-12 to 3-8-1, Interior(1) 3-8-1 to 15-0-0, Exterior(2) 15-0-0 to 19-4-13 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members, with BCCL = 10.0psf.
 - 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.

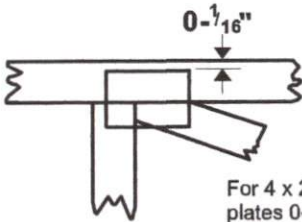


Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

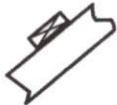
* Plate location details available in MITek 20/20 software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING

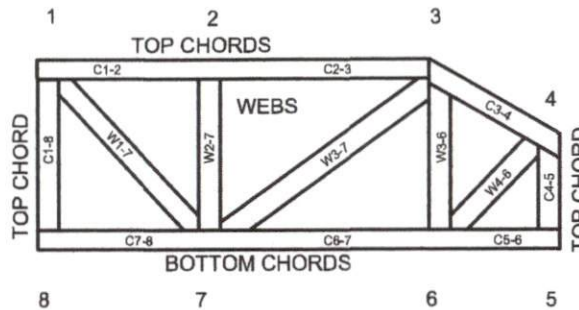
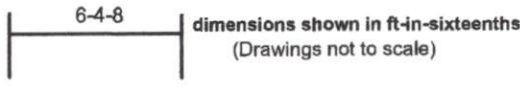


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

- ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
 DSB-89: Design Standard for Bracing.
 BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR-1988
 ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015

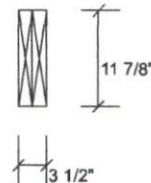
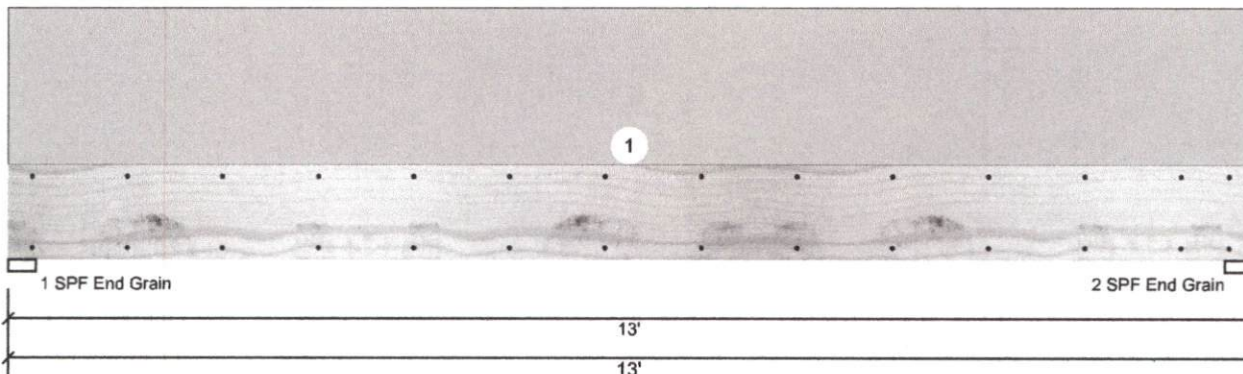
General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

GDH1 Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal
Temperature:	Temp <= 100°F

Application:	Floor
Design Method:	ASD
Building Code:	IBC/IRC 2015
Load Sharing:	No
Deck:	Not Checked

Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind	Const
1	0	3960	0	0	0
2	0	3960	0	0	0

Bearings

Bearing	Length	Cap. React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	37%	3960 / 0	Uniform	D
2 - SPF End Grain	3.500"	37%	3960 / 0	Uniform	D

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	11979 ft-lb	6'6"	17919 ft-lb	0.668 (67%)	D	Uniform
Unbraced	11979 ft-lb	6'6"	12018 ft-lb	0.997 (100%)	D	Uniform
Shear	3218 lb	1'2 5/8"	7980 lb	0.403 (40%)	D	Uniform
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
TL Defl inch	0.380 (L/396)	6'6"	0.418 (L/360)	0.910 (91%)	D	Uniform

Design Notes

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at a maximum of 6'10 1/8" o.c.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform Self Weight			Top	600 PLF	0 PLF	0 PLF	0 PLF	0 PLF	GABLE WEIGHT

Notes
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Lumber
 1. Dry service conditions, unless noted otherwise
 2. LVL not to be treated with fire retardant or corrosive chemicals

Handling & Installation
 1. LVL beams must not be cut or drilled
 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 3. Damaged Beams must not be used
 4. Design assumes top edge is laterally restrained
 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 12/11/2021

Manufacturer Info
 Metsä Wood
 301 Merritt 7 Building, 2nd Floor
 Norwalk, CT 06851
 (800) 622-5850
 www.metsawood.com/us
 ICC-ES: ESR-3633

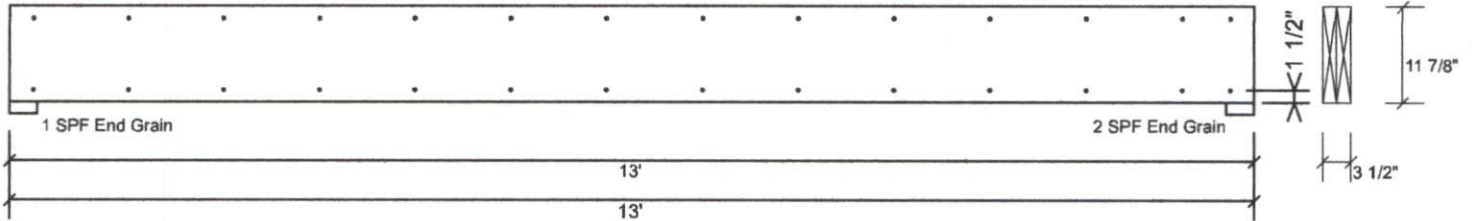
Comtech, Inc.
 1001 S. Reilly Road, Suite #639
 Fayetteville, NC
 USA
 28314
 910-864-TRUS





GDH1 Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

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chemicals

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Manufacturer Info

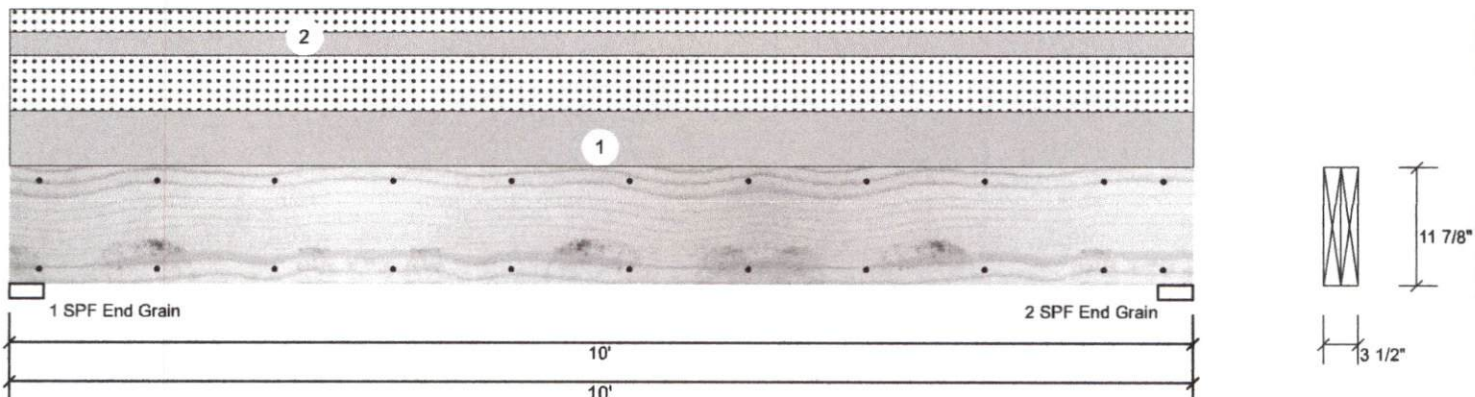
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GDH2 Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED

Level: Level



Member Information

Reactions UNPATTERNED lb (Uplift)

Type:	Girder	Application:	Floor
Plies:	2	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC/IRC 2015
Deflection LL:	480	Load Sharing:	No
Deflection TL:	360	Deck:	Not Checked
Importance:	Normal		
Temperature:	Temp <= 100°F		

Brg	Live	Dead	Snow	Wind	Const
1	0	2841	2795	0	0
2	0	2841	2795	0	0

Bearings

Bearing	Length	Cap. React	D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	53%	2841 / 2795	5636	L	D+S
2 - SPF End Grain	3.500"	53%	2841 / 2795	5636	L	D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	12828 ft-lb	5'	22897 ft-lb	0.560 (56%)	D+S	L
Unbraced	12828 ft-lb	5'	12856 ft-lb	0.998 (100%)	D+S	L
Shear	4262 lb	1'2 5/8"	10197 lb	0.418 (42%)	D+S	L
LL Defl inch	0.124 (L/921)	5'	0.239 (L/480)	0.520 (52%)	S	L
TL Defl inch	0.251 (L/457)	5'	0.318 (L/360)	0.790 (79%)	D+S	L

Design Notes

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at a maximum of 6'7 1/2" o.c.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	395 PLF	0 PLF	395 PLF	0 PLF	0 PLF	A2 TRUSS
2	Uniform			Top	164 PLF	0 PLF	164 PLF	0 PLF	0 PLF	MONO
	Self Weight				9 PLF					

Notes
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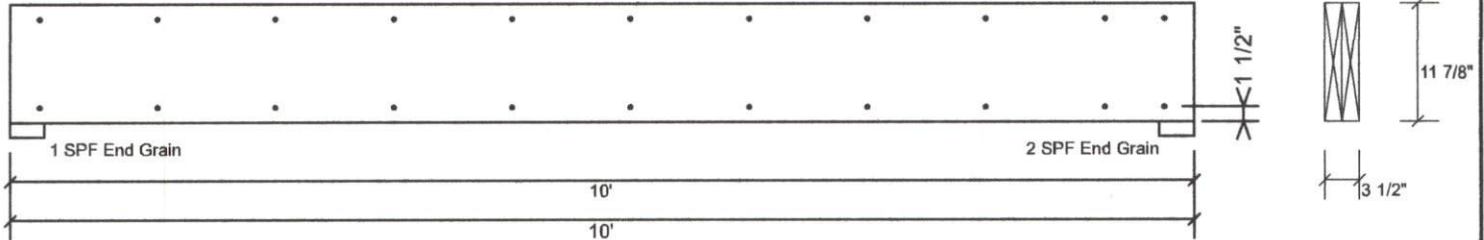
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GDH2 Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED

Level: Level


Multi-Ply Analysis

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Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

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