

Trenco 818 Soundside Rd Edenton, NC 27932

Re: J0718-3290 Cary Reconstruction/25 Crutchfield Dr.

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: E12170064 thru E12170076

My license renewal date for the state of North Carolina is December 31, 2018.

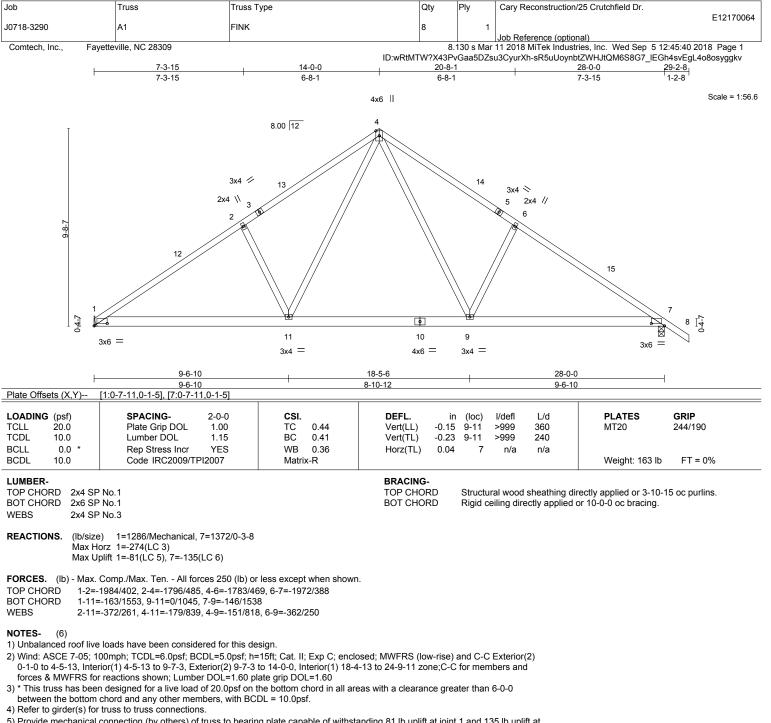
North Carolina COA: C-0844



September 5,2018

Strzyzewski, Marvin

**IMPORTANT NOTE:** Truss Engineer's responsibility is solely for design of individual trusses based upon design parameters shown on referenced truss drawings. Parameters have not been verified as appropriate for any use. Any location identification specified is for file reference only and has not been used in preparing design. Suitability of truss designs for any particular building is the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.



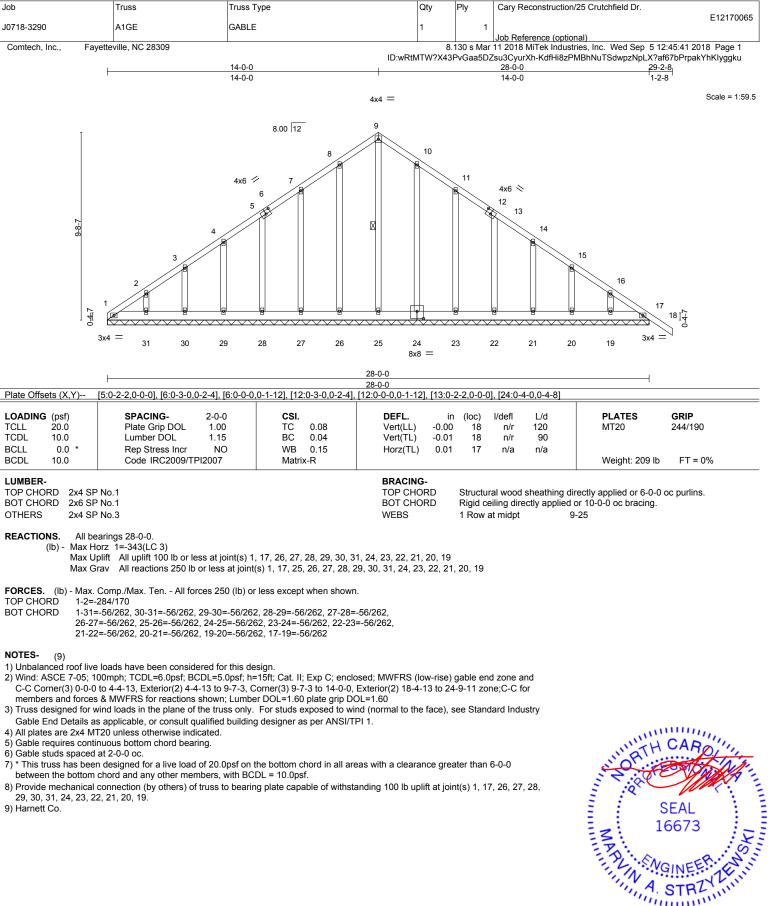
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 81 lb uplift at joint 1 and 135 lb uplift at joint 7.

6) Harnett Co.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
 Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not
 a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall
 building designer. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
 is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
 fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
 NoISITPI1 Quality Criteria, DSB-89 and BCSI Building Component
 Safety Information
 available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

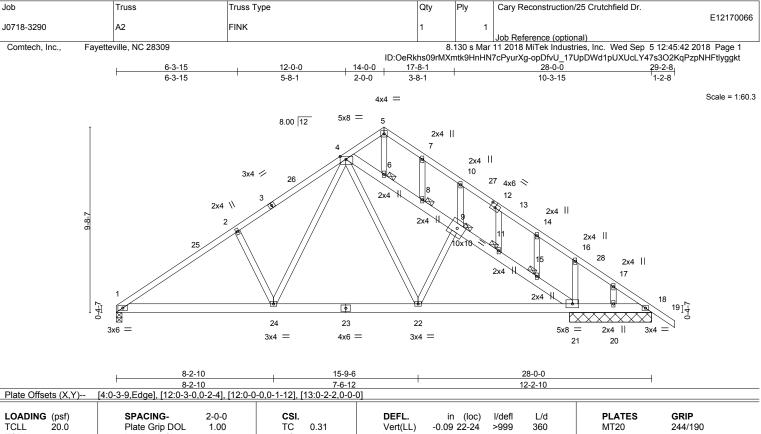






WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not besign value to be only with with these contractions. This besign is based only upon parameters shown, and is to rain individual outdarg component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

Edenton, NC 27932



TCLL 20.0 TCDL 10.0	Plate Grip DOL 1.00 Lumber DOL 1.15	TC 0.31 BC 0.32	Vert(LL) -0.09	9 22-24 >999 4 22-24 >999	360 240	MT20	244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2009/TPI2007	WB 0.32 Matrix-R	Horz(TL) 0.03	3 18 n/a	n/a	Weight: 211 lb	FT = 0%
LUMBER- TOP CHORD 2x4 3 4-21	SP No.1 *Except* 2x6 SP No.1		BRACING- TOP CHORD BOT CHORD			rectly applied or 4-6-0 or 10-0-0 oc bracing.	oc purlins.
	SP No.1 SP No.3		JOINTS	1 Brace at Jt(s)	): 6, 8, 9, 11, 1	5	

REACTIONS. All bearings 4-3-8 except (jt=length) 1=0-3-8.

(lb) - Max Horz 1=-343(LC 3)

Max Uplift All uplift 100 lb or less at joint(s) 20 except 1=-211(LC 5), 21=-476(LC 6) Max Grav All reactions 250 lb or less at joint(s) 20 except 1=1111(LC 1), 21=1253(LC 1), 18=302(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD 1-2=-1706/313, 2-4=-1546/394, 16-17=-274/0, 17-18=-329/0, 4-6=-1172/352,
  - 6-8=-1136/347, 8-9=-1187/374, 9-11=-1232/401, 11-15=-1284/429, 15-21=-1342/467
- BOT CHORD 1-24=-257/1331, 22-24=-77/895, 21-22=-111/1212, 20-21=0/359, 18-20=0/359
- WEBS 2-24=-303/283, 4-24=-217/705, 4-22=-125/566

2 24- 000/200, 4 24- 211/

NOTES- (6)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 9-7-3, Exterior(2) 9-7-3 to 14-0-0, Interior(1) 18-4-13 to 24-9-11 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

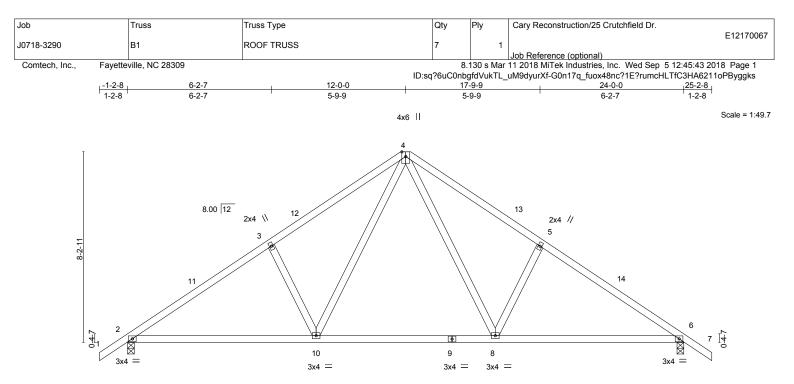
3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members, with BCDL = 10.0psf.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 20 except (jt=lb) 1=211, 21=476.

5) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.6) Harnett Co.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
 Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not
 a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall
 building designer. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
 is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
 fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
 MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component
 Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



		8-1-10			15-10-6				24-0-0	
		8-1-10			7-8-12				8-1-10	
LOADING	G (psf)	SPACING- 2-0	0-0 CSI		DEFL.	in (l	loc) l/	/defl L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL 1.	00 TC	0.32	Vert(LL)	-0.24 8	-10 >	999 360	MT20	244/190
TCDL	10.0	Lumber DOL 1.	15 BC	0.58	Vert(TL)	-0.32 8	-10 >	898 240		
BCLL	0.0 *	Rep Stress Incr YE	ES WB	0.29	Horz(TL)	0.05	6	n/a n/a		
BCDL	10.0	Code IRC2009/TPI200	7 Mat	rix-R					Weight: 122 lb	FT = 0%

BRACING-

TOP CHORD

BOT CHORD

# LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

2x4 SP No.3 WEBS

REACTIONS. (lb/size) 2=1184/0-3-8, 6=1184/0-3-8 Max Horz 2=-223(LC 3) Max Uplift 2=-123(LC 5), 6=-123(LC 6)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 2-3=-1665/330, 3-4=-1503/401, 4-5=-1503/401, 5-6=-1665/330

2-10=-118/1293, 8-10=0/877, 6-8=-118/1293 BOT CHORD

3-10=-303/217, 4-10=-132/689, 4-8=-132/689, 5-8=-303/217 WEBS

NOTES-(5)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2)

-1-2-8 to 3-2-5, Interior(1) 3-2-5 to 7-7-3, Exterior(2) 7-7-3 to 12-0-0, Interior(1) 16-4-13 to 20-9-11 zone; C-C for members and forces

& MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0

between the bottom chord and any other members, with BCDL = 10.0psf.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=123, 6=123.

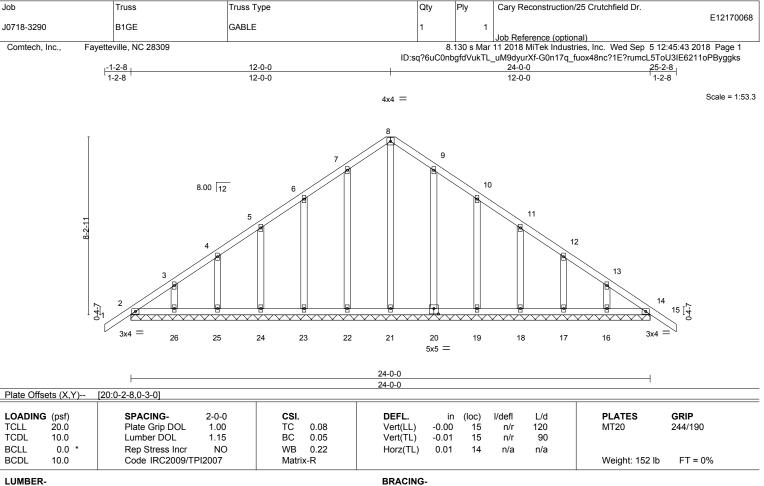
5) Harnett Co.



Structural wood sheathing directly applied or 4-5-13 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not besign value to be only with with these contractions. This besign is based only upon parameters shown, and is to rain individual outdarg component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



TOP CHORD

BOT CHORD

### LUMBER-

TOP CHORD 2x4 SP No.1 2x4 SP No.1 BOT CHORD 2x4 SP No 3 OTHERS

REACTIONS. All bearings 24-0-0

(lb) - Max Horz 2=-284(LC 3)

Max Uplift All uplift 100 lb or less at joint(s) 2, 14, 22, 23, 24, 25, 26, 20, 19, 18, 17, 16

Max Grav All reactions 250 lb or less at joint(s) 2, 14, 21, 22, 23, 24, 25, 26, 20, 19, 18, 17, 16

NOTES-(9)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Corner(3) -1-2-8 to 3-2-5, Exterior(2) 3-2-5 to 7-7-3, Corner(3) 7-7-3 to 12-0-0, Exterior(2) 16-4-13 to 20-9-11 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 2-0-0 oc.

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members, with BCDL = 10.0psf.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 14, 22, 23, 24, 25, 26, 20, 19, 18, 17, 16.

9) Harnett Co.



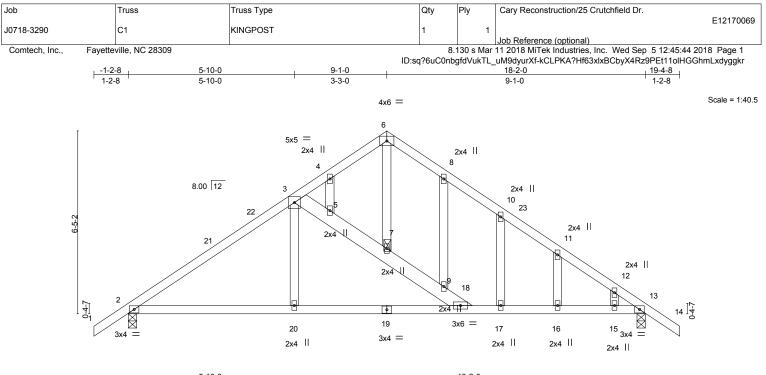
Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not besign value to be only with with these contractions. This besign is based only upon parameters shown, and is to rain individual outdarg component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-251/176



	5-10-0		18-2-0		
	5-10-0		12-4-0		
LOADING (psf)	<b>SPACING-</b> 2-0-0	CSI.	DEFL. in (loc) I/defl	L/d PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.00	TC 0.57	Vert(LL) 0.19 16-17 >999	360 MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.48	Vert(TL) -0.32 16-17 >679	240	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.23	Horz(TL) 0.02 13 n/a	n/a	
BCDL 10.0	Code IRC2009/TPI2007	Matrix-R		Weight: 1	109 lb FT = 0%
LUMBER-		·	BRACING-		

TOP CHORD

BOT CHORD

JOINTS

TOP CHORD	2x4 SP No.1 *Except
	3-18: 2x6 SP No.1
BOT CHORD	2x4 SP No.1
WEBS	2x4 SP No.3

REACTIONS. (lb/size) 13=796/0-3-8, 2=796/0-3-8 Max Horz 2=216(LC 4) Max Uplift 13=-244(LC 6), 2=-244(LC 5)

6-7=-243/556

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

2-3=-972/233, 3-4=-771/299, 4-6=-759/331, 6-8=-745/327, 8-10=-776/265, TOP CHORD

10-11=-793/210, 11-12=-837/165, 12-13=-884/125, 5-7=-273/152 BOT CHORD 2-20=-126/713, 18-20=-127/711, 17-18=-46/646, 16-17=-46/646, 15-16=-46/646, 13-15=-46/646

WEBS

NOTES-(6)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) -1-2-8 to 3-2-5, Interior(1) 3-2-5 to 4-8-3, Exterior(2) 4-8-3 to 9-1-0, Interior(1) 13-5-13 to 14-11-11 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 13=244, 2=244,

5) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord. 6) Harnett Co.

HER SO MILLIN ORT Manual International Providence of the International Providenc SEAL 16673 STP 11 STOR September 5,2018

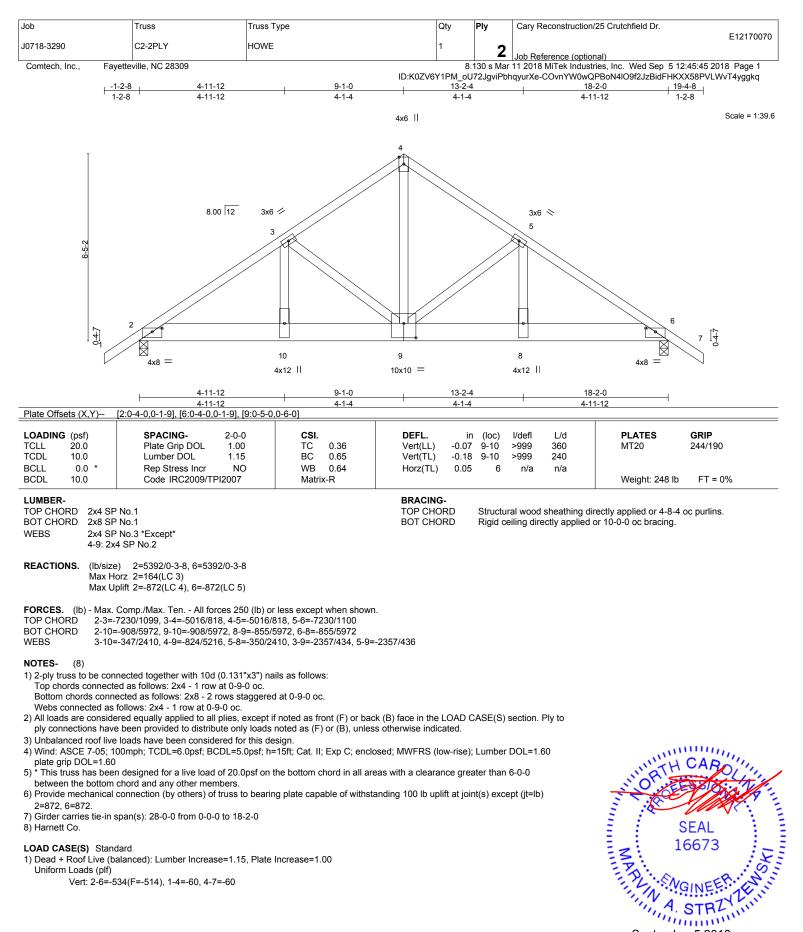
Structural wood sheathing directly applied or 5-7-5 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Brace at Jt(s): 7

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not besign value to be only with with these contractions. This besign is based only upon parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



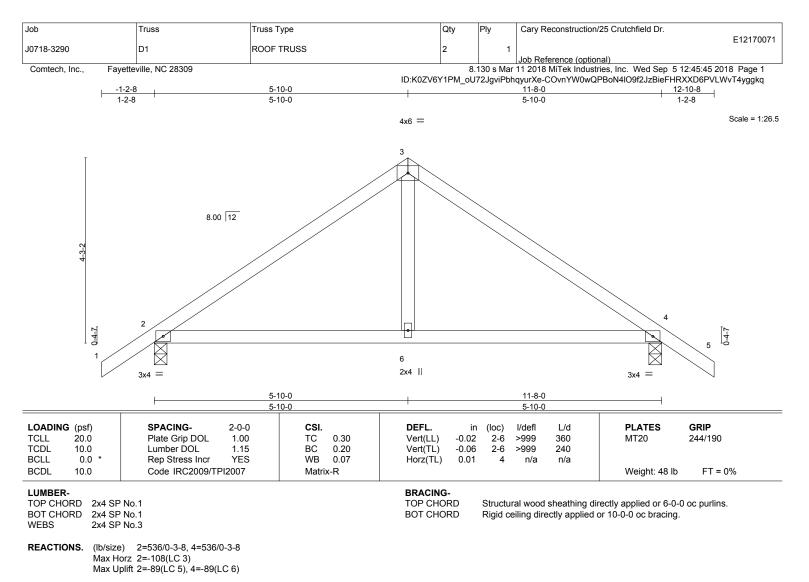


September 5,2018

Edenton, NC 27932



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
 Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not
 a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall
 building designer. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
 is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
 fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
 NoISITPI1 Quality Criteria, DSB-89 and BCSI Building Component
 Safety Information
 available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-557/160, 3-4=-557/160

BOT CHORD 2-6=0/387, 4-6=0/387

### **NOTES-** (5)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0

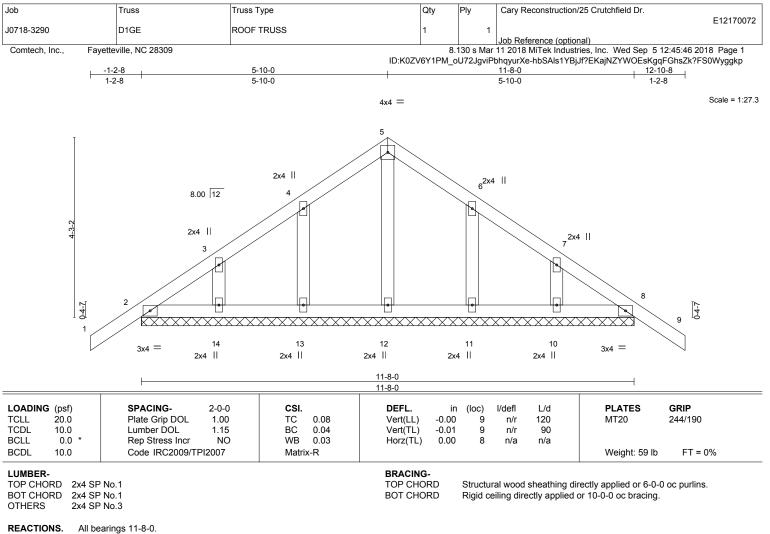
between the bottom chord and any other members.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4. 5) Harnett Co.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
 Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not
 a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall
 building designer. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
 is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
 fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
 MSNITP11 Quality Criteria, DSB-89 and BCSI Building Component
 Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





(lb) - Max Horz 2=-141(LC 3)

Max Uplift All uplift 100 lb or less at joint(s) 2, 8, 13, 14, 11, 10 Max Grav All reactions 250 lb or less at joint(s) 2, 8, 12, 13, 14, 11, 10

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

# NOTES- (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Corner(3) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Gable requires continuous bottom chord bearing.

5) Gable studs spaced at 2-0-0 oc.

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8, 13, 14, 11,

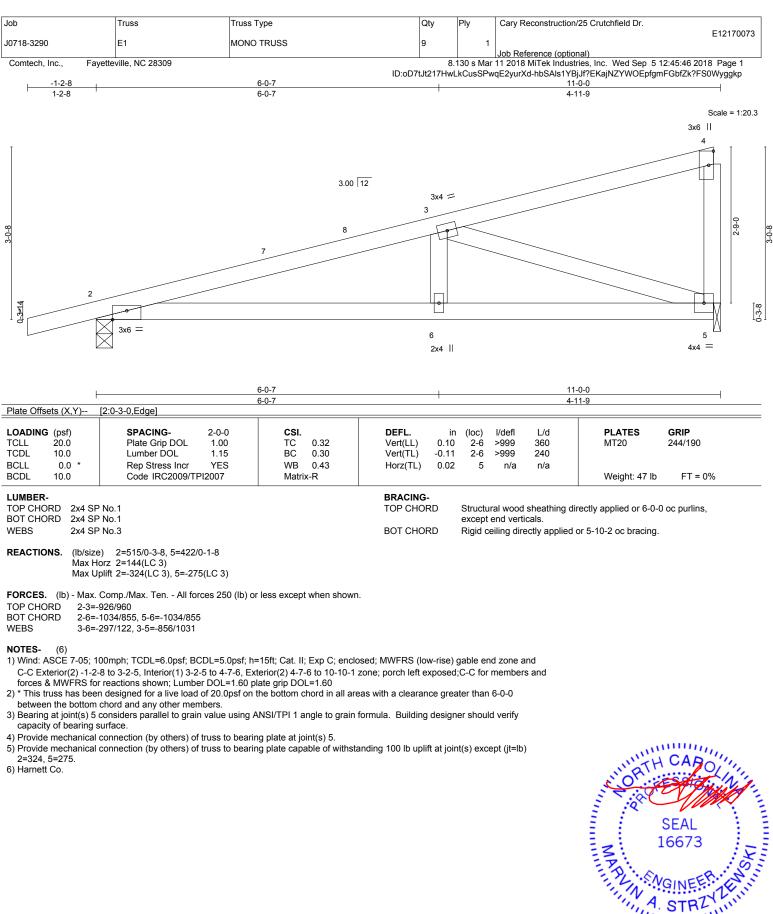
8) Harnett Co.

10.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
 Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not
 a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall
 building designer. This design in to the overall
 building designer must verify the applicability of design parameters and properly incorporate this design into the overall
 building designer. Strategies and the applicability of design parameters and properly incorporate this design into the overall
 building designer. Strategies and the applicability of design parameters and properly incorporate this design into the overall
 building designer. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
 is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
 fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
 NoISITP11 Quality Criteria, DSB-89 and BCSI Building Component
 Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

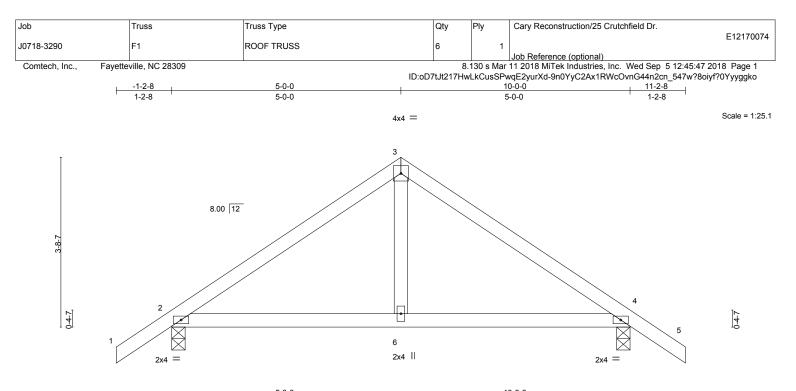
A MiTek Affili 818 Soundside Road Edenton, NC 27932





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not besign value to be only with with these contractions. This besign is based only upon parameters shown, and is to rain individual outdarg component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





	5-0-0 5-0-0				+			10-0-0 5-0-0	 			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	Lumbe	Grip DOL	2-0-0 1.00 1.15 YES	CSI. TC BC WB	0.21 0.14 0.05	DEFL. Vert(LL) Vert(TL) Horz(TL)	in -0.01 -0.03 0.01	(loc) 2-6 2-6 4	l/defl >999 >999 n/a	L/d 360 240 n/a	PLATES MT20	<b>GRIP</b> 244/190
BCDL 10.0		RC2009/TF		Matri		1012(12)	0.01	-	n/a	1// 4	Weight: 42 lb	FT = 0%

BRACING-

TOP CHORD

BOT CHORD

## LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

2x4 SP No.3 WEBS

(lb/size) 2=470/0-3-8, 4=470/0-3-8 REACTIONS. Max Horz 2=-93(LC 3) Max Uplift 2=-85(LC 5), 4=-85(LC 6)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 2-3=-467/133, 3-4=-467/133

BOT CHORD 2-6=0/321, 4-6=0/321

#### NOTES-(5)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0

between the bottom chord and any other members.

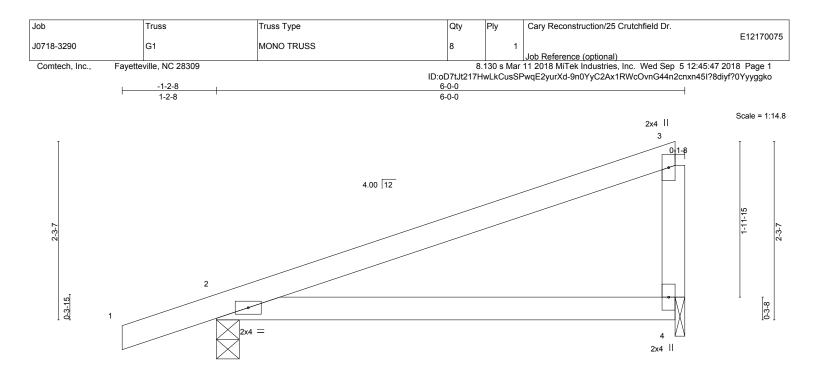
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4. 5) Harnett Co.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



OADING (psf)	<b>SPACING-</b> 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.00	TC 0.42	Vert(LL) 0.00 2 **** 360	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.31	Vert(TL) -0.08 2-4 >820 240	
3CLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(TĽ) 0.00 4 n/a n/a	
BCDL 10.0	Code IRC2009/TPI2007	Matrix-P	· ·	Weight: 22 lb FT = 0%

TOP CHORD

BOT CHORD

# LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 2x4 SP No.3 WEBS

REACTIONS. (lb/size) 2=319/0-3-8, 4=217/0-1-8 Max Horz 2=82(LC 3)

Max Uplift 2=-149(LC 3), 4=-104(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-(6)

1) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2)

zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0

between the bottom chord and any other members.

3) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

4) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=149, 4=104.

6) Harnett Co.

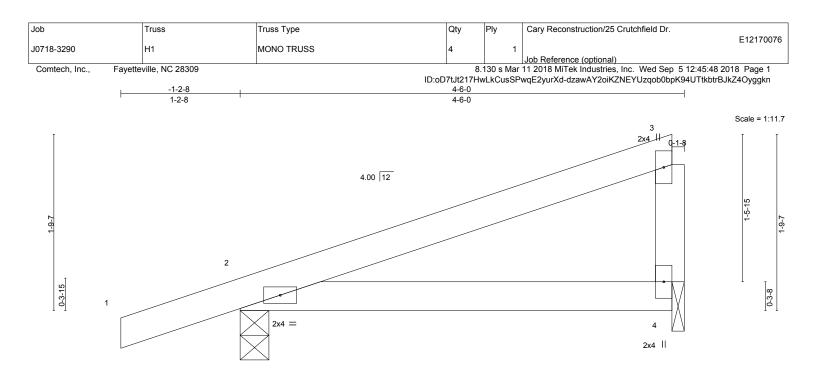


Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



		4-6-0										
	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.00	TC	0.20	Vert(LL)	0.00	2	****	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.16	Vert(TL)	-0.02	2-4	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	4	n/a	n/a		
BCDL	10.0	Code IRC2009/TF	PI2007	Matri	x-P						Weight: 17 lb	FT = 0%

## LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD WEBS

2x4 SP No.1 2x4 SP No.3 BRACING-TOP CHORD

Structural wood sheathing directly applied or 4-6-0 oc purlins, except end verticals BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 2=263/0-3-8, 4=154/0-1-8 Max Horz 2=66(LC 3) Max Uplift 2=-127(LC 3), 4=-73(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-(6)

1) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2)

zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0

between the bottom chord and any other members.

3) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

4) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=127

6) Harnett Co.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



