

ERECTION NOTES

1. All bracing shown and provided by the Metal Building Provider (MBP) for this building is required and shall be installed by the erector as a permanent part of the structure ("Code of Standard Practice for Steel Buildings" in the ANSI/AISC 303-16; Section 7.10).
2. Temporary supports, such as guys, braces, falsework, cribbing or other elements required for the erection operation shall be determined and furnished by the erector ("Code of Standard Practice for Steel Buildings and Bridges " in the ANSI/AISC 303-16; Section 7.10.3).
3. Normal erection operations include the correction of minor misfits by moderate amounts of reaming, grinding, welding or cutting, and the drawing of elements into line through use of drift pins. Errors which require major changes in the member configuration are to be reported immediately to the Metal Building Provider by the customer to enable whoever is responsible either to correct the error or to approve the most efficient and economic method of correction to be used by others ("Code of Standard Practice for Steel Buildings and Bridges "in the ANSI/AISC 303-16; Section 7.14).
4. Erection tolerances are set forth in the "Code of Standard Practice for Steel Buildings and Bridges "in the ANSI/AISC 303-16; Section 7.13 note that individual members are considered plump, level and aligned if the deviation does not exceed 1:500. Variations in finished overall dimensions of structure steel framing are deemed within the limits of good practice when they do not exceed the cumulative effect of rolling, fabricating, and erection tolerances.
- 4.1. When crane support systems are part of the metal building system erection tolerances Section 6.8, Erection Tolerances, 2018 MBMA Metal Building Systems manual shall apply. To achieve the required tolerances grouting of the columns and shimming of the runway beams may be required. The customer shall provide grout if required. The contractor erecting the runway beams is responsible for shimming, plumbing, and leveling of the runway system. When aligning the runway beams the alignment shall be with respect to the beam webs so that the center of the aligned rail is over the runway web.
5. As a general rule field welding is not used to assemble a metal building system. In cases where the drawings indicate field welding and in cases where approved corrections are to be made by field welding the following requirements shall be met;
- 5.1. welders must be qualified by an independent testing agency, with suitable documentation to AWS D1.1 Structural Welding Code – Steel or AWS D1.3 Structural Welding Code – Sheet as applicable, for the processes, positions, and materials involved.
- 5.2. All welds must be made in conformance to a documented and approved Welding Procedure Specification (WPS). All joints which are not prequalified must be supported by a certified Procedure Qualification Record (PQR) by an independent testing agency.
6. All documentation and records shall be the responsibility of the customer.
7. Any claims or shortages by buyer must be made to the Metal Building Provider within seven (7) working days after delivery, or such claims will be considered to have been waived by the customer and disallowed. All claims should be directed to the Metal Building Provider's Customer Service Department.
8. Claims for correction of alleged misfits will be disallowed unless the Metal Building Provider shall have received prior notice thereof and allowed reasonable inspection of such misfits. Ordinary inaccuracies of shop work shall not be construed as misfits. No part of the building may be returned or charges assessed for alleged misfits without prior approval from the Metal Building Provider.
9. Neither the Metal Building Provider nor the customer will cut, drill or otherwise alter their work, or the work of other trades to accommodate other trades unless such work is clearly specified in the contract documents. Whenever such work is specified the customer is responsible for furnishing complete information as to materials, size, location, and number of alterations prior to preparation of shop drawings ("Code of Standard Practice for Steel Buildings and Bridges "in the ANSI/AISC 303-16, Section 7.15).
10. The Metal Building Provider Field Modifications Policy:
- 10.1. The Metal Building Provider will only be responsible for the field-modified parts designed and approved by the Metal Building Provider's Customer Service Department.
- 10.2. Any field modifications designed by third parties may not be approved by the Metal Building Provider and may limit the Metal Building Provider's warranty and liability.
- 10.3. The Metal Building Provider makes no warranty and hereby disclaims any responsibility with respect to the design, engineering, or construction of any field-modified parts performed by third parties.
11. WARNING – SOME PANELS AND TRIM PARTS ARE FURNISHED WITH A PROTECTIVE PEEL-OFF FILM. PARTS PROVIDED WITH THIS FILM CANNOT BE EXPOSED TO SUNLIGHT WITHOUT FIRST REMOVING THE FILM. THIS FILM MUST BE REMOVED PRIOR TO INSTALLATION. FILM MUST ALSO BE REMOVED FROM ALL NON EXPOSED PARTS WITHIN SIX MONTHS FROM FILM APPLICATION OR IRREPARABLE DAMAGE WILL OCCUR TO THE SURFACE CLAIMS WILL NOT BE ACCEPTED FOR THIS ISSUE.

RESPONSIBILITIES

1. The Metal Building Provider Customer, hereafter referred to as the "customer," obtains and pays for all building permits, licenses, public assessments, paving or utility pro rata, utility connections, occupancy fees and other fees required by any governmental authority or utility in connection with the work provided for in the Contract Documents. The customer provides at his expense all plans and specifications required to obtain a building permit. it is the customer's responsibility to ensure that all plans and specifications comply with the applicable requirements of any governing building authorities.
2. The customer is responsible for identifying all applicable building codes, zoning codes, or other regulations applicable to the Construction Project, including the Metal Building system.
3. It is the responsibility of the customer to interpret all aspects of the End User's specifications and incorporate the appropriate specifications, design criteria, and design loads into the Order Documents submitted to the Metal Building Provider.
4. It is the responsibility of the Metal Building Provider to furnish the metal building system to meet the specifications including the design criteria and design loads incorporated by the Contractor into the Order Documents. The Metal Building Provider is not responsible for making an independent determination of any local codes or any other requirements not part of the Order Document.
5. The Metal Building Provider's standard specifications apply unless stipulated otherwise in the Contract Documents. The Metal Building Provider design, fabrication, quality criteria, standards, practice, methods and tolerances shall govern the work any other interpretations to the contrary not with standing. it is understood by both parties that the customer is responsible for clarifications of inclusions or exclusions from the Architectural plans.
6. In case of discrepancies between the Metal Building Provider's structural steel plans and plans for other trades, the Metal Building Provider's shall govern ("Code of Standard Practice for Steel Buildings and Bridges" in the AISC 303-16; Section 3.3).
7. The customer is responsible for overall project coordination. All interface, compatibility and design considerations concerning any materials not furnished by the Metal Building Provider and the Metal Building Provider's steel system are to be considered and coordinated by the customer. Specific design criteria concerning this interface between materials must be furnished by the customer before release for fabrication or the Metal Building Provider's assumptions will govern.
8. Foundations, anchor rods, and anchor rod embedment are designed, furnished, and set by the customer in accordance with an approved drawing. Dimensional accuracy shall satisfy the requirements of Section 7.5 1 of "Code of Standard Practice for Steel Buildings and Bridges" in the AISC 303-16.
9. All other embedded items or connection materials between the structural steel and the work of other trades are located and set by the customer in accordance with approved location on erection drawings. Accuracy of these items must satisfy the erection tolerance requirements.
10. The Metal Building Provider does not investigate the influence of the metal building system on existing buildings or structures. The End Customer assures that such buildings and structures are adequate to resist snow drifts, wind loads, or other conditions as a result of the presence of the metal building system.

GENERAL SPECIFICATIONS

1. Wall and liner panels are an integral part of the structural system. Unauthorized removal of panels or cutting panels for framed openings not shown is prohibited.
2. Oil-canning, a perceived waviness inherent to light gauge metal, may exist. This condition does not affect the structural integrity or the finish of the panel, and therefore is not a cause for rejection.
3. The Metal Building Provider's red-oxide and gray-oxide primer are designed for short term field protection from exposure to ordinary atmospheric conditions. Primed steel which is stored in the field pending erection should be kept free of the ground, and so positioned as to minimize water-holding pockets, dust, mud, and other contamination of the primer film. Repairs of damage to primed surfaces and/or removal of foreign material due to transportation (e.g. road salt, de-icing chemicals and other substances encountered during transportation that may accelerate deterioration of the primer or corrosion of the underlying steel), improper field storage, or site conditions are not the responsibility of the Metal Building Provider. (MBMA, 2018 MBSM, Section 4.2.4)
4. All bolts are 1/2" x 1-1/4" A307 unless noted. Refer to the erection drawings for specific framing connections and the cross-section(s) for main frame connections.
5. Unless noted otherwise on the frame cross section(s), all bolted joints with ASTM F3125 Grade A325 bolts are specified as snug-tightened joints in accordance with the specification for Structural Joints Using High-Strength Bolts, June 11, 2020. Installation inspection requirements for Snug-Tight Bolts (Specification for Structural joints, Section 9.1) is suggested.
6. Unless noted otherwise, all bolted connections are designed as bearing type connections with bolt threads not excluded from the shear plane.
7. Any type of suspended or load inducing system(s) is prohibited if zero collateral and zero sprinkler loads are designated on the contract. This would include lights, duct work, piping, and insulation types other than 3" standard duty fiberglass blanket insulation, etc.

BUILDING DESIGN CODES

Building Code: NORTH CAROLINA BUILDING CODE 2018
Hot-rolled version: AISC 360-10
Cold-formed version: AISI S100-12

GENERAL LOADS

Dead Load: 2.50 psf
Roof Collateral Load: 3.00 psf (Misc.)
Roof Live Load: 20.00 psf
Tributary Live Load Reduction: YES
Rainfall Intensity: 6.54 in/hr (5-minute duration 5-year recurrence)

WIND LOAD

Wind Speed (3-sec gust) Vult: 120 mph
Vasd: 93 mph
V service: 76 mph
Exposure Factor: B
Wind Condition: Enclosed
Internal Pressure Coefficient : +/- 0.18
Edge Zone Width: 6.00 Ft

SNOW LOAD

Ground Snow Load : 15.00 psf
Roof Snow Load : 10.50 psf
Importance Factor: 1.00
Exposure Factor: 1.00
Thermal Factor: 1.00
Slope Factor: 1.00

DEFLECTION CRITERIA : (Building B)

Endwall Column : H/120 Wind Framing (Wind) : L/60
Endwall Rafter (Live) : L/180 Wind Framing (Seismic) : L/50
Endwall Rafter (Wind) : L/180
Wall Girts : L/90
Roof Purlin (Live) : L/180
Roof Purlin (Wind) : L/180 Roof Purlin (Total) : L/120
Wall Panel : L/60 Endwall Rafter (Total) : L/120
Roof Panel (Live) : L/60 Rigid Frame (Total) : L/120
Roof Panel (Wind) : L/60
Rigid Frame (Horz) : H/60
Rigid Frame (Live) : L/180
Rigid Frame (Wind) : L/180
Rigid Frame (Seismic) : L/50
Rigid Frame (Crane) : L/100

For components,claddings and MWFRS, deflections involving wind are based on 10 year serviceability wind pressures.

SEISMIC LOAD

Risk Category: II - Normal
Seismic Importance Factor : 1.0000
Structural Response Acceleration (Ss): 0.1750
Structural Response Acceleration(S1): 0.0840
Site Class: D
Design Spectral Response (Sds): 0.1867
Design Spectral Response (Sd1): 0.1344
Seismic Design Category: C

Framing Direction: Lateral Longitudinal
Structural Syst: 'Structural Steel Systems Not Specifically Detailed for Seismic Resistance'

Response Modification Factor : 3.0 3.0
Deflection Amplification : 3.0 3.0
Seismic Response Coefficient (Cs): 0.623 0.623
Design Base Shear V : 5.36 5.35
Analysis Procedure: Equivalent Lateral Force

ROOF PANEL

Profile: Super Seam-Plus Gauge: 24 Color: SMP Polar White
UL580 Class 90: Yes
Clip Type if Standing Seam: High Floating

WALL PANEL

Profile: Super Span X Gauge: 26 Color: SMP Sahara Tan

PRIMARY FRAMING

Built-Up & Hot-Rolled: Gray Oxide Primer

SECONDARY FRAMING

Purlins, Eave Struts: Pre-Galvanized
Girts, Light Gage Columns: Pre-Galvanized
Light Gage Jambs & Headers: Pre-Galvanized
Base Channel Finish: Pre-Galvanized

Hot-Dip Galvanizing conforms to the ASTM A123 specification.
Pre-Galvanized members conform to the ASTM A653, Grade 50,
Coating G-90 specification.

APPROVAL SPECIFICATIONS

1. Approval of the Metal Building Provider drawings and/or calculations indicate that the Metal Building Provider has correctly interpreted the contact requirements. This approval constitutes the customer acceptance of the Metal Building Provider design, concepts, assumptions, and loadings.
2. Failure to respond to clouded areas and areas to verify may result in additional costs and/or schedule delays for which the Metal Building Provider will not be responsible.
3. Any changes made after the Metal Building Provider's customer has signed and returned the Metal Building Provider drawings and/or calculations and the project is released for fabrication shall be billed to the Metal Building Provider customer including material, engineering, and other costs. An additional fee may be charged if the project must be moved in the fabrication and/or the shipping schedule.
4. It is the responsibility of the customer to field verify all existing conditions prior to fabrication.
5. It is imperative that any changes to these drawings:
- 5.1. Be made in contrasting ink.
- 5.2. Be legible and unambiguous.
- 5.3. Have all instances of changes clearly indicated.
6. A dated signature, in the designated areas, is required on all pages. The signature must be from the person authorized on the contract or a person authorized, in writing, by the Metal Building Provider customer.
7. The Metal Building Provider reserves the right to resubmit drawings with extensive or complex changes required to avoid misfabrication. This may impact the delivery schedule.
8. Any changes noted on the drawings not in conformance with the terms and requirements of the contract between the Metal Building Provider and its customer are not binding on the Metal Building Provider unless subsequently acknowledged and agreed to in writing by change order or separate documentation.
9. Waiving the approval process by designating the order "For Production" supercedes notes 1,2,5,6, and 8 in this section, and constitutes the customer acceptance of the Metal Building Provider's design, concepts, assumptions, and loadings.

DRAWING SCHEDULE

DWG NO.	ISSUE	DATE	DESCRIPTION
C1	P1	12.01.25	COVER SHEET
F1	0	12.01.25	ANCHOR BOLT PLAN
F2	0	12.01.25	ANCHOR BOLT DETAILS
F3	0	12.01.25	ANCHOR BOLT REACTIONS
P1	P1	12.01.25	RIGID FRAME ELEVATION
W1	P1	12.01.25	PORTAL FRAME ELEVATION
W2	P1	12.01.25	PORTAL FRAME ELEVATION
E1	P1	12.01.25	ROOF FRAMING PLAN
E2	P1	12.01.25	ROOF SHEETING PLAN
E3	P1	12.01.25	ENDWALL FRAME & SHEETING ELEVATION
E4	P1	12.01.25	ENDWALL FRAME & SHEETING ELEVATION
E5	P1	12.01.25	SIDEWALL FRAME & SHEETING ELEVATION
E6	P1	12.01.25	SIDEWALL FRAME & SHEETING ELEVATION
E7	P1	12.01.25	BUILDING SECTIONS
E8	P1	12.01.25	BUILDING SECTIONS
D1	P1	12.01.25	STANDARD DETAIL PAGE
D2	P1	12.01.25	STANDARD DETAIL PAGE
D3	P1	12.01.25	STANDARD DETAIL PAGE
D4	P1	12.01.25	STANDARD DETAIL PAGE

TRIM COLOR:

SHADOW GUTTER: SMP Black GAUGE: 26
SHADOW RAKE: SMP Black GAUGE: 26
DOWNSPOUTS: SMP Black GAUGE: 26
CORNER: SMP Black GAUGE: 26
ACCESSORY: SMP Black GAUGE: 26
BASE TRIM: SMP Sahara Tan GAUGE: 26

THE JOBSITE IS LOCATED IN A HURRICANE PRONE REGION WITH WIND SPEEDS OF 120 MPH OR GREATER. TO MAINTAIN THE ENCLOSED BUILDING CLASSIFICATION AND DESIGN FOR WIND, ALL OVERHEAD OR ROLL-UP TYPE DOORS, PERSONNEL DOORS, AND WINDOWS PROVIDED BY OTHERS SHALL BE DESIGNED TO BE IMPACT RESISTANT OR COVERED BY IMPACT RESISTANT MATERIALS. THE BUYER'S DESIGN PROFESSIONAL, NOT THE METAL BUILDING PROVIDER'S ENGINEER, IS RESPONSIBLE FOR DETERMINING THE ADEQUACY OF MATERIAL ACTING AS THE IMPACT RESISTANT COVERING BY OTHERS AND ATTACHMENT OF SUCH TO THE MATERIAL PROVIDED BY THE BUILDING PROVIDER. THIS STRUCTURE HAS NOT BEEN DESIGNED TO WITHSTAND THE ADDITIONAL INTERNAL PRESSURE REQUIRED BY CODE AS A PARTIALLY ENCLOSED CONDITION IN THE ABSENCE OF IMPACT RESISTANT COVERINGS.

NOTICE TO CONTRACTOR

All construction must comply with current NC Building Codes and is subject to field inspections and verification.

Reviewed for Code Compliance

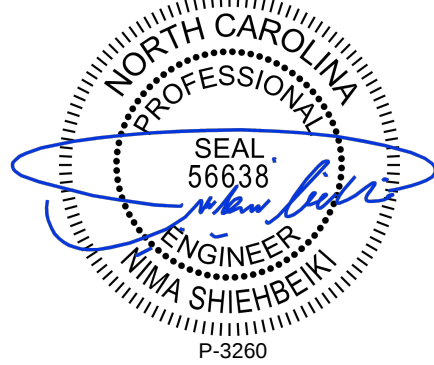
09/23/2026



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This document has been digitally signed and sealed by Nima Shiehbeiki on 12/04/2025

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☐ FOR ERECTOR INSTALLATION: Final drawings for construction.

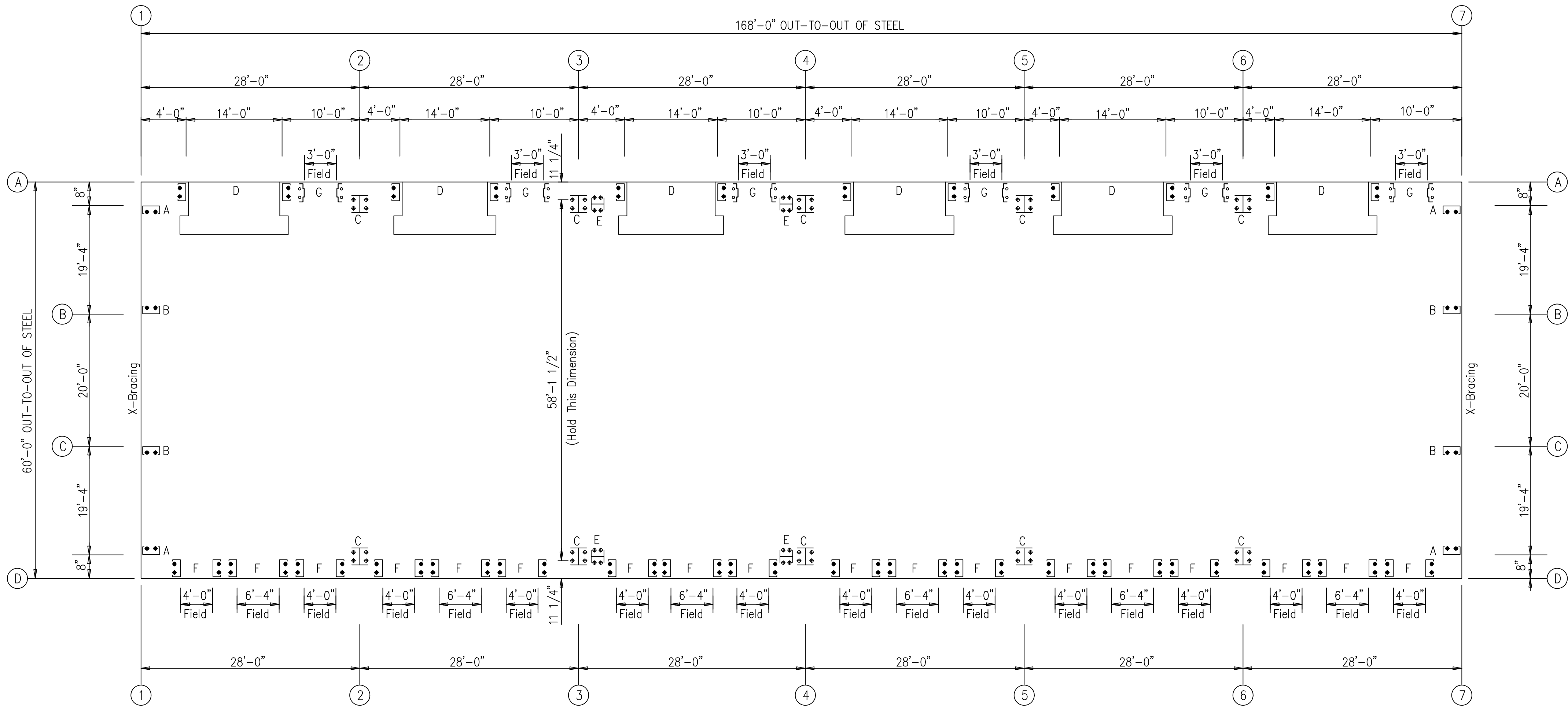


ISSUE	DATE	DESCRIPTION	BY	CHK	SHEET DESCRIPTION:	BLDG SIZE:
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					PROJECT REFERENCE: PROGRESS VENTURES LLC	
					JOBSITE LOCATION: FUQUAY VARINA, NC 27526	JOBSITE COUNTY: WAKE
					DWN: PND	CHK: PNC
					DATE: 12.01.25	ENG: MKK
					JOB NO: 15130-38808	DWG NO: C1
						ISSUE: P1

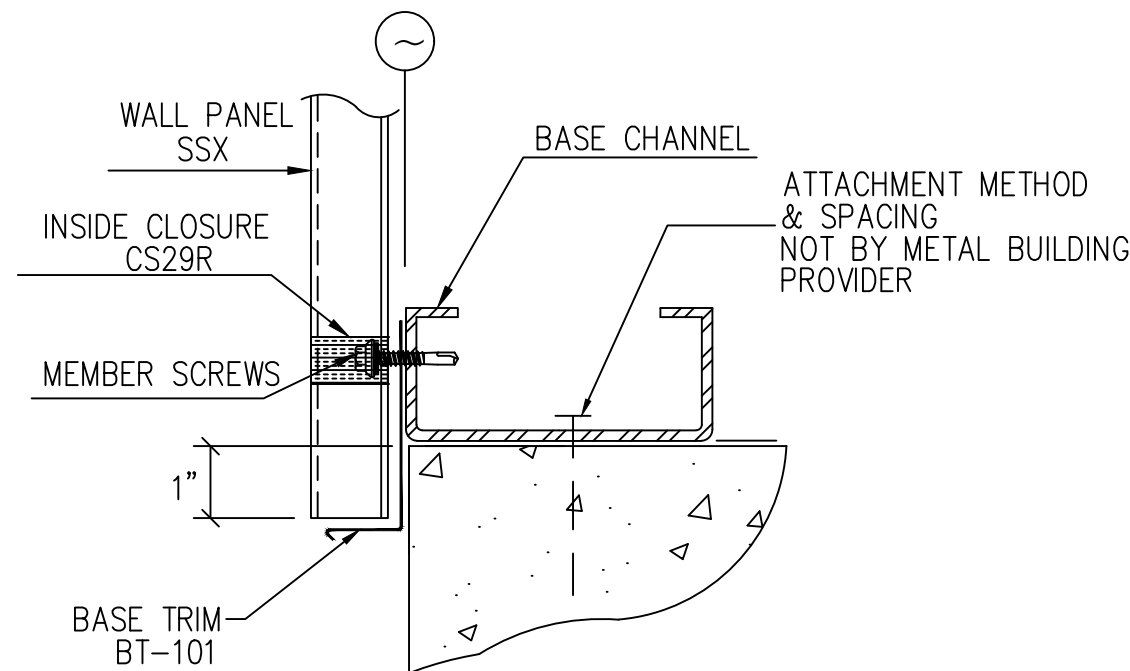
● Dia= 5/8"

⊕ Dia= 3/4"

○ Dia= 1/2"



ANCHOR BOLT PLAN
NOTE: All Base Plates @ Finished Floor (U.N.)



BASE TRIM DETAIL W/ BASE CHANNEL NO SHEETING RECESS

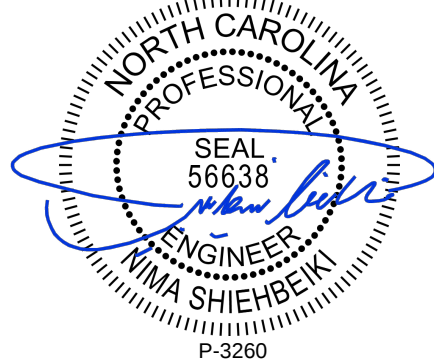
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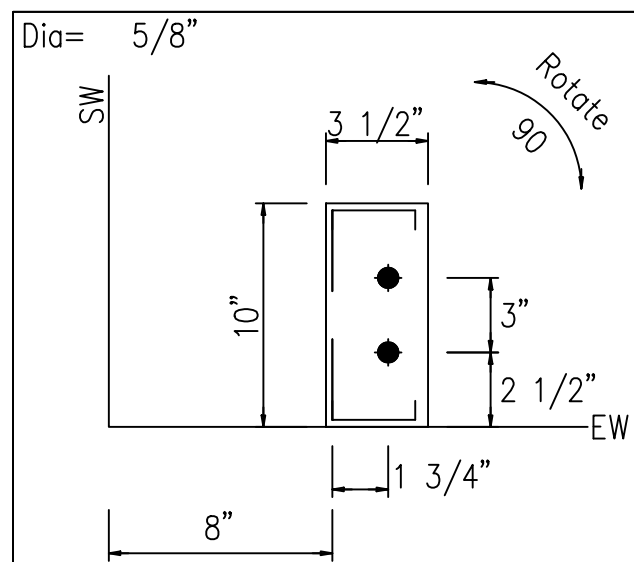
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CUSTOMER:						CUSTOMER LOCATION:
PROGRESS VENTURES LLC						FUQUAY VARINA, NC 27526
PROJECT REFERENCE:						
PROGRESS VENTURES LLC						
JOBSITE LOCATION:						JOBSITE COUNTY:
FUQUAY VARINA, NC 27526						WAKE
DWN:	CHK:	DATE:	ENG:	JOB NO:	DWG NO:	ISSUE:
PND	PNC	12.01.25	MVK	15130-38808	F1	0

This document has been digitally signed and sealed by Nima Shiehbeiki on 12/04/2025

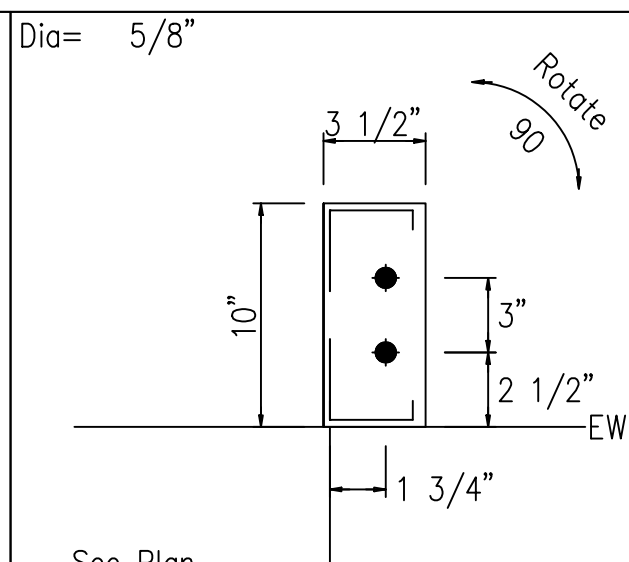
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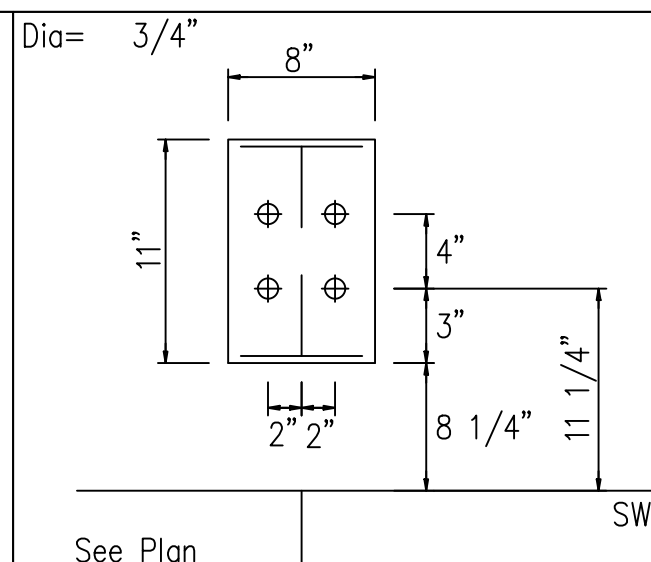
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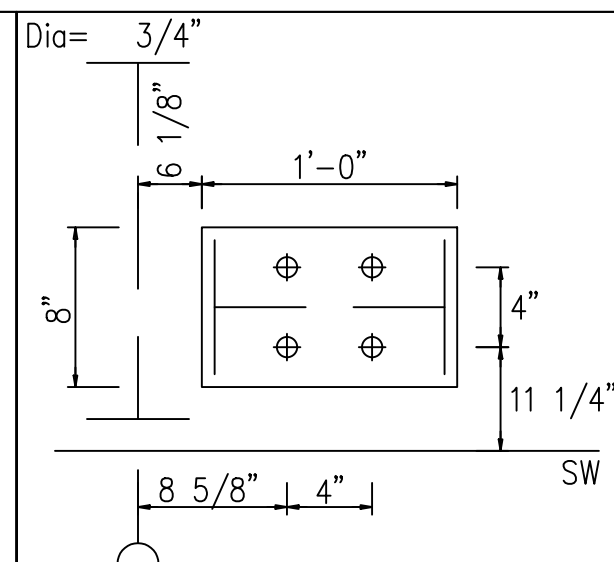
DETAIL A



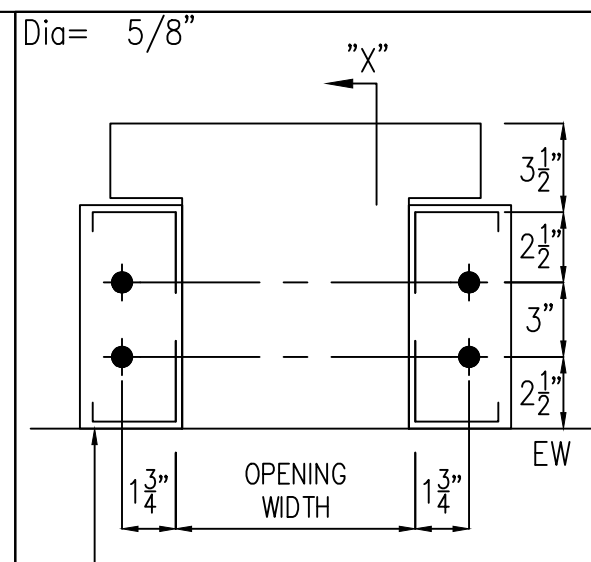
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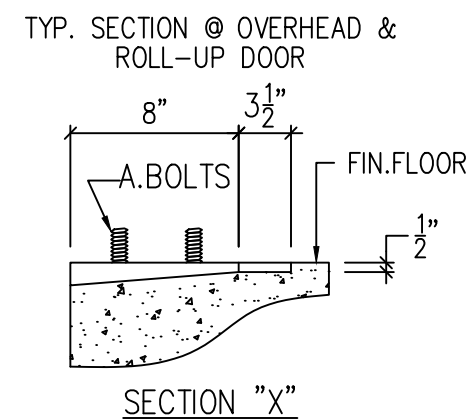
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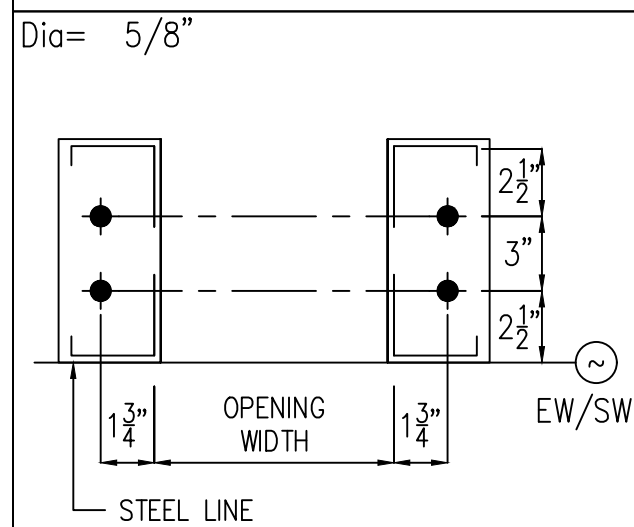
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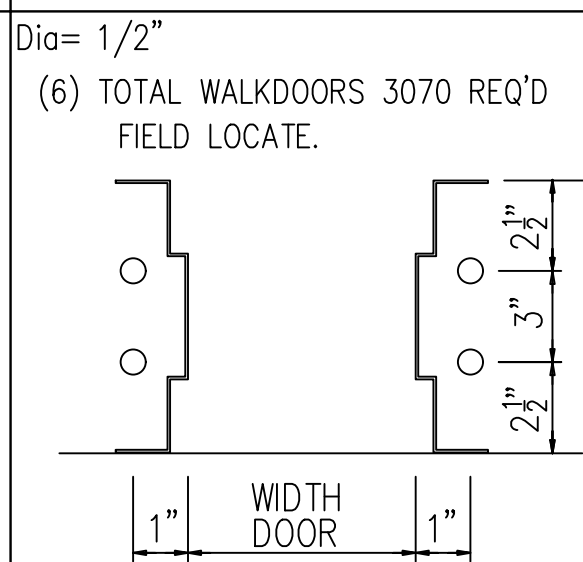
DETAIL D



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DETAIL F



DETAIL G

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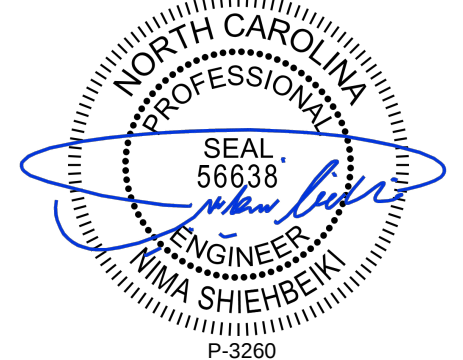


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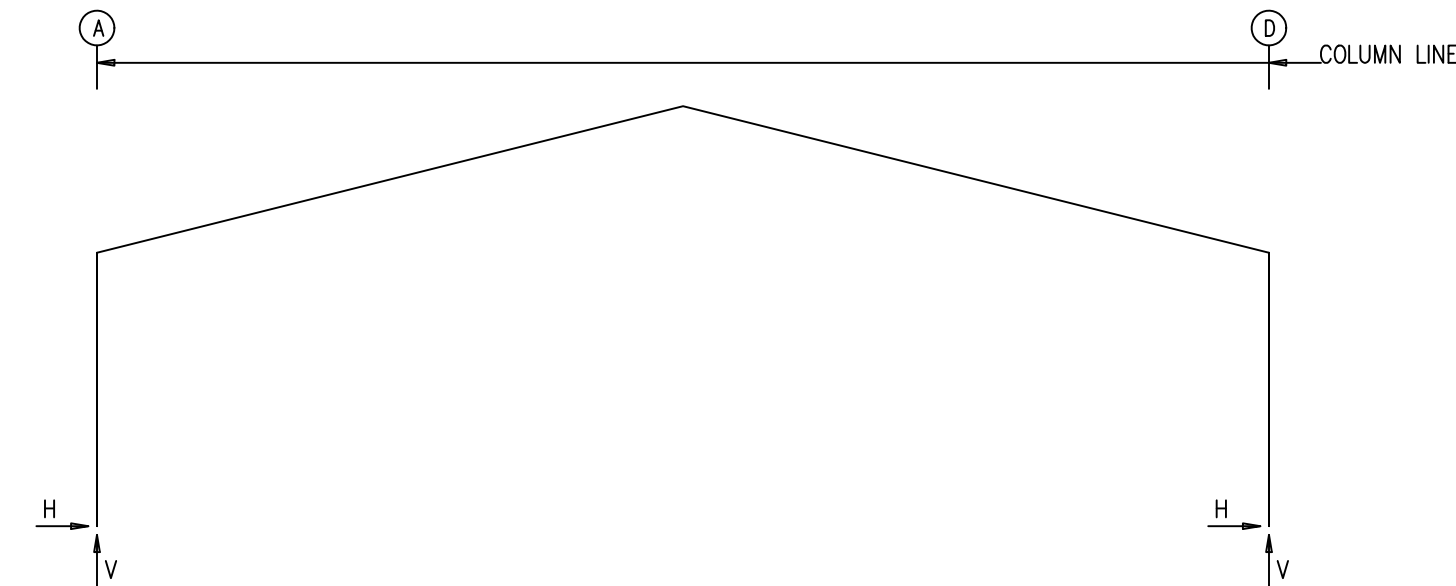
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FRAME LINES: 2 3 4 5 6



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)						Bolt(in) Dia		Base_Plate(in)			Elev. (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin			Width	Length	Thick	
2*	A	3	10.6	18.1	1	−5.7	−7.6	4	0.750	8.000	11.00	0.375	0.0
2*	D	2	5.7	−7.6	3	−10.6	18.1	4	0.750	8.000	11.00	0.375	0.0
		3	−10.6	18.1	2	5.7	−7.6						
2*	Frame lines:		2	3	4	5	6						

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead	Collat	Live	Snow	Wind_Left1	Wind_Right1	Wind_Left2	Wind_Right2	Wind Press
1	A	0.4	0.4	2.1	1.2	0.0	-2.8	0.0	-2.5	0.0
1	B	1.1	1.0	5.7	3.2	-2.4	-9.7	0.0	-2.3	-2.4
1	C	1.1	1.0	5.7	3.2	0.0	-2.3	2.4	-9.7	0.0
1	D	0.4	0.4	2.1	1.2	0.0	-2.5	0.0	-2.8	0.0

Frm Line	Col Line	Wind Suct	Wind_Long1	Wind_Long2	Seis_Left	Seis_Right	Seis Long	-MIN_SNOW--
1	A	1.5	0.0	-3.5	0.0	-2.2	0.0	0.0
1	B	3.9	0.0	-5.9	-1.1	-5.0	-0.5	-0.5
1	C	3.9	1.1	-5.0	0.0	-5.9	0.0	0.5
1	D	1.5	0.0	-2.2	0.0	-3.5	0.0	0.0

Frm Line	Col Line	E1UNB_SL_L--	E1UNB_SL_R--	E1PAT_LL_1--	E1PAT_LL_2--	E1PAT_LL_3--	E1PAT_LL_4--
1	A	0.0	1.1	0.0	0.3	0.0	2.0
1	B	0.0	4.2	0.0	1.5	0.0	6.2
1	C	0.0	1.5	0.0	4.2	0.0	2.4
1	D	0.0	0.3	0.0	1.1	0.0	-0.2

Frm Line	Col Line	Dead	Collat	Live	Snow	Wind_Left1	Wind_Right1	Wind_Left2	Wind_Right2	Wind Press
7	D	0.4	0.4	2.1	1.2	0.0	-2.8	0.0	-2.5	0.0
7	C	1.1	1.0	5.7	3.2	-2.4	-9.7	0.0	-2.3	-2.4
7	B	1.1	1.0	5.7	3.2	0.0	-2.3	2.4	-9.7	0.0
7	A	0.4	0.4	2.1	1.2	0.0	-2.5	0.0	-2.8	0.0

Frm Line	Col Line	Wind Suct	Wind_Long1	Wind_Long2	Seis_Left	Seis_Right	Seis Long	-MIN_SNOW--
7	D	1.5	0.0	-3.5	0.0	-2.2	0.0	0.0
7	C	3.9	0.0	-5.9	-1.1	-5.0	-0.5	-0.5
7	B	3.9	1.1	-5.0	0.0	-5.9	0.0	0.5
7	A	1.5	0.0	-2.2	0.0	-3.5	0.0	0.0

Frm Line	Col Line	E2UNB_SL_L--	E2UNB_SL_R--	E2PAT_LL_1--	E2PAT_LL_2--	E2PAT_LL_3--	E2PAT_LL_4--
7	D	0.0	1.1	0.0	0.3	0.0	2.0
7	C	0.0	4.2	0.0	1.5	0.0	6.2
7	B	0.0	1.5	0.0	4.2	0.0	2.4
7	A	0.0	0.3	0.0	1.1	0.0	-0.2

ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)						Bolt(in) Qty	Dia	Base_Plate(in)			Elev. (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin			Width	Length	Thick	
1	A	4	0.9	-1.9	5	-0.8	-1.9	2	0.625	3.500	10.00	0.250	0.0
		6	0.0	3.2	4	0.9	-1.9						
1	B	7	2.3	-5.2	5	-2.1	-2.9	2	0.625	3.500	10.00	0.250	0.0
		8	0.0	8.2	7	2.3	-5.2						
1	C	9	2.3	-5.2	10	-2.1	-2.9	2	0.625	3.500	10.00	0.250	0.0
		11	0.0	8.2	9	2.3	-5.2						
1	D	12	0.9	-1.9	10	-0.8	-1.9	2	0.625	3.500	10.00	0.250	0.0
		6	0.0	3.2	12	0.9	-1.9						
7	D	4	0.9	-1.9	5	-0.8	-1.9	2	0.625	3.500	10.00	0.250	0.0
		13	0.0	3.2	4	0.9	-1.9						
7	C	7	2.3	-5.2	5	-2.1	-2.9	2	0.625	3.500	10.00	0.250	0.0
		14	0.0	8.2	7	2.3	-5.2						
7	B	9	2.3	-5.2	10	-2.1	-2.9	2	0.625	3.500	10.00	0.250	0.0
		15	0.0	8.2	9	2.3	-5.2						
7	A	12	0.9	-1.9	10	-0.8	-1.9	2	0.625	3.500	10.00	0.250	0.0
		13	0.0	3.2	12	0.9	-1.9						

6	5	6
---	---	---

6	4	3	4
5	2	1	2
	4	3	4
	4	3	4
	2	1	2
6	4	3	4

6	5	6
---	---	---

Zone	Width (ft)	Length (ft)	Components & Cladding			
			Pressure(psf Member)	Panel	Suction(psf Member)	Panel
1			16.00	16.00	-21.50	-23.69
2			16.00	16.00	-30.31	-40.98
3	6.00	6.00	16.00	16.00	-30.31	-40.98
4	6.00	6.00	16.00	16.00	-47.73	-60.88
5			20.86	25.88	-23.05	-28.08
6	6.00		20.86	25.88	-24.66	-34.54

(+) wind towards surface
(-) wind away from surface

6	5	6
---	---	---

NOTE: Per AISI S-100-12 Appendix A Section I6.3.1a, the wind uplift loading in edge and corner zones is permitted to be multiplied by a factor of 0.67 for the standing seam roof panel used on this building. This reduction factor is not expressed in the ultimate wind uplift values tabulated on this page.

PORTAL FRAME REACTIONS

Frm Line	Col Line	Dead	Collat	Live	Snow	Wind_Left1	Wind_Right1	Wind_Left2	Wind_Right2	Wind Press
1	A	0.4	0.4	2.1	1.2	0.0	-2.8	0.0	-2.5	0.0
1	B	1.1	1.0	5.7	3.2	-2.4	-9.7	0.0	-2.3	-2.4
1	C	1.1	1.0	5.7	3.2	0.0	-2.3	2.4	-9.7	0.0
1	D	0.4	0.4	2.1	1.2	0.0	-2.5	0.0	-2.8	0.0

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead		Collateral		Live		Snow		Wind_Left1		Wind_Right1	
2*	A	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
2*	D	1.6	2.9	1.5	2.6	6.0	10.1	5.2	8.8	-11.0	-15.6	-3.0	-11.7
2*	A	-1.6	2.9	-1.5	2.6	-6.0	10.1	-5.2	8.8	3.0	-11.7	11.0	-15.6

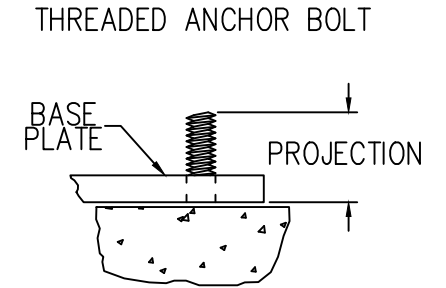
Frame Line	Column Line	Wind_Left2		Wind_Right2		Wind_Long1		Wind_Long2		Seismic_Left		Seismic_Right	
2*	A	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
2*	D	-9.4	-9.0	-1.4	-5.1	-3.7	-14.2	-5.1	-12.0	-0.4	-0.2	0.4	0.2
2*	A	1.4	-5.1	9.4	-9.0	5.1	-12.0	3.7	-14.2	-0.4	0.2	0.4	-0.2

Frame Line	Column Line	MIN_SNOW		F1UNB_SL_L		F1UNB_SL_R	
2*	A	Horz	Vert	Horz	Vert	Horz	Vert
2*	D	7.5	12.6	4.6	8.8	4.6	5.3
2*	A	-7.5	12.6	-4.6	5.3	-4.6	8.8

2* Frame lines: 2 3 4 5 6

GENERAL NOTES

- All anchor bolts (by others) to have nuts and flat washers.
- All anchor bolts are designed to full S.A.E. diameters with cut threads. No substitutions are allowed.
- The Metal Building Provider is not responsible for the design, materials and workmanship of the foundation. Anchor bolt plans prepared by the Metal Building Provider are intended to show only location, diameter, and projection of anchor bolts required to attach the Metal Building System to the foundation. It is the responsibility of the End Customer to ensure that adequate provisions are made for specifying bolt embedment, bearing angles, tie rods, and/or other associated items embedded in the concrete foundation, as well as foundation design for the loads imposed by the Metal Building System, other imposed loads, and the bearing capacity of the soil and other conditions of the building site. This is typically the responsibility of the Design Professional or Engineer of Record, which is another reason that their involvement in the Construction Project from the outset is highly recommended. (2012 MBMA Metal Building Systems Manual, Section 3.2.2)
- The projection is based from the bottom of the base plate. Adjustments must be made for grout and/or leveling plates.



NOTE: PROJECTION BASED FROM BOTTOM OF BASE PLATE. ADJUSTMENTS SHOULD BE MADE FOR GROUT AND/OR LEVELING PLATES.

NOTES FOR REACTIONS

- All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
- Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
- Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
- Loading conditions are:
 - 0.6Dead+0.6Wind_Left1
 - 0.6Dead+0.6Wind_Right1
 - Dead+Collateral+MIN_SNOW
 - 0.6Dead+0.6Wind_Suction+0.6Wind_Long1L
 - 0.6Dead+0.6Wind_Pressure+0.6Wind_Long1L
 - Dead+Collateral+E1PAT_LL_3
 - 0.6Dead+0.6Wind_Left1+0.6Wind_Suction
 - Dead+Collateral+E1PAT_LL_1
 - 0.6Dead+0.6Wind_Right1+0.6Wind_Suction
 - 0.6Dead+0.6Wind_Pressure+0.6Wind_Long2L
 - Dead+Collateral+E1PAT_LL_2
 - 0.6Dead+0.6Wind_Suction+0.6Wind_Long2L
 - Dead+Collateral+E2PAT_LL_3
 - Dead+Collateral+E2PAT_LL_1
 - Dead+Collateral+E2PAT_LL_2

BUILDING BRACING REACTIONS

Loc	Wall Line	Col Line	± Reactions(k)				Panel Shear (lb/ft)		Note
			Wind	Seismic	Wind	Seismic	Wind	Seis	
L_EW	1	B,C	2.4	2.1	0.5	0.5			(a)
F_SW	D	3,4							
R_EW	7	C,B	2.4	2.1	0.5	0.5			(a)
B_SW	A	3,4							

(a)Wind bent in bay

Reactions for seismic represent shear force, Eh
Reaction values shown are unfactored

ANCHOR BOLT SUMMARY (GRADE 36)				
Qty	Locate	Dia (in)	Type	Proj (in)
96	Jamb	5/8"	F1554	2.50
16	Endwall	5/8"	F1554	2.50
40	Frame	3/4"	F1554	3.00
16	Portal Frame	3/4"	F1554	3.00
24	Walk Door	1/2"	F1554	2.00

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NORTH CAROLINA
PROFESSIONAL
SEAL
56638
ENGINEER
NIMA SHIEHBEIKI
P-3260

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☒ FOR ERECTOR INSTALLATION: Final drawings for construction.



ISSUE	DATE	DESCRIPTION	BY	CHK
0	12.01.25	FOR ERECTOR INSTALLATION	PND	PNC

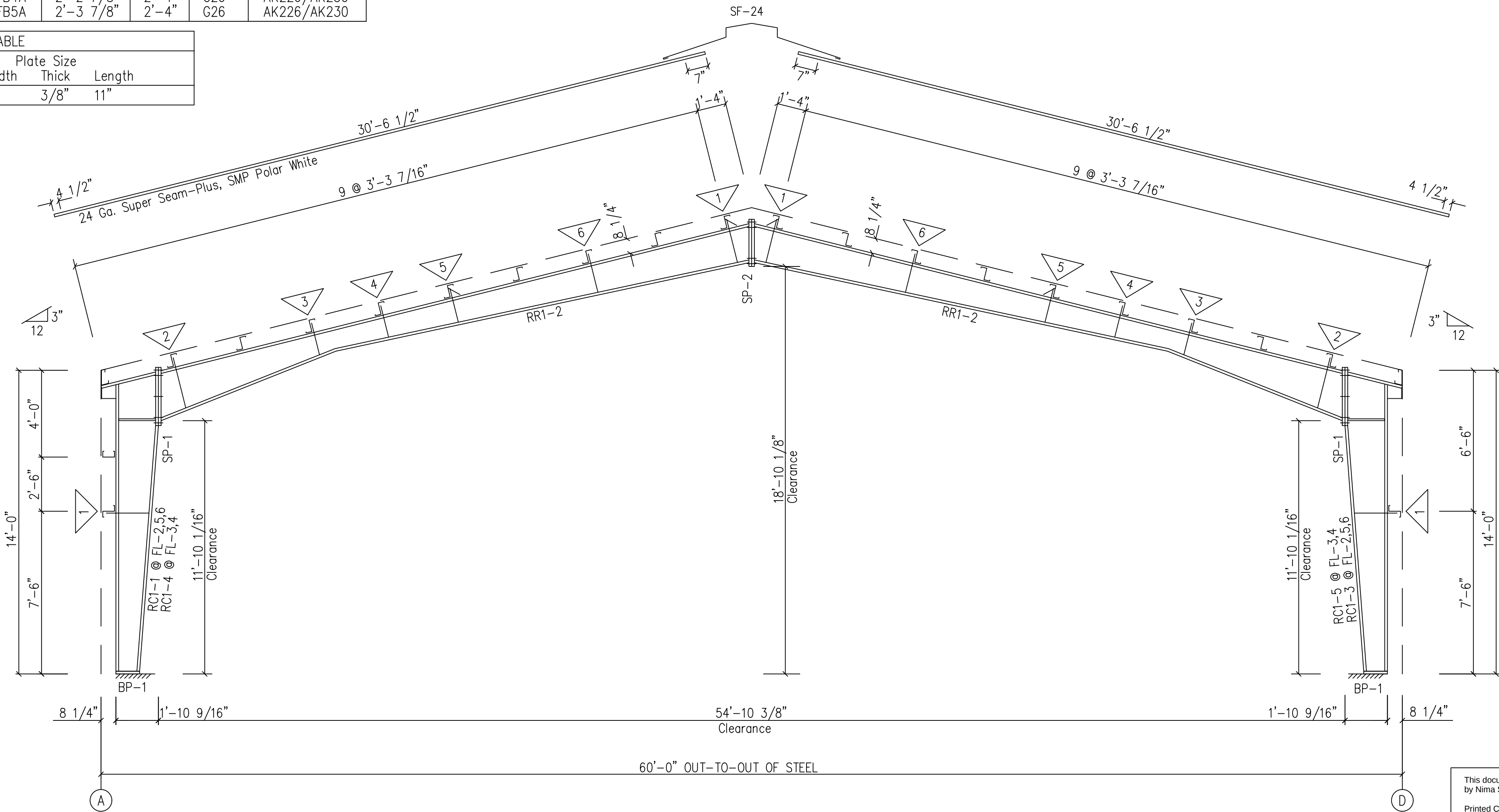
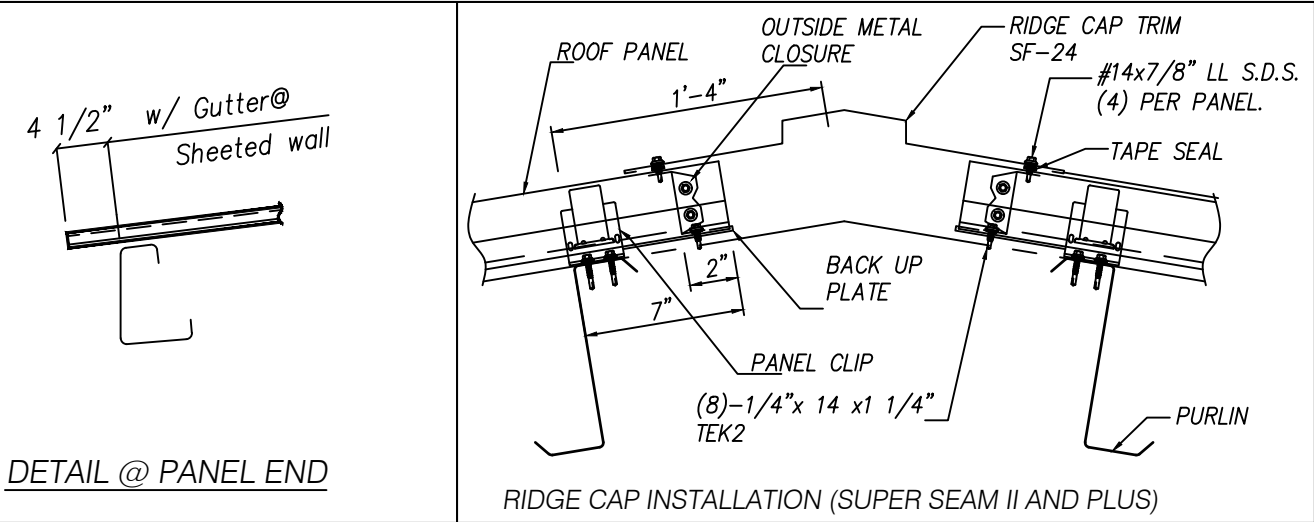
SHEET DESCRIPTION:		BLDG SIZE:	
ANCHOR BOLT REACTIONS		60'-0" x 168'-0" x 14'-0"	
CUSTOMER:		CUSTOMER LOCATION:	
PROGRESS VENTURES LLC		FUQUAY VARINA, NC 27526	
PROJECT REFERENCE:		PROGRESS VENTURES LLC	
JOB SITE LOCATION:		JOB SITE COUNTY:	
FUQUAY VARINA, NC 27526		WAKE	
OWN:	CHK:	DATE:	ENG:
PND	PNC	12.01.25	MVK
JOB NO:		DWG NO:	
15130-38808		F3	
ISSUE:		0	

SPlice PLATE & BOLT TABLE									
Mark	Qty		Int	Type	Dia	Length	Width	Thick	Length
SP-1	4	4	2	A325	3/4"	2"	6"	1/2"	2'-9 3/4"
SP-2	4	4	0	A325	3/4"	1 3/4"	6"	3/8"	2'-3 1/2"

FLANGE BRACE TABLE						
A=L2x2x14GA B=L2x2x12GA C=L2x2x1/8 D=L3x3x3/16						
FRAME LINE: 2 3 4 5 6						
▽ ID	# Sides	MARK	LENGTH	OFFSET	DETAIL	CLIP
1	1	FB6A	2'-5 1/8"	2'-4"	G26	AK226/AK230
2	1	FB7A	2'-7 3/8"	2'-4"	G26	AK226/AK230
3	1	FB2A	2'-2 5/8"	2'-4"	G26	AK226/AK230
4	1	FB1A	2'-2 1/2"	2'-4"	G26	AK226/AK230
5	1	FB4A	2'-2 7/8"	2'-4"	G26	AK226/AK230
6	1	FB5A	2'-3 7/8"	2'-4"	G26	AK226/AK230

BASE PLATE TABLE			
Col Mark	Plate Size		
	Width	Thick	Length
BP-1	8"	3/8"	11"

MEMBER TABLE				
Mark	Web Depth Start/End	Web Plate Thick	Outside Flange W x Thk	Inside Flange W x Thk
RC1-1/RC1-4	10.0/22.0	0.161	6 x 1/4" 6 x 5/16"	6 x 5/16"
RR1-2	24.0/10.0 10.0/18.0	0.184 0.133	5 x 1/4" 5 x 1/4"	5 x 1/4" 5 x 1/4"
RC1-3/RC1-5	22.0/10.0	0.161	6 x 5/16" 6 x 1/4"	6 x 5/16"



BOLT TIGHTENING (Snug-Tight)

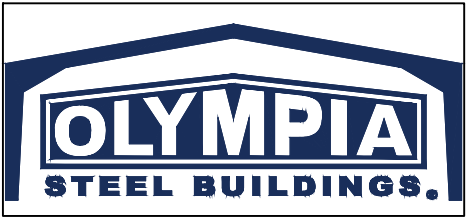
All bolted joints with ASTM F3125 Grade A325 bolts are specified as Snug-Tightened Joints in accordance with the Specification of Structural Joints Using High-Strength Bolts, June 11, 2020, installation as given in Section 7.1 Washers are not required for Snug-Tightened Joints using standard standard size holes per Section 6.1 of the Specification

Pretensioning methods, including Turn-of-Nut, calibrated wrench, twist-off tension control bolts or direct tension indicator are not required. Installation inspection requirements for Snug-Tight Bolt is found in Section 9.1 of the Specification.

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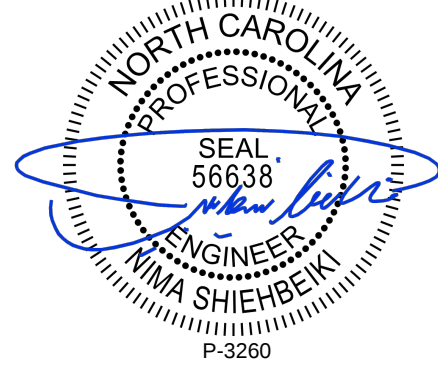


ISSUE	DATE	DESCRIPTION	BY	CHK
P1	12.01.25	FOR CONSTRUCTION PERMIT	PND	PNC

SHEET DESCRIPTION:		BLDG SIZE:	
RIGID FRAME ELEVATION		60'-0" x 168'-0" x 14'-0"	
CUSTOMER:		CUSTOMER LOCATION:	
PROGRESS VENTURES LLC		FUQUAY VARINA, NC 27526	
PROJECT REFERENCE:		PROGRESS VENTURES LLC	
JOBSITE LOCATION:		JOBSITE COUNTY:	
FUQUAY VARINA, NC 27526		WAKE	
DWN:	CHK:	DATE:	ENG:
PND	PNC	12.01.25	MVK
JOB NO:		DWG NO:	
15130-38808		P1	
ISSUE:		P1	

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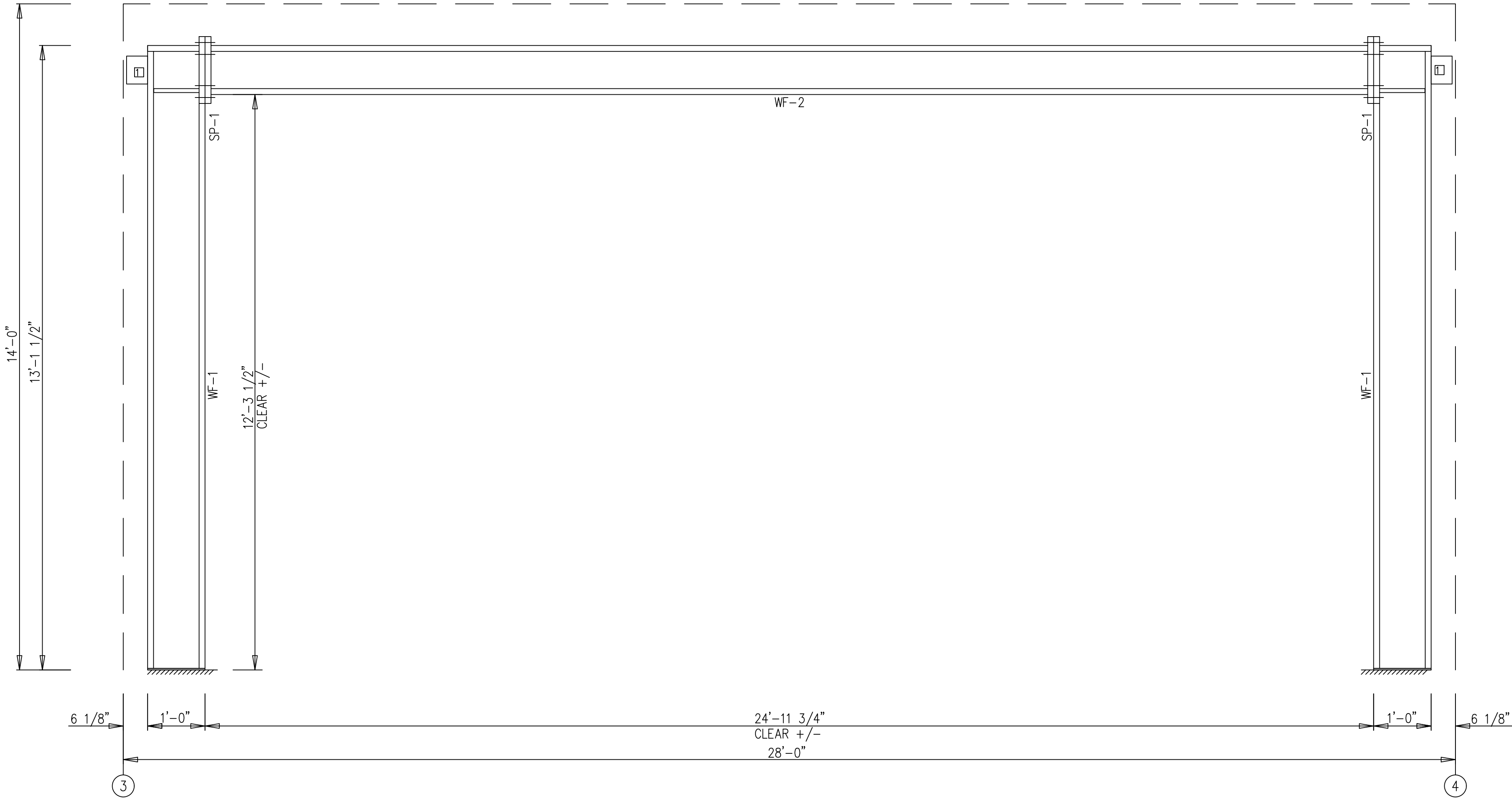
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SPlice PLATES & BOLTS								
Splice Mark	Quan Top/ Bot		-----Bolt----- Type Dia Length	Plate Size Width Thick Length				
SP- 1	4	4	A325 0.750 2.00	8"	1/2"	1'-6 1/4"		

MEMBER SIZE TABLE	
MARK	MEMBER
WF-2	W10651
WF-1	W12841

CONNECTION PLATES
FRAME LINE D

ID	MARK/PART
1	AK508



BOLT TIGHTENING (Snug-Tight)

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Final drawings for construction.



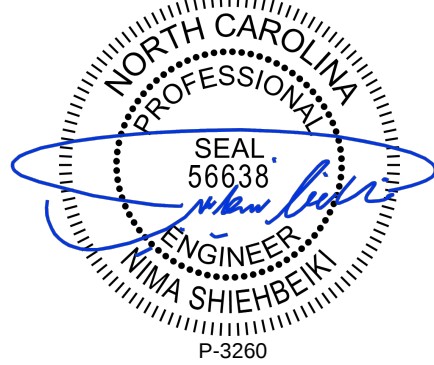
ISSUE	DATE	DESCRIPTION	BY	CHK
P1	12.01.25	FOR CONSTRUCTION PERMIT	PND	PNC

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SHEET DESCRIPTION:		BLDG SIZE:	
PORTAL FRAME ELEVATION		60'-0" x 168'-0" x 14'-0"	
CUSTOMER:		CUSTOMER LOCATION:	
PROGRESS VENTURES LLC		FUQUAY VARINA, NC 27526	
PROJECT REFERENCE:		PROJECT REFERENCE:	
PROGRESS VENTURES LLC		PROGRESS VENTURES LLC	
JOBSITE LOCATION:		JOBSITE COUNTY:	
FUQUAY VARINA, NC 27526		WAKE	
DWN:	CHK:	DATE:	ENG:
PND	PNC	12.01.25	MVK
JOB NO:	DWG NO:	WAKE	ISSUE:
15130-38808	W1		P1

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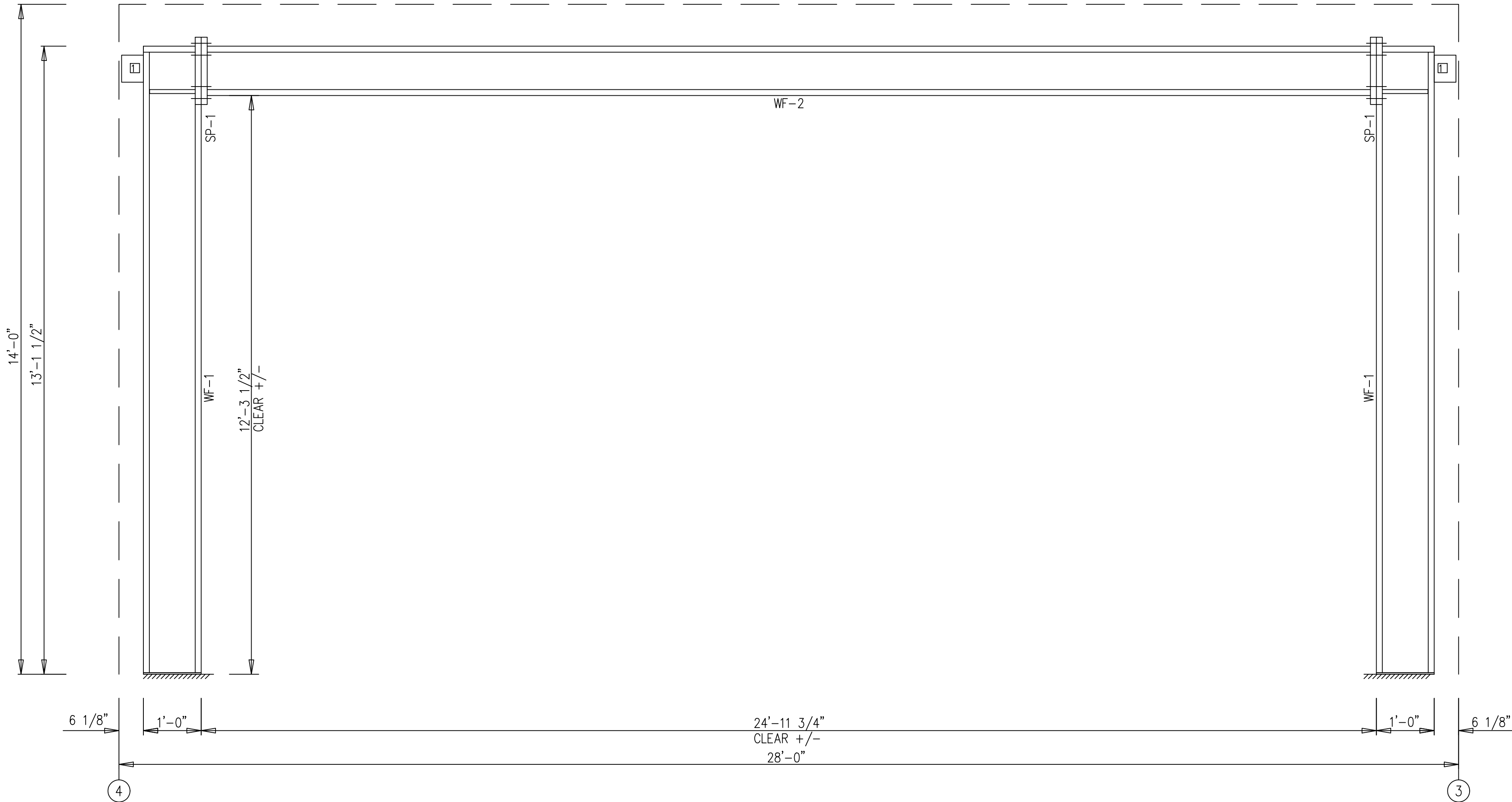
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SPLICE PLATES & BOLTS								
Splice Mark	Quan		-----Bolt-----			Plate Size		
	Top/	Bot	Type	Dia	Length	Width	Thick	Length
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MEMBER SIZE TABLE	
MARK	MEMBER
WF-2	W10651
WF-1	W12841

CONNECTION PLATES	
FRAME LINE A	
ID	MARK/PART
1	AK508



PORTAL FRAME ELEVATION: FRAME LINE A

BOLT TIGHTENING (Snug-Tight)

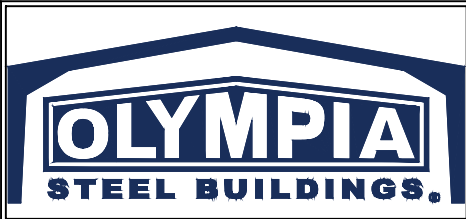
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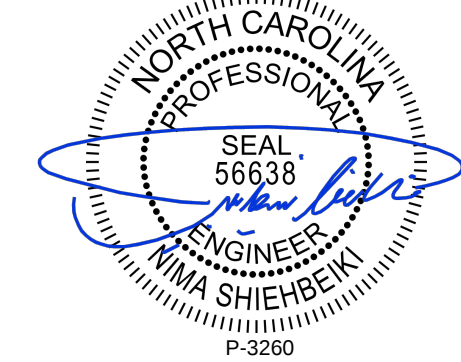
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ISSUE	DATE	DESCRIPTION	BY	CHK	SHEET DESCRIPTION:		BLDG SIZE:	
P1	12.01.25	FOR CONSTRUCTION PERMIT	PND	PNC	PORTAL FRAME ELEVATION		60'-0" x 168'-0" x 14'-0"	
					CUSTOMER:		CUSTOMER LOCATION:	
					PROGRESS VENTURES LLC		FUQUAY VARINA, NC 27526	
					PROJECT REFERENCE:			
					PROGRESS VENTURES LLC			
					JOBSITE LOCATION:		JOBSITE COUNTY:	
					FUQUAY VARINA, NC 27526		WAKE	
DWN:		CHK:	DATE:	ENG:	JOB NO:	DWG NO:	ISSUE:	
PND		PNC	12.01.25	MVK	15130-38808	W2	P1	

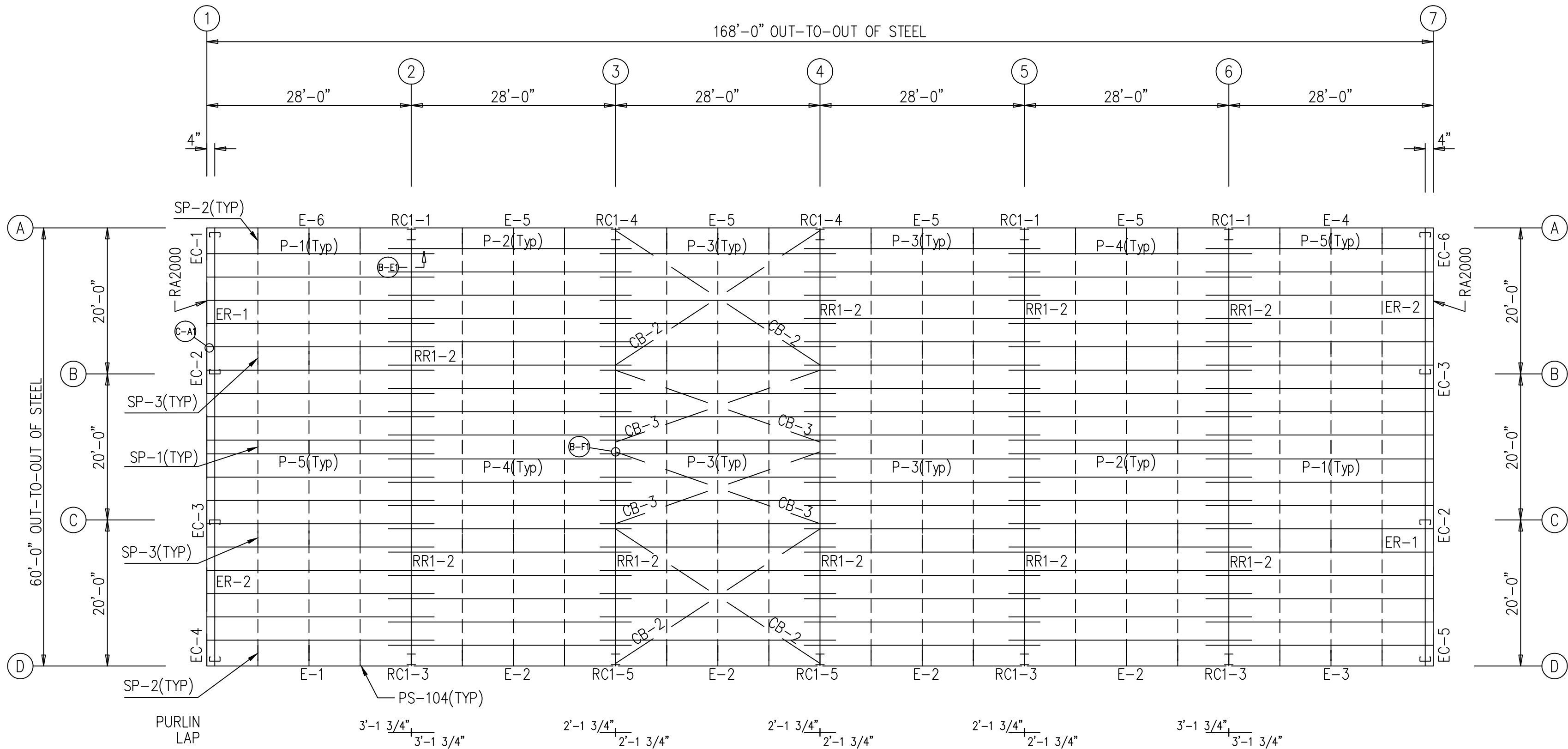
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MEMBER TABLE	
ROOF PLAN	
MARK	PART
P-1	8X25Z14
P-2	8X25Z14
P-3	8X25Z16
P-4	8X25Z14
P-5	8X25Z14
E-1	8ES143
E-2	8ES143
E-3	8ES143
E-4	8ES143
E-5	8ES143
E-6	8ES143
CB-2	0.25_CBL
CB-3	0.25_CBL
SP-1	6X25C14
SP-2	6X25C14
SP-3	6X25C14



ROOF FRAMING PLAN

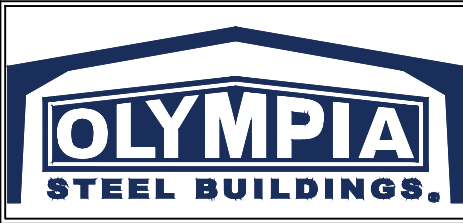
UL580, CLASS 90 CONST. NUMBER 538

NOTE!
For clarity tape sealant, closures, caulking, etc. may not be shown. Refer to the standing Seam Erection Manual for additional installation instructions.

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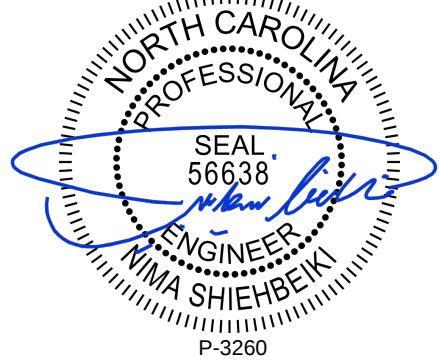
☐ FOR ERECTOR INSTALLATION:
Final drawings for construction.



ISSUE	DATE	DESCRIPTION	BY	CHK	PNC	SHEET DESCRIPTION:	BLDG SIZE:
P1	12.01.25	FOR CONSTRUCTION PERMIT	PND	PNC		ROOF FRAMING PLAN	60'-0" x 168'-0" x 14'-0"
						CUSTOMER:	CUSTOMER LOCATION:
						PROGRESS VENTURES LLC	FUQUAY VARINA, NC 27526
						PROJECT REFERENCE:	
						PROGRESS VENTURES LLC	
						JOB SITE LOCATION:	JOB SITE COUNTY:
						FUQUAY VARINA, NC 27526	WAKE
DWN:	CHK:	DATE:	ENG:	JOB NO:	DWG NO:	ISSUE:	
PND	PNC	12.01.25	MVK	15130-38808	E1	P1	

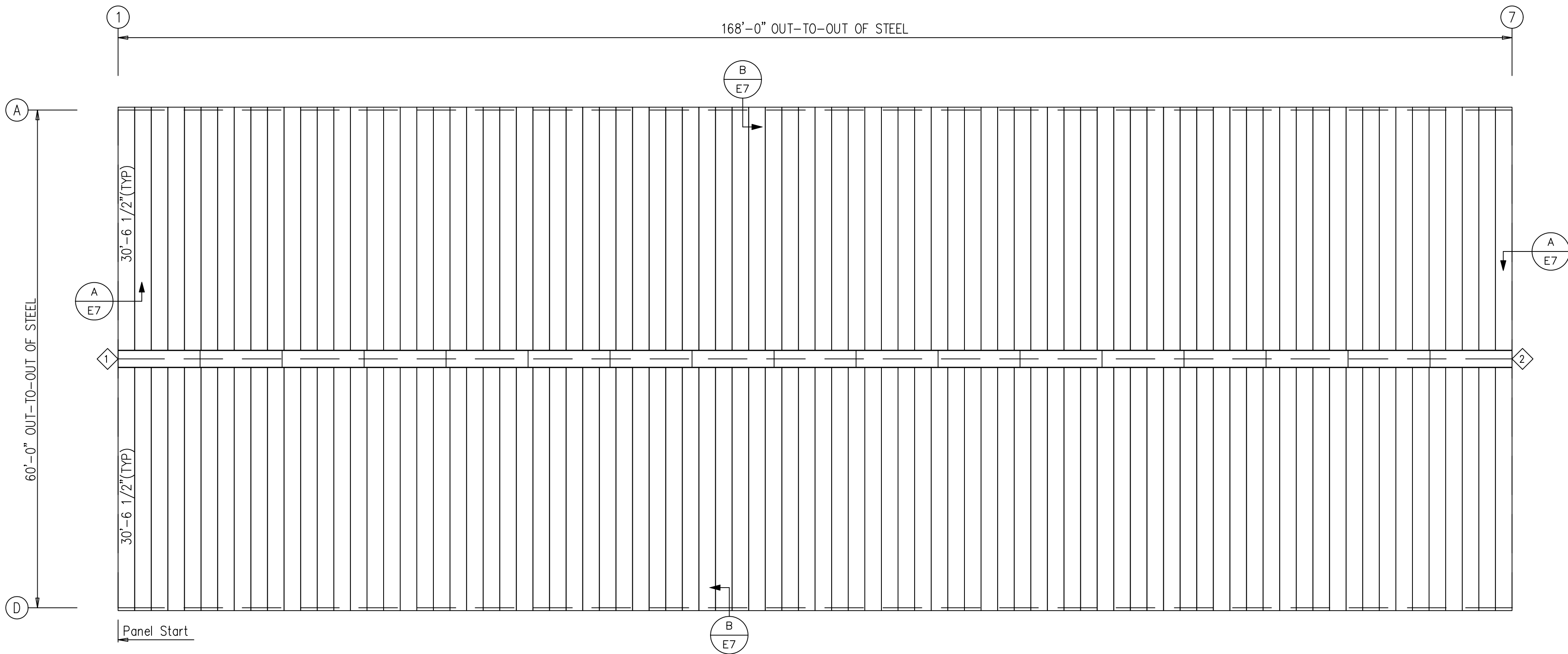
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ROOF SHEETING TRIM TABLE		
◇ID	PART	LENGTH
1	SF-24	10'-3"
2	RC-600	



ROOF SHEETING PLAN

PANELS: 24 Ga. Super Seam-Plus - SMP Polar White

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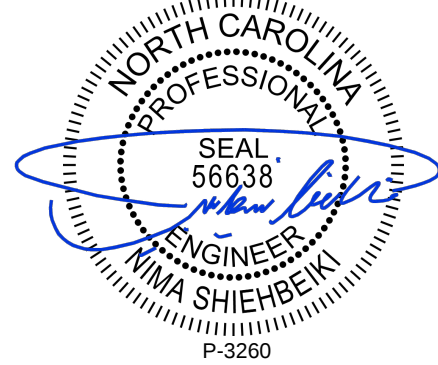
☐ FOR ERECTOR INSTALLATION:
Final drawings for construction.



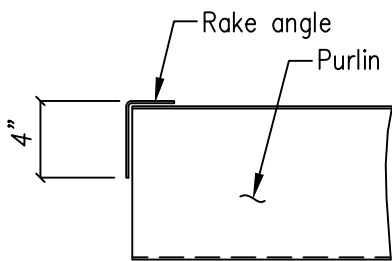
ISSUE	DATE	DESCRIPTION	BY	CHK	SHEET DESCRIPTION:	BLDG SIZE:
P1	12.01.25	FOR CONSTRUCTION PERMIT	PND	PNC	ROOF SHEETING PLAN	60'-0" x 168'-0" x 14'-0"
					CUSTOMER:	CUSTOMER LOCATION:
					PROGRESS VENTURES LLC	FUQUAY VARINA, NC 27526
					PROJECT REFERENCE:	
					PROGRESS VENTURES LLC	
					JOB SITE LOCATION:	JOB SITE COUNTY:
					FUQUAY VARINA, NC 27526	WAKE
					DWN:	CHK:
					PND	PNC
					DATE:	ENG:
					12.01.25	MVK
					JOB NO:	DWG NO:
					15130-38808	E2
					ISSUE:	
					P1	

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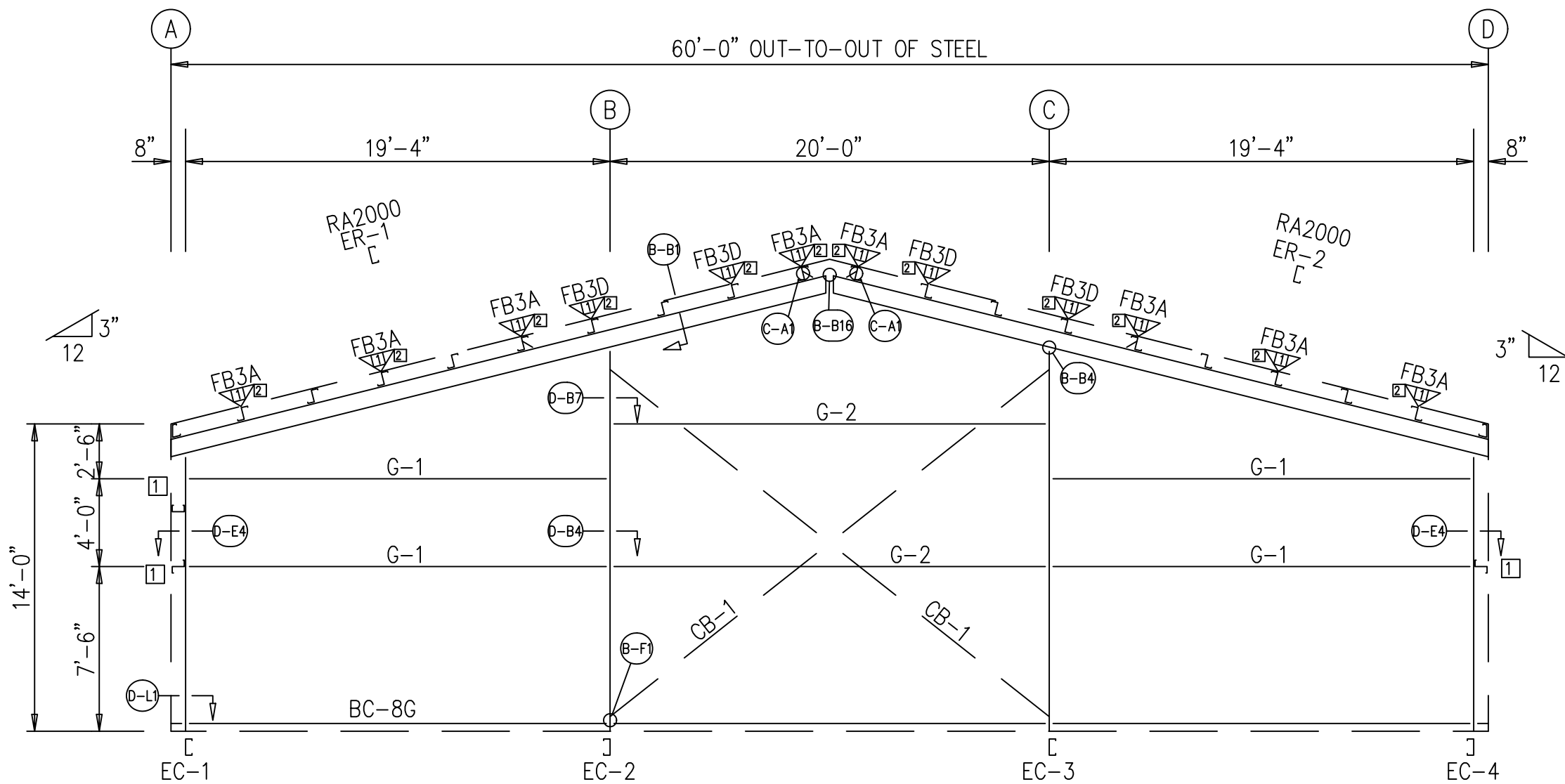


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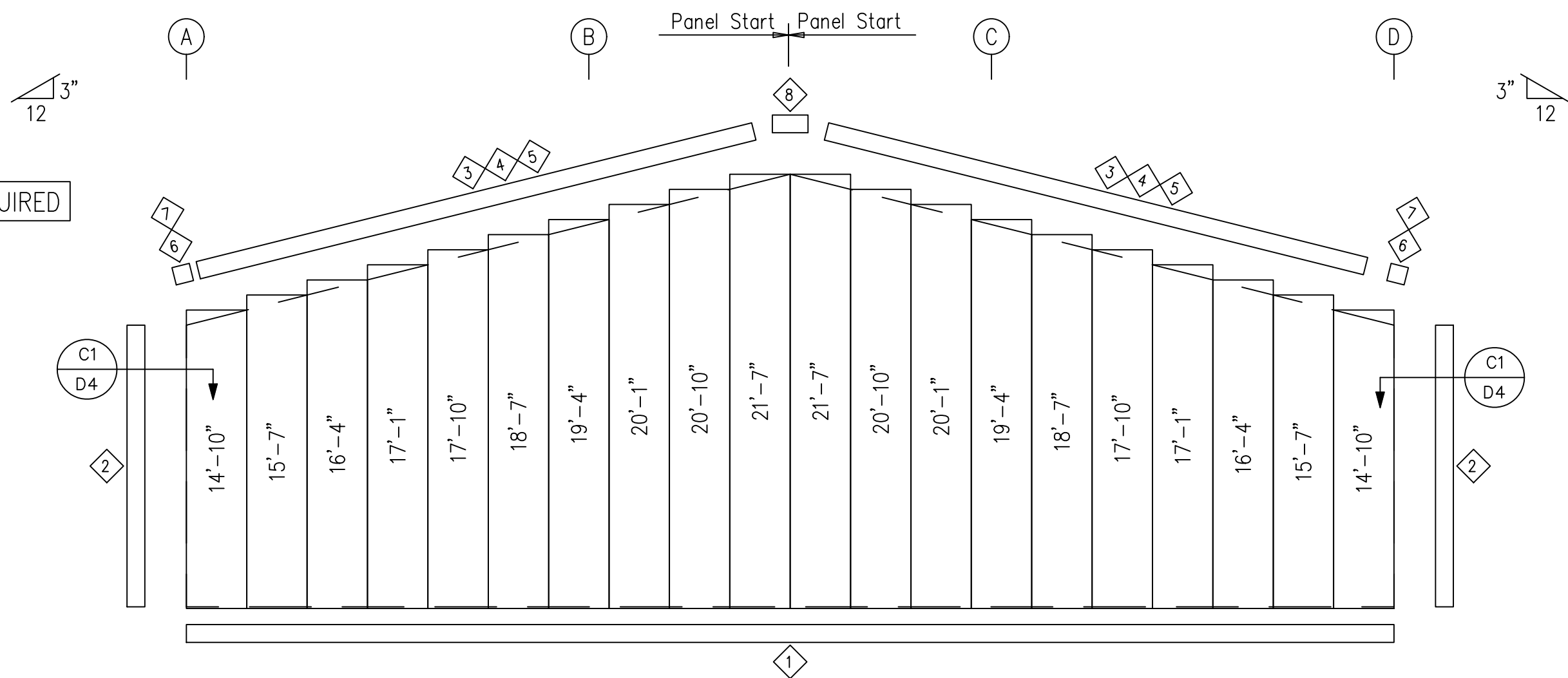


Detail at Rake Angle

D=3"x3"x3/16" FB WITH BACK UP PLATE (AA301)



ENDWALL FRAMING: FRAME LINE 1



ENDWALL SHEETING & TRIM: FRAME LINE 1

PANELS: 26 Ga. Super Span X – SMP Sahara Tan

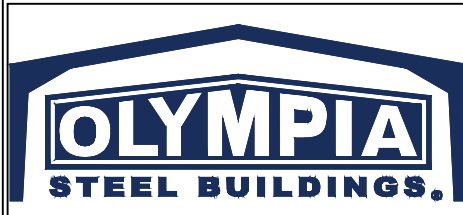
GENERAL SHEETING & TRIM NOTES

1. Refer to erection drawings for rake angle locations.
2. Roof member screws are at 12" o.c. Eave end lap and peak screws are as shown.
3. Wall member screws are at 6" o.c. at the base member and 12" o.c. at all remaining members.
4. Roof stitch screws are located at each member with two between members (20" max. spacing).
5. Wall stitch screws are located at each member with one between members (20" max. spacing).
6. Skylight stitch screws are at 6" o.c.
7. Start endwall panels at centerline of bldg, unless noted.
8. Gutter, rake, & eave trim lap 2". All other trims lap 1".
9. Field cut or lap panels as required to fit.
10. Field cut panels for all openings.
11. Pop rivet gutter counterflashing to wall panel on 3'-0 centers and caulk all laps.
12. Gutter support strap spacing: Super Span 3'-0, Super Seam 4'-0, Weather Lok-16 2'-8".
13. Corner and/or peak boxes are not furnished with special rake or gutter profiles. Field miter as req'd.
14. Downspout straps are located 6" from base and at every girt location.
15. Hot-rolled or built-up members must be pre-drilled before attaching members screws.
16. Metal shavings must be swept from the roof each day to avoid surface rusting.
17. Windows and louvers must be installed before sheeting the walls.
18. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.

GENERAL FRAMING NOTES

1. Angles are marked by their length in feet and inches.
2. Field cut or lap angles as required to fit.
3. Flange braces are marked by their length in decimal inches.
4. Outside flange of girt turns down unless noted.
5. Endwall girts and eave struts do not lap.
6. Field cut and self-top girts at walk doors.
7. Field slot girts for brace rods or cables.
8. Field locate windows and walk doors.
9. Field weld all splices at 14 gauge valley gutters.
10. Field bolt AK400 base clip to endwall columns:
 - (1) 5/8" x 1-1/2" A325 bolts if (1) AK400 req'd
 - (2) 5/8" x 1-3/4" A325 bolts if (2) AK400 req'd
11. Locate top of roof framed openings flush with the pan of the roof panel.
12. Some field drilling at framed openings may be required. Field drill 9/16" diameter holes.
13. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.
14. Sub-jambs for overhead doors, if required, is not furnished by Metal Building Provider

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- ☒ FOR CONSTRUCTION PERMIT:
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- ☐ FOR ERECTOR INSTALLATION:
Final drawings for construction.



ISSUE	DATE	DESCRIPTION	BY	CHK
P1	12.01.25	FOR CONSTRUCTION PERMIT	PND	PNC

SHEET DESCRIPTION:		BLDG SIZE:	
ENDWALL FRAME & SHEETING ELEVATION		60'-0" x 168'-0" x 14'-0"	
CUSTOMER:		CUSTOMER LOCATION:	
PROGRESS VENTURES LLC		FUQUAY VARINA, NC 27526	
PROJECT REFERENCE:		JOB SITE LOCATION:	
PROGRESS VENTURES LLC		FUQUAY VARINA, NC 27526	
JOB SITE LOCATION:		WAKE	
DWN:	CHK:	DATE:	ENG:
PND	PNC	12.01.25	MVK
JOB NO:		DWG NO:	
15130-38808		E3	
ISSUE:		P1	

BOLT TABLE FRAME LINE 1				
LOCATION	QUAN	TYPE	DIA	LENGTH
ER-1/ER-2	4	A325	5/8"	2"
Columns/Raf	4	A325	5/8"	1 1/2"

FLANGE BRACE TABLE FRAME LINE 7		
VID	MARK	LENGTH
1	FB3A	2'-0"
2	FB3D	2'-0"

CONNECTION PLATES FRAME LINE 1	
ID	MARK/PART
1	AK226
2	AK230

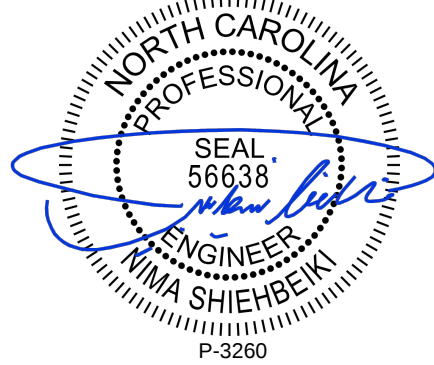
MEMBER TABLE FRAME LINE 1	
MARK	PART
EC-1	10M35C14
EC-2	10M35C12
EC-3	10M35C12
EC-4	10M35C14
ER-1	12M35C12
ER-2	12M35C12
G-1	8X25Z14
G-2	8X25Z12
CB-1	0.25_CBL

TRIM TABLE – THIS WALL ONLY FRAME LINE – 1		
VID	PART	LENGTH
1	BT-101	10'-3"
2	CT-102	14'-4"
3	RT-540	20'-3"
4	RT-540	15'-3"
5	RS-501	10'-3"
6	RT-508	
7	SCB	
8	SSPB	

CONNECTION PLATES FRAME LINE 1	
ID	MARK/PART
1	SC-5

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P-3260



1. Refer to erection drawings for rake angle locations.
2. Roof member screws are at 12" o.c. Eave end lap and peak screws are as shown.
3. Wall member screws are at 6" o.c. at the base member and 12" o.c. at all remaining members.
4. Roof stich screws are located at each member with two between members (20" max. spacing).
5. Wall stich screws are located at each member with one between members (20" max. spacing).
6. Skylight stich screws are at 6" o.c.
7. Start endwall panels at centerline of bldg. unless noted.
8. Gutter, rake, & eave trim lap 2". All other trims lap 1".
9. Field cut or lap panels as required to fit.
10. Field cut panels at all openings.
11. Pop rivet gutter counterflashing to wall panel on 3'-0" centers and caulk all laps.
12. Gutter support strap spacing: Super Seam 3'-0", Super Seam 4'-0", Weather Lok-16 2'-8".
13. Corner and/or peak screws are not furnished with special rake or gully profiles. Field mitre as req'd.
14. Downspout straps are located 6" from base and at every gully location.
15. Hot-rolled or built-up members must be pre-drilled before attaching members screws.
16. Metal shavings must be swept from the roof each day to avoid surface rusting.
17. Windows and louvers must be installed before sheeting the walls.
18. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.

PANELS: 26 Ga. Super Span X – SMP Sahara Tan

1. Angles are marked by their length in feet and inches.
2. Field cut or lap angles as required to fit.
3. Flange braces are marked by their length in decimal inches.
4. Outside flange of girt turns down unless noted.
5. Endwall girts and eave struts do not lap.
6. Field cut and self-tap girts at walk doors.
7. Field slot girts for brace rods or cables.
8. Field locate windows and walk doors.
9. Field weld all splices at 14 gauge valley gutters.
10. Field bolt AK400 base clip to endwall columns:
 - (1) 5/8" x 1-1/2" A325 bolts if (1) AK400 req'd
 - (2) 5/8" x 1-3/4" A325 bolts if (2) AK400 req'd
11. Locate top of roof framed openings flush with the pan of the roof panel.
12. Some field drilling at framed openings may be required. Field drill 9/16" diameter holes.
13. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.
14. Sub-jams for overhead doors, if required, is not furnished by Metal Building Provider



ISSUE	DATE	DESCRIPTION	BY	CHK
P1	12.01.25	FOR CONSTRUCTION PERMIT	PND	PNC

SHEET DESCRIPTION: ENDWALL FRAME & SHEETING ELEVATION				BLDG SIZE: 60'-0" x 168'-0" x 14'-0"			
CUSTOMER: PROGRESS VENTURES LLC				CUSTOMER LOCATION: FUQUAY VARINA, NC 27526			
PROJECT REFERENCE: PROGRESS VENTURES LLC							
JOBSITE LOCATION: FUQUAY VARINA, NC 27526						JOBSITE COUNTY:	
DWN:	CHK:	DATE:	ENG:	JOB NO:	DWG NO:	ISSUE:	
PMD	PNC	12.01.25	MWK	15130-3680R	E4	P1	

BOLT TABLE				
FRAME LINE 7				
LOCATION	QUAN	TYPE	DIA	LENGTH
ER-1/ER-2	4	A325	5/8"	2"
Columns/Raf	4	A325	5/8"	1 1/2"

FLANGE BRACE TABLE		
FRAME LINE 7		
▽ID	MARK	LENGTH
1	FB3A	2'-0"
2	FB3D	2'-0"

CONNECTION PLATES	
FRAME LINE 7	
☐ ID	MARK/PART
1	AK226
2	AK230

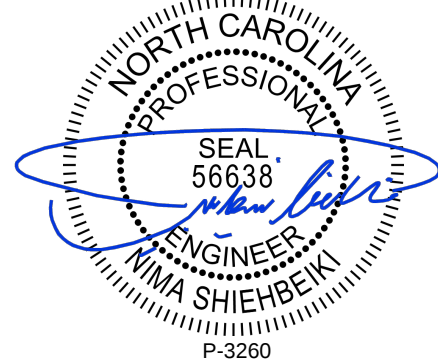
MEMBER TABLE	
FRAME LINE 7	
MARK	PART
EC-2	10M35C12
EC-3	10M35C12
EC-5	10M35C14
EC-6	10M35C14
ER-1	12M35C12
ER-2	12M35C12
G-1	8X25Z14
G-2	8X25Z12
CB-1	0.25_CBL

◇ID	PART	LENGTH
1	BT-101	10'-3"
2	CT-102	14'-4"
3	RT-540	20'-3"
4	RT-540	15'-3"
5	RS-501	10'-3"
6	RT-508	
7	SCB	
8	SSPB	

CONNECTION PLATES	
FRAME LINE 7	
☐ ID	MARK/PART
1	SC-5

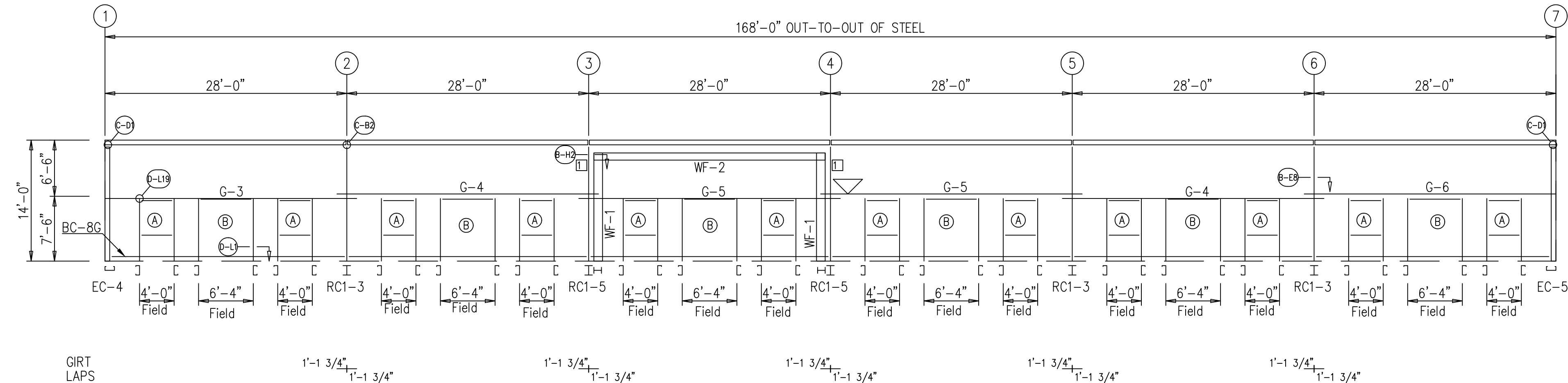
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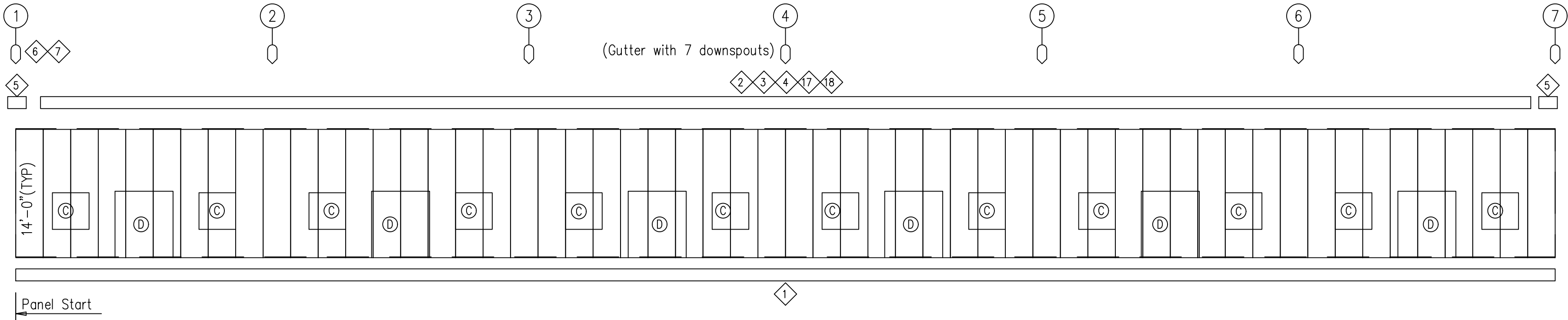


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DOWNSPOUT LOCATIONS



SIDEWALL FRAMING: FRAME LINE D



SIDEWALL SHEETING & TRIM: FRAME LINE D

PANELS: 26 Ga. Super Span X - SMP Sahara Tan

GENERAL SHEETING & TRIM NOTES

- Refer to erection drawings for rake angle locations.
- Roof member screws are at 12" o.c. Eave end lap and peak screws are as shown.
- Wall member screws are at 6" o.c. at the base member and 12" o.c. at all remaining members.
- Roof stitch screws are located at each member with two between members (20" max. spacing).
- Wall stitch screws are located at each member with one between members (20" max. spacing).
- Skylight stitch screws are at 6" o.c.
- Start endwall panels at centerline of bldg. unless noted.
- Gutter, rake, & eave trim lap 2". All other trims lap 1".
- Field cut or lap panels as required to fit.
- Field cut panels for all openings.
- Pop rivet gutter counterflashing to wall panel on 3'-0 centers and caulk all laps.
- Gutter support strap spacing: Super Span 3'-0, Super Seam 4'-0, Weather Lok-16 2'-8".
- Corner and/or peak boxes are not furnished with special rake or gutter profiles. Field miter as req'd.
- Downspout straps are located 6" from base and at every girt location.
- Hot-rolled or built-up members must be pre-drilled before attaching members screws.
- Metal shavings must be swept from the roof each day to avoid surface rusting.
- Windows and louvers must be installed before sheeting the walls.
- For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.

GENERAL FRAMING NOTES

- Angles are marked by their length in feet and inches.
- Field cut or lap angles as required to fit.
- Flange braces are marked by their length in decimal inches.
- Outside flange of girt turns down unless noted.
- Endwall girts and eave struts do not lap.
- Field cut and self-tap girts at walk doors.
- Field slot girts for brace rods or cables.
- Field locate windows and walk doors.
- Field weld all splices at 14 gauge valley gutters.
- Field bolt AK400 base clip to endwall columns:
(1) 5/8" x 1-1/2" A325 bolts if (1) AK400 req'd
(2) 5/8" x 1-3/4" A325 bolts if (2) AK400 req'd
- Locate top of roof framed openings flush with the pan of the roof panel.
- Some field drilling at framed openings may be required. Field drill 9/16" diameter holes.
- For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.
- Sub-jams for overhead doors, if required, is not furnished by Metal Building Provider

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☐ FOR ERECTOR INSTALLATION:
Final drawings for construction.



ISSUE	DATE	DESCRIPTION	BY	CHK	CHK	SHEET DESCRIPTION:	BLDG SIZE:
P1	12.01.25	FOR CONSTRUCTION PERMIT	PND	PNC		SIDEWALL FRAME & SHEETING ELEVATION	60'-0" x 168'-0" x 14'-0"
						CUSTOMER:	CUSTOMER LOCATION:
						PROGRESS VENTURES LLC	FUQUAY VARINA, NC 27526
						PROJECT REFERENCE:	
						PROGRESS VENTURES LLC	
						JOB SITE LOCATION:	JOB SITE COUNTY:
						FUQUAY VARINA, NC 27526	WAKE
						DWN:	WAKE
						CHK:	DWG NO:
						PNC	E5
						DATE:	ISSUE:
						12.01.25	P1
						ENG:	
						MVK	
						JOB NO:	
						15130-38808	

BOLT TABLE FRAME LINE D				
LOCATION	QUAN	TYPE	DIA	LENGTH
WF-1 - WF-2	8	A325	3/4"	2"
WF-1 - RC1-3	8	A325	5/8"	1 3/4"

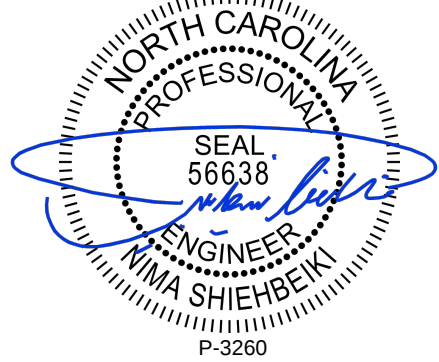
MEMBER TABLE FRAME LINE D	
MARK	PART
WF-1	W12841
WF-2	W10651
DJ-1	8M25C14
DJ-2	8M25C14
PM106-1	PM106
PM106-2	PM106
DS-1	8M25C14
G-3	8X25Z12
G-4	8X25Z12
G-5	8X25Z14
G-6	8X25Z12

CONNECTION PLATES FRAME LINE D	
ID	MARK/PART
1	AK508

TRIM TABLE - THIS WALL ONLY FRAME LINE -D		
ID	PART	LENGTH
1	BT-101	10'-3"
2	GU-523	20'-3"
3	ET-803	20'-3"
4	GS-501	
5	EC-523	
6	DS-101	13'-8"
7	DS-105	
8	FL-22	4'-4"
9	MT-116B	4'-4"
10	HT-101	4'-4"
11	MT-216B	4'-4"
12	FL-26	4'-4"
13	FL-22	7'-6"
14	MT-116B	7'-6"
15	HT-101	6'-8"
16	MT-216B	6'-8"
17	GU-523	15'-3"
18	ET-803	15'-3"

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P-3260

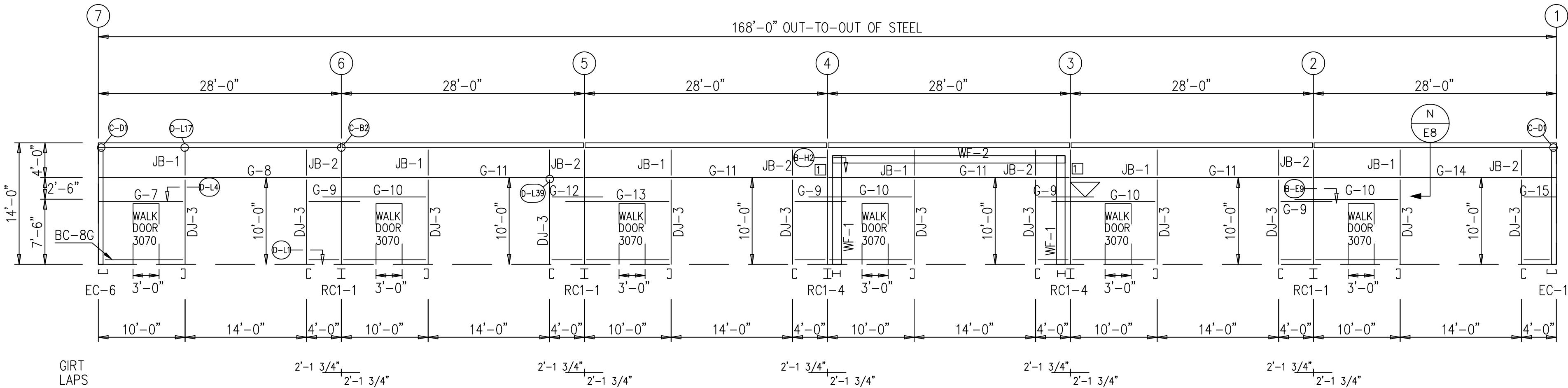
DOWNSPOUT LOCATIONS

BOLT TABLE FRAME LINE A				
LOCATION	QUAN	TYPE	DIA	LENGTH
WF-1 - WF-2	8	A325	3/4"	2"
WF-1 - RC1-1	8	A325	5/8"	1 3/4"

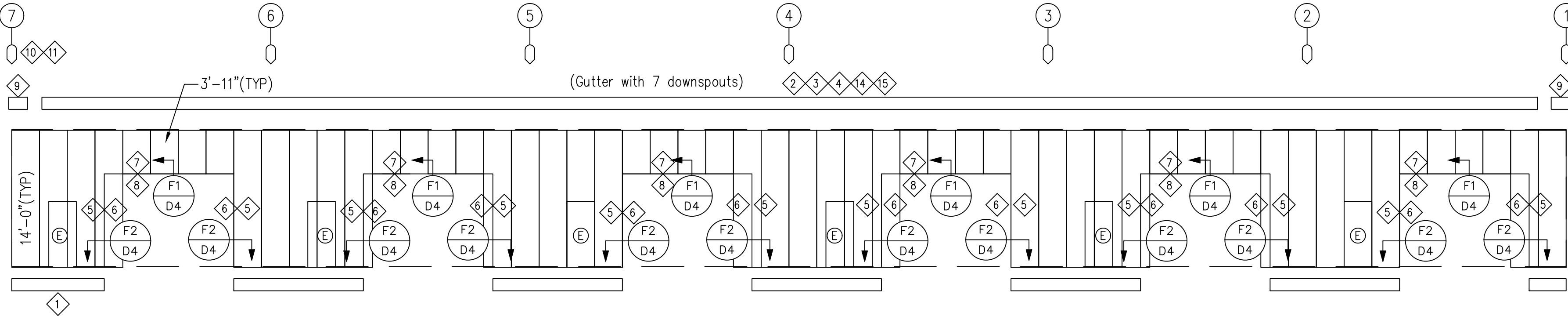
MEMBER TABLE FRAME LINE A	
MARK	PART
WF-1	W12841
WF-2	W10651
DJ-3	8M35C14
JB-1	8M35C14
JB-2	8M35C14
G-7	8X25Z16
G-8	8X25C12
G-9	8X25Z14
G-10	8X25Z14
G-11	8X25C14
G-12	8X25Z16
G-13	8X25Z16
G-14	8X25C12
G-15	8X25Z16

CONNECTION PLATES FRAME LINE A	
ID	MARK/PART
1	AK508

TRIM TABLE - THIS WALL ONLY FRAME LINE - A		
ID	PART	LENGTH
1	BT-101	10'-3"
2	GU-523	20'-3"
3	ET-803	20'-3"
4	GS-501	
5	MT-116B	10'-4"
6	FL-22	10'-4"
7	MT-116B	14'-4"
8	HT-101	14'-4"
9	EC-523	
10	DS-101	13'-8"
11	DS-105	
12	FL-22	7'-6"
13	HT-101	3'-8"
14	GU-523	15'-3"
15	ET-803	15'-3"



SIDEWALL FRAMING: FRAME LINE A



SIDEWALL SHEETING & TRIM: FRAME LINE A

PANELS: 26 Ga. Super Span X - SMP Sahara Tan

GENERAL SHEETING & TRIM NOTES

- Refer to erection drawings for rake angle locations.
- Roof member screws are at 12" o.c. Eave end lap and peak screws are as shown.
- Wall member screws are at 6" o.c. at the base member and 12" o.c. at all remaining members.
- Roof stitch screws are located at each member with two between members (20" max. spacing).
- Wall stitch screws are located at each member with one between members (20" max. spacing).
- Skylight stitch screws are at 6" o.c.
- Start endwall panels at centerline of bldg. unless noted.
- Gutter, rake, & eave trim lap 2". All other trims lap 1".
- Field cut or lap panels as required to fit.
- Field cut panels for all openings.
- Pop rivet gutter counterflashing to wall panel on 3'-0" centers and caulk all laps.
- Gutter support strap spacing: Super Span 3'-0", Super Seam 4'-0", Weather Lok-16 2'-8".
- Corner and/or peak boxes are not furnished with special rake or gutter profiles. Field miter as req'd.
- Downspout straps are located 6" from base and at every girt location.
- Hot-rolled or built-up members must be pre-drilled before attaching members screws.
- Metal shavings must be swept from the roof each day to avoid surface rusting.
- Windows and louvers must be installed before sheeting the walls.
- For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.

GENERAL FRAMING NOTES

- Angles are marked by their length in feet and inches.
- Field cut or lap angles as required to fit.
- Flange braces are marked by their length in decimal inches.
- Outside flange of girt turns down unless noted.
- Endwall girts and eave struts do not lap.
- Field cut and self-tap girts at walk doors.
- Field slot girts for brace rods or cables.
- Field locate windows and walk doors.
- Field weld all splices at 14 gauge valley gutters.
- Field bolt AK400 base clip to endwall columns:
(2) 5/8" x 1-1/2" A325 bolts if (1) AK400 req'd
(2) 5/8" x 1-3/4" A325 bolts if (2) AK400 req'd
- Locate top of roof framed openings flush with the pan of the roof panel.
- Some field drilling at framed openings may be required. Field drill 9/16" diameter holes.
- For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.
- Sub-jams for overhead doors, if required, is not furnished by Metal Building Provider

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- ☐ FOR ERECTOR INSTALLATION: Final drawings for construction.

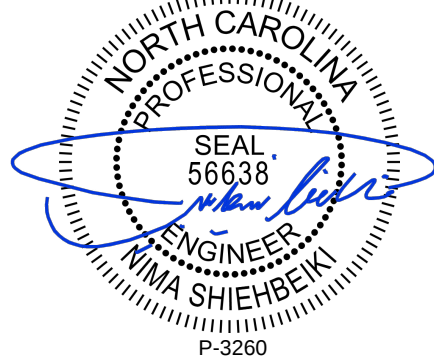


ISSUE	DATE	DESCRIPTION	BY	CHK
P1	12.01.25	FOR CONSTRUCTION PERMIT	PND	PNC

SHEET DESCRIPTION:		BLDG SIZE:	
SIDEWALL FRAME & SHEETING ELEVATION		60'-0" x 168'-0" x 14'-0"	
CUSTOMER:		CUSTOMER LOCATION:	
PROGRESS VENTURES LLC		FUQUAY VARINA, NC 27526	
PROJECT REFERENCE:		PROJECT:	
PROGRESS VENTURES LLC			
JOBSITE LOCATION:		JOBSITE COUNTY:	
FUQUAY VARINA, NC 27526		WAKE	
DWN:	CHK:	DATE:	ENG:
PND	PNC	12.01.25	MVK
JOB NO:	DWG NO:	ISSUE:	
15130-38808	E6	P1	

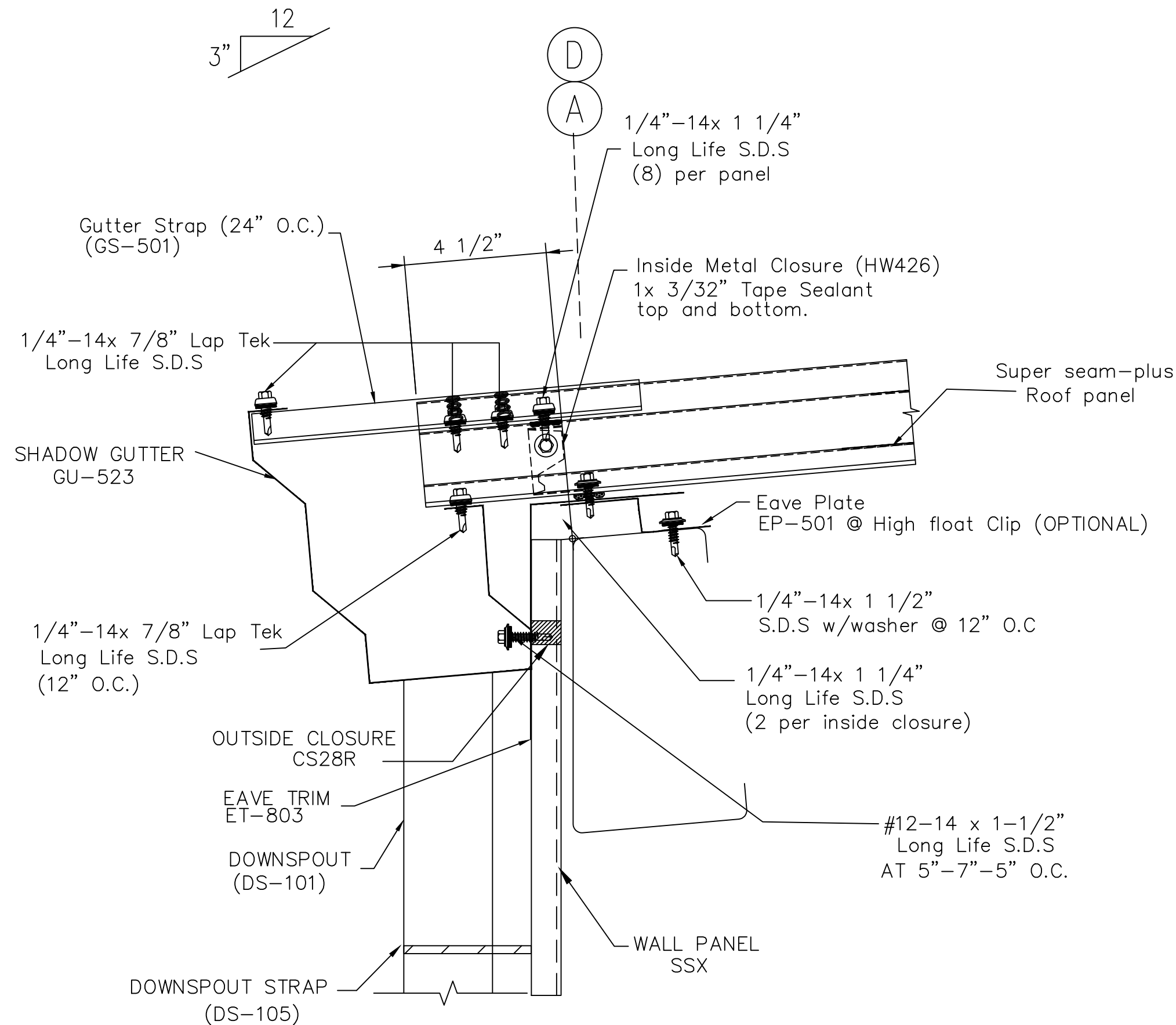
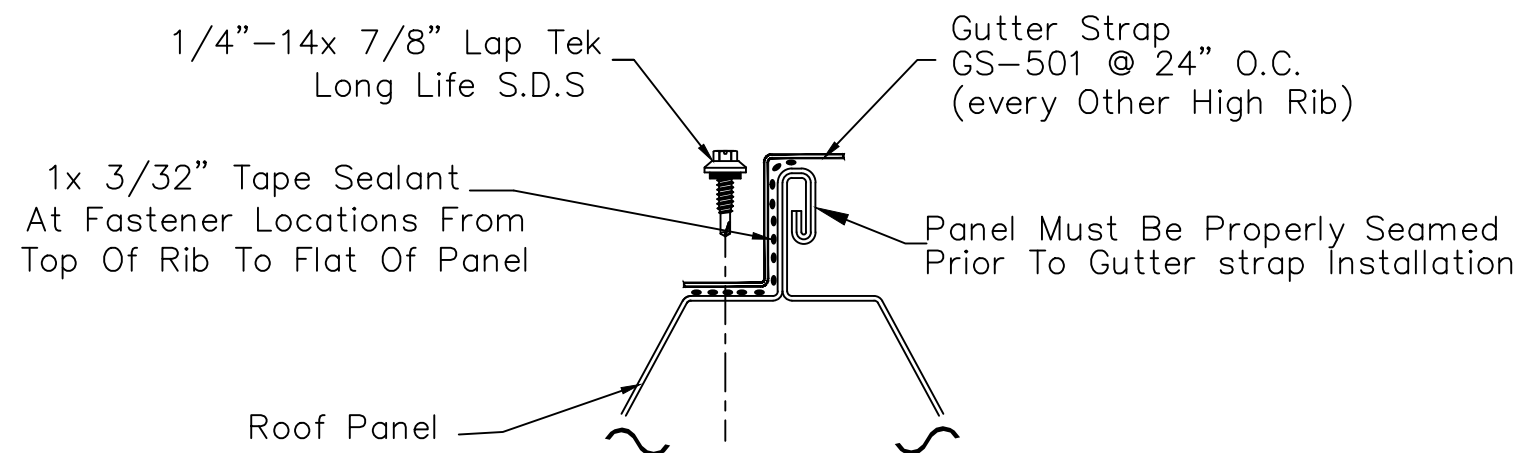
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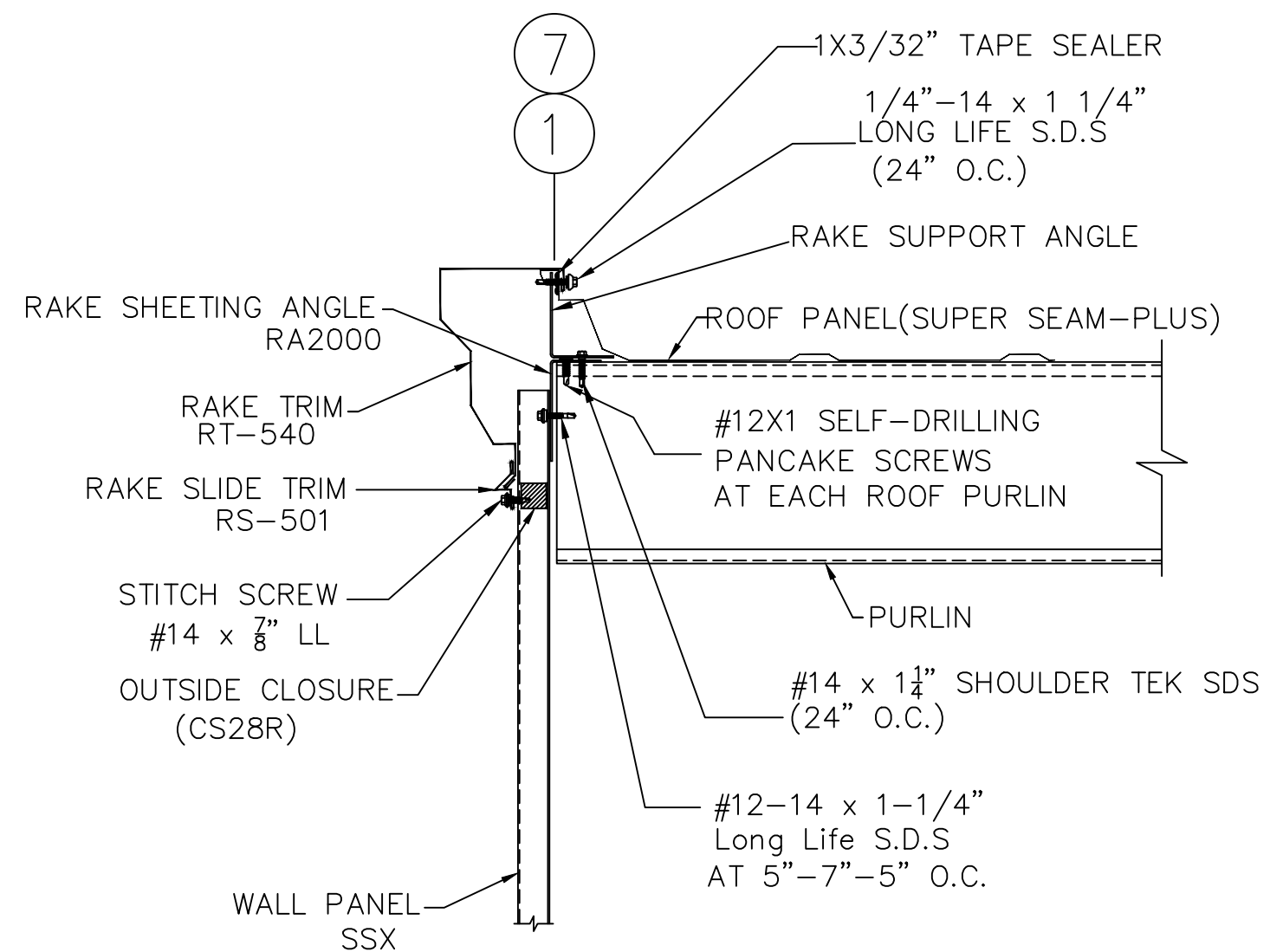


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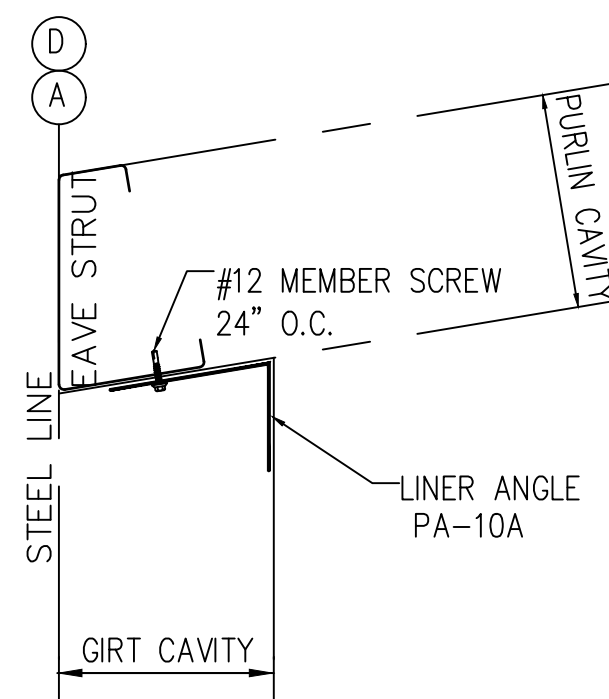
P-3260



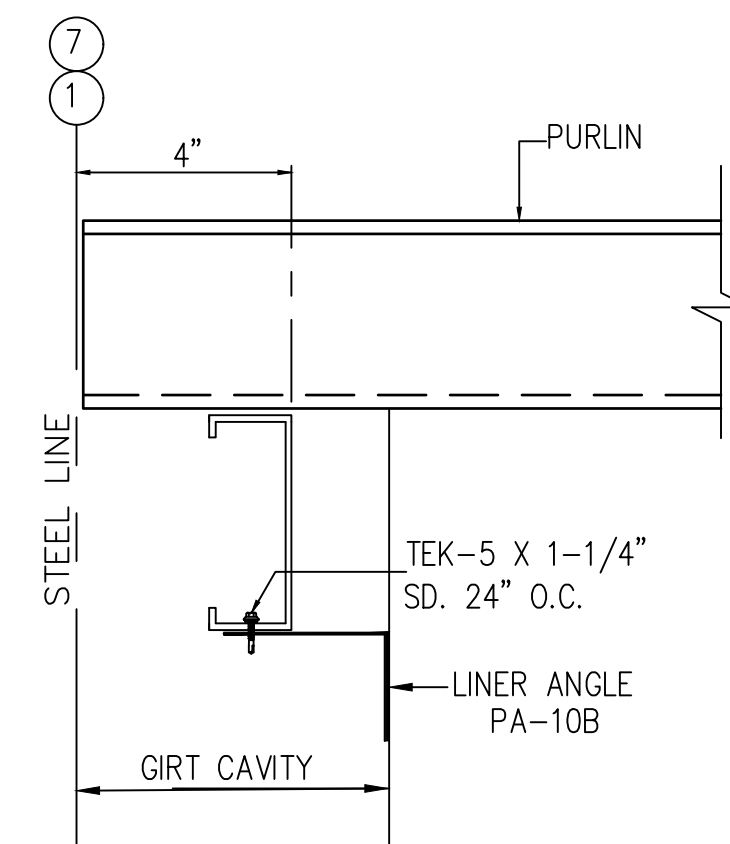
SECTION "B"



SECTION "A"



LINER ANGLE AT LOW-SIDE
EAVE STRUT



LINER ANGLE AT ENDWALL WITH
4" SETBACK

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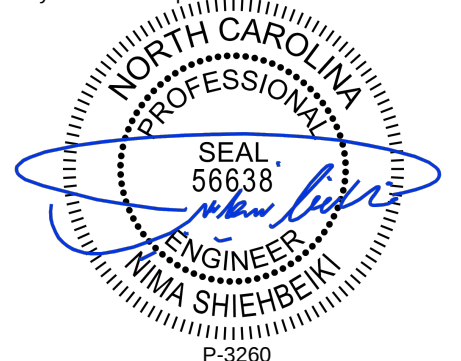
☐ FOR ERECTOR INSTALLATION:
Final drawings for construction.



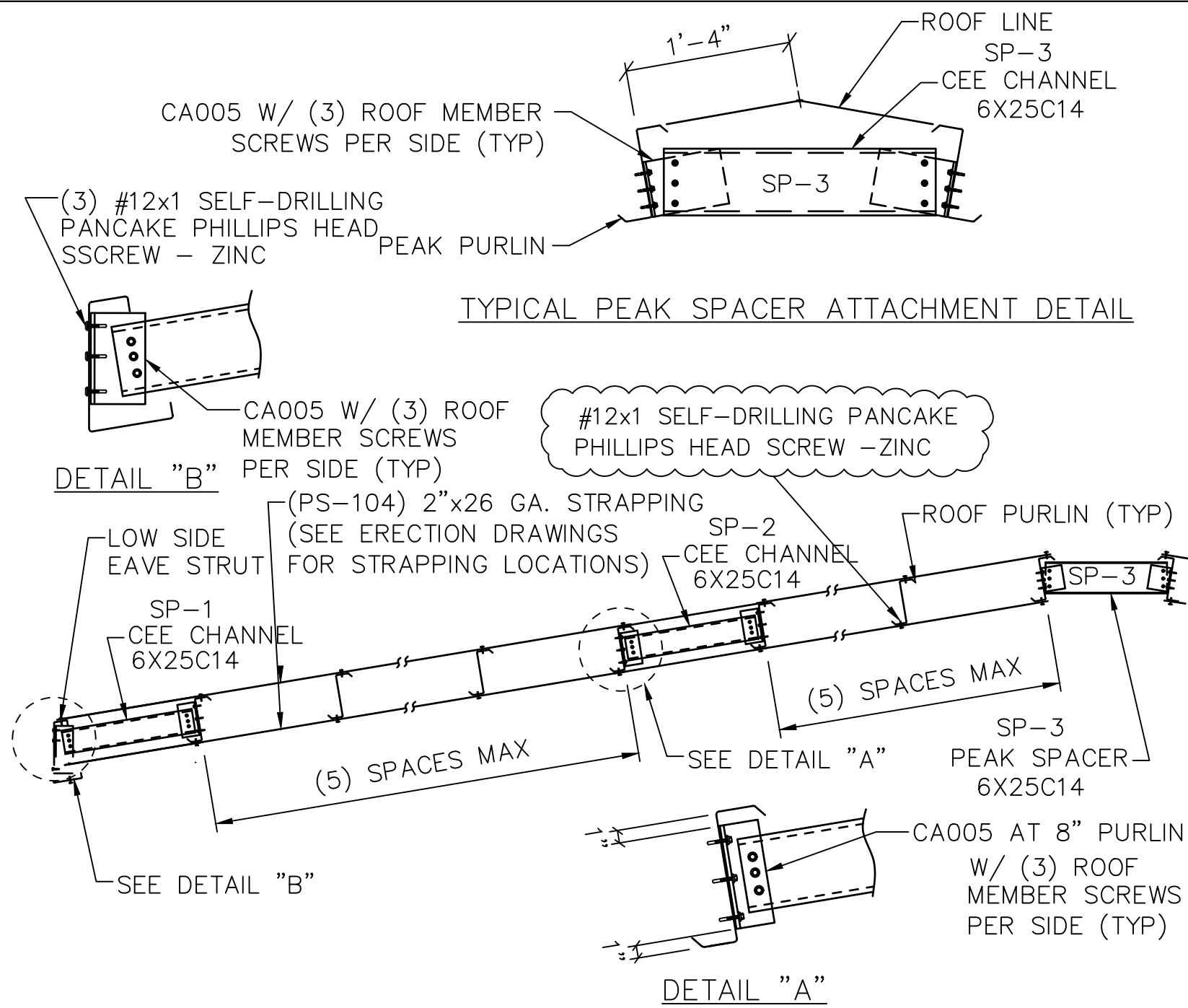
ISSUE	DATE	DESCRIPTION	BY	CHK	SHEET DESCRIPTION:	BLDG SIZE:
P1	12.01.25	FOR CONSTRUCTION PERMIT	PND	PNC	BUILDING SECTIONS	60'-0" x 168'-0" x 14'-0"
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					PROGRESS VENTURES LLC	FUQUAY VARINA, NC 27526
					PROJECT REFERENCE:	
					PROGRESS VENTURES LLC	
					JOB SITE LOCATION:	JOB SITE COUNTY:
					FUQUAY VARINA, NC 27526	WAKE
					DWN:	CHK:
					PND	PNC
					DATE:	ENG:
					12.01.25	MVK
					JOB NO:	DWG NO:
					15130-38808	E7
					ISSUE:	
					P1	

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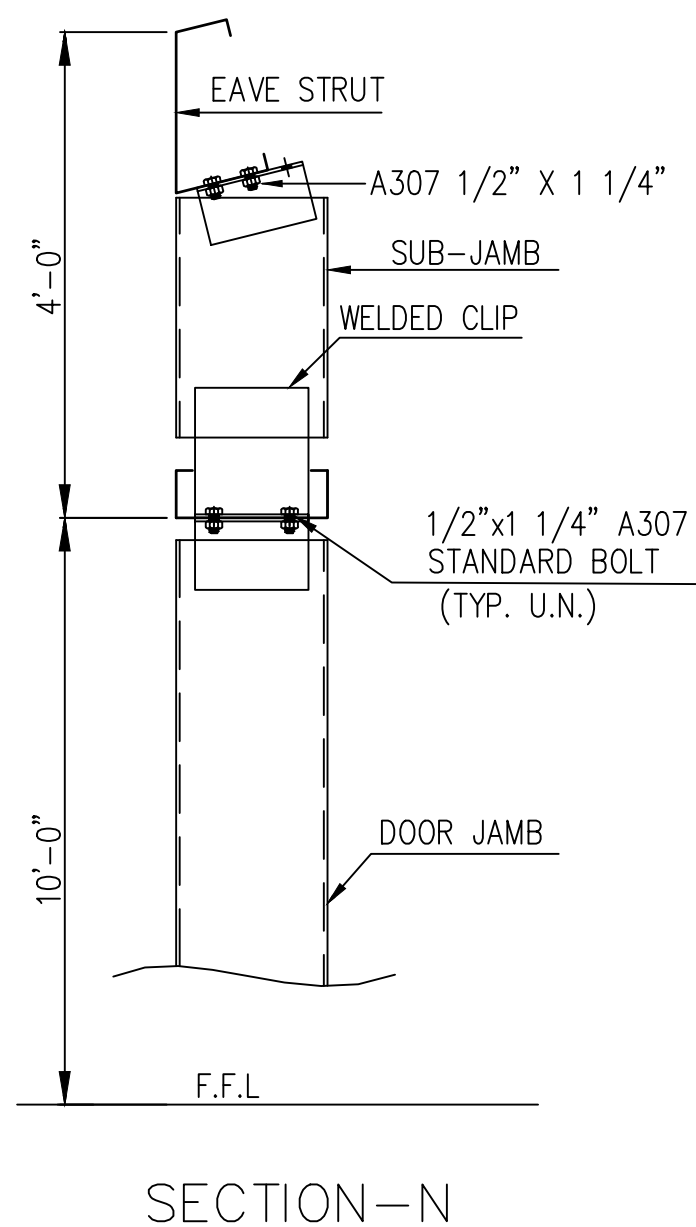
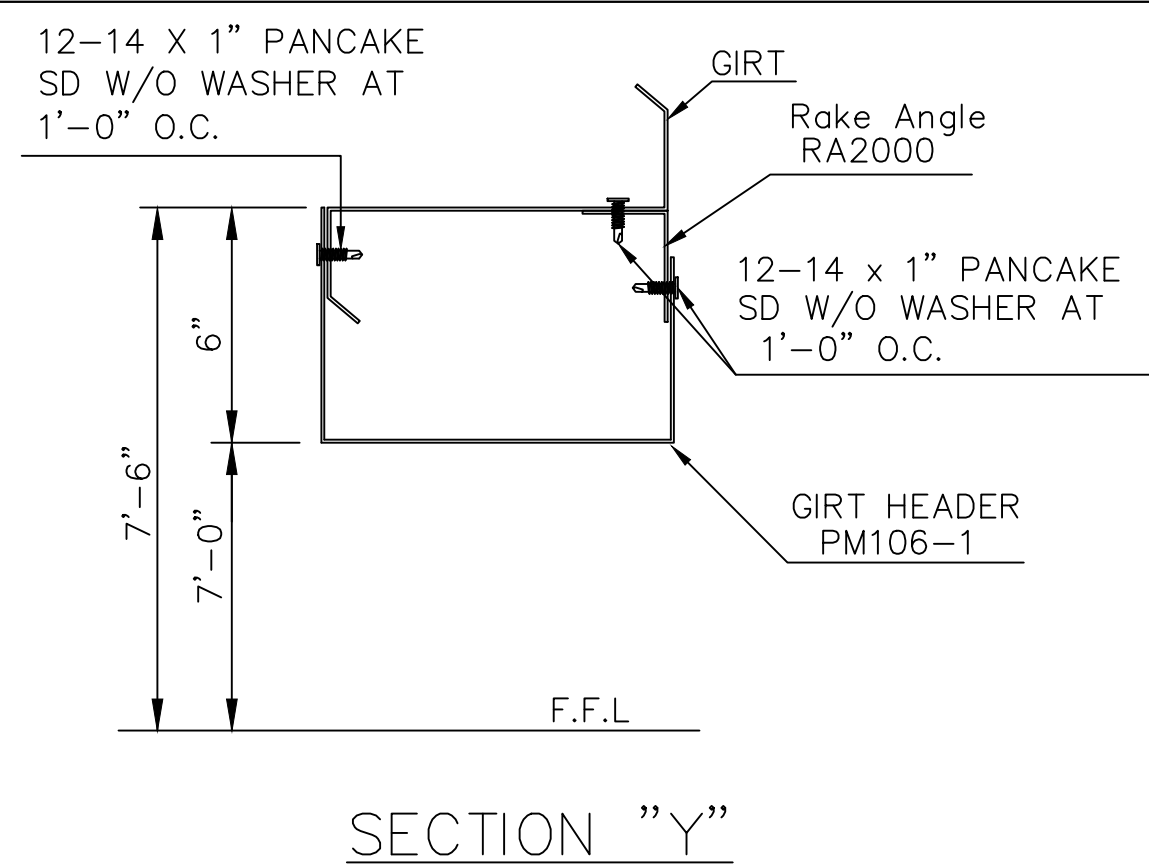
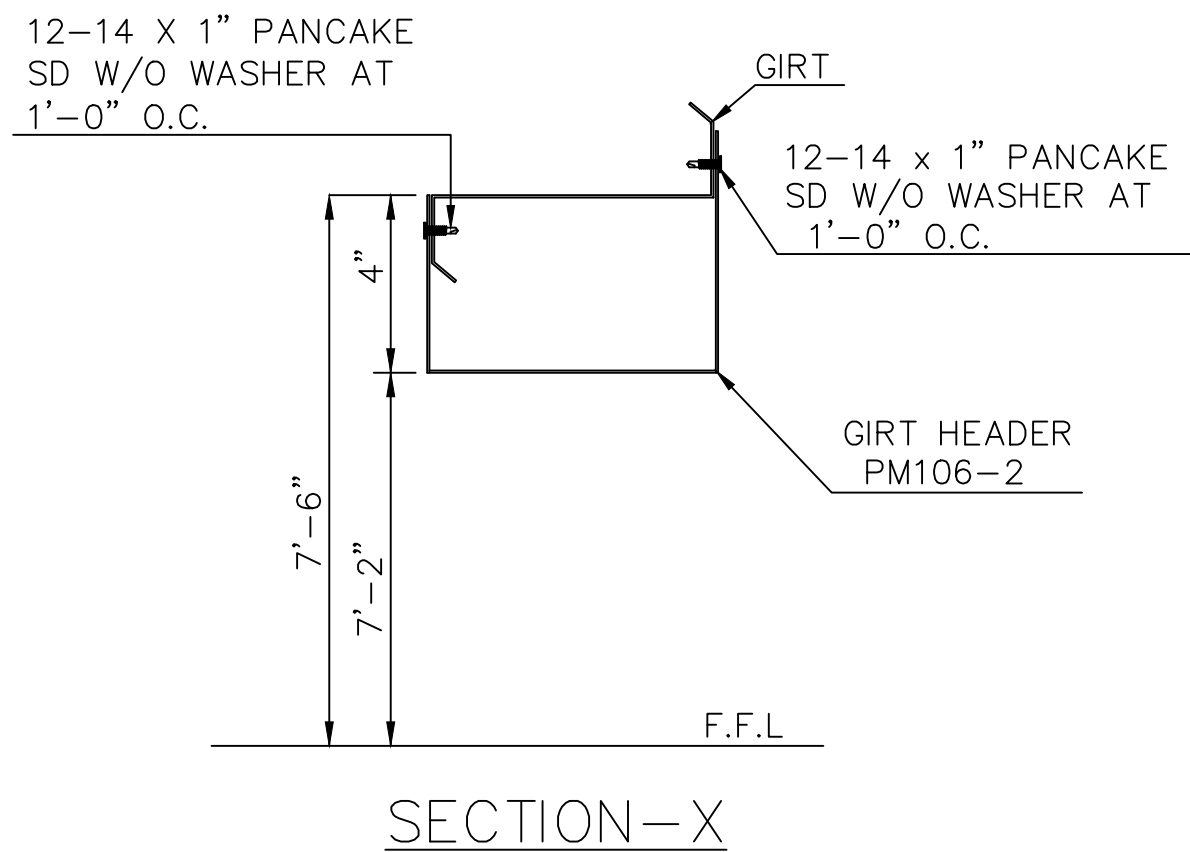
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STANDARD ROOF STRAPPING DETAIL STANDARD SEAM ROOF WITHOUT LINER - GABLE (WITH R-30 INSULATION)



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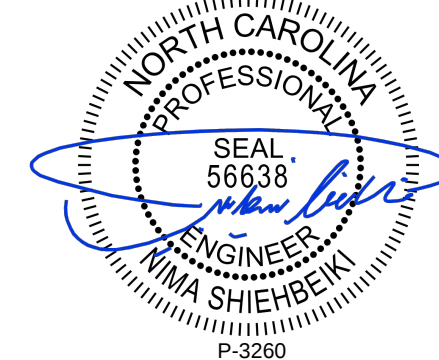
☐ FOR ERECTOR INSTALLATION: Final drawings for construction.



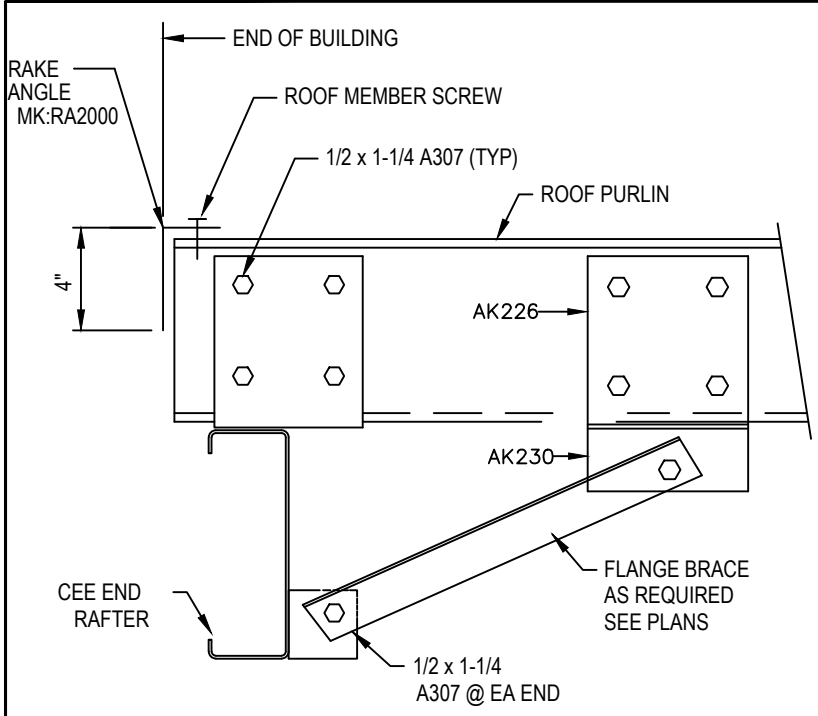
ISSUE	DATE	DESCRIPTION	BY	CHK	SHEET DESCRIPTION:	BLDG SIZE:
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					CUSTOMER: PROGRESS VENTURES LLC	CUSTOMER LOCATION: FUQUAY VARINA, NC 27526
					PROJECT REFERENCE: PROGRESS VENTURES LLC	
					JOB SITE LOCATION: FUQUAY VARINA, NC 27526	JOB SITE COUNTY: WAKE
					DWN: PND	CHK: PNC
					DATE: 12.01.25	ENG: MVK
					JOB NO: 15130-38808	DWG NO: E8
						ISSUE: P1

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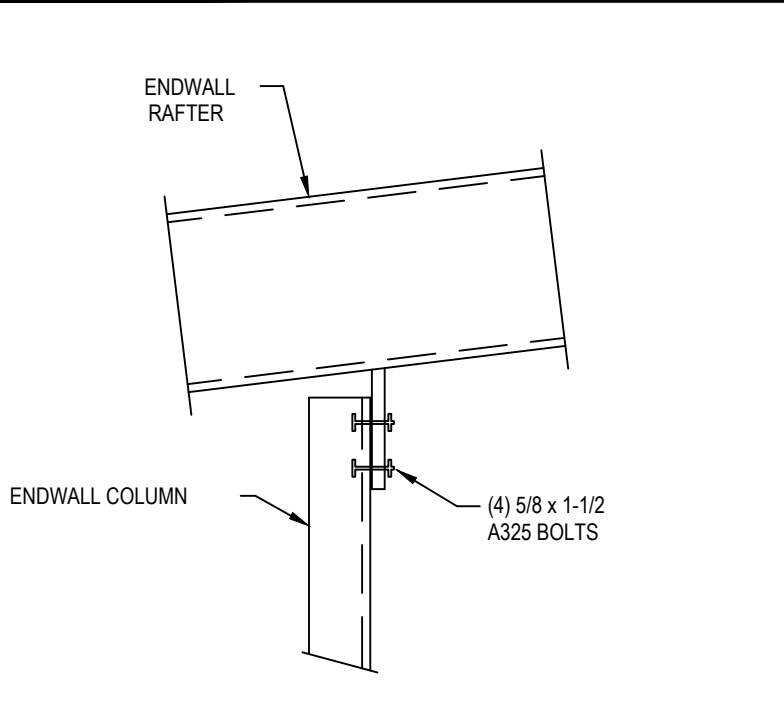
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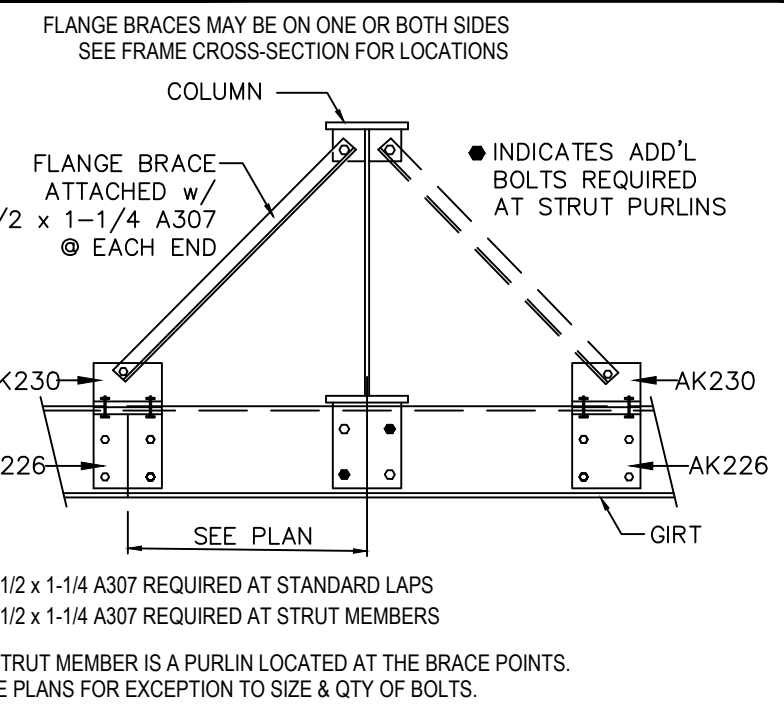
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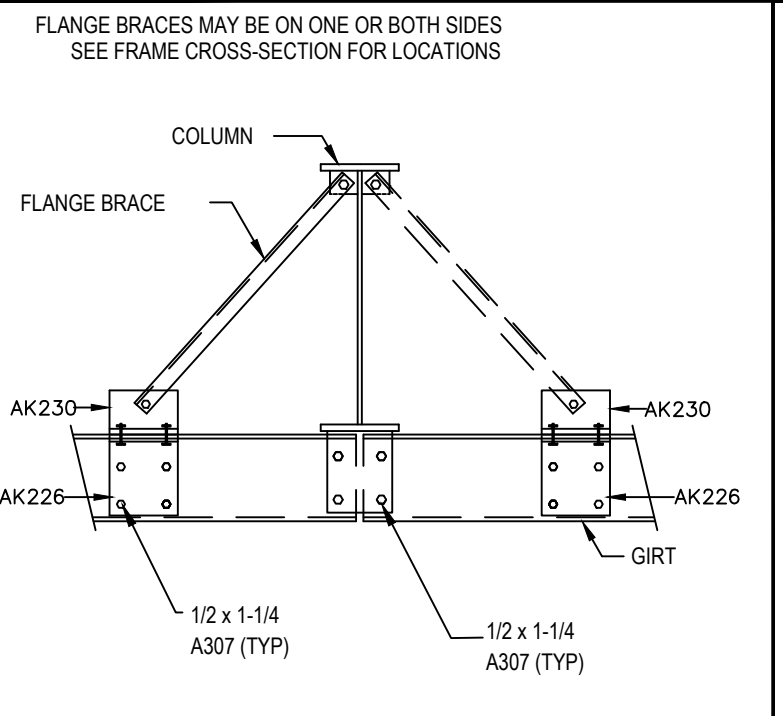
DETAIL
B-B1
REV 1
CEE RAFTER TO PURLIN CONNECTION



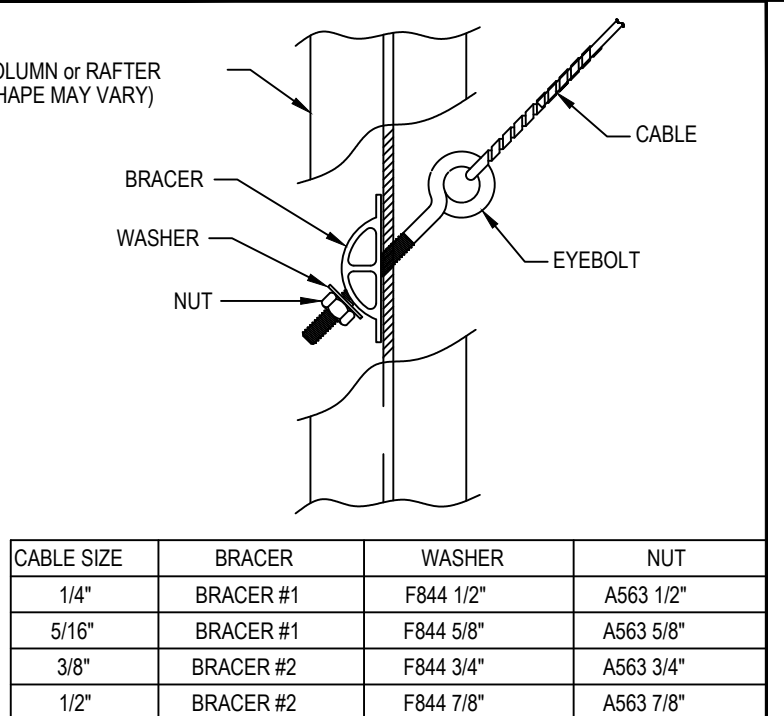
DETAIL
B-B4
REV 1
ENDWALL RAFTER TO COLUMN



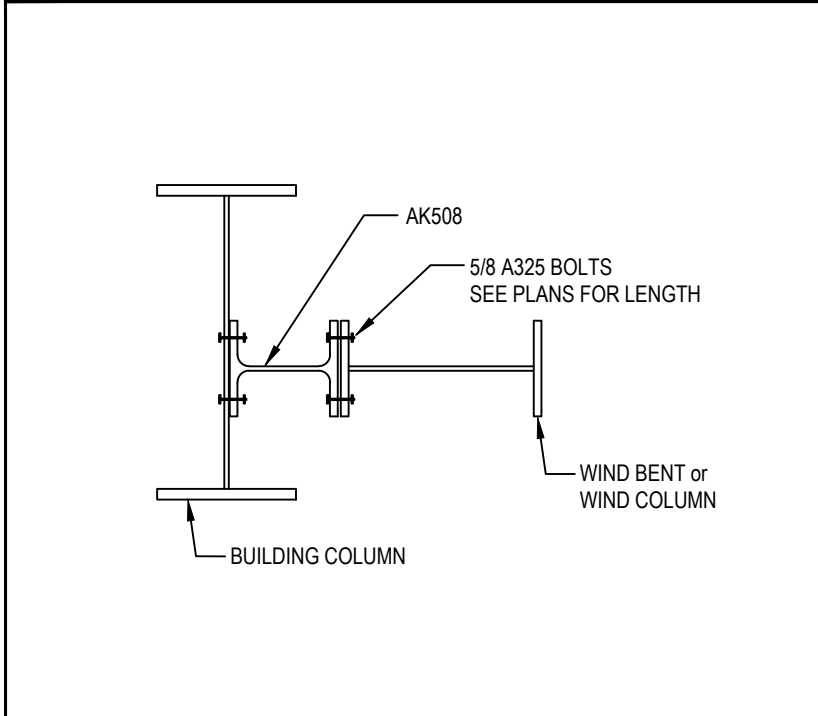
DETAIL
B-E8
REV 1
WALL GIRT TO FRAME COLUMN



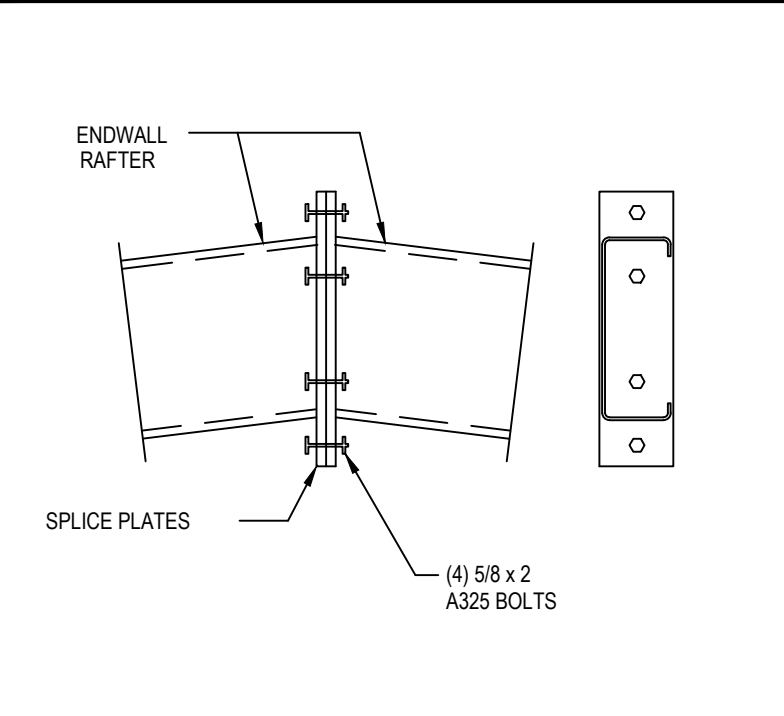
DETAIL
B-E9
REV 1
WALL GIRT TO INTERIOR FRAME COLUMN



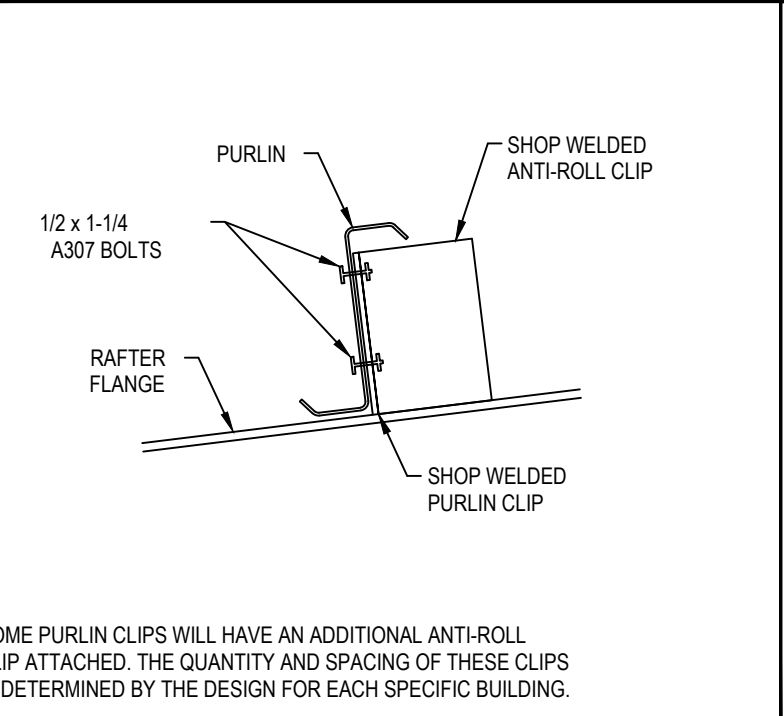
DETAIL
B-F1
REV 1
DIAGONAL CABLE BRACING INSTALLATION



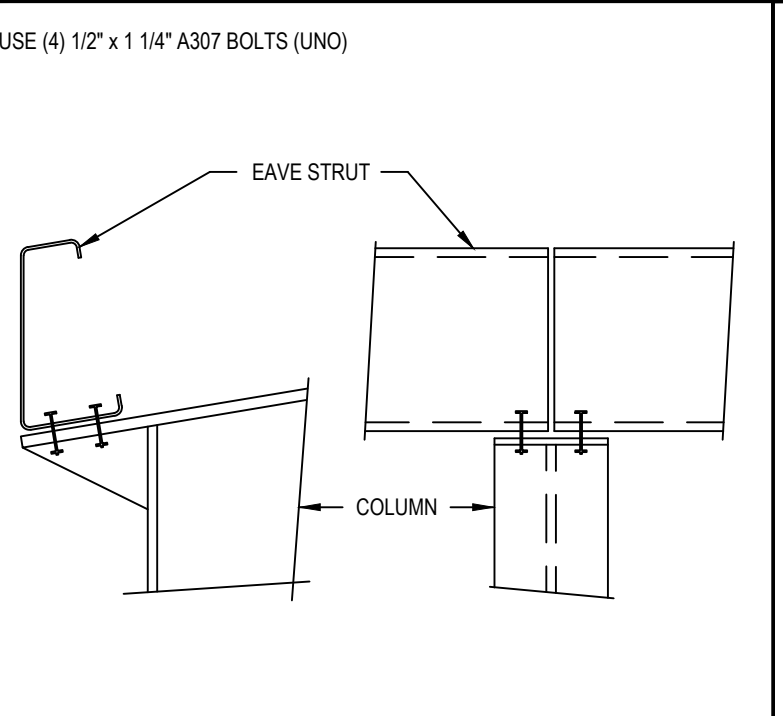
DETAIL
B-H2
REV 1
WIND BENT OR WIND COLUMN TO BUILDING COLUMN



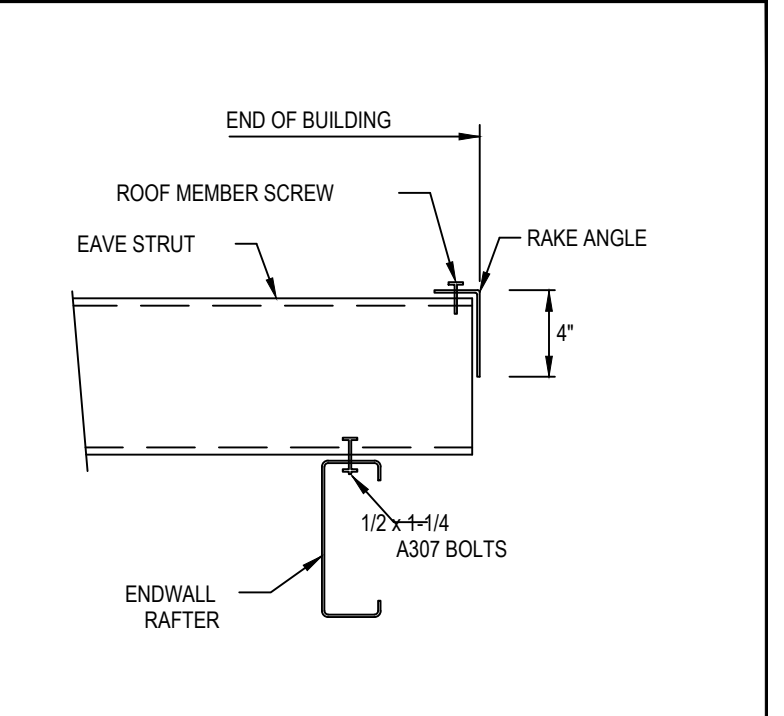
DETAIL
B-B16
REV 1
RAFTER SPLICE AT SURFACE CHANGE



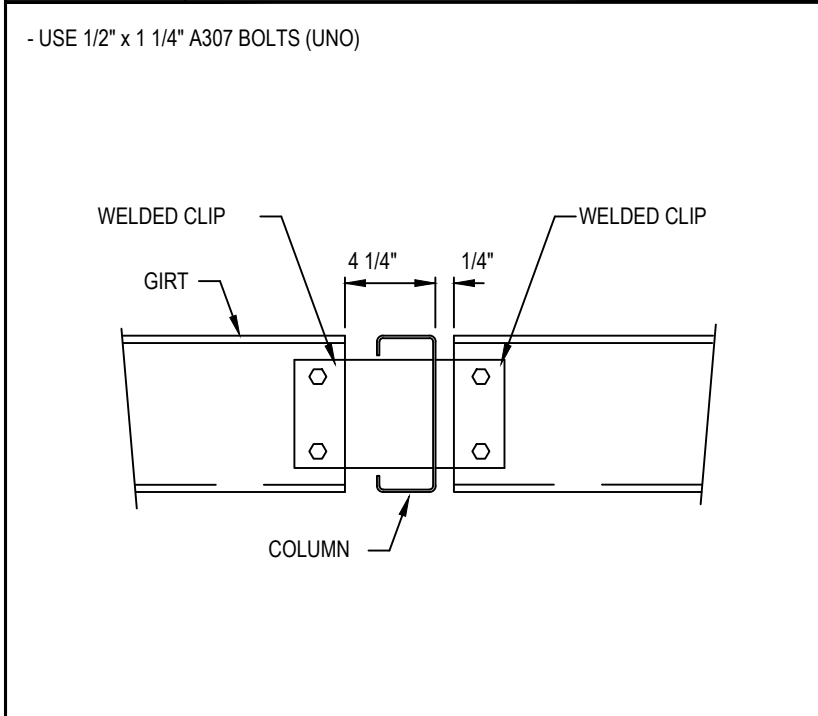
DETAIL
C-A1
REV 1
DETAIL AT ANTI-ROLL CLIP



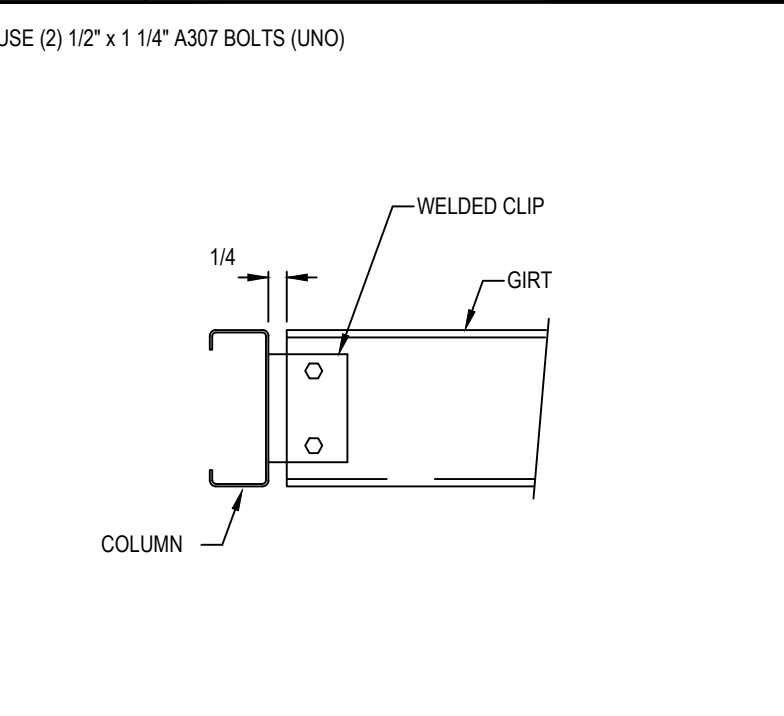
DETAIL
C-B2
REV 1
EAVE STRUT TO RIGID FRAME BYPASS CONDITION



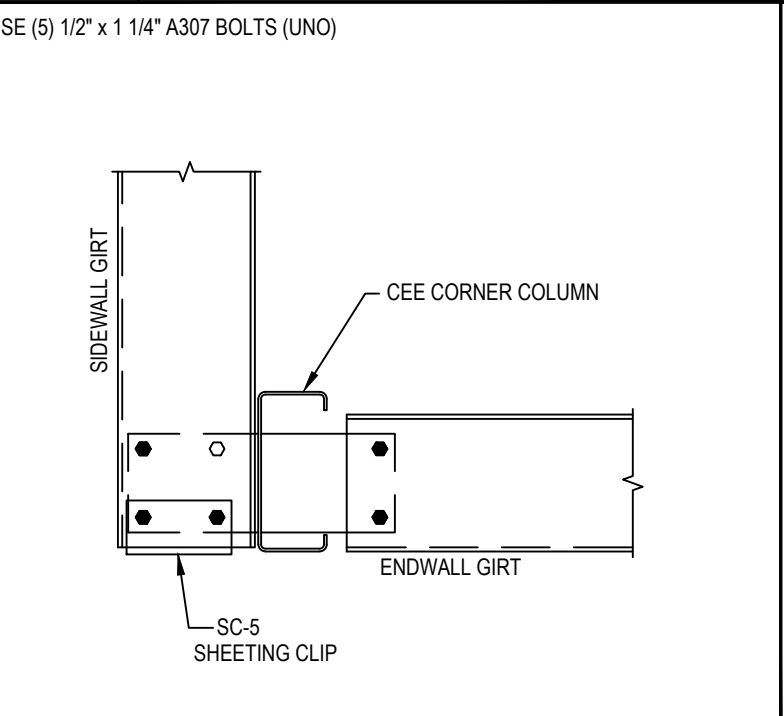
DETAIL
C-D1
REV 1
EAVE STRUT TO ENDWALL RAFTER



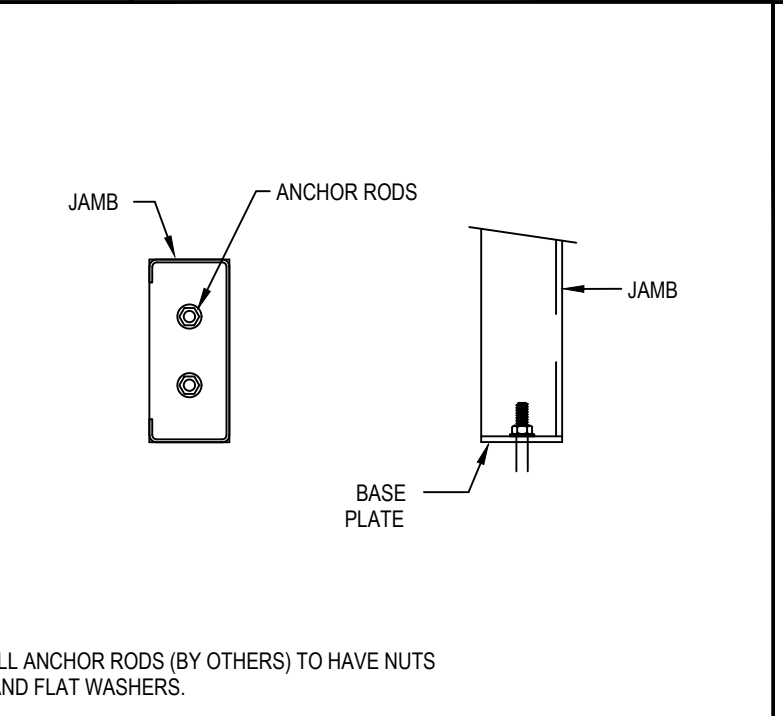
DETAIL
D-B4
REV 1
FLUSH ENDWALL GIRT TO CEE COLUMN



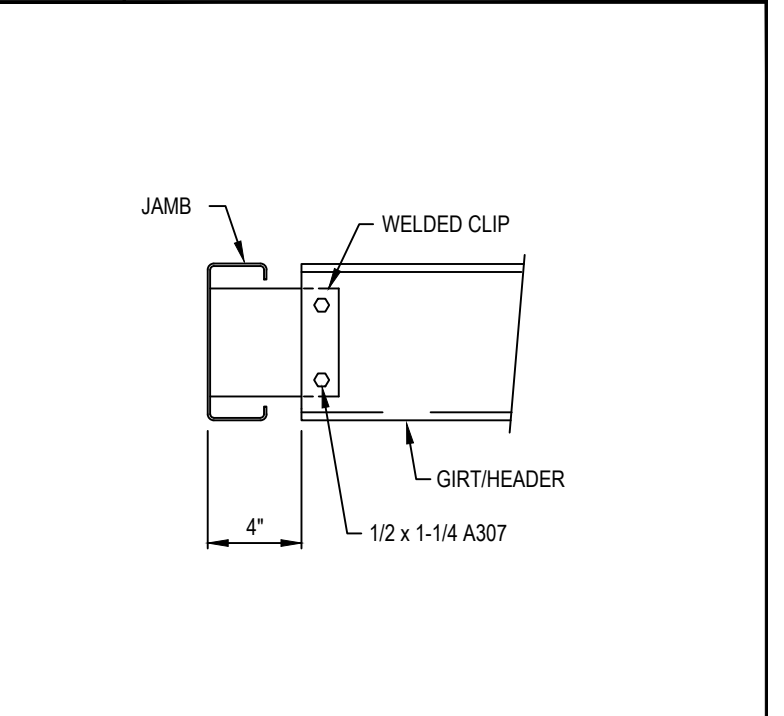
DETAIL
D-B7
REV 1
GIRT/HEADER TO CEE COLUMN



DETAIL
D-E4
REV 1
FLUSH ENDWALL GIRT TO CORNER CEE COLUMN



DETAIL
D-L1
REV 1
BASE PLATE FOR DOOR JAMB

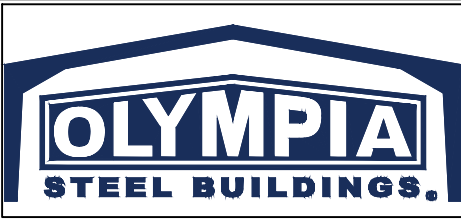


DETAIL
D-L4
REV 1
WALL GIRT TO DOOR JAMB

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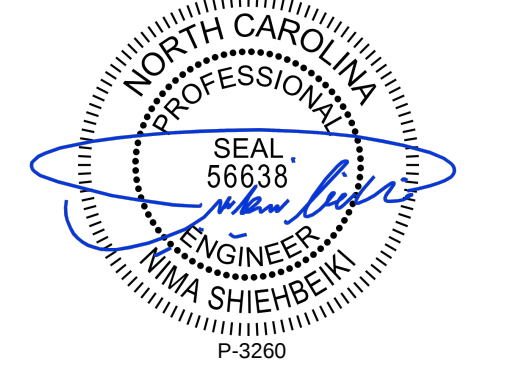
☐ FOR ERECTOR INSTALLATION:
Final drawings for construction.



ISSUE	DATE	DESCRIPTION	BY	CHK	PNC	SHEET DESCRIPTION:	BLDG SIZE:
P1	12.01.25	FOR CONSTRUCTION PERMIT	PND	PNC		STANDARD DETAIL PAGE	60'-0" x 168'-0" x 14'-0"
						CUSTOMER:	CUSTOMER LOCATION:
						PROGRESS VENTURES LLC	FUQUAY VARINA, NC 27526
						PROJECT REFERENCE:	
						PROGRESS VENTURES LLC	
						JOB SITE LOCATION:	JOB SITE COUNTY:
						FUQUAY VARINA, NC 27526	WAKE
						DWN:	ENG:
						PND	12.01.25
						CHK:	MVK
						DATE:	15130-38808
						JOB NO:	DWG NO:
							D2
						ISSUE:	P1

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SEE ROOF PLAN FOR LOCATIONS & PART MARKS											
DETAIL D-L17	DOOR JAMB TO EAVE STRUT (OPTIONAL REINFORCEMENT SHOWN)	DETAIL D-L19	DOOR JAMB TO WALL GIRT	DETAIL D-L40	DOOR JAMB TO WALL GIRT	DETAIL B-E6	FLANGE BRACE TO PURLIN/GIRT CLIPS OPTIONAL USE WITH VAPOR BARRIER	SEE PLANS FOR PART MARKS			
REV 1		REV 1		REV 1		REV 1					
								BOLT REQUIREMENTS			
DETAIL V-A2	GUTTER END CAP ATTACHMENT	DETAIL E-G1	CORNER BOX ATTACHMENT	DETAIL V-A3	DOWNSPOUT ATTACHMENT @ GUTTER	DETAIL V-A4	DOWNSPOUT STRAP ATTACHMENT	SEE PLANS FOR PART MARKS			
REV 1		REV 1		REV 1		REV 1					
				SEE PLANS FOR PART MARKS		BOLT REQUIREMENTS					
DETAIL B-A11	BOLTED END PLATE CONNECTION AT BUILDING PEAK	DETAIL B-A12	BOLTS FOR RAFTER TO COLUMN CONNECTION								
REV 1		REV 1									

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Final drawings for construction.



ISSUE	DATE	DESCRIPTION	BY	CHK	SHEET DESCRIPTION:	BLDG SIZE:
P1	12.01.25	FOR CONSTRUCTION PERMIT	PND	PNC	STANDARD DETAIL PAGE	60'-0" x 168'-0" x 14'-0"
					CUSTOMER:	CUSTOMER LOCATION:
					PROGRESS VENTURES LLC	FUQUAY VARINA, NC 27526
					PROJECT REFERENCE:	
					PROGRESS VENTURES LLC	
					JOB SITE LOCATION:	JOB SITE COUNTY:
					FUQUAY VARINA, NC 27526	WAKE
					DWN:	CHK:
					PND	PNC
					DATE:	ENG:
					12.01.25	MVK
					JOB NO:	DWG NO:
					15130-38808	03
					ISSUE:	
					P1	

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C1 CORNER TRIM INSTALLATION (SUPER SPAN X)

F1 HEAD TRIM INSTALLATION (SUPER SPAN X)

F2 JAMB TRIM INSTALLATION (SUPER SPAN X)

F3 SILL TRIM DETAIL (SUPER SPAN X)

F4 HEAD TRIM INSTALLATION (SUPER SPAN X PANEL)

F5 HEAD TRIM INSTALLATION (SUPER SPAN X PANEL)

Z1 WALK DOOR STD HEAD TRIM (SUPER SPAN X)

Z2 WALK DOOR STD JAMB TRIM (SUPER SPAN X)

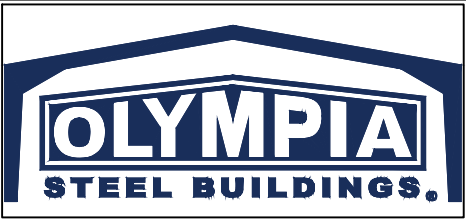
WALK DOOR STANDARD WITH EXTENSION CLIPS

WALK DOOR STANDARD WITH EXTENSION CLIPS

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Final drawings for construction.



ISSUE	DATE	DESCRIPTION	BY	CHK
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SHEET DESCRIPTION:		BLDG SIZE:	
STANDARD DETAIL PAGE		60'-0" x 168'-0" x 14'-0"	
CUSTOMER:		CUSTOMER LOCATION:	
PROGRESS VENTURES LLC		FUQUAY VARINA, NC 27526	
PROJECT REFERENCE:			
PROGRESS VENTURES LLC			
JOBSITE LOCATION:		JOBSITE COUNTY:	
FUQUAY VARINA, NC 27526		WAKE	
DWN:	CHK:	DATE:	ENG:
PND	PNC	12.01.25	MVK
JOB NO:		DWG NO:	ISSUE:
15130-38808		D4	P1

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