

Harnett County Central Permitting

PO Box 65 Lillington, NC 27546 - Ph: 910-893-7525 - Fx: 910-893-2793 - www.harnett.org/permits

**Certification of Work Performed By Owner/Contractor
(Individual Trade Application)**

Owner (s) of Structure: Donnie Pritt Phone: 919-291-7251

Owner (s) Mailing Address: 3365 OAK Ridge River Rd.
Fuquay Varin NC 27524

Land Owner Name (s): SAME Phone: _____

Construction or Site Address: _____

PIN # _____ Parcel # _____

Job Cost: 25⁰⁰⁰ Description of Work to be done 24x30 pool house

Mechanical: New Unit With Ductwork ___ New Unit Without Ductwork ___ Gas Piping ___ Other ___

Electrical*: 200 Amp ___ <200 Amp ___ Service Change ___ Service Reconnect ___ Other ___

* For Progress Energy customers we need the premise number

Plumbing: Water/Sewer Tap ___ Number of Baths 1 Water Heater ___

Specific Directions to Job from Lillington:

401 North Left on Christian Light 9 mile take Left
on OAK Ridge job on Right 1/2 mile

Subdivision: _____ Lot #: 4

I Howell Builders will provide the construction labor on this structure.
(Contractors Name) (Trade)

I am the building owner or my NC state license number is 41779, which entitles me to perform such work on the above structure legally. All work shall comply with the State Building Code and all other applicable State and local laws, ordinances and regulations.

Howell Builders Inc.
Contractor's Company Name

101 Mels Meadows Dr. Fuquay Varin
Address NC 27524

41779
License #

919-427-0679
Telephone

brayan@howell-builders.com
Email Address

Structure Owner / Contractor Signature: [Signature] Date: 2-13-19

By signing this application you affirm that you have obtained permission from the above listed license holder to purchase permits on their behalf. If doing the work as owner you understand that you cannot rent, lease or sell the listed property for 12 months after completion of the listed work.

***Company name, address, & phone must match information on license**

BUILDER SHALL VERIFY ALL DIMENSIONS AND CONDITIONS AT TIME OF CONSTRUCTION.

ALL CONSTRUCTION SHALL CONFORM TO THE LATEST EDITION OF THE NC STATE BUILDING CODE.

CODES GOVERN OVER DRAWINGS.

DIMENSIONS GOVERN OVER SCALE.

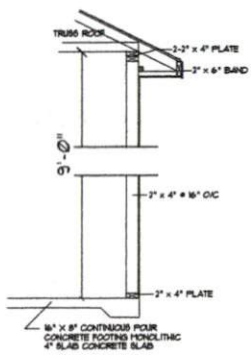
VERIFY ALL MECHANICAL REQUIREMENTS BEFORE FRAMING.

VINCANNON DESIGNS DOES NOT ASSUME LIABILITY FOR ANY DEVIATION OF OR CONSTRUCTION METHODS OF THESE PLANS.

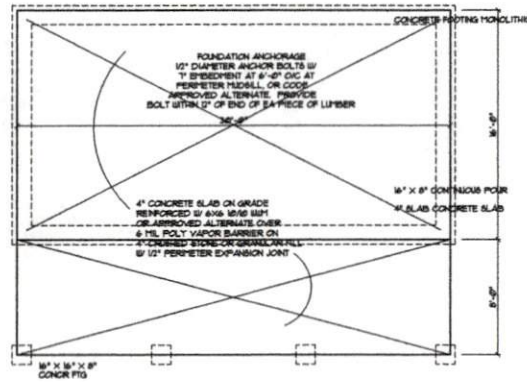


Approved

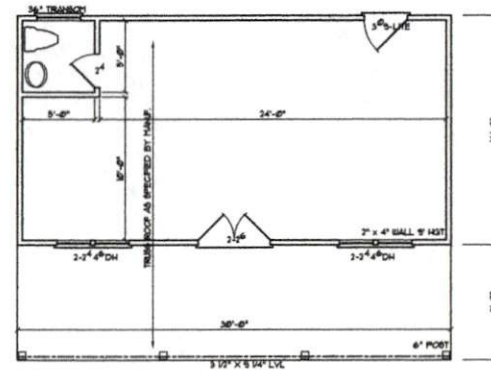
cutton 02/13/2019



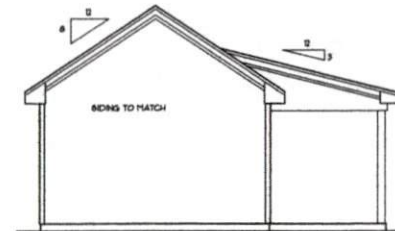
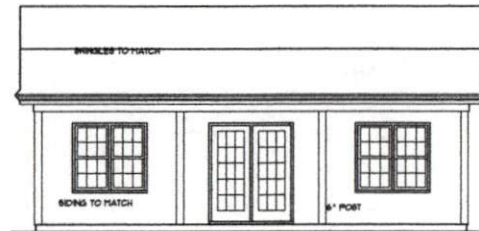
CROSS SECTION
SCALE: 1/4" = 1'-0"



FOUNDATION PLAN
SCALE: 1/4" = 1'-0"



FLOOR PLAN
SCALE: 1/4" = 1'-0"



ELEVATIONS
SCALE: 1/4" = 1'-0"

ATTIC SPACE VENTILATION

MAIN ROOF
ATTIC AREA = 120 SQ. FT.
REQUIRED AREA = A/AMB = 2.4 SQ. FT.

NOTE:
ALL EAVES TO HAVE 1" CONTINUOUS SOFFIT VENT.
ALLOW 1" AIR SPACE ABOVE INSULATION FOR AIR FLOW.
CONTINUOUS RIDGE ROOF VENTILATION.

Pritt Residence
3365 Oak Ridge River Rd
Fuquay Varina, NC

VINCANNON DESIGNS
CUSTOM HOME PLANS
RDU-11-VIR-14-C-154

DRAWN BY: JRV
CHECKED BY: JRV
DATE: 02/13/19

SHEET
A-1

Trenco

818 Soundside Rd
Edenton, NC 27932

Re: 1680354_Pritt_Pool_House
Howell Builders

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Builders FirstSource (Albermarle,NC).

Pages or sheets covered by this seal: E12692886 thru E12692887

My license renewal date for the state of North Carolina is December 31, 2019.

North Carolina COA: C-0844



February 12, 2019

Gilbert, Eric

IMPORTANT NOTE: Truss Engineer's responsibility is solely for design of individual trusses based upon design parameters shown on referenced truss drawings. Parameters have not been verified as appropriate for any use. Any location identification specified is for file reference only and has not been used in preparing design. Suitability of truss designs for any particular building is the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.

Job	Truss	Truss Type	Qty	Ply	Howell Builders	E12692886
1680354_Pritt_Pool_House	A1	GABLE	2	1		
					Job Reference (optional)	

Builders FirstSource, Albemarle, NC 28001

8.220 s Nov 16 2018 MiTek Industries, Inc. Tue Feb 12 08:21:09 2019 Page 1
ID:8K7E1EZuf_ usmh9PEFpzjzlyOO-g4xOFnbMy_kroeeMOSIQL6ch5g6NaSwvRF4FcXNzlxQe

0-10-8 8-0-0 12-2-6 24-0-0 24-10-8
0-10-8 8-0-0 4-2-6 11-9-10 0-10-8

Scale = 1:45.2

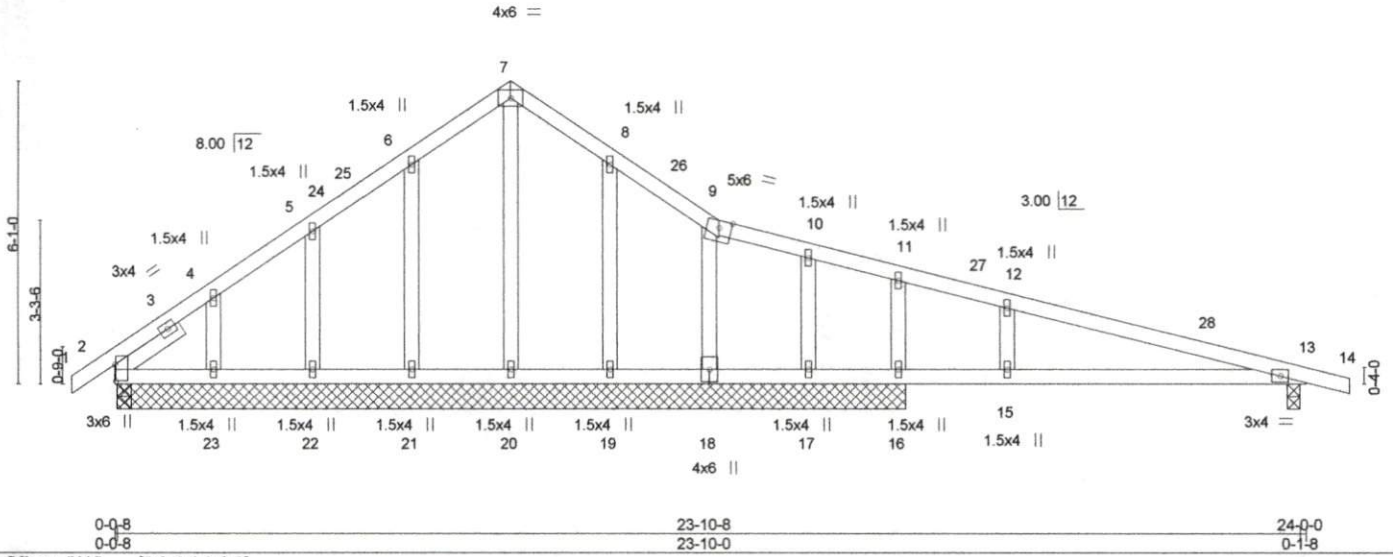


Plate Offsets (X, Y) [2:0-4-1,0-0-3]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.62	in (loc) l/defl L/d	MT20	244/190
Snow (Pf/Pg) 15.4/20.0	Plate Grip DOL 1.15	BC 0.67	Vert(LL) -0.10 13-15 >929 360		
TCDL 10.0	Lumber DOL 1.15	WB 0.08	Vert(CT) -0.23 13-15 >418 240		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.00 13 n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014		Wind(LL) 0.14 13-15 >706 240	Weight: 121 lb	FT = 20%

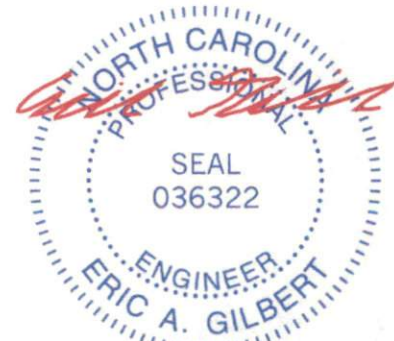
LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
OTHERS 2x4 SP No.3
SLIDER Left 2x4 SP No.3 1-7-3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 15-11-0 except (jt=length) 13=0-3-0.
(lb) - Max Horz 2=92(LC 14)
Max Uplift All uplift 100 lb or less at joint(s) 2, 21, 22, 23, 19, 18, 13 except 17=278(LC 43), 16=116(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 2, 2, 20, 21, 22, 23, 19, 17 except 18=250(LC 2), 16=807(LC 43), 13=329(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 11-16=399/174

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-10-8 to 2-0-0, Interior(1) 2-0-0 to 8-0-0, Exterior(2) 8-0-0 to 11-0-0, Interior(1) 11-0-0 to 24-10-8 zone; cantilever left and right exposed; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 - TCLL: ASCE 7-10; Pr=20.0 psf (roof live load; Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow; Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10; Min. flat roof snow load governs.
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - Gable studs spaced at 2-0-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 21, 22, 23, 19, 18, 13 except (jt=lb) 17=278, 16=116.



February 12, 2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Howell Builders	E12692887
1680354_Pritt_Pool_House	A2	ROOF SPECIAL	14	1		

Builders FirstSource, Albemarle, NC 28001 8.220 s Nov 16 2018 MiTek Industries, Inc. Tue Feb 12 08:21:11 2019 Page 1
 ID:8K7E1EZuf_usmh9PEFpzjzlyOO-cS39gSdcUb_Z1xWmZjSpC1nQRw5_woukInKj0GzlxQc

0-10-8	8-0-0	12-2-6	15-9-12	24-0-0	24-10-8
0-10-8	8-0-0	4-2-6	3-7-6	8-2-4	0-10-8

Scale = 1:42.9

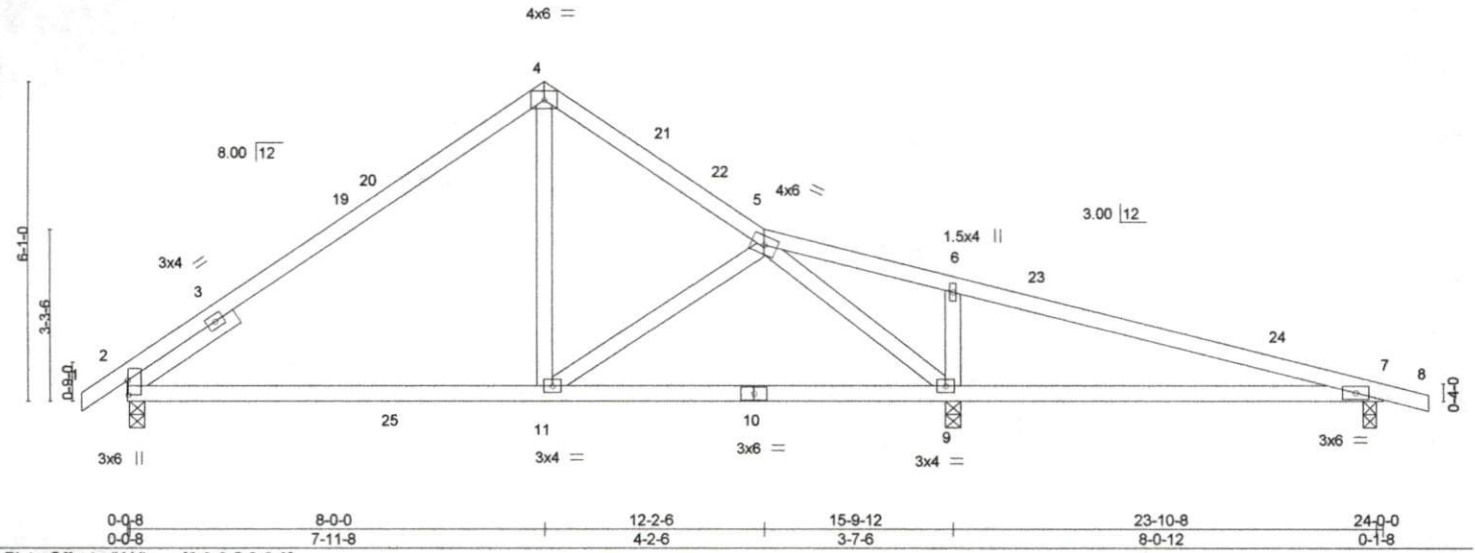


Plate Offsets (X, Y) -- [2:0-3-5, 0-0-3]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.66	in (loc) l/defl L/d	MT20	244/190
Snow (Pf/Pg) 15.4/20.0	Plate Grip DOL 1.15	BC 0.54	Vert(LL) -0.11 9-18 >916 360		
TCDL 10.0	Lumber DOL 1.15	WB 0.25	Vert(CT) -0.23 9-18 >421 240		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-AS	Horz(CT) 0.04 2 n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014		Wind(LL) 0.17 9-18 >574 240		
				Weight: 107 lb	FT = 20%

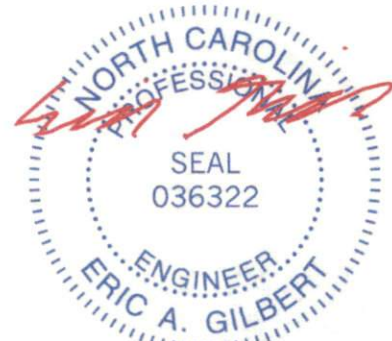
LUMBER-
 TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3
 SLIDER Left 2x4 SP No.3 2-6-0

BRACING-
 TOP CHORD Structural wood sheathing directly applied.
 BOT CHORD Rigid ceiling directly applied.

REACTIONS. (lb/size) 2=594/0-3-8, 9=881/0-3-8, 7=314/0-3-0
 Max Horz 2=-91(LC 14)
 Max Uplift 7=-79(LC 13)
 Max Grav 2=674(LC 2), 9=993(LC 2), 7=358(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-4=-572/56, 4-5=-619/70
 BOT CHORD 2-11=0/522, 9-11=-13/543
 WEBS 4-11=0/316, 6-9=-465/134, 5-9=-574/0

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-10-8 to 2-1-8, Interior(1) 2-1-8 to 8-0-0, Exterior(2) 8-0-0 to 11-0-0, Interior(1) 11-0-0 to 24-10-8 zone; cantilever left and right exposed; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10; Min. flat roof snow load governs.
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7.
 - This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



February 12, 2019

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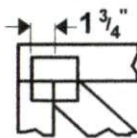
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



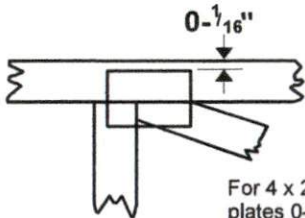
818 Soundside Road
 Edenton, NC 27932

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- $\frac{1}{16}$ " from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING

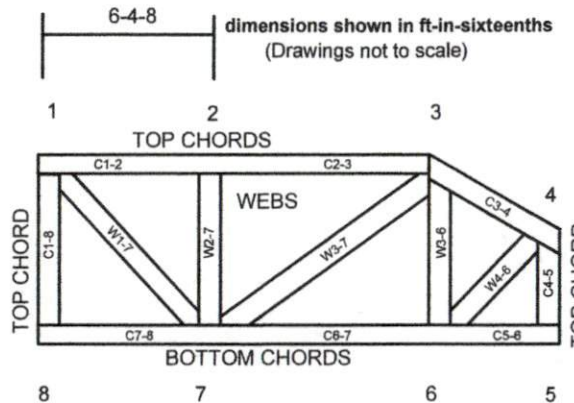


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

- ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
 DSB-89: Design Standard for Bracing.
 BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988
 ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.