

BUILDING PROFILE

Width (ft) = 40 Eave Height (ft) = 19.33 H.S.
Length (ft) = 60 Roof Slope (Rise/12) = 1.0:12

BUILDING LOADS

- A) THIS IS TO CERTIFY THAT THIS STRUCTURE IS DESIGNED UTILIZING THE LOADS INDICATED AND APPLIED AS REQUIRED BY NCBC 18 / IBC 15
- B) THIS CERTIFICATION IS LIMITED TO THE STRUCTURAL DESIGN OF THE FRAMING AND COVERING PARTS MANUFACTURED BY THE BUILDING MANUFACTURER AND AS SPECIFIED IN THE CONTRACT. ACCESSORY ITEMS SUCH AS DOORS, WINDOWS, LOUVERS, TRANSLUCENT PANELS, VENTILATORS ARE NOT INCLUDED. ALSO EXCLUDED ARE OTHER PARTS OF THE PROJECT NOT PROVIDED BY THE BUILDING MANUFACTURER SUCH AS FOUNDATIONS, MASONRY WALLS, MECHANICAL EQUIPMENT AND THE ERECTION AND INSPECTION OF THE BUILDING. THE BUILDING SHOULD BE ERECTED ON A PROPERLY DESIGNED FOUNDATION IN ACCORDANCE WITH THE BUILDING MANUFACTURER'S DESIGN MANUAL, THE ATTACHED DRAWINGS, AND GOOD ERECTION PRACTICES. THE END USER AND/OR ENGINEER OF RECORD IS TO CONFIRM THAT THESE LOADS COMPLY WITH REQUIREMENTS OF THE LOCAL BUILDING DEPT.

OCCUPANCY/RISK CATEGORY II - Normal Is 1.0000 Ie 1.00

WIND LOAD ULTIMATE 120 MPH NOMINAL 92.95 MPH WIND EXPOSURE B

CLOSURE TYPE Enclosed INTERNAL WIND COEF. -0.18 / 0.18

GROUND SNOW LOAD 10.00 PSF ROOF SNOW LOAD 7 PSF Ce 1.0000 Ct 1.00

SNOW BANKING LOADS PER CODE

COLLATERAL DEAD LOAD 0.5 PSF

ROOF LIVE LOAD 20.00 PSF (REDUCIBLE Yes)

DEAD LOAD 2.000 PSF (FOR ROOF PANELS AND PURLINS)

SEISMIC

SPECTRAL RESPONSE Ss 0.1810 S1 0.0850 Sds 0.1931 Sd1 0.1360

SITE CLASS D DESIGN RISK CATEGORY C Cs 0.0644

RESPONSE MODIFICATION FACTOR, R 3.000* FRAMES 3.000* BRACING

BASIC SEISMIC FORCE RESISTING SYSTEM (LATERAL DIRECTIONS) = ORDINARY STEEL MOMENT FRAMES

BASIC SEISMIC FORCE RESISTING SYSTEM (ENDWALLS) = ORDINARY STEEL CONCENTRICALLY BRACED FRAMES

BASIC SEISMIC FORCE RESISTING SYSTEM (LONGITUDINAL DIRECTIONS) = ORDINARY STEEL MOMENT FRAMES

ANALYSIS PROCEDURE = EQUIVALENT LATERAL FORCE PROCEDURE

SERVICEABILITY CRITERIA

STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE.

MINIMUM DESIGN DEFLECTIONS			
Endwall Column	= 120	Roof Panel (Live)	= 60
Endwall Rafter (Live)	= 180	Roof Panel (Wind)	= 60
Endwall Rafter (Wind)	= 180	Rigid Frame (Horz)	= 60
Wall Girt	= 90	Rigid Frame (Vert)	= 180
Roof Purlin (Live)	= 150	Rigid Frame (Seismic)	= 50
Roof Purlin (Wind)	= 150		
Wall Panel	= 60		

GENERAL NOTES

- A) THE STRUCTURE UNDER THIS CONTRACT HAS BEEN DESIGNED AND DETAILED FOR THE LOADS AND CONDITIONS STIPULATED IN THE CONTRACT AND SHOWN ON THESE DRAWINGS. ANY ALTERATIONS TO THE STRUCTURAL SYSTEM OR REMOVAL OF ANY COMPONENT PARTS, OR THE ADDITION OF OTHER CONSTRUCTION MATERIALS OR LOADS MUST BE DONE UNDER THE ADVICE AND DIRECTION OF A REGISTERED ARCHITECT, CIVIL OR STRUCTURAL ENGINEER. THE BUILDING MANUFACTURER WILL ASSUME NO RESPONSIBILITY FOR ANY LOADS NOT INDICATED.
- B) THIS METAL BUILDING IS DESIGNED WITH THE BUILDING MANUFACTURER'S STANDARD PRACTICES WHICH ARE BASED ON PERTINENT PROCEDURES AND RECOMMENDATIONS OF THE FOLLOWING ORGANIZATIONS AND CODES.

1. AMERICAN INSTITUTE OF STEEL CONSTRUCTION: "AISC SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS-ALLOWABLE STRESS DESIGN"
2. AMERICAN IRON AND STEEL INSTITUTE: "SPECIFICATION FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS"
3. AMERICAN WELDING SOCIETY: "STRUCTURAL WELDING CODE" AWS D1.1.
4. METAL BUILDING MANUFACTURER'S ASSOCIATION: "LOW RISE BUILDING SYSTEMS MANUAL"

- C) 1) MATERIAL PROPERTIES OF STEEL PLATE USED IN THE FABRICATION OF PRIMARY RIGID FRAMES, AND OTHER PRIMARY STRUCTURAL EXCLUSIVE OF COLD-FORMED SECTIONS, CONFORM TO ASTM-A529 OR A572 . FLANGES WITH THICKNESS OF ONE INCH OR LESS AND WIDTH OF 12" OR LESS CONFORM TO A529 WITH A MINIMUM YIELD POINT OF 55,000 psi. FLANGES GREATER THAN 1" IN THICKNESS OR 12" IN WIDTH CONFORM TO A572 WITH A MINIMUM YIELD POINT OF 50,000 psi. WEB MATERIAL CONFORMS TO ASTM-A529 WITH A MINIMUM YIELD POINT OF 55,000 psi.
- 2) MATERIAL PROPERTIES OF PIPE SECTIONS CONFORM TO ASTM-A500, GRADE B WITH A MINIMUM YIELD POINT OF 42,000 psi.
- 3) MATERIAL PROPERTIES OF TUBE SECTIONS CONFORM TO ASTM-A500, GRADE B WITH A MINIMUM YIELD POINT OF 46,000 psi.
- 4) MATERIAL PROPERTIES OF HOT ROLLED CHANNEL AND ANGLE MEMBERS CONFORM TO THE REQUIREMENTS OF ASTM-A529 WITH MINIMUM YIELD POINT OF 50,000 PSI. HOT ROLLED W-SHAPED MEMBERS CONFORM TO THE REQUIREMENTS OF ASTM-A992 WITH MINIMUM YIELD POINT OF 50,000 PSI.
- 5) MATERIAL PROPERTIES OF COLD FORMED LIGHT GAGE STEEL MEMBERS CONFORM TO EITHER ASTM A653-06 GR 55 OR A1011-04 HSLAS GRADE 55 WITH YIELD OF 55,000 psi.
- 6) MATERIAL PROPERTIES OF ROOF/WALL SHEETING, BASE METAL CONFORM TO ASTM-A792 GRADES 80 CLASS 1, 2 OR 3 WITH A MINIMUM YIELD STRENGTH OF 80,000 PSI. COATING OF BASE MATERIAL IS 55% ALUMINUM-ZINC ALLOY IN ACCORDANCE WITH A255 SPECIFICATIONS.
- 7) CABLE UTILIZED FOR BRACING CONFORMS TO ASTM A475. CABLE BRACING IS TO BE INSTALLED TO A TAUT CONDITION.
- 8) ROD UTILIZED FOR BRACING MEMBERS CONFORM TO ASTM-A36 WITH MINIMUM YIELD POINT OF 36,000 PSI.
- 9) IT IS THE RESPONSIBILITY OF ERECTOR TO ENSURE PROPER BOLT TIGHTNESS IN ACCORDANCE WITH APPLICABLE "RCS" SPECIFICATION FOR STRUCTURAL JOINTS USING A-325 OR A-490 BOLTS". ALL A-325 BOLTS IN PRIMARY FRAMING MUST BE "SNUG-TIGHT", EXCEPT AS FOLLOWS:
- "FULLY-PRETENSION" A-325 BOLTS IF:
- a) BUILDING LOCATED IN A HIGH SEISMIC AREA. FOR IBC-BASED CODE, "HIGH SEISMIC AREA" IS DEFINED AS "SEISMIC DESIGN CATEGORY" OF "D", "E" OR "F".
- b) BUILDING SUPPORTS A CRANE SYSTEM WITH A CAPACITY GREATER THAN 5.00 TONS.
- c) BUILDING SUPPORTS MACHINERY THAT CREATES VIBRATION, IMPACT OR STRESS - REVERSALS ON THE CONNECTIONS.
- d) ANY CONNECTION DESIGNATED IN THESE DRAWINGS AS "A-325 - SC".

10) SECONDARY MEMBERS AND FLANGE BRACE CONNECTIONS SHALL ALWAYS BE SNUG TIGHT, UNO.

11) ANCHOR BOLTS 3/4" IN DIAMETER THRU 1 1/4" IN DIAMETER CONFORM TO A.S.T.M. F1554 GR. 36. ANCHOR BOLTS 1/2" IN DIAMETER CONFORM TO A.S.T.M. A-307.

- D) UNLESS NOTED OTHERWISE ON FRAMING COLOR CHART: ALL STEEL MEMBERS EXCEPT BOLTS, FASTENERS, CABLE AND RODS SHALL RECEIVE ONE COAT OF STANDARD RED OXIDE SHOP PRIMER.
- E) SHOP AND FIELD INSPECTIONS AND ASSOCIATED FEES ARE THE RESPONSIBILITY OF THE CONTRACTOR, UNLESS STIPULATED OTHERWISE IN THE CONTRACT.

APPROVAL NOTES

THE FOLLOWING CONDITIONS APPLY IN THE EVENT THAT THESE DRAWINGS ARE USED AS APPROVAL DRAWINGS:

A) IT IS IMPERATIVE THAT ANY CHANGES TO THESE DRAWINGS:

- 1) BE MADE IN CONTRASTING INK.
- 2) HAVE ALL INSTANCES OF CHANGE CLEARLY INDICATED.
- 3) BE LEGIBLE AND UNAMBIGUOUS.

B) DATED SIGNATURE IS REQUIRED ON ALL PAGES.

C) MANUFACTURER RESERVES THE RIGHT TO RESUBMIT DRAWINGS WITH EXTENSIVE OR COMPLEX CHANGES REQUIRED TO AVOID MISFABRICATION. THIS MAY IMPACT THE DELIVERY SCHEDULE.

D) APPROVAL OF THESE DRAWINGS INDICATES CONCLUSIVELY THAT THE MANUFACTURER HAS CORRECTLY INTERPRETED THE CONTRACT REQUIREMENTS, AND FURTHER CONSTITUTES AGREEMENT THAT THE BUILDING AS DRAWN, OR AS DRAWN WITH INDICATED CHANGES REPRESENTS THE TOTAL OF THE MATERIALS TO BE SUPPLIED BY MANUFACTURER.

E) ANY CHANGES NOTED ON THE DRAWINGS NOT IN CONFORMANCE WITH THE TERMS AND REQUIREMENTS OF THE CONTRACT BETWEEN MANUFACTURER AND ITS CUSTOMER ARE NOT BINDING ON MANUFACTURER UNLESS SUBSEQUENTLY SPECIFICALLY ACKNOWLEDGED AND AGREED TO IN WRITING BY CHANGE ORDER OR SEPARATE DOCUMENTATION. MANUFACTURER RECOGNIZES THAT RUBBER STAMPS ARE ROUTINELY USED FOR INDICATING APPROVAL, DISAPPROVAL, REJECTION, OR MERE REVIEW OF THE DRAWINGS SUBMITTED. HOWEVER, MANUFACTURER DOES NOT ACCEPT CHANGES OR ADDITIONS TO CONTRACTUAL TERMS AND CONDITIONS THAT MAY APPEAR WITH USE OF A STAMP OR SIMILAR INDICATION OF APPROVAL, DISAPPROVAL, ETC. SUCH LANGUAGE APPLIED TO MANUFACTURER'S DRAWINGS BY THE CUSTOMER, ARCHITECT, ENGINEER, OR ANY OTHER PARTY WILL BE CONSIDERED AS UNACCEPTABLE ALTERATIONS TO THESE DRAWING NOTES, AND WILL NOT ALTER THE CONTRACTUAL RIGHTS AND OBLIGATIONS EXISTING BETWEEN MANUFACTURER AND ITS CUSTOMER.

SAFETY COMMITMENT

- A) THE BUILDING MANUFACTURER HAS A COMMITMENT TO MANUFACTURE QUALITY BUILDING COMPONENTS THAT CAN BE SAFELY ERECTED. HOWEVER, THE SAFETY COMMITMENT AND JOB SITE PRACTICES OF THE ERECTOR ARE BEYOND THE CONTROL OF THE BUILDING MANUFACTURER.
- B) IT IS STRONGLY RECOMMENDED THAT SAFE WORKING CONDITIONS AND ACCIDENT PREVENTION PRACTICES BE THE TOP PRIORITY OF ANY JOB SITE.
- C) LOCAL, STATE AND FEDERAL SAFETY AND HEALTH STANDARDS SHOULD ALWAYS BE FOLLOWED TO HELP INSURE WORKER SAFETY.
- D) MAKE CERTAIN ALL EMPLOYEES KNOW THE SAFEST AND MOST PRODUCTIVE WAY OF ERECTING A BUILDING. EMERGENCY PROCEDURES SHOULD BE KNOWN TO ALL EMPLOYEES.
- E) DAILY MEETINGS HIGHLIGHTING SAFETY PROCEDURES ARE ALSO RECOMMENDED. THE USE OF HARD HATS, RUBBER SOLE SHOES FOR ROOF WORK, PROPER EQUIPMENT FOR HANDLING MATERIAL, AND SAFETY NETS WHERE APPLICABLE, ARE RECOMMENDED.

ERECTOR / CONTRACTOR RESPONSIBILITIES

- A) IT IS THE RESPONSIBILITY OF THE ERECTOR/CONTRACTOR TO INSURE THAT ALL PROJECT PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT THE BUILDING MANUFACTURER OR ITS DESIGN ENGINEER IS ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT.
- B) THE CONTRACTOR MUST SECURE ALL REQUIRED APPROVALS AND PERMITS FROM THE APPROPRIATE AGENCY AS REQUIRED.
- C) APPROVAL OF THE MANUFACTURER'S DRAWINGS AND CALCULATIONS INDICATE THAT THE BUILDING MANUFACTURER CORRECTLY INTERPRETED AND APPLIED THE REQUIREMENTS OF THE CONTRACT DRAWINGS AND SPECIFICATIONS. (SECT. 4.4.1 AISC CODE OF STANDARD PRACTICES, 13TH ED.)
- D) WHERE DISCREPANCIES EXIST BETWEEN THE MANUFACTURER'S STRUCTURAL STEEL PLANS AND THE PLANS FOR OTHER TRADES, THE STRUCTURAL STEEL PLANS SHALL GOVERN. (SECT. 3.3 AISC CODE OF STANDARD PRACTICE 13TH ED.)
- E) DESIGN CONSIDERATIONS OF ANY MATERIALS IN THE STRUCTURE WHICH ARE NOT FURNISHED BY THE BUILDING MANUFACTURER ARE THE RESPONSIBILITY OF THE CONTRACTORS AND ENGINEERS OTHER THAN THE BUILDING MANUFACTURER'S ENGINEERS UNLESS SPECIFICALLY INDICATED.
- F) THE ERECTOR/CONTRACTOR IS RESPONSIBLE FOR ALL ERECTION OF STEEL AND ASSOCIATED WORK IN COMPLIANCE WITH THE BUILDING MANUFACTURER'S "FOR CONSTRUCTION" DRAWINGS.
- G) PRODUCTS SHIPPED TO ERECTOR/CONTRACTOR OR HIS CUSTOMER SHALL BE INSPECTED BY ERECTOR/CONTRACTOR IMMEDIATELY UPON ARRIVAL. CLAIMS FOR SHORTAGES OR DEFECTIVE MATERIAL IF NOT PACKAGED MUST BE SENT TO THE MANUFACTURER IN WRITING WITHIN FIVE (5) DAYS AFTER RECEIPT OF THE SHIPMENT. HOWEVER, IF A DEFECT IS OF SUCH A NATURE THAT REASONABLE VISUAL INSPECTION WOULD FAIL TO DISCLOSE IT, THEN THE CLAIM MUST BE MADE WITHIN FIVE (5) DAYS AFTER THE ERECTOR/CONTRACTOR LEARNS OF THE DEFECT. THE MANUFACTURER WILL NOT BE LIABLE FOR ANY DEFECT UNLESS CLAIM IS MADE WITHIN ONE (1) YEAR AFTER DATE OF THE ORIGINAL SHIPMENT BY THE MANUFACTURER TO CONTRACTOR OR HIS CUSTOMER. THE MANUFACTURER WILL BE GIVEN A REASONABLE OPPORTUNITY TO INSPECT DEFECTIVE MATERIALS UPON RECEIPT OF CLAIM BY CONTRACTOR.
- IF A DEFECT IS OF SUCH NATURE THAT IT CAN BE REMEDIED BY A FIELD OPERATION AT THE JOB SITE WITHOUT THE NECESSITY OF RETURNING THE MATERIAL TO THE MANUFACTURER, THEN UPON WRITTEN AUTHORIZATION OF THE MANUFACTURER THE CONTRACTOR MAY REPAIR OR CAUSE THE MATERIAL TO BE REPAIRED AND THE MANUFACTURER WILL REIMBURSE THE CONTRACTOR FOR THE COST OF THE REPAIR IN ACCORDANCE WITH THE WRITTEN AUTHORIZATION.
- THE CORRECTION OF MINOR MISRITS BY THE USE OF DRIFT PINS TO DRAW THE COMPONENTS IN TO LINE, MODERATE AMOUNTS OF REAMING, CHIPPING AND CUTTING, AND THE REPLACEMENT OF MINOR SHORTAGES OF MATERIAL ARE A NORMAL PART OF ERECTION AND ARE NOT SUBJECT TO CLAIM.

H) ALL BRACING AS SHOWN AND PROVIDED BY THE MANUFACTURER FOR THIS BUILDING IS REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE STRUCTURE.

I) TEMPORARY SUPPORTS, SUCH AS TEMPORARY GUYS, BRACES, FALSE WORK, CRIBBING OR OTHER ELEMENTS REQUIRED FOR THE ERECTION OPERATION WILL BE DETERMINED AND FURNISHED AND INSTALLED BY THE ERECTOR. THESE TEMPORARY SUPPORTS WILL SECURE THE STEEL FRAMING, OR ANY PARTLY ASSEMBLED STEEL FRAMING, AGAINST LOADS COMPARABLE IN INTENSITY TO THOSE FOR WHICH THE STRUCTURE WAS DESIGNED, RESULTING FROM WIND, SEISMIC FORCES AND ERECTION OPERATIONS, BUT NOT THE LOADS RESULTING FROM THE PERFORMANCE OF WORK BY OR THE ACTS OF OTHERS, NOR SUCH UNPREDICTABLE LOADS AS THOSE DUE TO TORNADO, EXPLOSION OR COLLISION. (SECT. 7.10.3 AISC CODE OF STANDARD PRACTICE, 13TH ED.)

- J) METAL BUILDING MANUFACTURER IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL AND WORKMANSHIP OF FOUNDATION. ANCHOR BOLT PLANS PREPARED BY MBM ARE INTENDED TO SHOW ONLY LOCATION, DIAMETER AND PROJECTION OF THE ANCHOR RODS REQUIRED TO ATTACH THE METAL BUILDING SYSTEM TO FOUNDATION. IT IS RESPONSIBILITY OF THE END CUSTOMER TO ENSURE THAT ADEQUATE PROVISIONS ARE MADE FOR SPECIFYING ROD EMBEDMENT, BEARING VALUES, TIE RODS AND OTHER ASSOCIATED ITEMS EMBEDDED IN THE CONCRETE FOUNDATION, AS WELL AS FOUNDATION DESIGN FOR THE LOADS IMPOSED BY MB SYSTEM, OTHER IMPOSED LOAD, AND THE BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE (MBMA 06 SECTIONS 3.2.2 AND A3)
- K) METAL BUILDING MANUFACTURER DOES NOT PROVIDE ANY FIELD SUPERVISION FOR THE ERECTION, NOR DOES MBM PERFORM ANY INSPECTIONS DURING OR AFTER ERECTION.

COMPONENTS & CLADDING (unfactored)

Wall Field Values = 23.689 psf / -25.663 psf
Wall Edge Values = 23.689 psf / -31.526 psf

RENEGADE

STEEL BUILDINGS, INC.

IT IS THE RESPONSIBILITY OF THE CUSTOMER TO PROVIDE ALL DOCUMENTATION REQUIRED FOR ANY ACCESSORIES NOT PROVIDED BY MBM TO THEIR LOCAL PERMITTING OFFICE. ALL ACCESSORIES MUST COMPLY AND MEET ALL DESIGN REQUIREMENTS PER LOCAL CODES.

ALL VEHICULAR FRAMED OPENINGS SUPPLIED ON THIS PROJECT HAVE BEEN DESIGNED TO SUPPORT WIND LOADS NORMAL TO A DOOR SYSTEM, BASED ON THE STANDARD BUILDING CODE CRITERIA. THE VEHICULAR FRAMED OPENING HAS NOT BEEN DESIGNED FOR ANY ADDITIONAL MOMENT OR CATENARY FORCE FROM THE DOOR SYSTEM. ANY CHANGES TO THE INFORMATION SHOWN HERE WOULD REQUIRE AN ENGINEERING INVESTIGATION AND POSSIBLE BUILDING REINFORCEMENT.

FRAMING COLORS

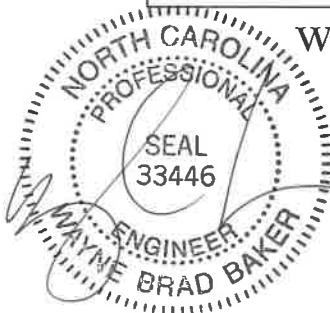
Rigid Frame: RO RO - Red Oxide
Flange brace: RO GP - Grey Primer
Angle: RO GZ - Galvanized

	Grt	Pur	Exst	Jmb	BB	Endwall Col	Ref
U SECTION:	RO	RO	RO	RO	RO	RO	RO
C SECTION:	RO	RO	RO	RO	RO	RO	RO
D SECTION:	RO	RO	RO	RO	RO	RO	RO
Z SECTION:	RO	RO	RO	RO	RO	RO	RO
E SECTION:	RO	RO	RO	RO	RO	RO	RO
R SECTION:	RO	RO	RO	RO	RO	RO	RO
W SECTION:	RO	RO	RO	RO	RO	RO	RO

WHEN GALVANIZED PROVIDED: ALL FINISHED PRIMARY BUILT-UP AND HOT ROLL MEMBERS ARE HOT DIPPED GALVANIZED. ALL SECONDARY COLD FORMED MEMBERS ARE PRE-GALVANIZED.



BUILDING DESIGNED & MANUFACTURED BY AN IAS ACCREDITED FACILITY.



2-3-23

DRAWING INDEX

REV.	PAGE	DESCRIPTION
	0	COVER PAGE
	1	ANCHOR BOLT LAYOUT
	1.1	ANCHOR BOLT DETAILS
	1.2	ANCHOR BOLT REACTIONS
	2	ROOF FRAMING LAYOUT
	2.1-2.2	RIGID FRAME CROSS SECTION
	3	SIDEWALL FRAMING LAYOUT
	4	ENDWALL FRAMING LAYOUT
	5-5.4	FRAMING DETAILS
	6	ROOF PANELS & TRIM
	6.1	ROOF PANEL DETAILS
	7	SIDEWALL PANELS & TRIM
	7.1	SIDEWALL PANEL DETAILS
	8	ENDWALL PANELS & TRIM
	8.1	ENDWALL PANEL DETAILS
	9	SPECIAL DETAILS

BUILDING CODE SPECIFICATIONS REQUIRE CONSIDERATION OF SNOW SURCHARGES FOR ANY LOWER ROOF OF A STRUCTURE LOCATED WITHIN 20ft. OF A HIGHER STRUCTURE. INFORMATION SUPPLIED TO THE METAL BUILDING SUPPLIER DOES NOT INDICATE PRESENCE OF A SHADOWING STRUCTURE WITHIN THIS 20ft. ENVELOPE, AND AS SUCH, SNOW SURCHARGES HAVE NOT BEEN CONSIDERED IN THE DESIGN OF THE BUILDING(S) SHOWN ON THESE PLANS.

THIS PROJECT IS DESIGNED AS AN ENCLOSED BUILDING. ACCESSORIES (DOORS, WINDOWS, ETC.) BY OTHERS MUST BE DESIGNED AS "COMPONENTS AND CLADDING" IN ACCORDANCE TO SPECIFIC WIND PROVISIONS OF REFERENCED BUILDING CODE.

FOR OCCUPANCY (RISK) CATEGORY I OR II, IBC PROVISIONS INDICATE THAT SINGLE-STORY BUILDINGS SHALL HAVE "NO DRIFT LIMIT" PROVIDED THAT INTERIOR WALLS, PARTITIONS, CEILINGS AND EXTERIOR WALL SYSTEMS HAVE BEEN DESIGNED TO ACCOMMODATE THE SEISMIC STORY DRIFTS. INTERIOR WALLS, PARTITIONS, CEILINGS OR EXTERIOR SYSTEMS NOT PROVIDED BY MBM SHALL BE DESIGNED AND DETAILED BY OTHERS TO ACCOMMODATE THE SEISMIC STORY DRIFTS.

0.5 PSF COLL ONLY ALLOW LIGHTING TO HANG FROM ROOF SYSTEMS. SUSPENSION OF ANY LOAD INDUCING SYSTEM IS EXPLICITLY PROHIBITED, UNLESS A CORRESPONDING REDUCTION IN CERTIFIED LIVE/SNOW LOADS CAN BE PERMITTED BY CODE.

Wayne Brad Baker, P.E.
235 Sanders Rd.
Hahira, GA 31632

DRAWING STATUS

FOR APPROVAL: THESE DRAWINGS, BEING FOR APPROVAL, ARE BY DEFINITION NOT FINAL, AND ARE FOR CONCEPTUAL REPRESENTATION ONLY. THEIR PURPOSE IS TO CONFIRM PROPER INTERPRETATION OF THE PROJECT DOCUMENTS. ONLY DRAWINGS ISSUED "FOR CONSTRUCTION" CAN BE CONSIDERED AS COMPLETE.

FOR PERMIT: THESE DRAWINGS, BEING FOR PERMIT, ARE BY DEFINITION NOT FINAL IN THAT, AS A MINIMUM, PIECE MARKINGS ARE NOT IDENTIFIED. ONLY DRAWINGS ISSUED "FOR CONSTRUCTION" CAN BE CONSIDERED AS COMPLETE.

FOR CONSTRUCTION: THESE DRAWINGS ARE FINAL AND ISSUED FOR FIELD USE FOR BUILDING ERECTION

DATE							
CHK							
DET							
ISSUE							

FOR:

JUNIOR FAIRCLOTH
1560 GEORGE PERRY LEE RD.
DUNN, NC 28334

JOBSITE: DUNN, NC 28334

FROM:

RENEGADE STEEL BUILDINGS, INC.
2151 HOMER RD.
COMMERCE, GA 30529

JOB NO : 7871

DATE : 1/31/23

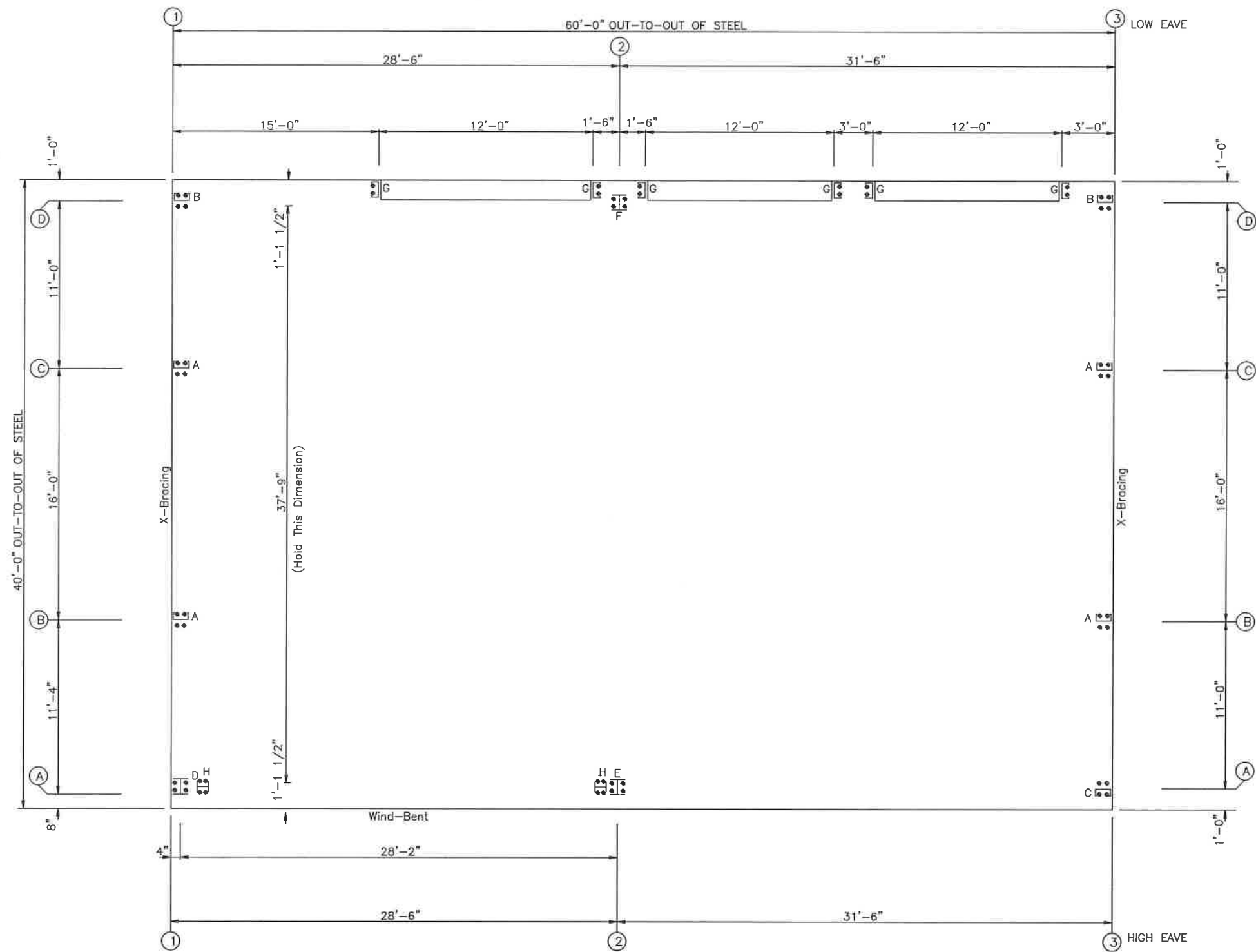
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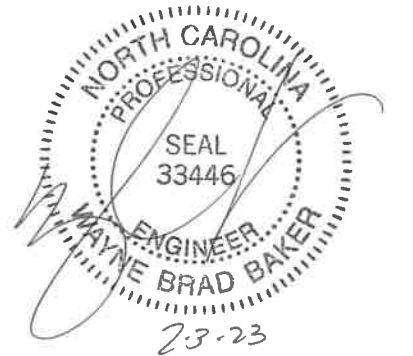
PAGE 0

⊕ Dia= 5/8"
⊗ Dia= 3/4"



ANCHOR BOLT PLAN
NOTE: All Base Plates ⌀ 100'-0" (Unless Noted)

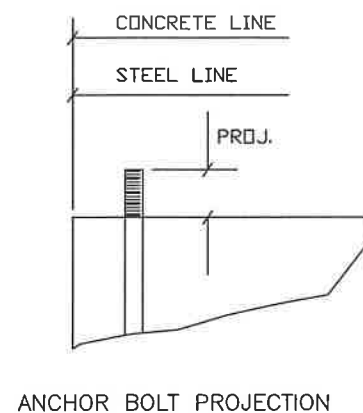
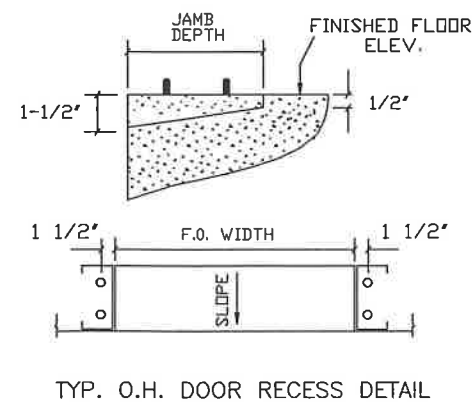
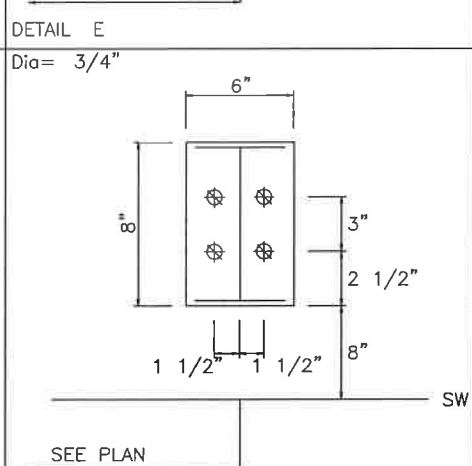
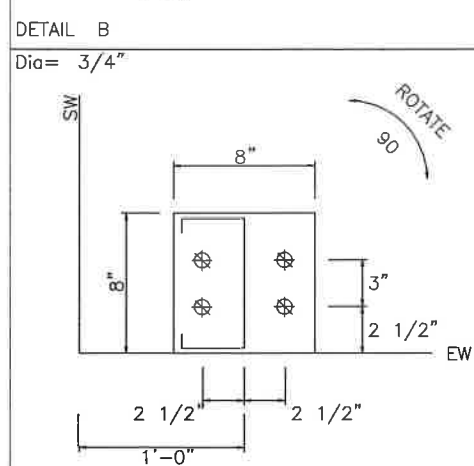
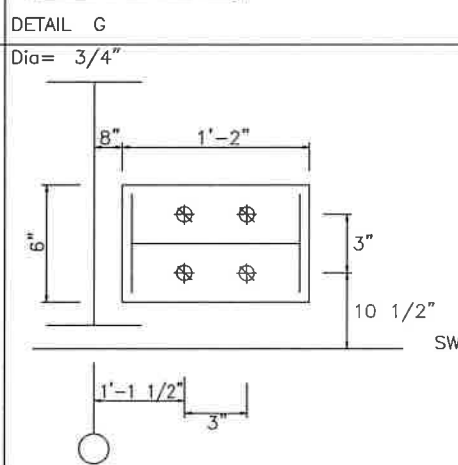
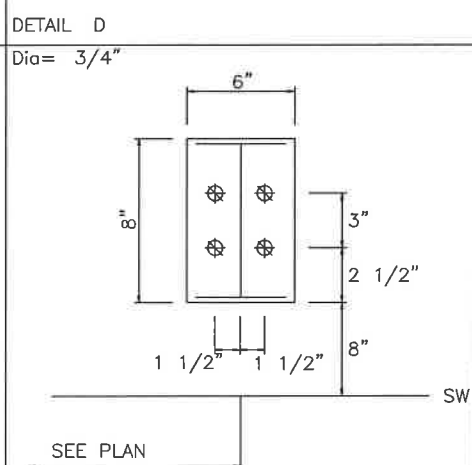
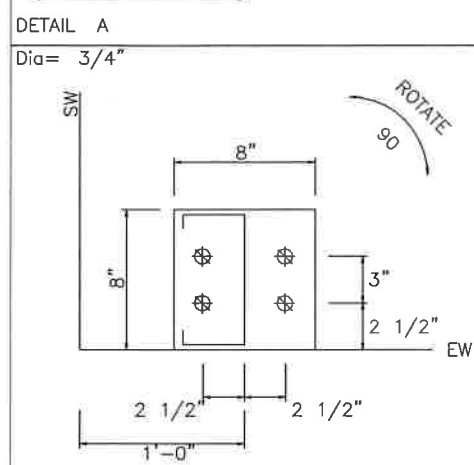
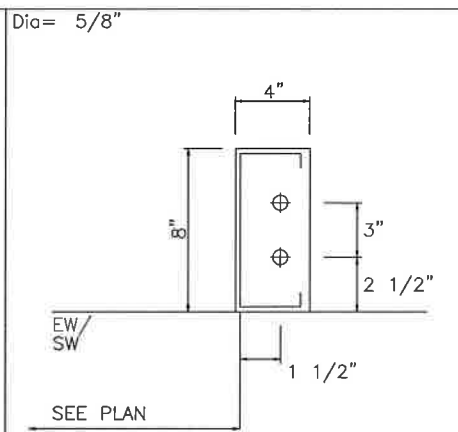
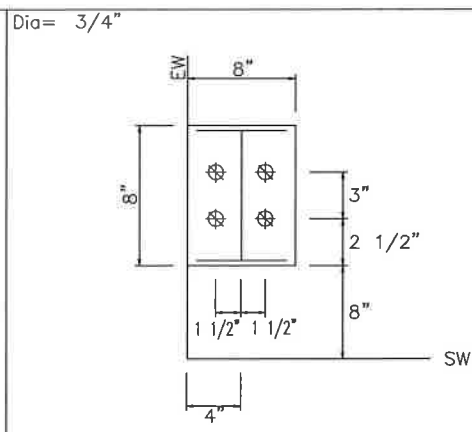
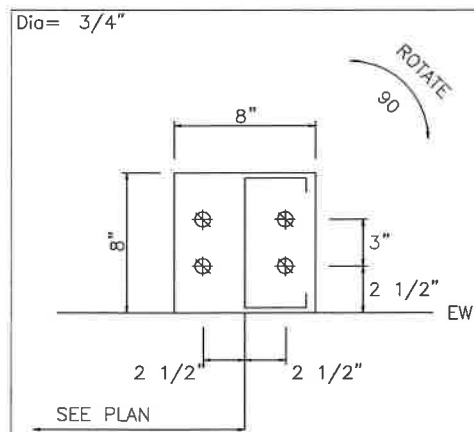
Wayne Brad Baker, P.E.
235 Sanders Rd.
Hahira, GA 31632



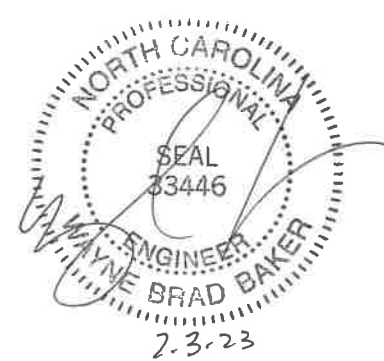
NOTE: ALL FIELD LOCATED FRAMED OPENING
LOCATIONS SHALL BE AT THE DISCRETION OF THE
ERECTOR/CUSTOMER. IT IS RECOMMENDED THAT
THESE ANCHORS BE LOCATED AT TIME OF ERECTION.

FIELD LOCATE:
(2) 3070 WALKDOORS

ISSUE		DET	CHK	DATE
RENEGADE STEEL BUILDINGS, INC.				
CUSTOMER: JUNIOR FAIRCLOTH				
JOB NO: 7871		DATE: 1/31/23		
LOCATION: DUNN, NC 28334				
DRAWING NAME: ANCHOR BOLT LAYOUT				
DRAWING NO: PAGE 1		DRAWN BY: DJH	CHECKED BY: SPW	SCALE: NONE

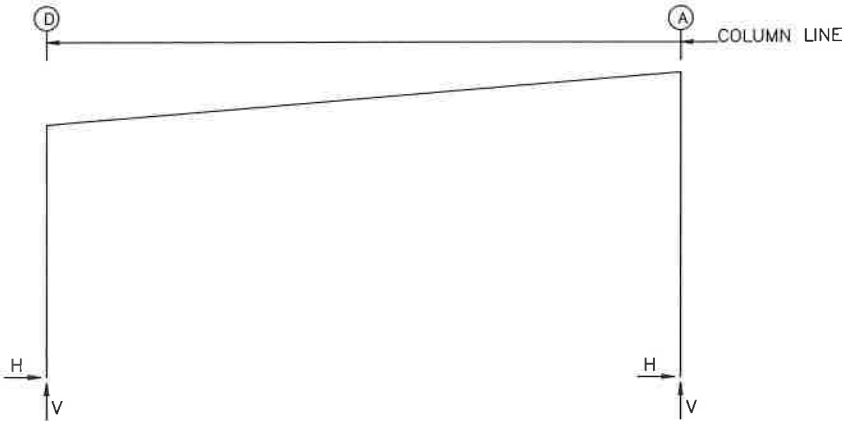


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ISSUE		DET	CHK	DATE
RENEGADE STEEL BUILDINGS, INC.				
CUSTOMER: JUNIOR FAIRCLOTH				
JOB NO: 7871		DATE: 1/31/23		
LOCATION: DUNN, NC 28334				
DRAWING NAME: ANCHOR BOLT DETAILS				
DRAWING NO: PAGE 1.1		DRAWN BY: DJH	CHECKED BY: SPW	SCALE: NONE

FRAME LINES: 2



RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Qty	Bolt Dia	Base_Plate (in)			Grout (in)
				Width	Length	Thick	
2	D	4	0.750	6.000	8.000	0.375	0.0
2	A	4	0.750	6.000	8.000	0.375	0.0

ENDWALL COLUMN: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Qty	Bolt Dia	Base_Plate (in)			Grout (in)
				Width	Length	Thick	
1	D	4	0.750	8.000	8.000	0.250	0.0
1	C	4	0.750	8.000	8.000	0.250	0.0
1	B	4	0.750	8.000	8.000	0.250	0.0
1	A	4	0.750	8.000	8.000	0.375	0.0
3	A	4	0.750	8.000	8.000	0.250	0.0
3	B	4	0.750	8.000	8.000	0.250	0.0
3	C	4	0.750	8.000	8.000	0.250	0.0
3	D	4	0.750	8.000	8.000	0.250	0.0

NOTES FOR REACTIONS

Building reactions are based on the following building data:

Width (ft)	=	40.0
Length (ft)	=	60.0
Eave Height (ft)	=	16.0/ 19.3
Roof Slope (Rise/12)	=	1.0
Dead Load (psf)	=	2.0
Collateral Load (psf)	=	0.5
Roof Live Load (psf)	=	20.0
Frame Live Load (psf)	=	12.0
Snow Load (psf)	=	7.0
Wind Speed (mph)	=	120.0
Wind Code	=	NCBC 18 (IBC 15)
Exposure	=	B
Enclosed/Open/Partial	=	ENCLOSED
Importance Wind	=	1.00
Importance Seismic	=	1.00
Seismic Zone	=	C
Seismic Coeff (Fa*Gs)	=	0.29

BUILDING BRACING REACTIONS

Wall Loc		Col Line	± Reactions(k)				Panel Shear (lb/ft)		Note
			Wind Horiz	Wind Vert	Seismic Horiz	Seismic Vert	Wind	Seis	
L_EW	1	C,B	5.2	5.6	0.7	0.7			
F_SW	A	1,2							(a)
R_EW	3	B,C	3.2	3.4	0.3	0.3			
B_SW	D	Torsional Bracing Used							

(a)Wind bent in bay

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Snow Horiz	Snow Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert
2	D	0.6	2.0	0.1	0.4	3.3	9.0	1.9	5.2	-9.6	-16.4	3.2	-6.2
2	A	-0.6	2.0	-0.1	0.4	-3.3	9.1	-1.9	5.3	-1.1	-12.3	8.0	-12.0

Frame Line	Column Line	Wind_Left2 Horiz	Wind_Left2 Vert	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	Seismic_Left Horiz	Seismic_Left Vert	Seismic_Right Horiz	Seismic_Right Vert
2	D	-10.3	-10.4	3.1	-0.2	-3.0	-15.7	2.8	-7.5	-0.3	-0.2	0.3	0.2
2	A	-0.3	-6.4	8.1	-6.0	-0.3	-12.9	0.9	-10.7	-0.2	0.2	0.2	-0.2

Frame Line	Column Line	Seismic_Long Horiz	Seismic_Long Vert	MIN_SNOW Horiz	MIN_SNOW Vert
2	D	-0.4	-0.3	2.7	7.5
2	A	-0.3	0.3	-2.7	7.6

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Snow Vert	Wind Left1 Vert	Wind Right1 Vert	Wind Left2 Vert	Wind Right2 Vert	Wind Press Horiz	Wind Suct Horiz	Wind Long1 Vert	Wind Long2 Vert
1	D	0.2	0.0	1.3	0.5	-1.4	-1.0	-0.8	-0.4	-0.9	1.0	-1.7	-1.1
1	C	0.6	0.1	4.4	1.6	-5.1	-2.8	-3.4	-1.1	-2.1	2.3	-5.1	-3.1
1	B	0.6	0.1	4.4	1.6	-5.3	-3.1	-3.6	-1.3	-2.2	2.5	-5.1	-3.1
1	A	0.4	0.0	1.4	0.5	-1.4	-1.0	-1.0	-0.6	-2.5	2.8	-1.4	-0.8

Frm Line	Col Line	Seis Left Vert	Seis Right Vert	Seis Long Vert	MIN_SNOW Horiz	E1PAT_SL_1 Horiz	E1PAT_SL_1 Vert	E1PAT_SL_2 Horiz	E1PAT_SL_2 Vert	E1PAT_SL_3 Horiz	E1PAT_SL_3 Vert	E1PAT_SL_4 Horiz	E1PAT_SL_4 Vert
1	D	0.0	0.0	0.0	0.0	0.7	0.3	0.0	0.0	0.0	0.2	0.0	-0.1
1	C	0.0	0.0	0.0	0.0	2.2	0.0	0.3	0.0	0.0	0.8	0.0	0.4
1	B	0.0	0.0	0.0	0.0	2.2	0.0	0.0	0.3	0.0	0.4	0.0	0.8
1	A	0.0	0.0	0.0	0.0	0.7	0.0	0.0	0.3	0.0	-0.1	0.0	0.2

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Snow Vert	Wind Left1 Vert	Wind Right1 Vert	Wind Left2 Vert	Wind Right2 Vert	Wind Press Horiz	Wind Suct Horiz	Wind Long1 Vert	Wind Long2 Vert
3	A	0.2	0.0	1.5	0.5	-1.1	-1.5	-0.6	-1.1	-1.1	1.3	-1.5	-0.8
3	B	0.7	0.1	4.9	1.7	-3.3	-5.8	-1.4	-3.9	-2.2	2.5	-5.6	-3.3
3	C	0.7	0.1	4.9	1.7	-3.1	-5.6	-1.2	-3.7	-2.1	2.3	-5.6	-3.3
3	D	0.2	0.0	1.5	0.5	-1.1	-1.6	-0.5	-0.9	-0.9	1.0	-1.8	-1.1

Frm Line	Col Line	Seis Left Vert	Seis Right Vert	MIN_SNOW Horiz	E2PAT_SL_1 Horiz	E2PAT_SL_1 Vert	E2PAT_SL_2 Horiz	E2PAT_SL_2 Vert	E2PAT_SL_3 Horiz	E2PAT_SL_3 Vert	E2PAT_SL_4 Horiz	E2PAT_SL_4 Vert
3	A	0.0	0.0	0.0	0.7	0.0	0.3	0.0	0.0	0.3	0.0	-0.1
3	B	0.0	0.0	0.0	2.4	0.0	0.4	0.0	0.0	0.9	0.0	0.5
3	C	0.0	0.0	0.0	2.4	0.0	0.0	0.4	0.0	0.5	0.0	0.9
3	D	0.0	0.0	0.0	0.7	0.0	0.0	0.3	0.0	-0.1	0.0	0.2

ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type	Projection (in)
⊕ 12	Jamb	5/8"	A307	1.50
⊕ 32	Endwall	3/4"	GR36	1.50
⊕ 8	Frame	3/4"	GR36	2.50
⊕ 8	WindCol	3/4"	GR36	2.50

GENERAL NOTES

- FOUNDATION DESIGN AND CONSTRUCTION ARE NOT THE RESPONSIBILITY OF METAL BUILDING MANUFACTURER.
- ALL REACTIONS ARE UNFACTORED.
- ULTIMATE WIND LOADS ARE USED TO DERIVE THE WIND REACTION.
- ANCHOR BOLTS SHALL BE ACCURATELY SET TO A TOLERANCE OF +/- 1/8" IN BOTH ELEVATION AND LOCATION.
- COLUMN BASE PLATES ARE DESIGNED NOT TO EXCEED A BEARING PRESSURE OF 1050 POUNDS PER SQUARE INCH.

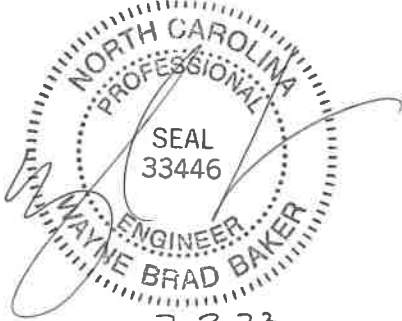
WIND BENT REACTIONS

Wall Loc		Col Line	± Reactions				Bolt(in)		Base_Plate(in)		
			Wind Horiz	Wind Vert	Seismic Horiz	Seismic Vert	Qty	Dia	Width	Length	Thick
F_SW	A	1	2.9	4.0	0.5	0.6	4	0.750	6.000	14.000	0.375
F_SW	A	2	2.9	4.0	0.5	0.6	4	0.750	6.000	14.000	0.375

NOTE: THE FRAMING AT BOTH ENDWALLS IS NOT DESIGNED TO ACCOMMODATE FUTURE ADDITIONS. REACTIONS CORRESPONDING TO THESE FRAME LINES REFLECT LOADINGS FOR ACTUAL TRIBUTARY AREA AND ARE NOT INTENDED TO INCLUDE ANY FUTURE MODIFICATIONS UNLESS NOTED OTHERWISE.

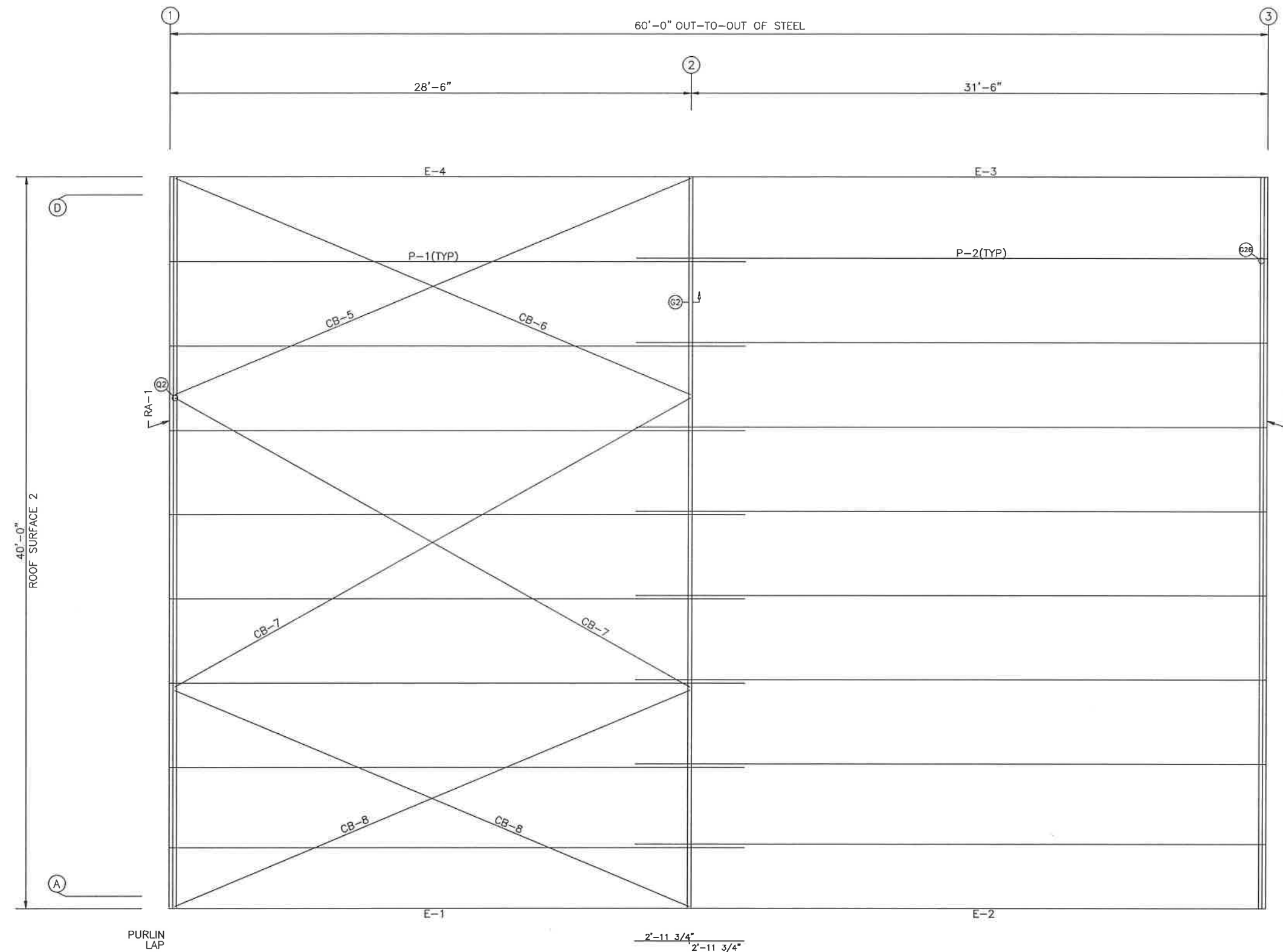
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Hahira, GA 31632



2.3.23

ISSUE		DET	CHK	DATE
RENEGADE STEEL BUILDINGS, INC.				
CUSTOMER: JUNIOR FAIRCLOTH				
JOB NO: 7871			DATE: 1/31/23	
LOCATION: DUNN, NC 28334				
DRAWING NAME: ANCHOR BOLT REACTIONS				
DRAWING NO: PAGE 1.2		DRAWN BY: DJH	CHECKED BY: SPW	SCALE: NONE



ROOF FRAMING PLAN

MEMBER TABLE		
ROOF PLAN		
MARK	PART	LENGTH
P-1	8x25Z12	31'-5 1/2"
P-2	8x25Z12	34'-5 1/2"
E-1	8HE14@1	28'-5 1/2"
E-2	8HE14@1	31'-5 1/2"
E-3	8LE12@1	31'-5 1/2"
E-4	8LE14@1	28'-5 1/2"
CB-5	1/4 CBL	30'-3"
CB-6	1/4 CBL	30'-11"
CB-7	1/4 CBL	32'-7"
CB-8	1/4 CBL	30'-6"

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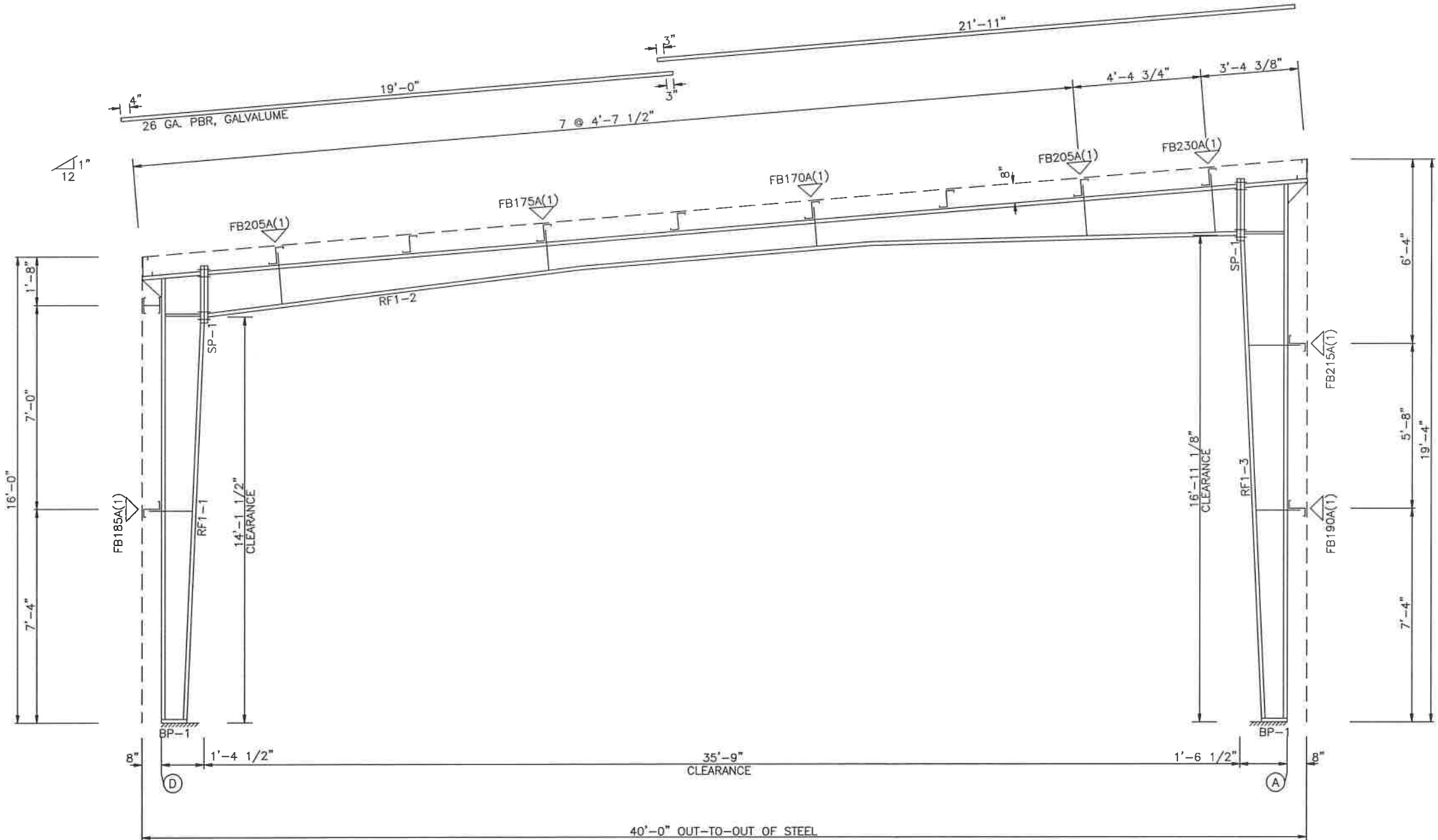
ISSUE		DET	CHK	DATE
RENEGADE STEEL BUILDINGS, INC.				
CUSTOMER: JUNIOR FAIRCLOTH				
JOB NO: 7871		DATE: 1/31/23		
LOCATION: DUNN, NC 28334				
DRAWING NAME: ROOF FRAMING LAYOUT				
DRAWING NO: PAGE 2	DRAWN BY: DJH	CHECKED BY: SPW	SCALE: NONE	

SPLICE BOLT TABLE						
MARK	Qty	Top	Bot	Int	TYPE	DIA Length
SP-1	4	4	0		A325	5/8" 2"

BASE PLATE TABLE			
COL	PLATE SIZE		
MARK	Width	THICK	Length
BP-1	6"	3/8"	8"

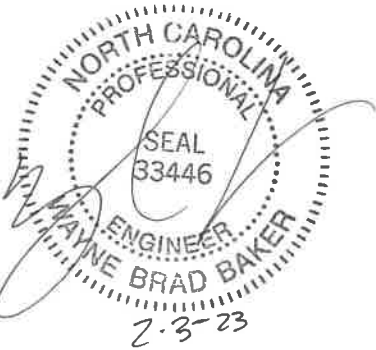
▽ FLANGE BRACES: (1) One Side; (2) Two Sides
FBxxA(1): xx=length(in)
A - L2x2x14

MEMBER TABLE		Web Depth	Web PLATE	Outside Flange	Inside Flange
MARK	Weight	Start/End	THICK Length	W x Thk x Length	W x Thk x Length
RF1-1	258	7.5/16.0	0.135 13'-9 3/8"	5 x 1/4" x 15'-4"	5 x 1/4" x 13'-9 9/16"
RF1-2	563	16.0/16.0	0.188 1'-8"	5 x 1/4" x 2'-0 5/16"	
		16.0/ 9.5	0.135 13'-0 11/16"	5 x 1/4" x 20'-0"	5 x 1/4" x 13'-0 13/16"
		9.5/ 9.5	0.135 10'-0"	5 x 1/4" x 15'-8 15/16"	5 x 1/4" x 10'-0"
RF1-3	316	9.5/18.0	0.135 12'-9 5/8"		5 x 1/4" x 12'-8 3/8"
		18.0/18.0	0.188 1'-11 9/16"	5 x 1/4" x 2'-2 5/16"	5 x 1/4" x 16'-7 5/16"
		18.0/ 8.8	0.135 14'-7 1/16"	5 x 1/4" x 18'-6 11/16"	
		8.8/ 7.5	0.135 2'-0"		



RIGID FRAME ELEVATION: FRAME LINE 2

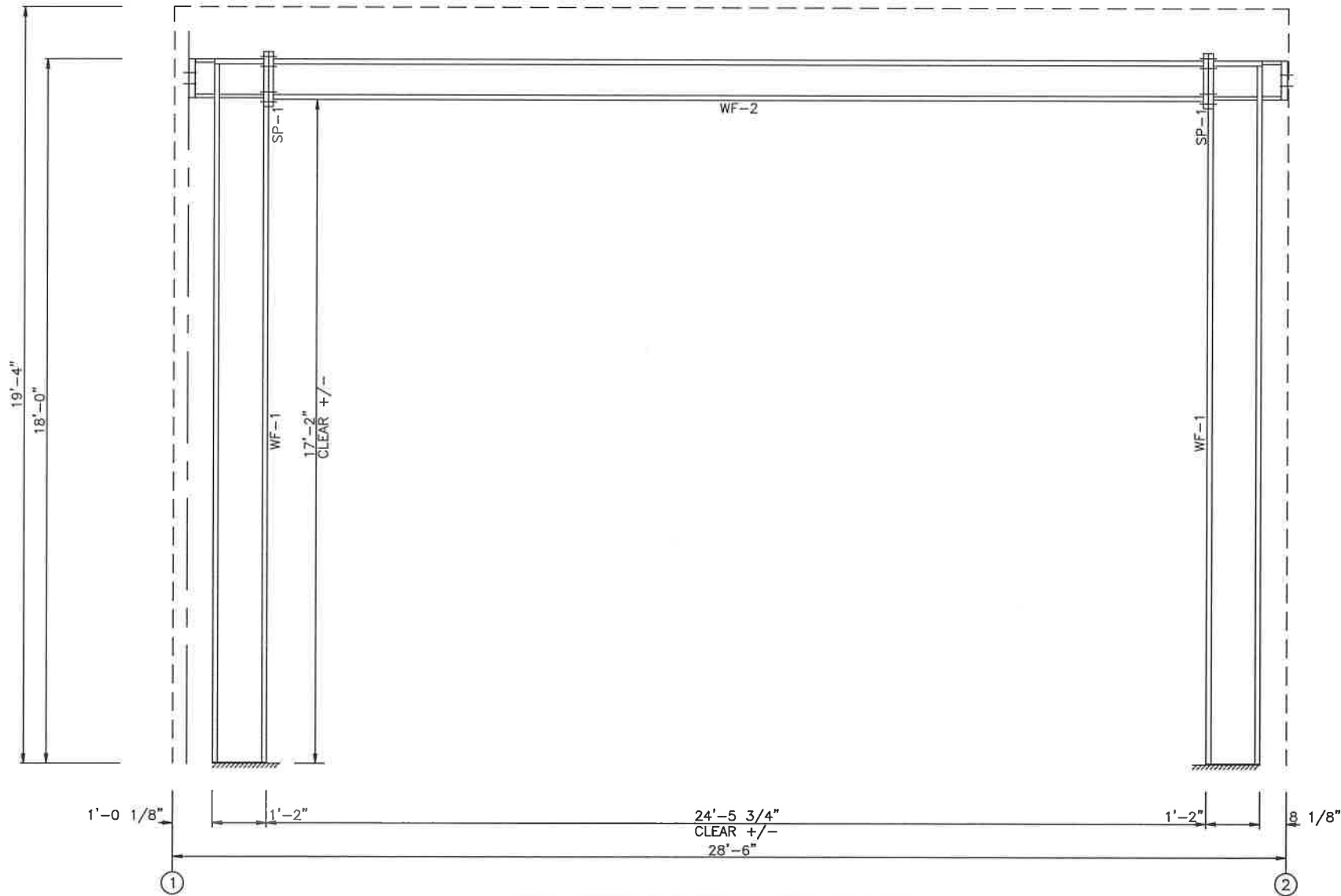
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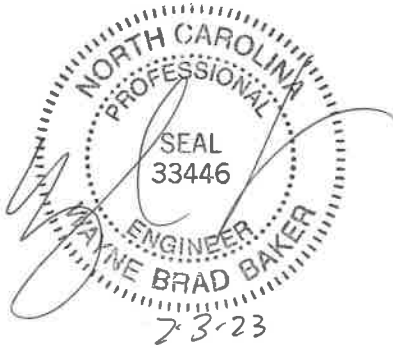
ISSUE		DET	CHK	DATE
RENEGADE STEEL BUILDINGS, INC.				
CUSTOMER: JUNIOR FAIRCLOTH				
JOB NO: 7871		DATE: 1/31/23		
LOCATION: DUNN, NC 28334				
DRAWING NAME: RIGID FRAME CROSS SECTION				
DRAWING NO: PAGE 2.1	DRAWN BY: DJH		CHECKED BY: SPW	SCALE: NONE

SPlice BOLTS					
Splice Mark	Quan	Type		Bolt	
Top/ Bot				Dia	Length
SP- 1	4	4	A325	5/8"	2"

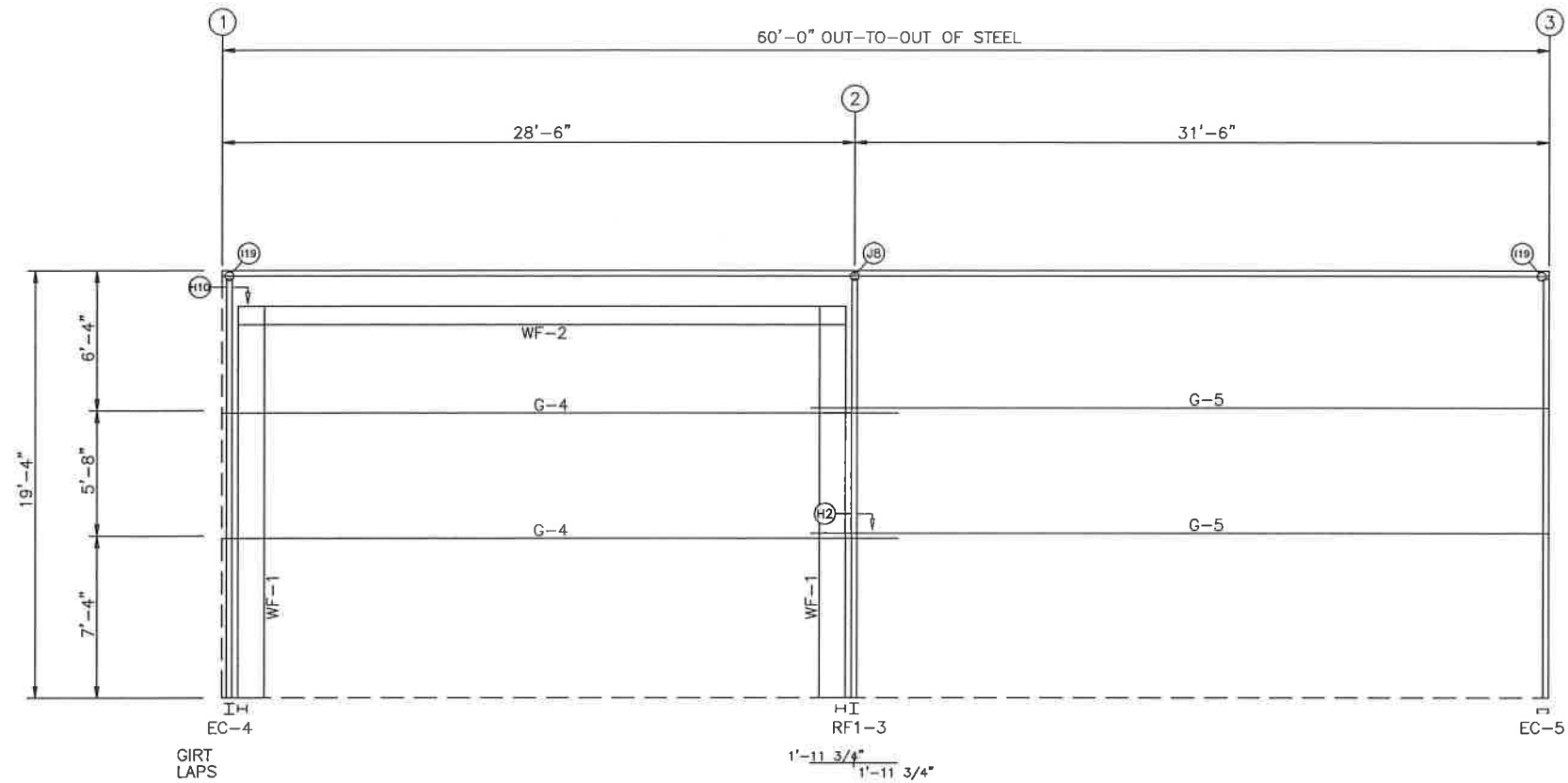
MEMBER SIZE TABLE		
MARK	MEMBER	LENGTH
WF-2	B10651	24'-5 1/4"
WF-1	B14641	18'-0"



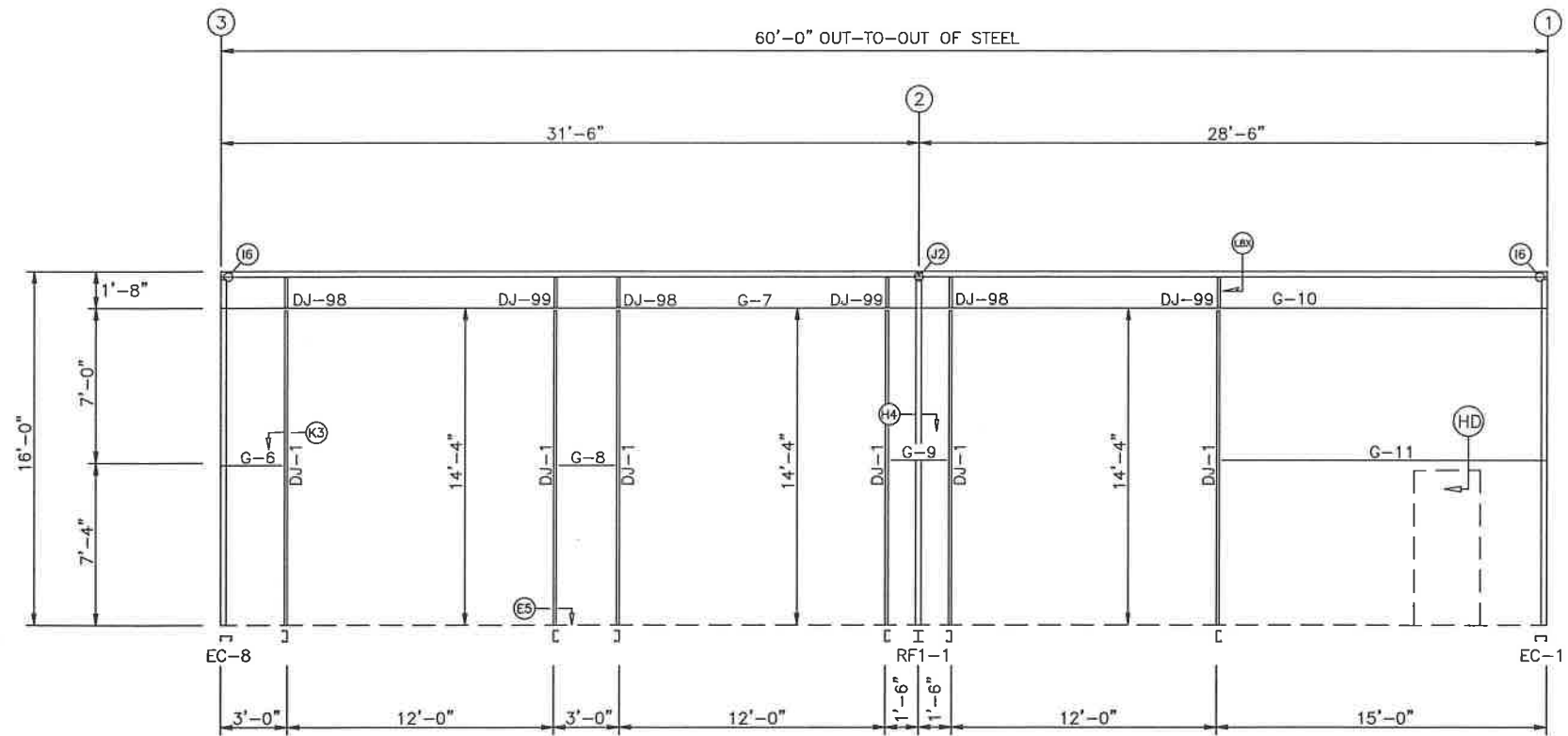
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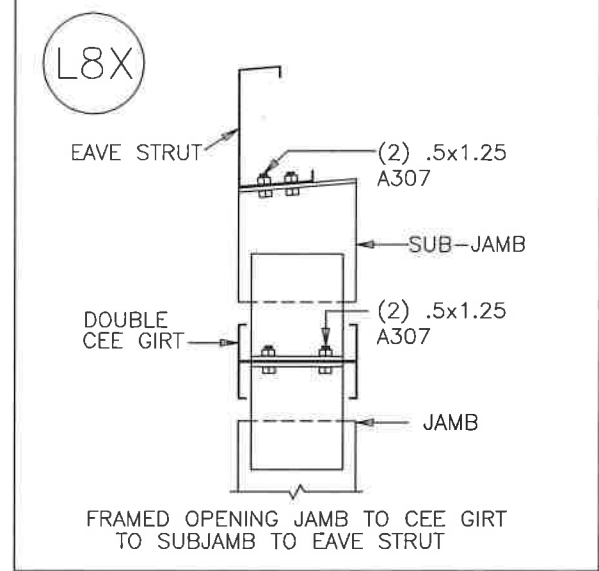
ISSUE		DET	CHK	DATE
RENEGADE STEEL BUILDINGS, INC.				
CUSTOMER: JUNIOR FAIRCLOTH				
JOB NO: 7871		DATE: 1/31/23		
LOCATION: DUNN, NC 28334				
DRAWING NAME: RIGID FRAME CROSS SECTION				
DRAWING NO: PAGE 2.2	DRAWN BY: DJH	CHECKED BY: SPW	SCALE: NONE	



SIDEWALL FRAMING: FRAME LINE A

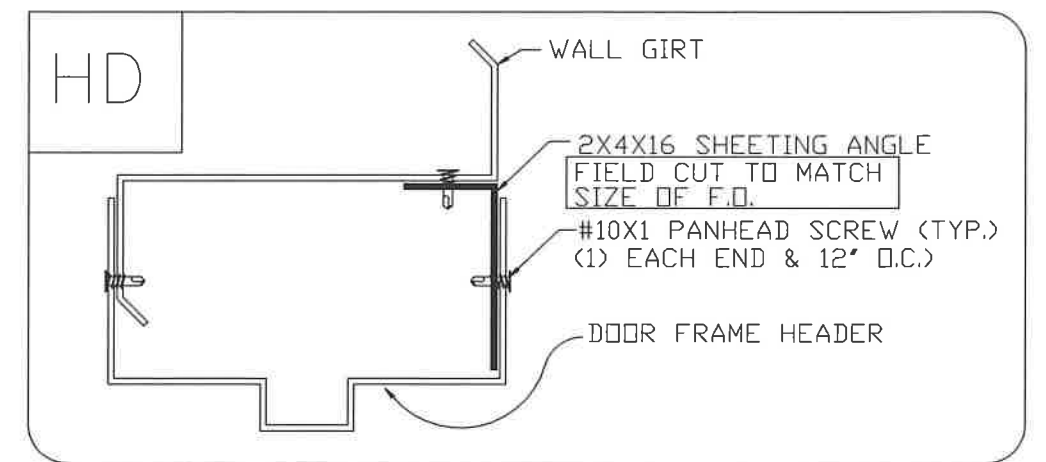


SIDEWALL FRAMING: FRAME LINE D

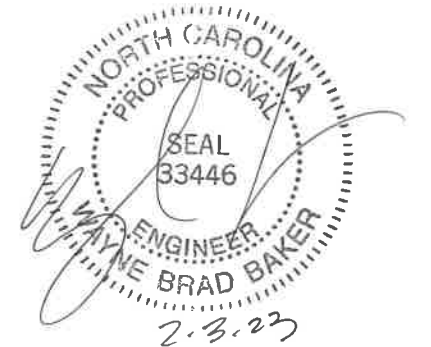


BOLT TABLE FRAME LINE A & D				
LOCATION	QUAN	TYPE	DIA	LENGTH
WF-1 - WF-2	8	A325	5/8"	2"
WF-1 - EC-4	8	A325	5/8"	2"
WF-1 - RF1-3	8	A325	5/8"	2"

MEMBER TABLE FRAME LINE A & D		
MARK	PART	LENGTH
WF-1	B14641	18'-0"
WF-2	B10651	24'-5 1/4"
DJ-1	8X35C16	14'-4"
DJ-98	8X35C16	1'-0 1/2"
DJ-99	8X35C16	1'-0 1/2"
G-4	8x25Z12	30'-5 1/2"
G-5	8x25Z12	33'-5 1/2"
G-6	8x25Z16	2'-7 1/2"
G-7	8X8DC14	31'-5 1/2"
G-8	8x25Z16	2'-3 1/2"
G-9	8x25Z16	2'-3 1/2"
G-10	8X7DC14	28'-5 1/2"
G-11	8x25Z16	14'-7 1/2"



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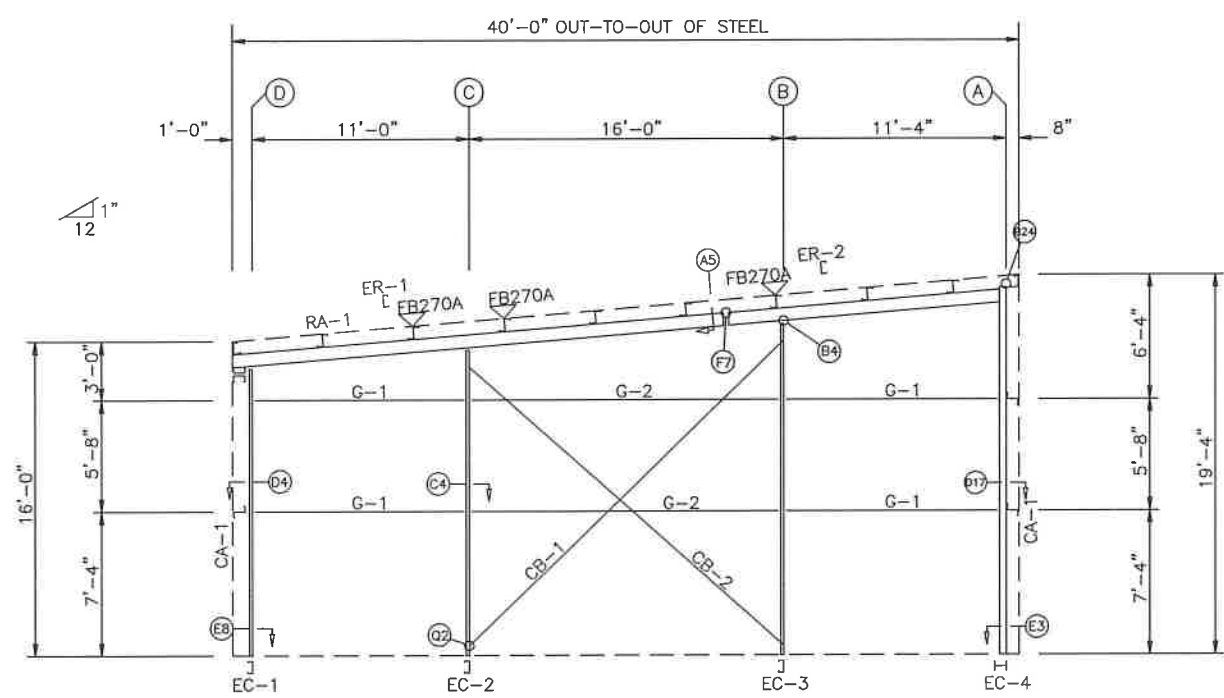


ALL VEHICULAR FRAMED OPENINGS SUPPLIED ON THIS PROJECT HAVE BEEN DESIGNED TO SUPPORT WIND LOADS NORMAL TO A DOOR SYSTEM, BASED ON THE STANDARD BUILDING CODE CRITERIA. THE VEHICULAR FRAMED OPENING HAS NOT BEEN DESIGNED FOR ANY ADDITIONAL MOMENT OR CATENARY FORCE FROM THE DOOR SYSTEM. ANY CHANGES TO THE INFORMATION SHOWN HERE WOULD REQUIRE AN ENGINEERING INVESTIGATION AND POSSIBLE BUILDING REINFORCEMENT.

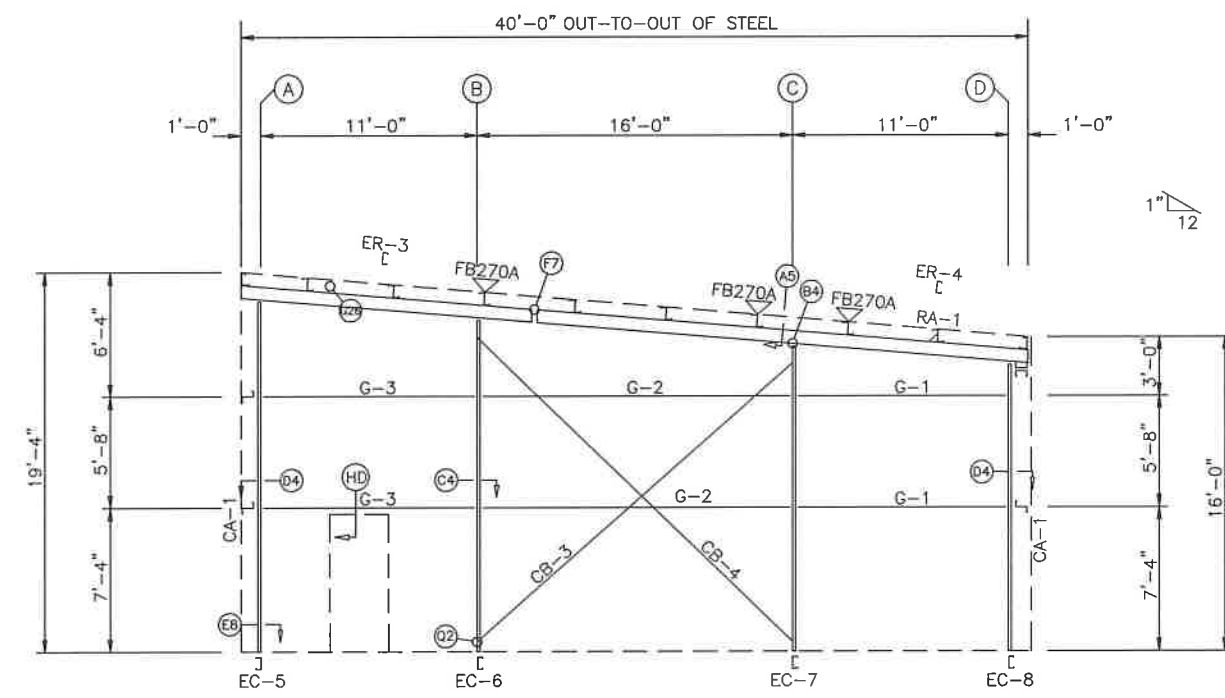
ISSUE		DET	CHK	DATE
RENEGADE STEEL BUILDINGS, INC.				
CUSTOMER: JUNIOR FAIRCLOTH				
JOB NO: 7871		DATE: 1/31/23		
LOCATION: DUNN, NC 28334				
DRAWING NAME: SIDEWALL FRAMING LAYOUT				
DRAWING NO: PAGE 3		DRAWN BY: DJH	CHECKED BY: SPW	SCALE: NONE

BOLT TABLE				
FRAME LINE 1 & 3				
LOCATION	QUAN	TYPE	DIA	LENGTH
EC-4/ER-2	4	A325	5/8"	2"
ER-1/ER-2	8	A325	5/8"	2"
ER-3/ER-4	8	A325	5/8"	2"
EC-1/ER-1	2	A325	5/8"	2"
Int_Column/Raf	2	A325	5/8"	2"
EC-5/ER-3	2	A325	5/8"	2"
EC-8/ER-4	2	A325	5/8"	2"

MEMBER TABLE		
FRAME LINE 1 & 3		
MARK	PART	LENGTH
EC-1	8X35C16	14'-6 15/16"
EC-2	8X35C12	15'-5 15/16"
EC-3	8X35C12	16'-9 15/16"
EC-4	B08541	18'-7 5/16"
EC-5	8X35C16	17'-8 15/16"
EC-6	8X35C12	16'-9 15/16"
EC-7	8X35C12	15'-5 15/16"
EC-8	8X35C16	14'-6 15/16"
ER-1	8X35C12	25'-1 9/16"
ER-2	8X35C12	13'-7 3/4"
ER-3	8X35C12	14'-11 9/16"
ER-4	8X35C12	25'-1 9/16"
G-1	8x25Z16	10'-7 1/2"
G-2	8x25Z16	15'-7 1/2"
G-3	8x25Z16	10'-11 1/2"
CB-1	5/16 CBL	23'-2"
CB-2	5/16 CBL	22'-3"
CB-3	1/4 CBL	22'-3"
CB-4	1/4 CBL	23'-2"



ENDWALL FRAMING: FRAME LINE 1



ENDWALL FRAMING: FRAME LINE 3

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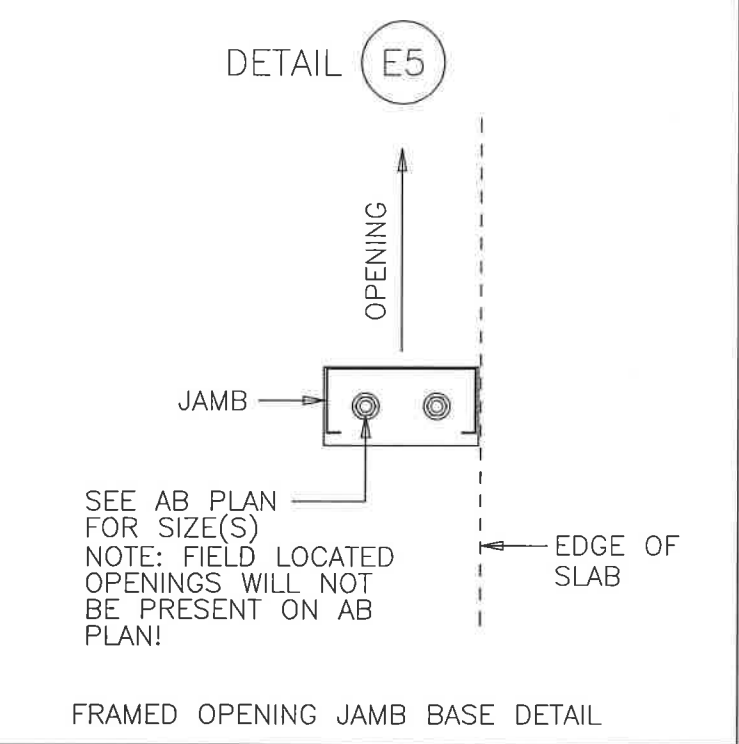
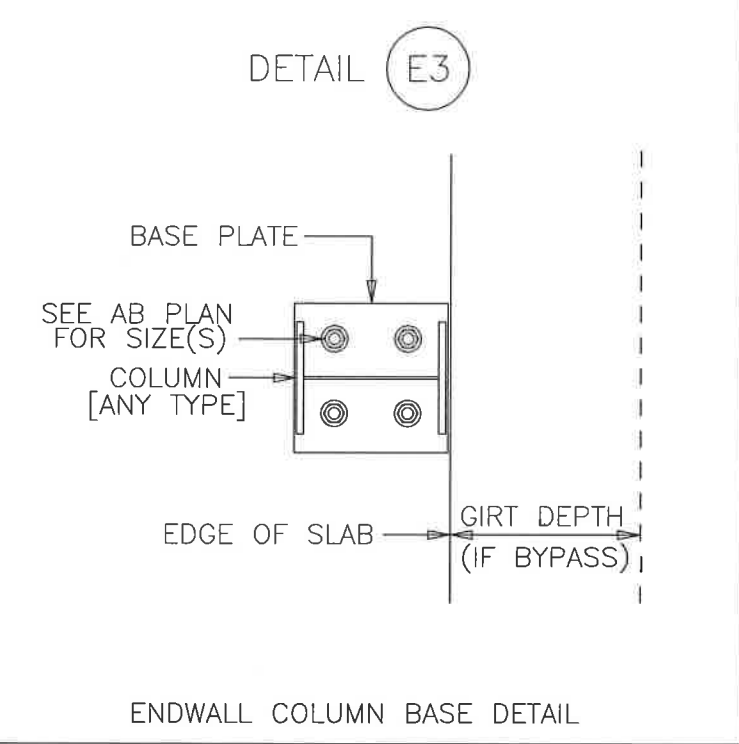
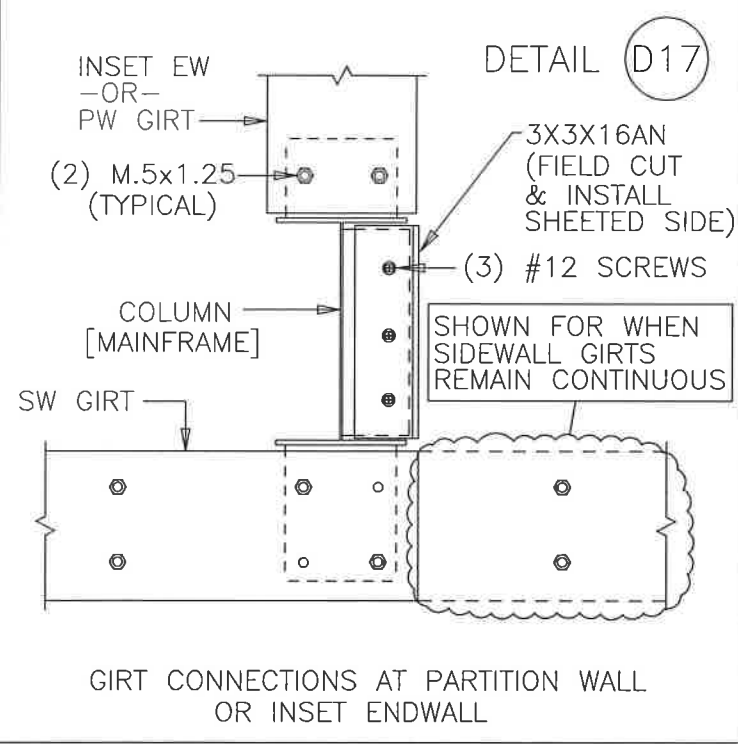
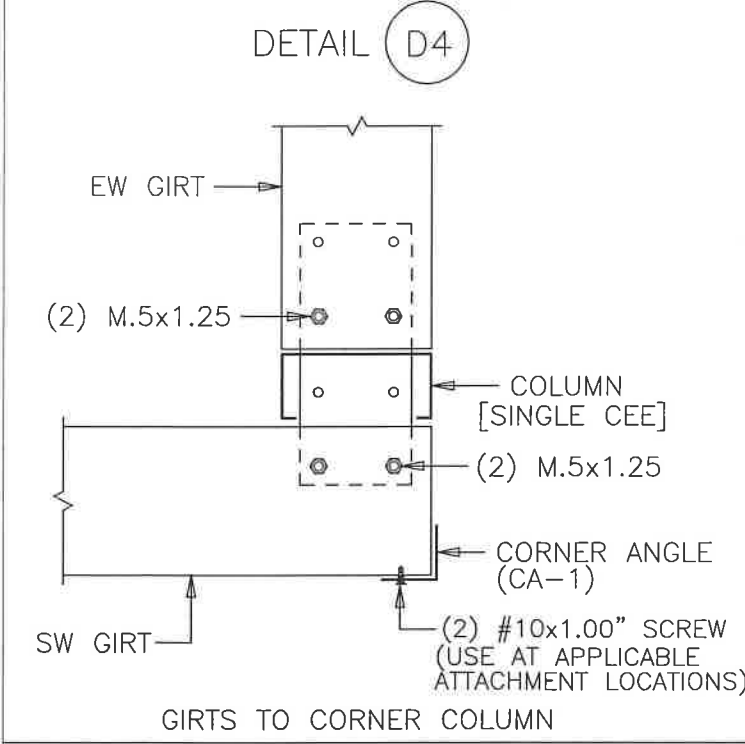
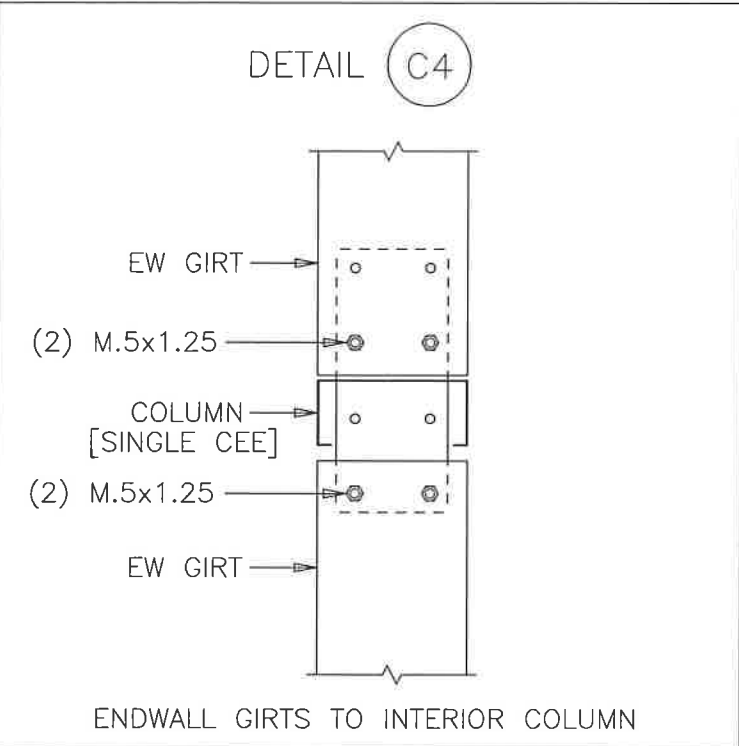
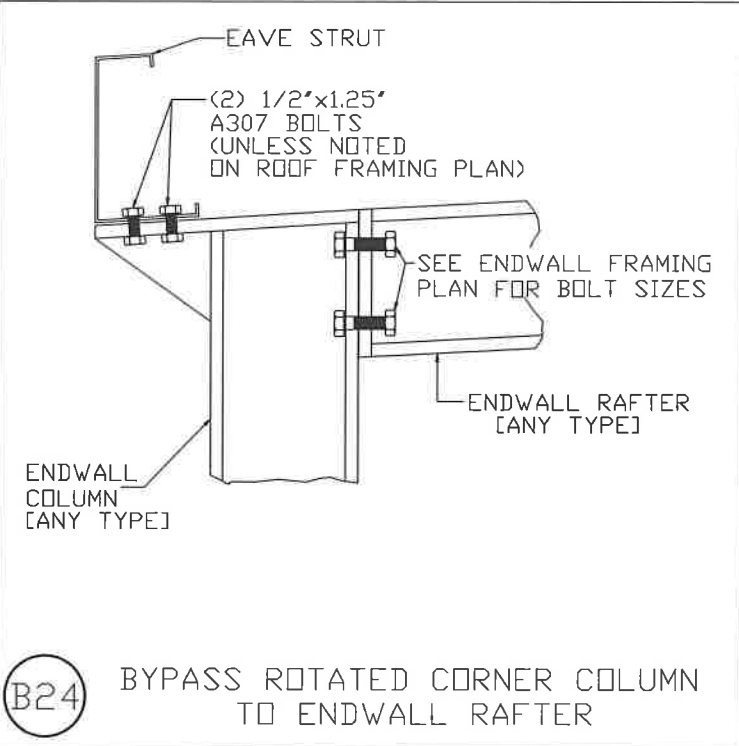
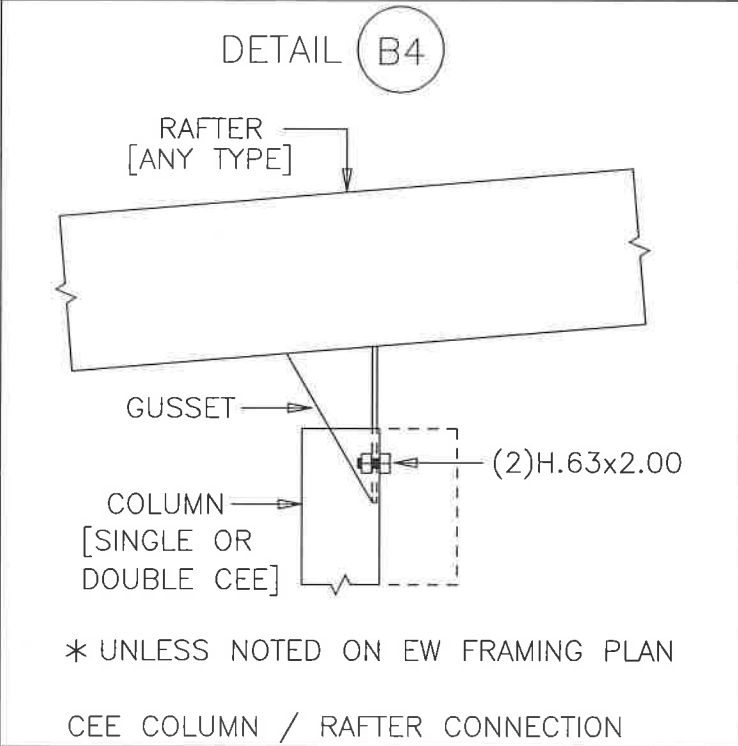
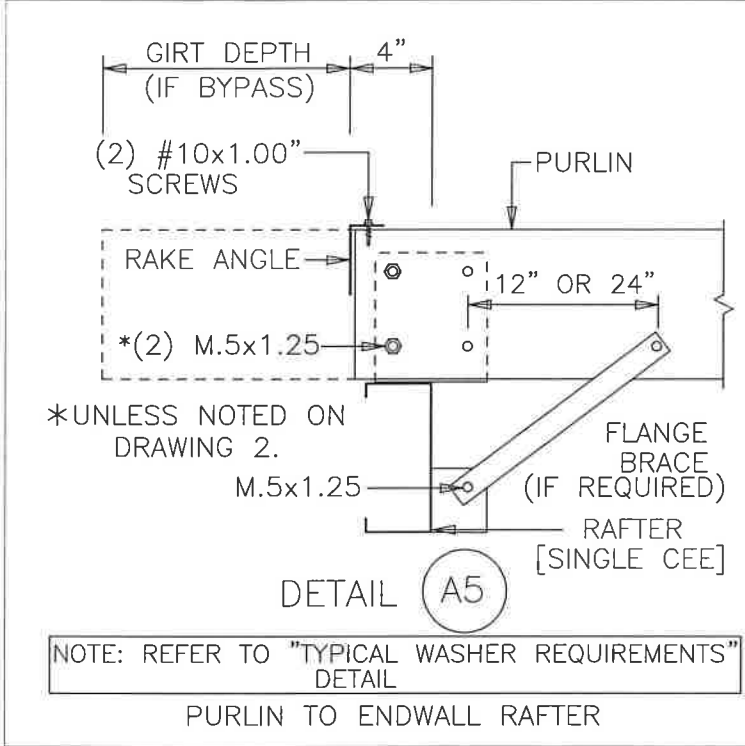


NOTE: FIELD SLOT GIRTS FOR CABLE PASSAGE

NOTE: THE FRAMING AS DEPICTED ABOVE IS NOT DESIGNED TO ACCOMMODATE ANY FUTURE EXPANSION.

ALL VEHICULAR FRAMED OPENINGS SUPPLIED ON THIS PROJECT HAVE BEEN DESIGNED TO SUPPORT WIND LOADS NORMAL TO A DOOR SYSTEM, BASED ON THE STANDARD BUILDING CODE CRITERIA. THE VEHICULAR FRAMED OPENING HAS NOT BEEN DESIGNED FOR ANY ADDITIONAL MOMENT OR CATENARY FORCE FROM THE DOOR SYSTEM. ANY CHANGES TO THE INFORMATION SHOWN HERE WOULD REQUIRE AN ENGINEERING INVESTIGATION AND POSSIBLE BUILDING REINFORCEMENT.

ISSUE		DET	CHK	DATE
RENEGADE STEEL BUILDINGS, INC.				
CUSTOMER: JUNIOR FAIRCLOTH				
JOB NO: 7871		DATE: 1/31/23		
LOCATION: DUNN, NC 28334				
DRAWING NAME: ENDWALL FRAMING LAYOUT				
DRAWING NO: PAGE 4		DRAWN BY: DJH	CHECKED BY: SPW	SCALE: NONE



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 235 Sanders Rd.
 Hahira, GA 31632

ISSUE	DET	CHK	DATE

RENEGADE STEEL BUILDINGS, INC.

CUSTOMER: JUNIOR FAIRCLOTH

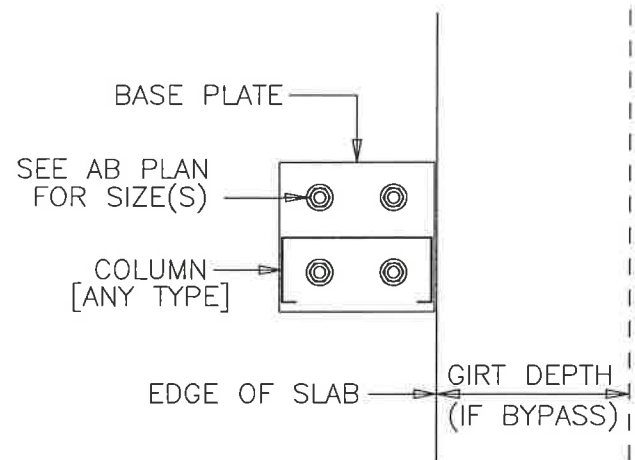
JOB NO: 7871 DATE: 1/31/23

LOCATION: DUNN, NC 28334

DRAWING NAME: FRAMING DETAILS

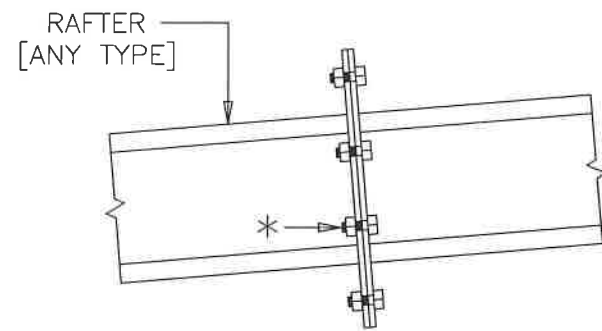
DRAWING NO: PAGE 5 DRAWN BY: DJH CHECKED BY: SPW SCALE: NONE

DETAIL (E8)



ENDWALL COLUMN BASE DETAIL

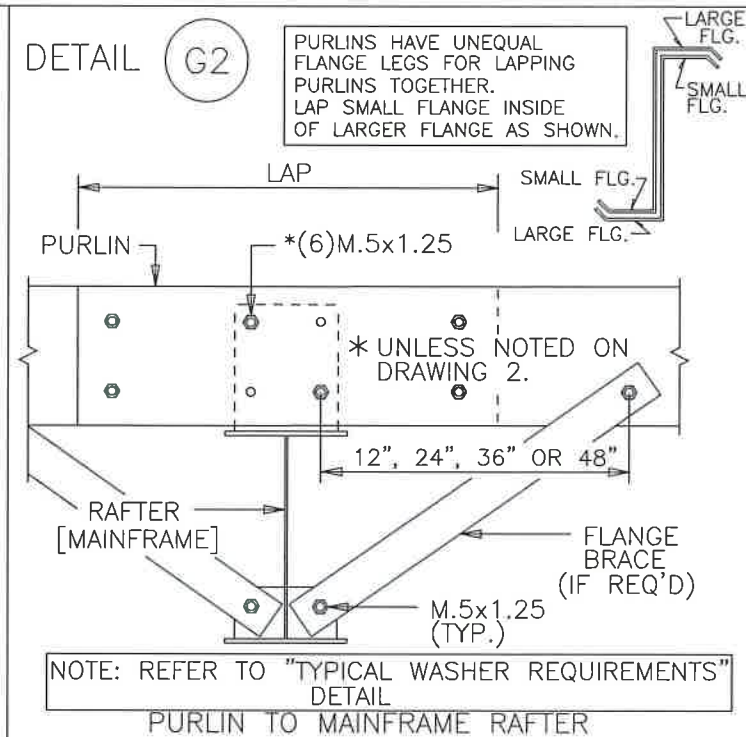
DETAIL (F7)



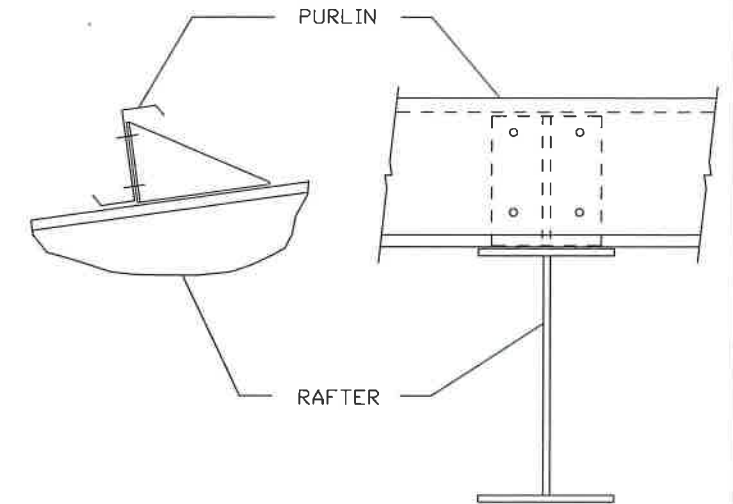
* SEE EW FRAMING PLAN FOR SIZE AND QUANTITY OF BOLTS.

RAFTER DETAIL AT SPLICE

DETAIL (G2)

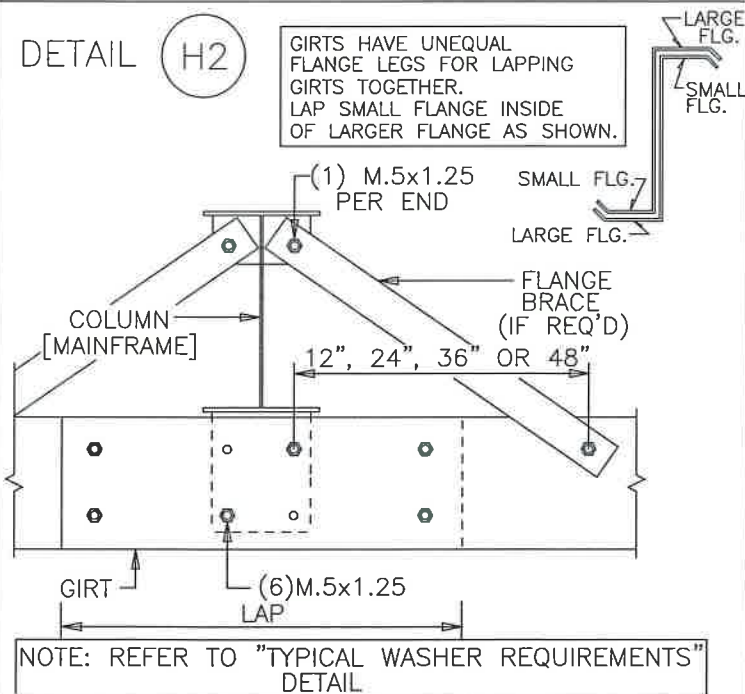


PURLIN TO MAINFRAME RAFTER



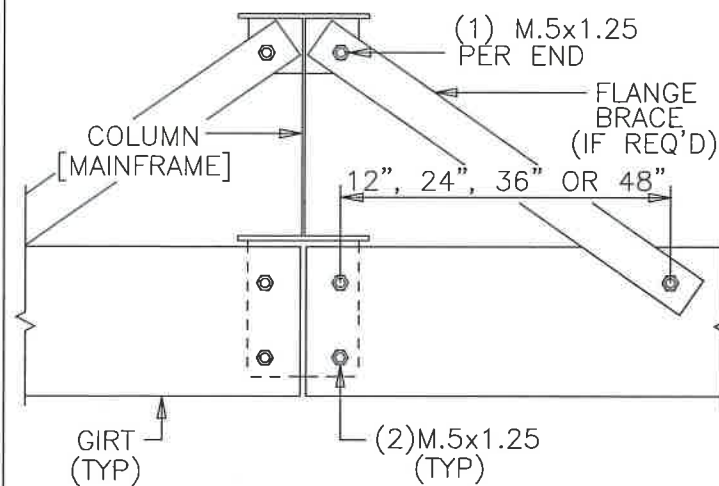
G26 WELDED ANTI-ROLL CLIP

DETAIL (H2)



GIRT TO MAINFRAME COLUMN

DETAIL (H4)

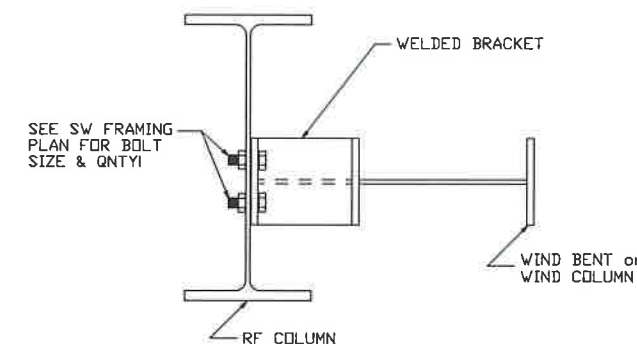


NOTE: REFER TO "TYPICAL WASHER REQUIREMENTS" DETAIL

GIRT TO MAINFRAME COLUMN

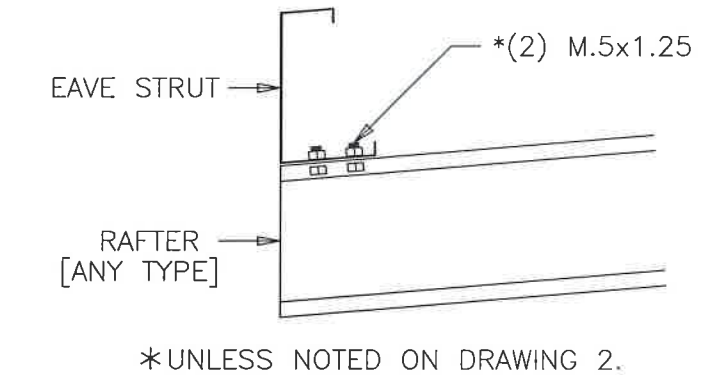
H10

WIND BENT OR WIND COLUMN TO BUILDING COLUMN



I6

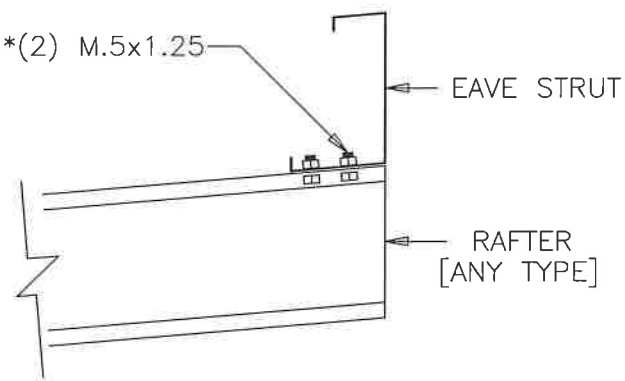
EAVE STRUT CONNECTION AT ENDWALL



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Hahira, GA 31632

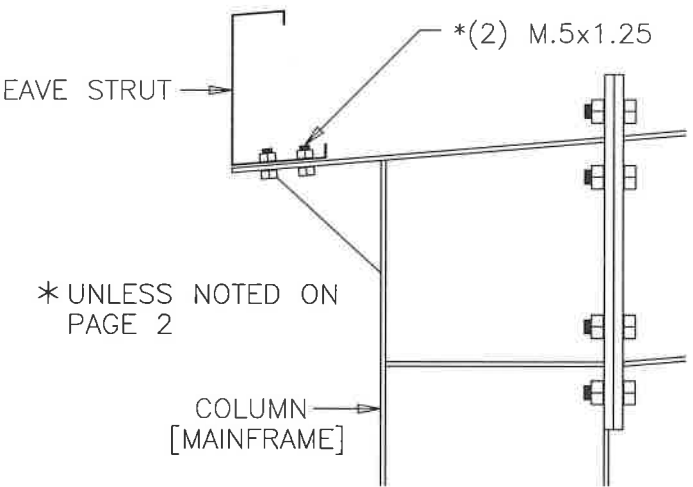
ISSUE		DET	CHK	DATE
RENEGADE STEEL BUILDINGS, INC.				
CUSTOMER: JUNIOR FAIRCLOTH				
JOB NO: 7871		DATE: 1/31/23		
LOCATION: DUNN, NC 28334				
DRAWING NAME: FRAMING DETAILS				
DRAWING NO: PAGE 5 1		DRAWN BY: D.IH	CHECKED BY: SPW	SCALE: NONE

DETAIL I19

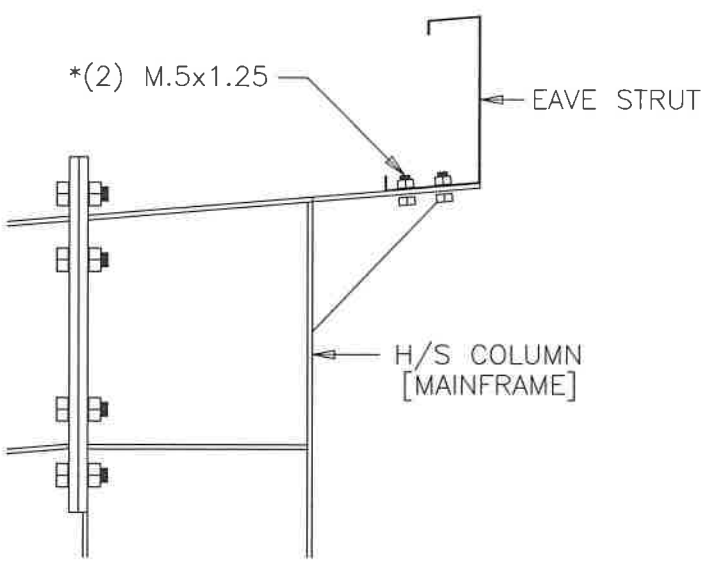


*UNLESS NOTED ON DRAWING 2.

EAVE STRUT CONNECTION AT ENDWALL

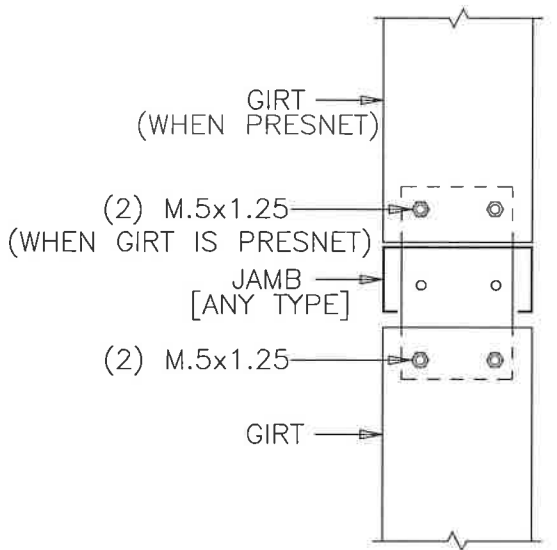


J2 EAVE STRUT CONNECTION AT MAINFRAME



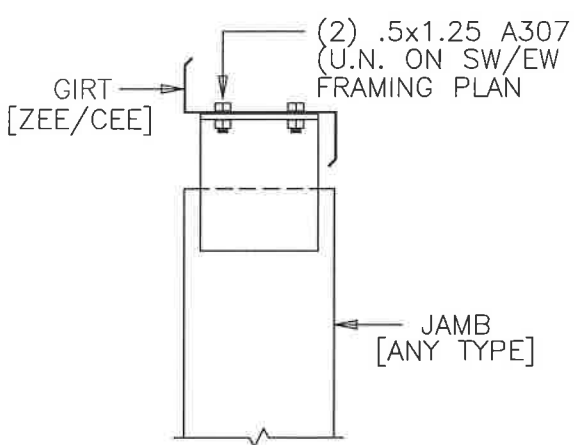
J8 EAVE STRUT CONNECTION AT MAINFRAME

DETAIL K3



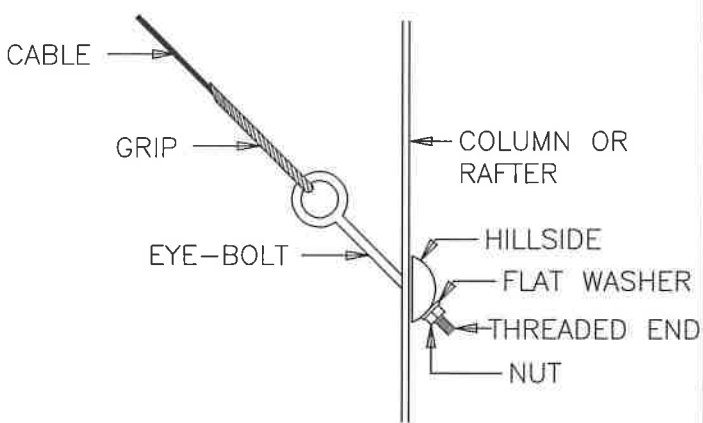
GIRTS TO JAMB

DETAIL L8



FRAMED OPENING JAMB TO GIRT

Q2 CABLE INSTALLATION DETAIL

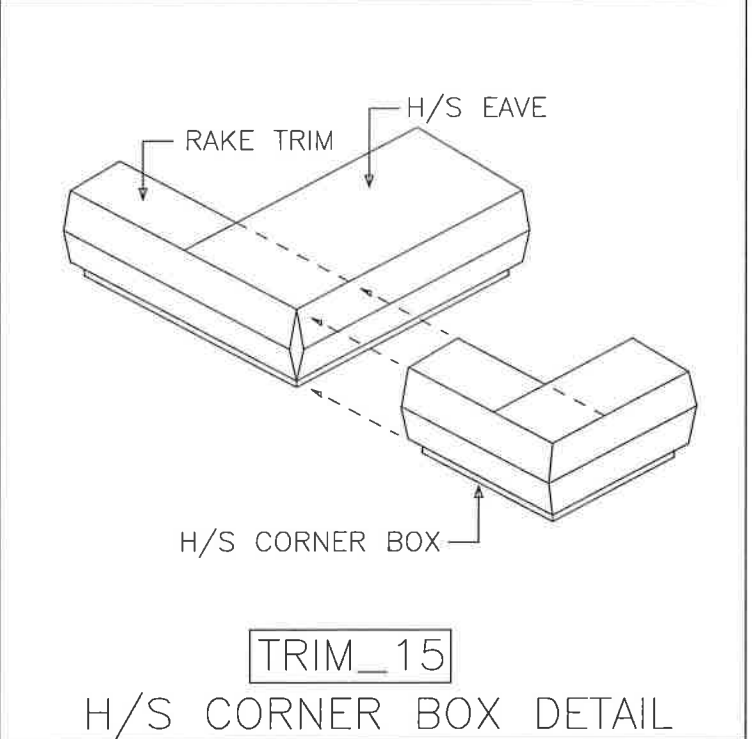
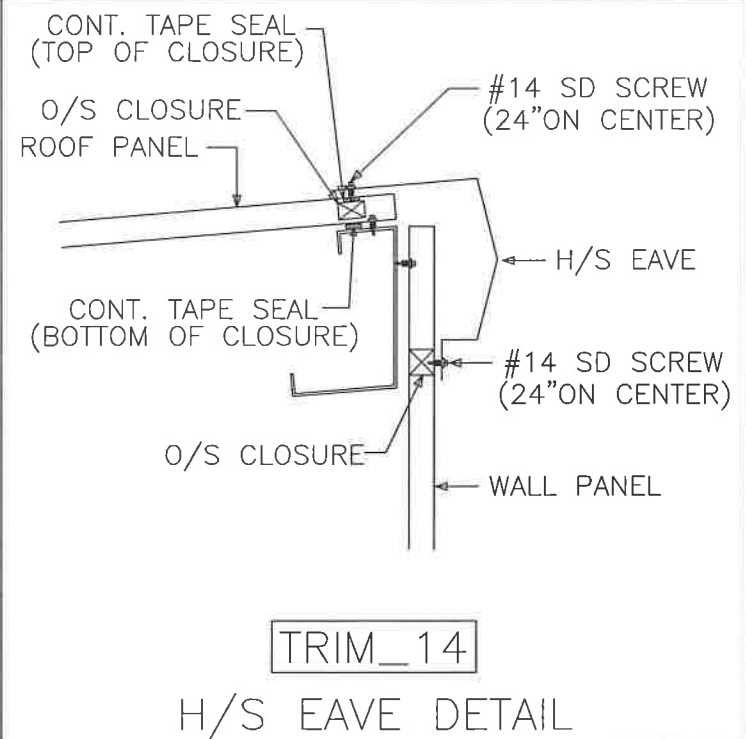
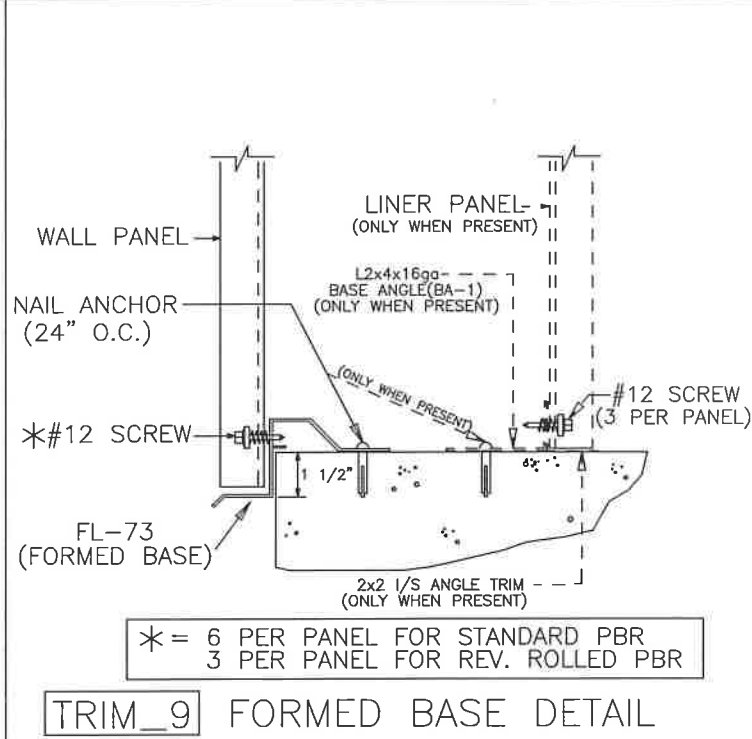
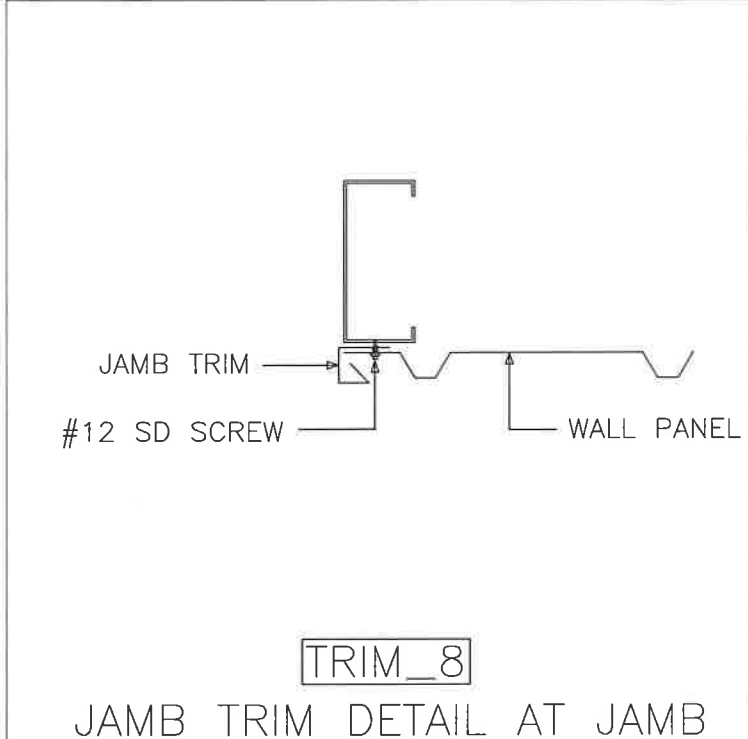
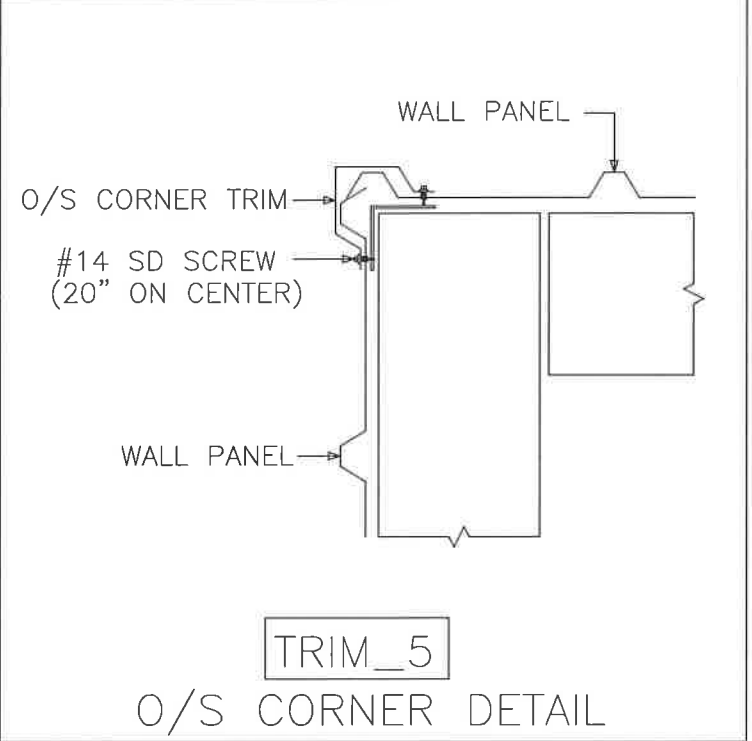
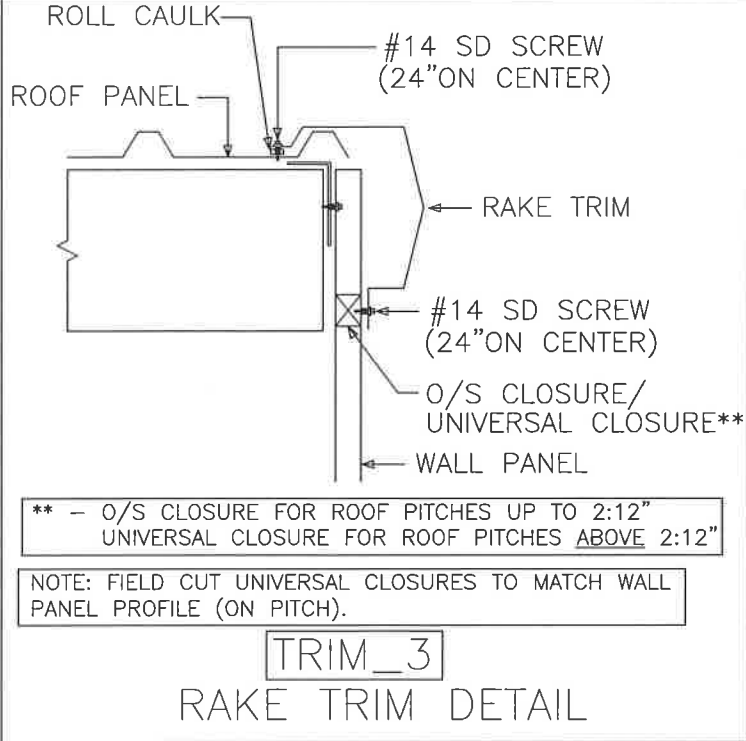
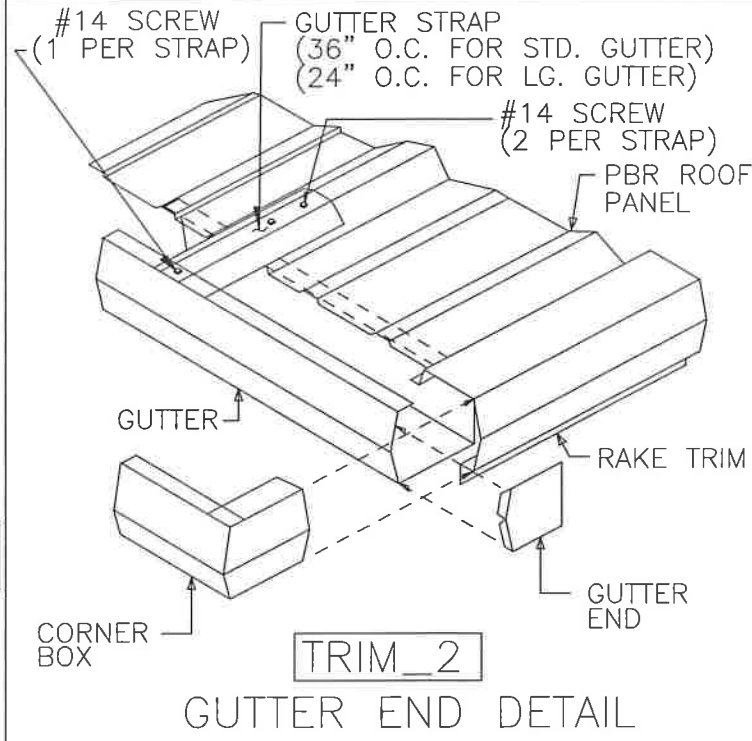
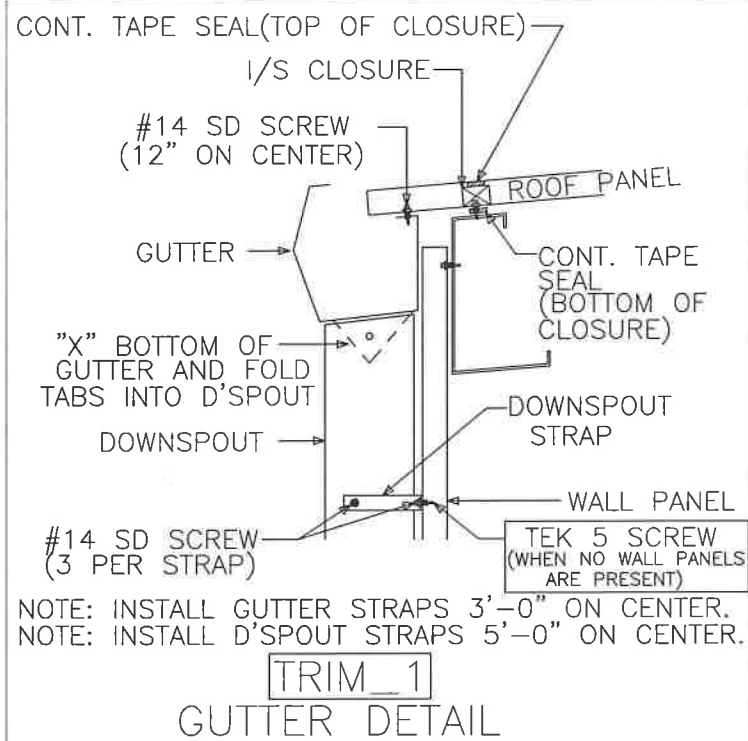


NOTE: WHEN FLUSH GIRTS/PURLINS ARE USED, FIELD SLOT GIRT OR PURLIN AS REQ'D FOR CABLE/ROD PASSAGE THROUGH PURLIN/GIRT.

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235 Sanders Rd.
Hahira, GA 31632

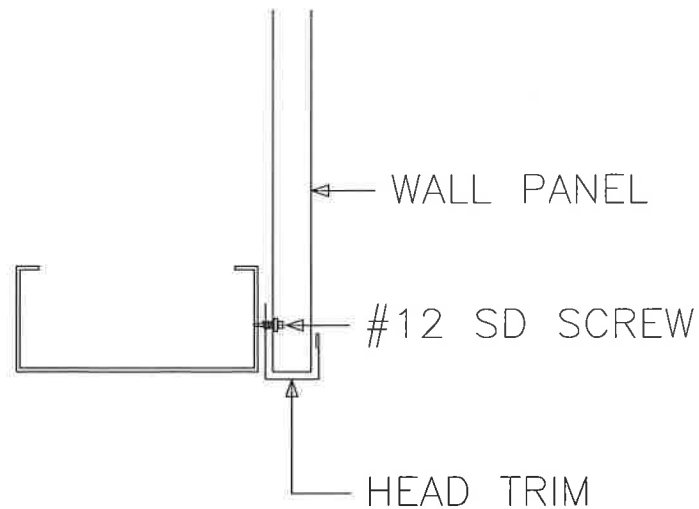


ISSUE		DET	CHK	DATE
RENEGADE STEEL BUILDINGS, INC.				
CUSTOMER: JUNIOR FAIRCLOTH				
JOB NO: 7871		DATE: 1/31/23		
LOCATION: DUNN, NC 28334				
DRAWING NAME: FRAMING DETAILS				
DRAWING NO: PAGE 5.2		DRAWN BY: DJH	CHECKED BY: SPW	SCALE: NONE



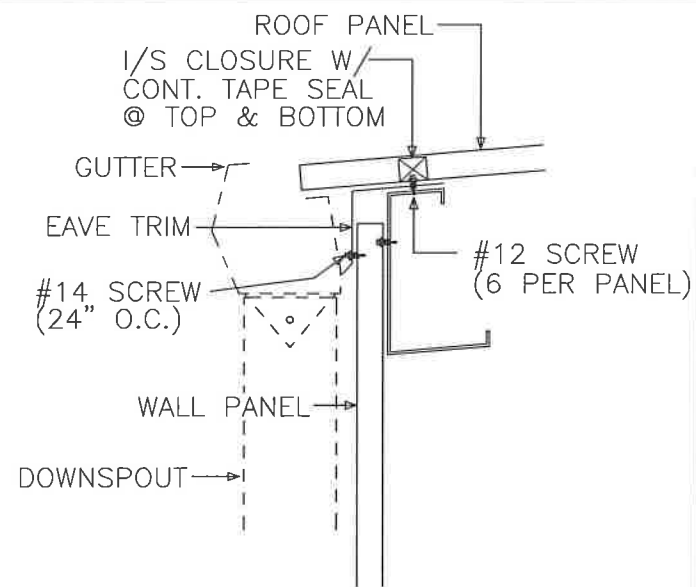
Wayne Brad Baker, P.E.
235 Sanders Rd.
Hahira, GA 31632

ISSUE		DET	CHK	DATE
RENEGADE STEEL BUILDINGS, INC.				
CUSTOMER: JUNIOR FAIRCLOTH				
JOB NO: 7871		DATE: 1/31/23		
LOCATION: DUNN, NC 28334				
DRAWING NAME: FRAMING DETAILS				
DRAWING NO: PAGE 5.3	DRAWN BY: DJH	CHECKED BY: SPW	SCALE: NONE	



TRIM_61

HEAD TRIM DETAIL AT HEADER



TRIM_120

EAVE/GUTTER TRIM DETAIL

STRUCTURAL BOLTED CONNECTIONS

REFER TO COVER PAGE "GENERAL NOTES" PARAGRAPH "C", SECTION "9" FOR INSTRUCTIONS ON TIGHTENING ALL A325 AND A490 CONNECTION BOLTS.

TRIM NOTES:

- [1] SEAL TRIM SPLICES WITH TUBE CAULK.
- [2] SECURE GUTTER SPLICES AND END PLUGS WITH RIVETS.
- [3] SECURE ALL OTHER ROOF TRIM SPLICES WITH TRIM SCREWS UNLESS NOTED OTHERWISE.
- [4] TRIM SCREWS ARE LOCATED 24" ON CENTER UNLESS NOTED OTHERWISE.
- [5] STD. TRIM SPLICES ARE 3" TOTAL UNLESS NOTED OTHERWISE.

MORTISE PREPPED PERSONNEL DOORS

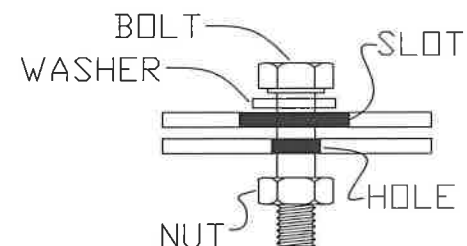
ALL MORTISE PREPPED PERSONNEL DOORS COME AS RIGHTHAND REVERSED SWING.

(i.e. STANDING ON THE OUTSIDE OF THE BUILDING FACING THE DOOR, THE LOCK WILL BE ON THE LEFTHAND SIDE OF THE DOOR AND THE DOOR WILL SWING OUTWARD FROM THE BUILDING.)

ANY FIELD MODIFICATIONS ARE THE RESPONSIBILITY OF THE ERECTOR AND MBM IS NOT LIABLE FOR LABOR CHARGES NOR DAMAGES DUE TO ERROR.

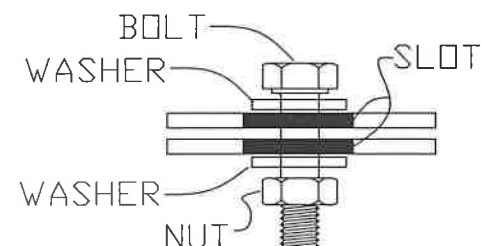
TYPICAL WASHER REQUIREMENTS (UNLESS NOTED OTHERWISE ON DRAWINGS)

SLOT TO HOLE CONN.
WASHER REQUIRED ON SLOTTED SIDE



HOLE TO HOLE CONN.
NO WASHERS REQ'D.

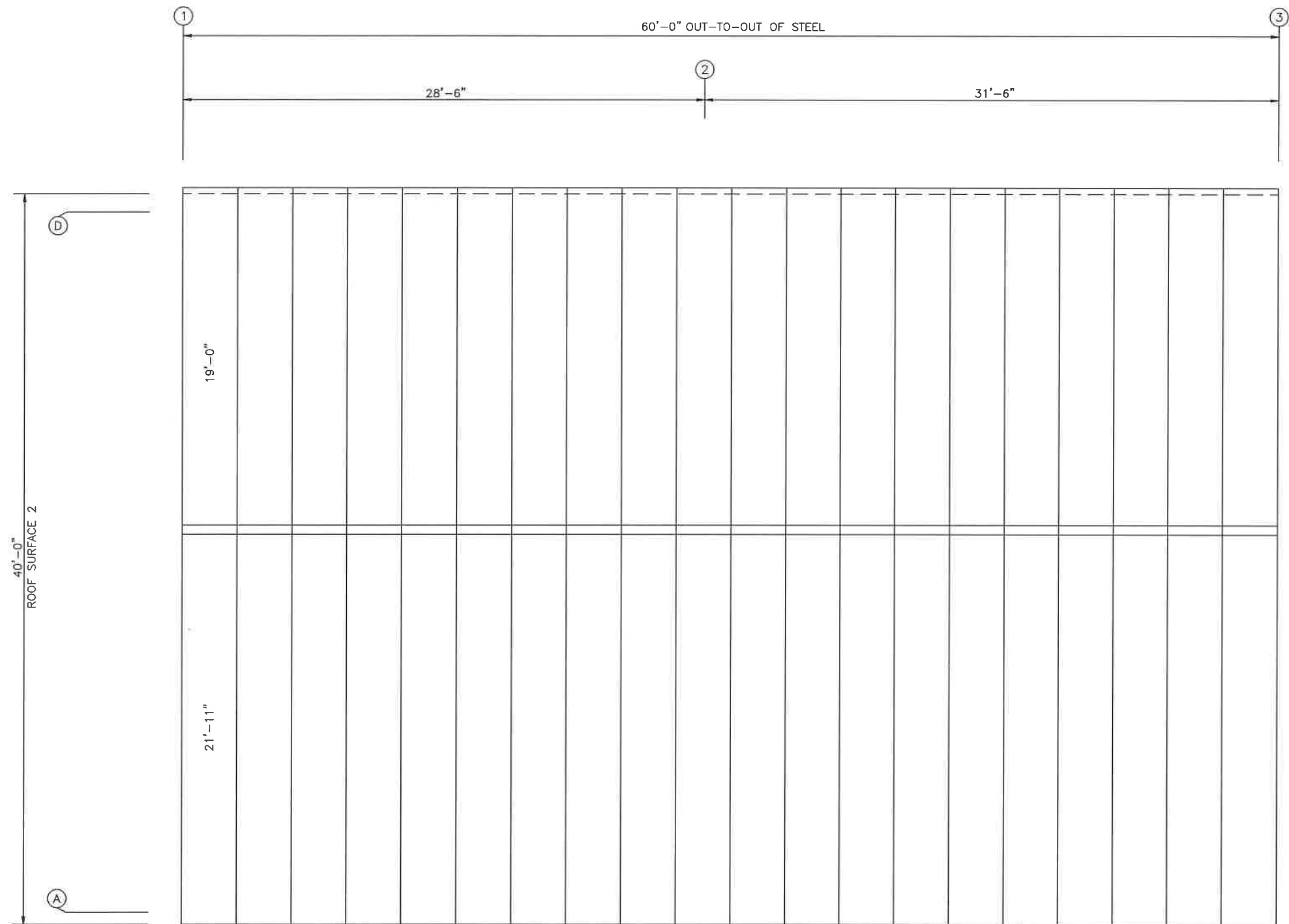
SLOT TO SLOT CONN.
WASHERS ARE REQUIRED ON EACH SLOTTED SIDE.
(**WASHER(S) NOT REQ'D WITHIN LAPPED ZEE PURLIN/GIRT AREAS**)



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RENEGADE STEEL BUILDINGS, INC.			
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JOB NO: 7871	DATE: 1/31/23		
LOCATION: DUNN, NC 28334			
DRAWING NAME: FRAMING DETAILS			
DRAWING NO: PAGE 5.4	DRAWN BY: DJH	CHECKED BY: SPW	SCALE: NONE



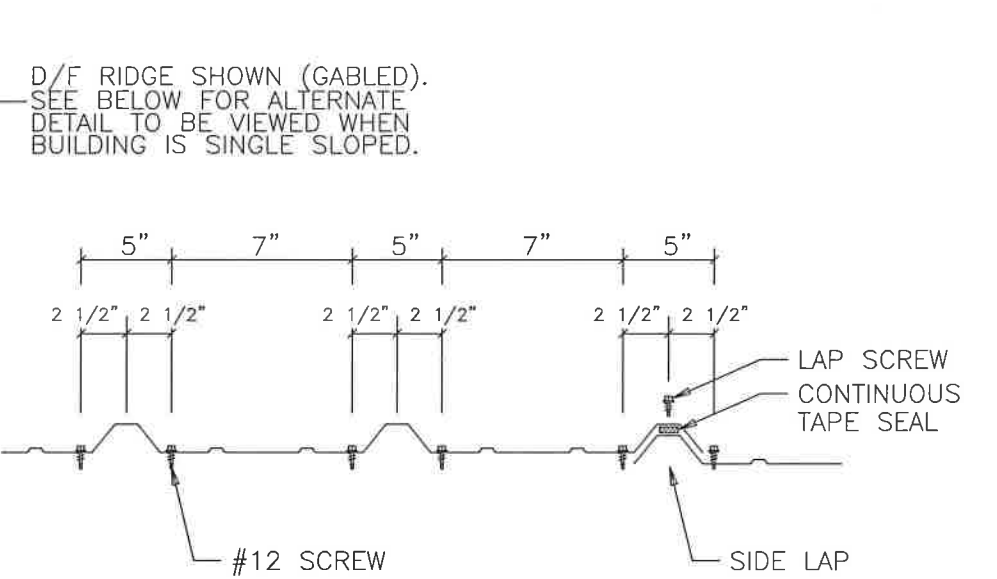
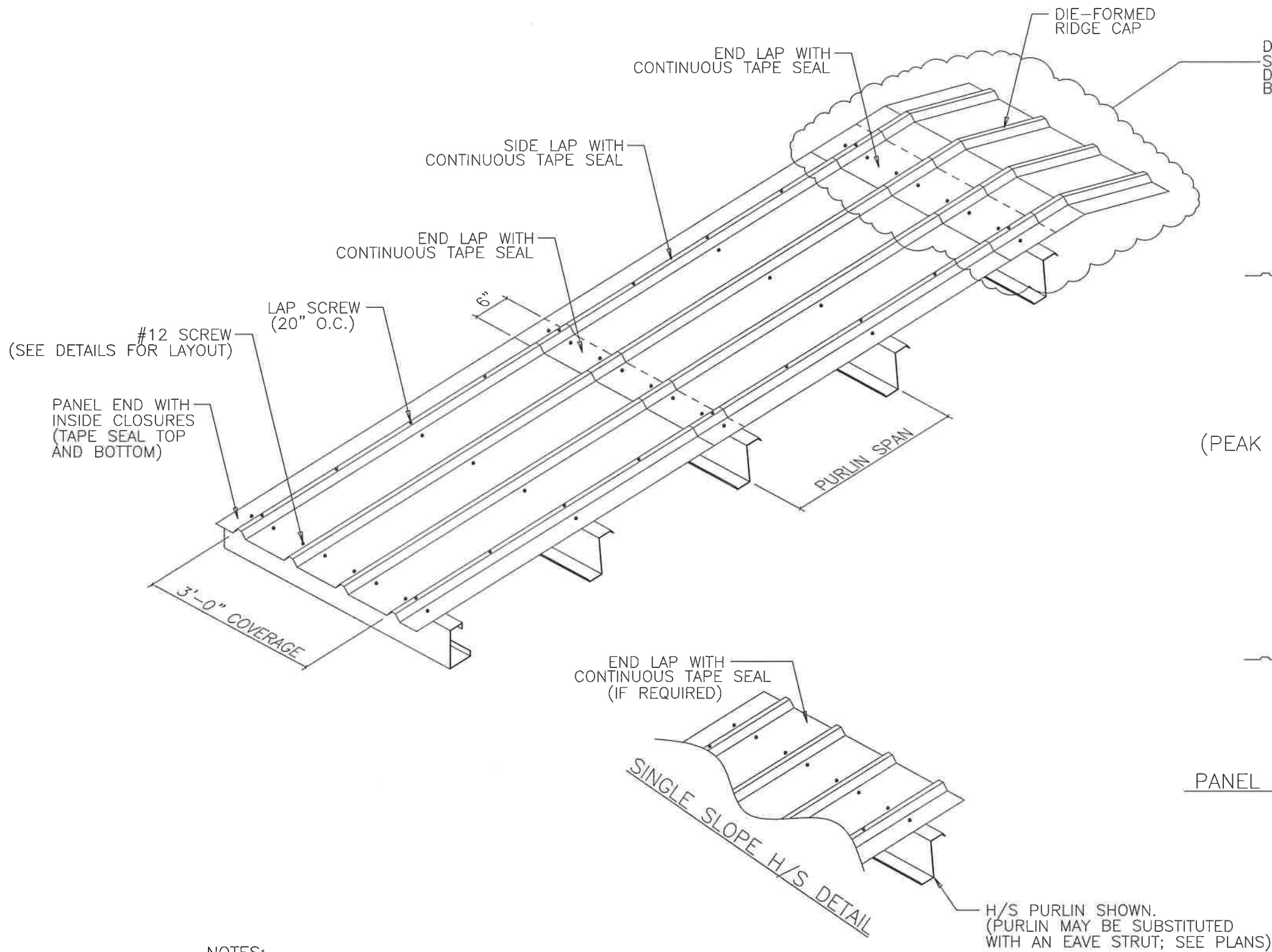
ROOF SHEETING PLAN
PANELS: 26 GA. PBR - GALVALUME

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235 Sanders Rd.
Hahira, GA 31632

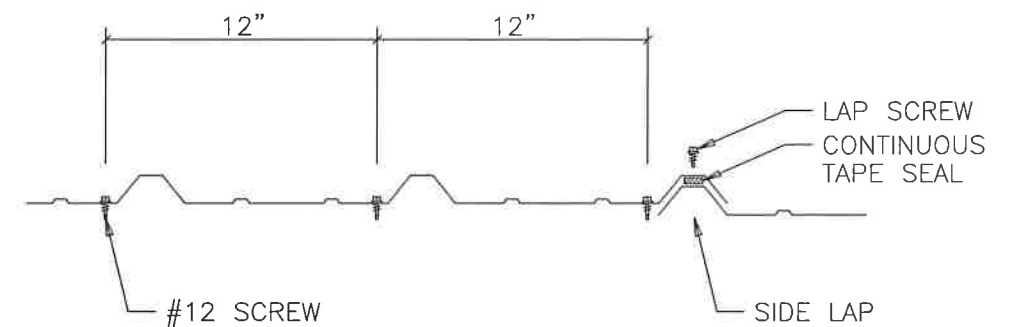


ISSUE		DET	CHK	DATE
RENEGADE STEEL BUILDINGS, INC.				
CUSTOMER: JUNIOR FAIRCLOTH				
JOB NO: 7871		DATE: 1/31/23		
LOCATION: DUNN, NC 28334				
DRAWING NAME: ROOF PANELS & TRIM				
DRAWING NO: PAGE 6		DRAWN BY: DJH	CHECKED BY: SPW	SCALE: NONE

HORNET STEEL BUILDINGS, INC.
GA COMP: PEF008160
EXPIRATION: 06/30/2024



PANEL ATTACHMENT AT PANEL END
(PEAK PURLIN, EAVE STRUT, AND PANEL END LAPS)



PANEL ATTACHMENT AT INTERMEDIATE MEMBERS

NOTES:

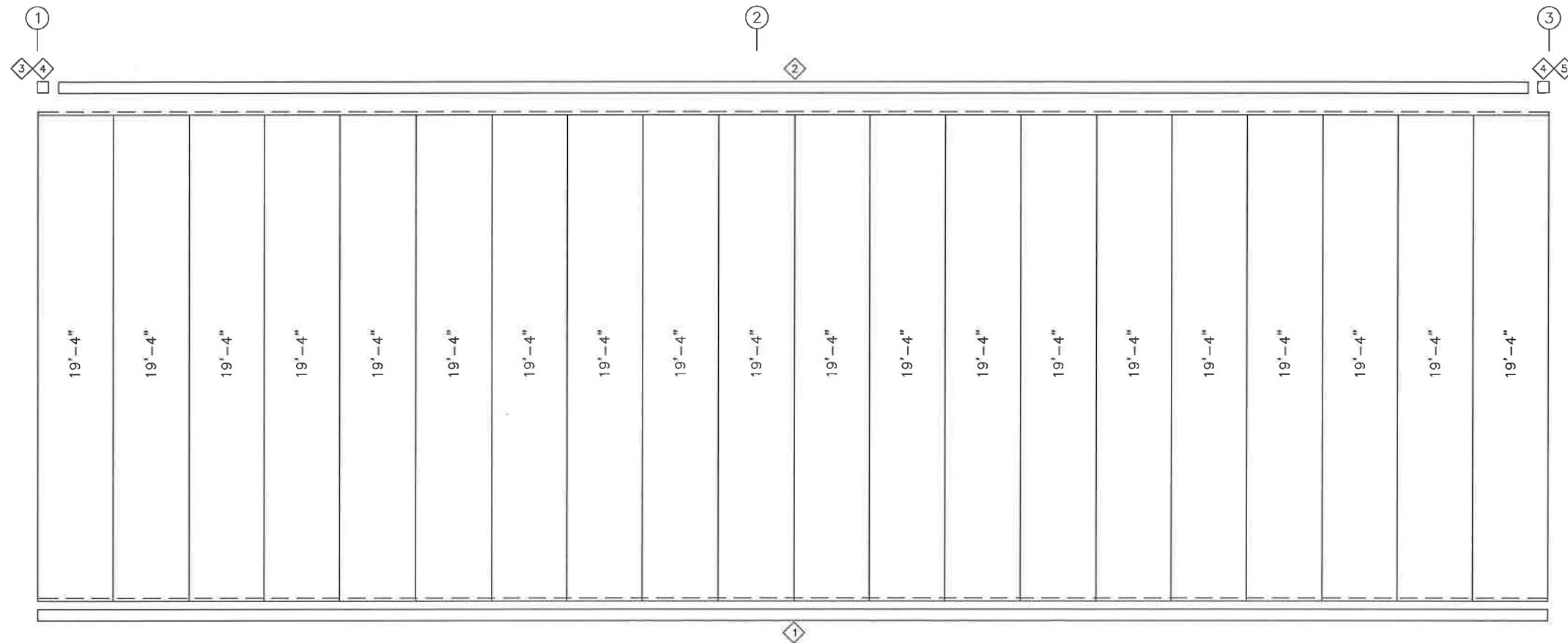
- [1] ALL END LAPS MUST BE A MINIMUM OF 6".
- [2] METAL SHAVINGS MUST BE SWEEPED FROM THE ROOF EACH DAY DURING ERECTION TO PREVENT SURFACE RUSTING.
- [3] TAPE SEAL MUST BE APPLIED WITH NO GAPS OR BREAKS.
- [4] #12 SCREWS ARE USED TO ATTACH THE PANEL TO THE PURLINS. #14 LAP SCREWS ARE USED AT THE PANEL-TO-PANEL ATTACHMENTS. ALL FASTENERS ARE SELF-DRILLING.

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Hahira, GA 31632

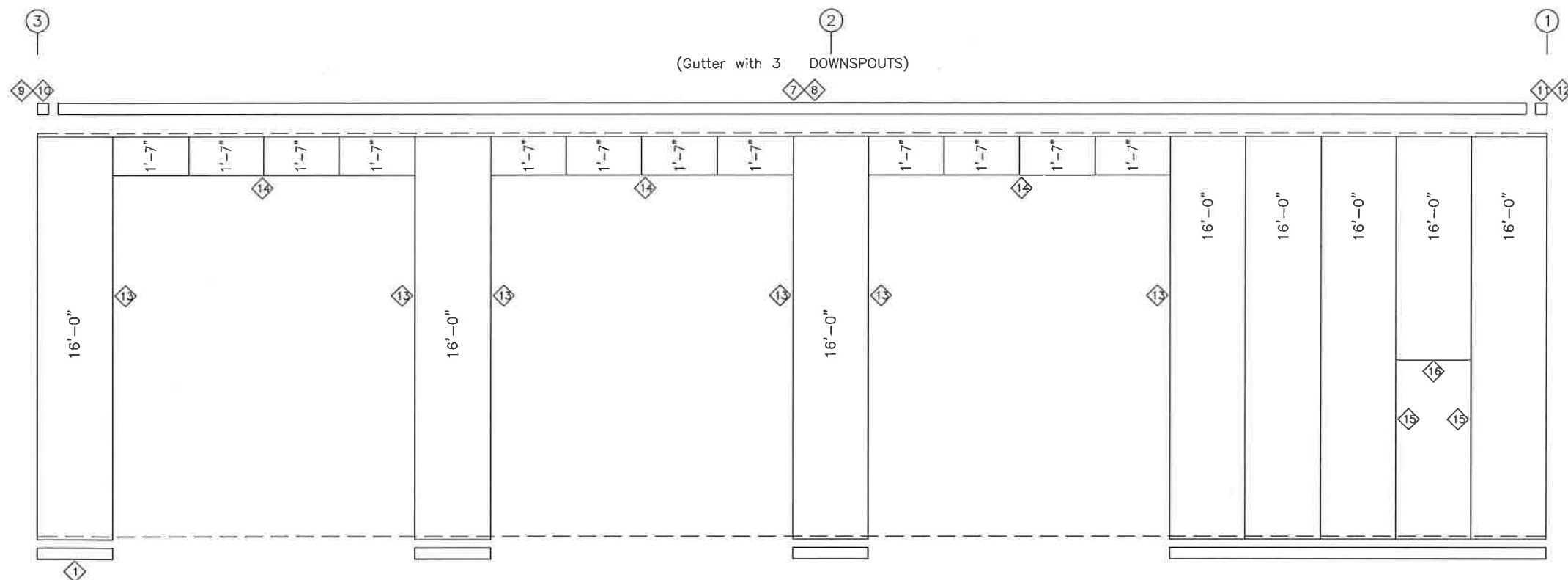


ISSUE		DET	CHK	DATE
RENEGADE STEEL BUILDINGS, INC.				
CUSTOMER: JUNIOR FAIRCLOTH				
JOB NO: 7871		DATE: 1/31/23		
LOCATION: DUNN, NC 28334				
DRAWING NAME: ROOF PANEL DETAILS				
DRAWING NO: PAGE 6.1		DRAWN BY: D.JH	CHECKED BY: SPW	SCALE: NONE

TRIM TABLE			
FRAME LINE A & D			
◇ID	PART	LENGTH	DETAIL
1	FL-73	20'-0"	TRIM_9
2	H/S EAVE	20'-3"	TRIM_14
3	HS COR L	1'-0"	TRIM_15
4	RAKE BOX	1'-4"	
5	HS COR R	1'-0"	TRIM_15
7	GUTTER	20'-3"	TRIM_1
8	EAVE TRM	20'-3"	TRIM_120
9	GUTEND L	1"	TRIM_2
10	CORBOX L	1'-0"	TRIM_2
11	GUTEND R	1"	TRIM_2
12	CORBOX R	1'-0"	TRIM_2
13	R JAMB	14'-7"	TRIM_8
14	R HEAD	12'-3"	TRIM_61
15	R JAMB	7'-3"	TRIM_8
16	R HEAD	3'-3"	TRIM_61



SIDEWALL SHEETING & TRIM: FRAME LINE A
PANELS: 26 GA. PBR - LIGHT STONE

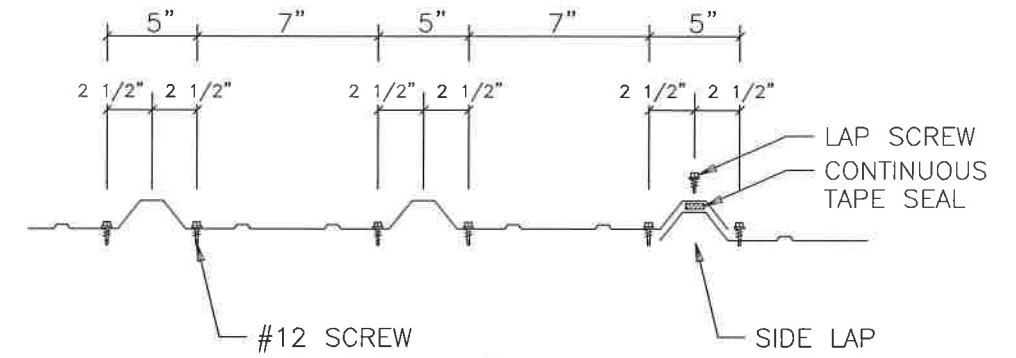
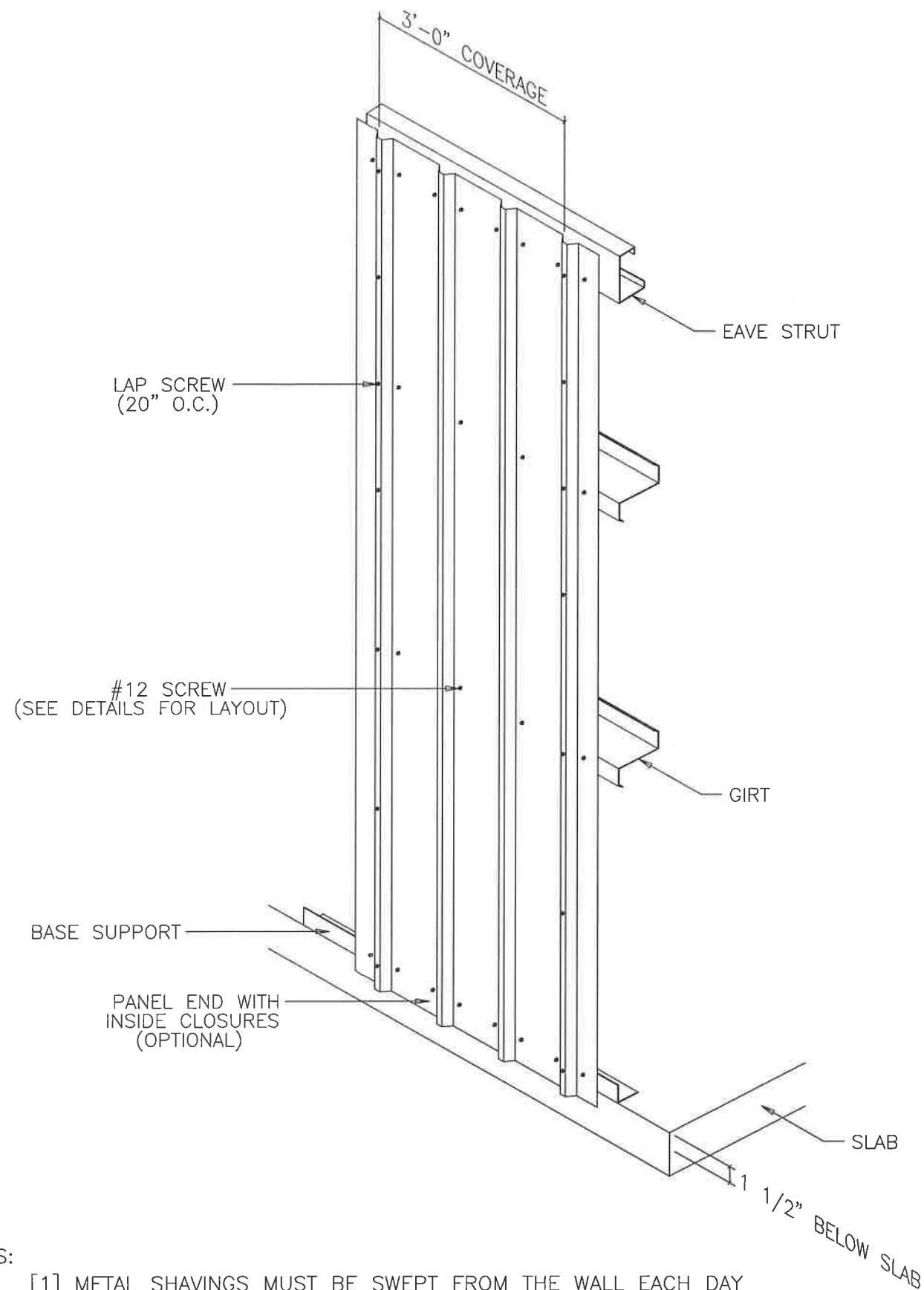


SIDEWALL SHEETING & TRIM: FRAME LINE D
PANELS: 26 GA. PBR - LIGHT STONE

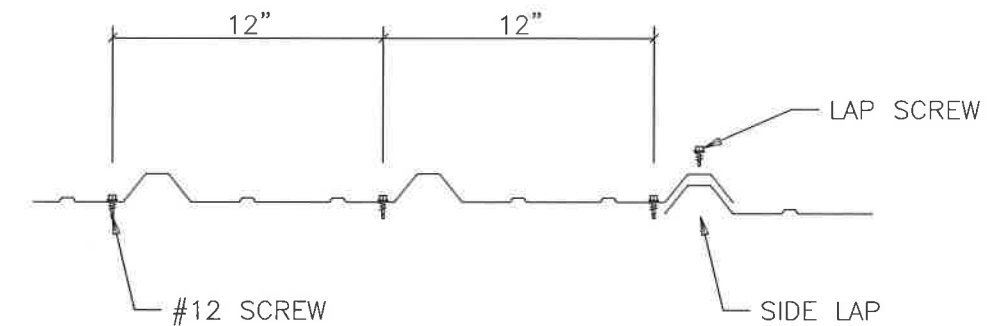
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235 Sanders Rd.
Hahira, GA 31632



ISSUE		DET	CHK	DATE
RENEGADE STEEL BUILDINGS, INC.				
CUSTOMER: JUNIOR FAIRCLOTH				
JOB NO: 7871		DATE: 1/31/23		
LOCATION: DUNN, NC 28334				
DRAWING NAME: SIDEWALL PANELS & TRIM				
DRAWING NO: PAGE 7		DRAWN BY: DJH	CHECKED BY: SPW	SCALE: NONE



PANEL ATTACHMENT AT PANEL END
(BASE, EAVE STRUT, HEADER, SILL, AND PANEL END LAPS)



PANEL ATTACHMENT AT INTERMEDIATE MEMBERS

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Hahira, GA 31632

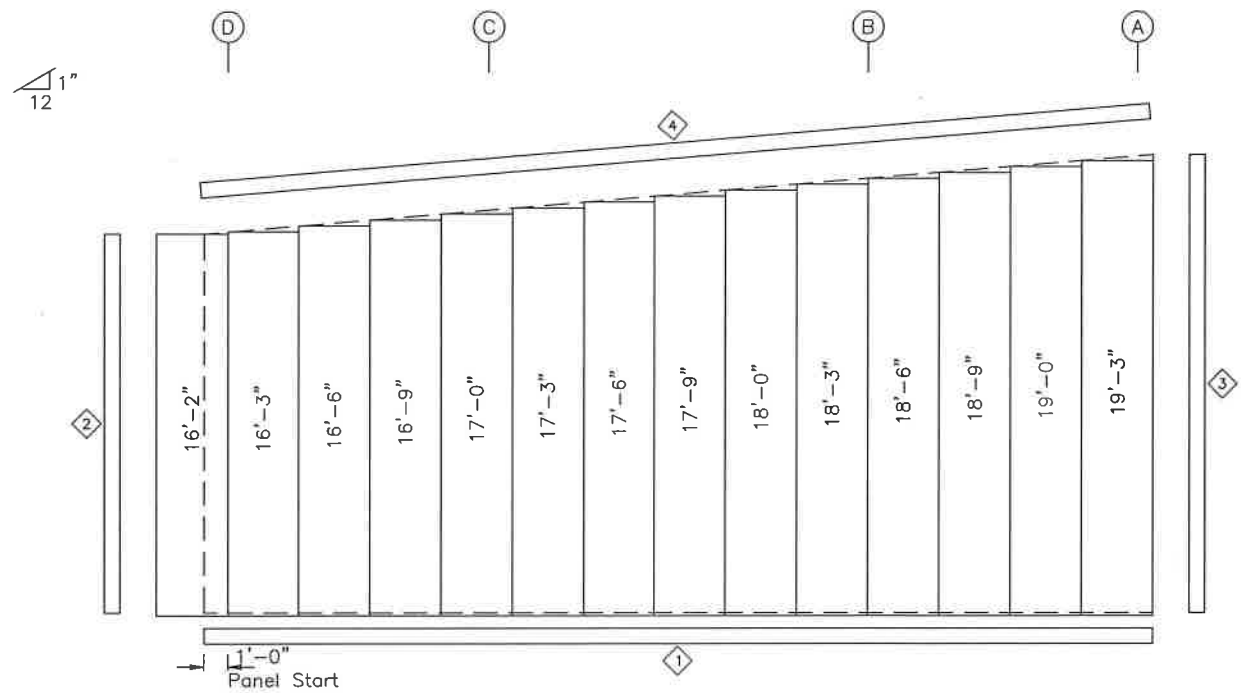


NOTES:

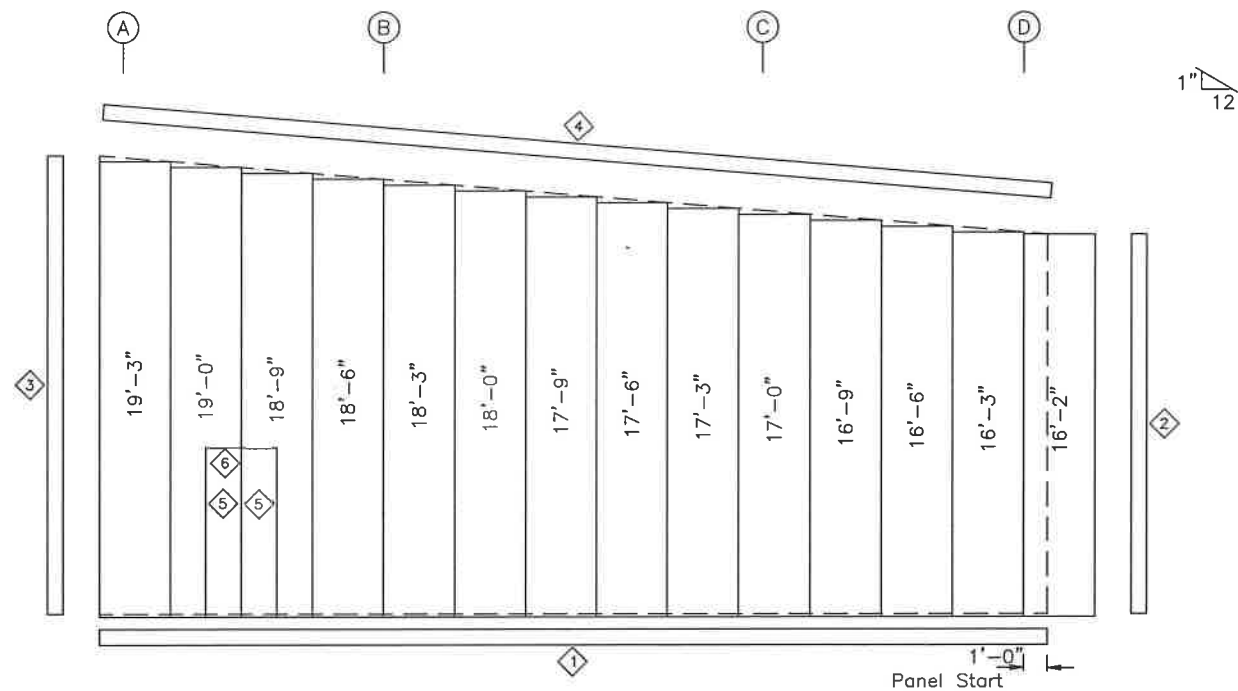
- [1] METAL SHAVINGS MUST BE SWEEPED FROM THE WALL EACH DAY DURING ERECTION TO PREVENT SURFACE RUSTING.
- [2] #12 SCREWS ARE USED TO ATTACH THE PANEL TO THE GIRTS. #14 LAP SCREWS ARE USED AT THE PANEL-TO-PANEL ATTACHMENTS. ALL FASTENERS ARE SELF-DRILLING.

ISSUE		DET	CHK	DATE
RENEGADE STEEL BUILDINGS, INC.				
CUSTOMER: JUNIOR FAIRCLOTH				
JOB NO: 7871		DATE: 1/31/23		
LOCATION: DUNN, NC 28334				
DRAWING NAME: SIDEWALL PANEL DETAILS				
DRAWING NO: PAGE 7.1		DRAWN BY: DJH	CHECKED BY: SPW	SCALE: NONE

TRIM TABLE			
FRAME LINE 1 & 3			
ID	PART	LENGTH	DETAIL
1	FL-73	20'-0"	TRIM_9
2	O/S CORN	16'-2"	TRIM_5
3	O/S CORN	19'-6"	TRIM_5
4	RAKE TRM	20'-3"	TRIM_3
5	R JAMB	7'-3"	TRIM_8
6	R HEAD	3'-3"	TRIM_61

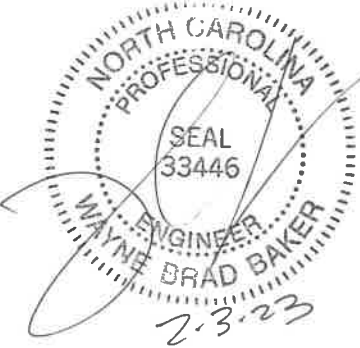


ENDWALL SHEETING & TRIM: FRAME LINE 1
PANELS: 26 GA. PBR - LIGHT STONE

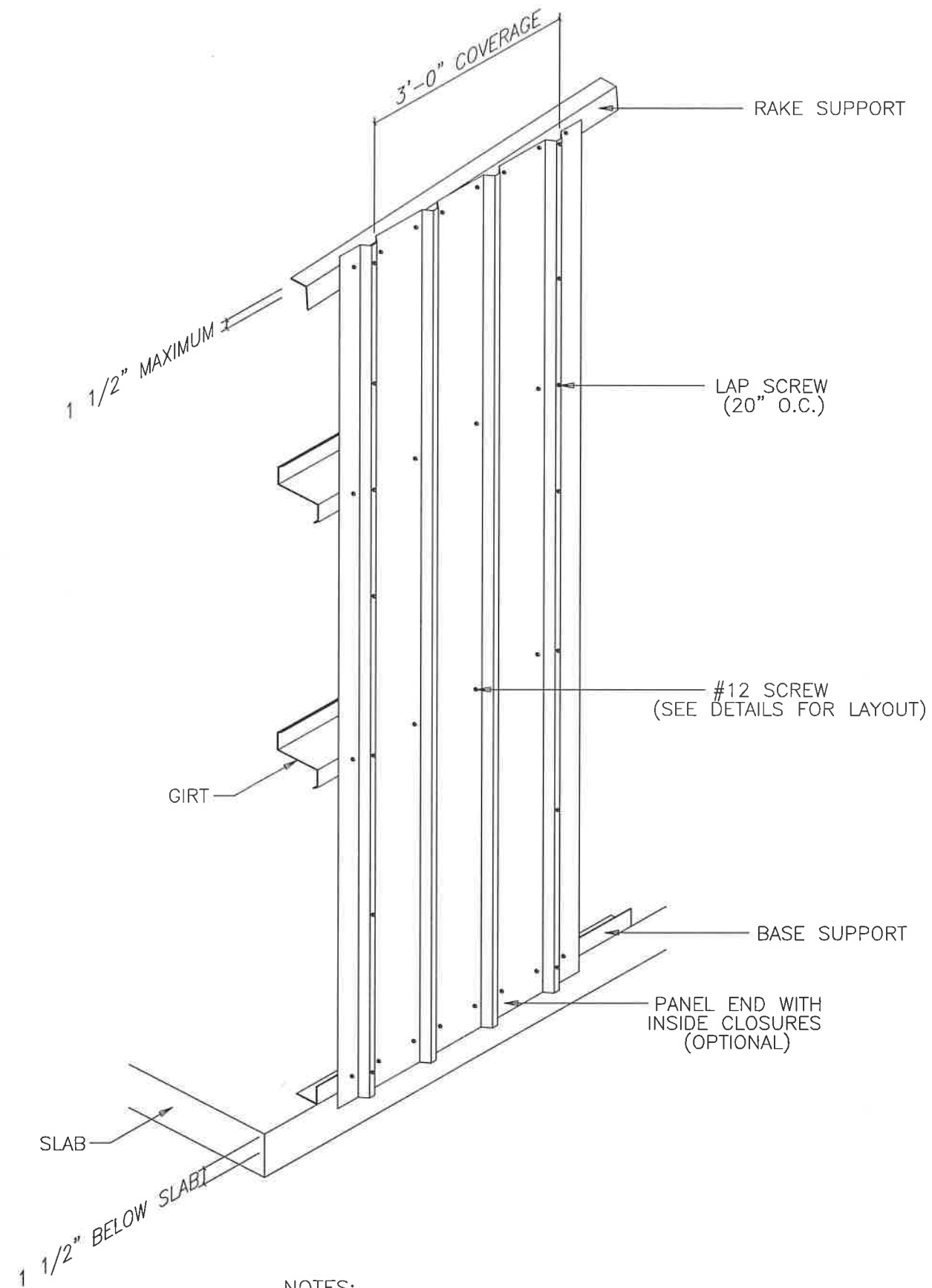


ENDWALL SHEETING & TRIM: FRAME LINE 3
PANELS: 26 GA. PBR - LIGHT STONE

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Hahira, GA 31632

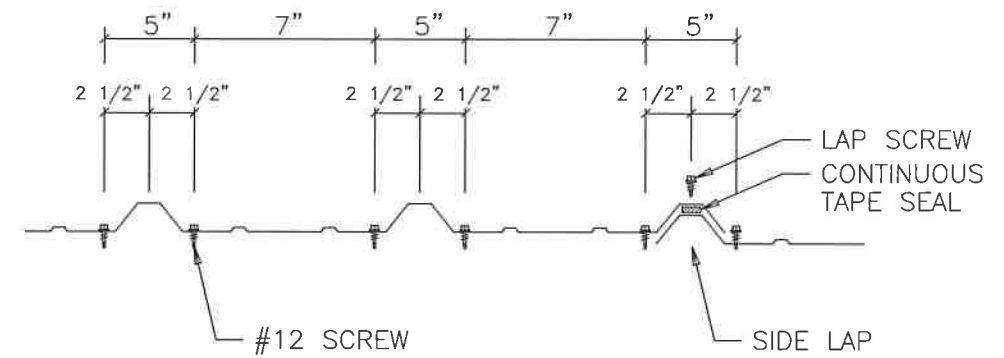


ISSUE		DET	CHK	DATE
RENEGADE STEEL BUILDINGS, INC.				
CUSTOMER:				
JUNIOR FAIRCLOTH				
JOB NO:		DATE:		
7871		1/31/23		
LOCATION:				
DUNN, NC 28334				
DRAWING NAME:				
ENDWALL PANELS & TRIM				
DRAWING NO:		DRAWN BY:	CHECKED BY:	SCALE:
PAGE 8		DJH	SPW	NONE

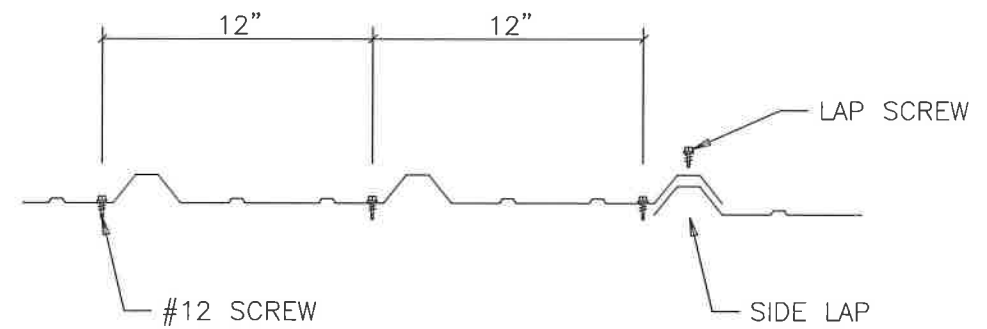


NOTES:

- [1] METAL SHAVINGS MUST BE SWEEPED FROM THE WALL EACH DAY DURING ERECTION TO PREVENT SURFACE RUSTING.
- [2] #12 SCREWS ARE USED TO ATTACH THE PANEL TO THE GIRTS. #14 LAP SCREWS ARE USED AT THE PANEL-TO-PANEL ATTACHMENTS. ALL FASTENERS ARE SELF-DRILLING.

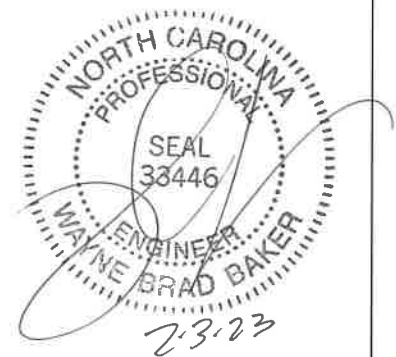


PANEL ATTACHMENT AT PANEL END
(BASE, EAVE STRUT, HEADER, SILL, AND PANEL END LAPS)

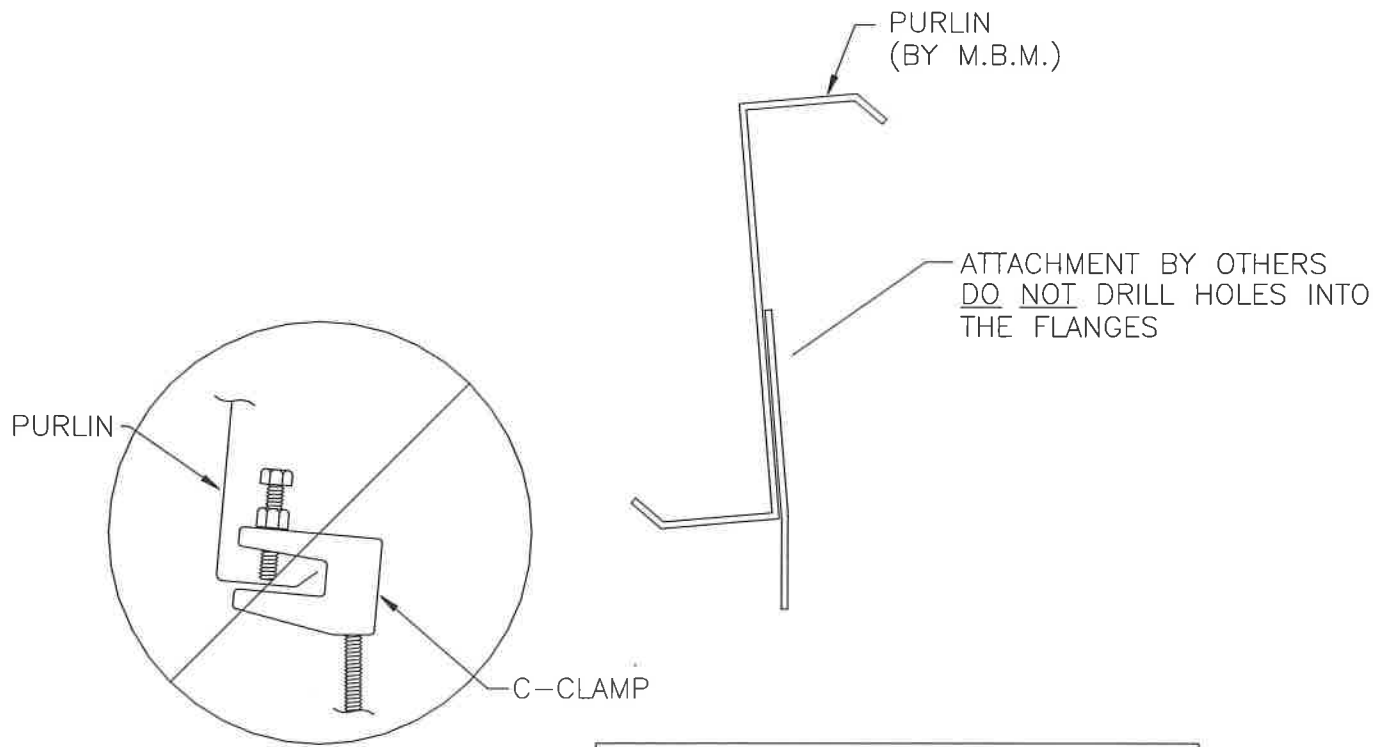


PANEL ATTACHMENT AT INTERMEDIATE MEMBERS

Wayne Brad Baker, P.E.
235 Sanders Rd.
Hahira, GA 31632



ISSUE		DET	CHK	DATE
RENEGADE STEEL BUILDINGS, INC.				
CUSTOMER: JUNIOR FAIRCLOTH				
JOB NO: 7871		DATE: 1/31/23		
LOCATION: DUNN, NC 28334				
DRAWING NAME: ENDWALL PANEL DETAILS				
DRAWING NO: PAGE 8.1	DRAWN BY: DJH	CHECKED BY: SPW	SCALE: NONE	



Flange C-Clamp is not an acceptable connection

NOTE: M.B.M. only provides the roof purlin. All other material and hardware is by others.

Recommended Connection Detail

NOTE

MANY FACTORS BEYOND THE CONTROL OF THE METAL BUILDING SUPPLIER AFFECT THE ABILITY OF A PURLIN TO SAFELY SUPPORT HANGING LOADS COMBINED WITH OTHER REQUIRED ROOF LOADS. DUE TO THE VARIABLES INVOLVED IN HANGING LOADS AND THEIR ATTACHMENTS TO THE PURLINS, THE METAL BUILDING SUPPLIER CANNOT ASSURE THAT THE PURLINS FOR A PARTICULAR BUILDING PROJECT CAN SAFELY SUPPORT THE MAXIMUM ALLOWABLE HANGING LOADS IN COMBINATION WITH OTHER ROOF LOADS.

IT IS THE RESPONSIBILITY OF THE HANGER SYSTEM INSTALLER TO COORDINATE WITH THE ENGINEER OF RECORD FOR THE OVERALL PROJECT TO ENSURE A SAFE HANGING LOAD INSTALLATION. THE METAL BUILDING ENGINEER IS NOT THE ENGINEER OF RECORD FOR THE OVERALL PROJECT. WITHOUT SPECIFIC CERTIFICATION FOR INDIVIDUAL HANGING LOADS, THE NET EFFECTS OF APPLIED HANGER LOADS INSTALLED ON A PARTICULAR PURLIN SHALL NOT EXCEED THE NET EFFECTS OF THE CERTIFIED UNIFORMLY APPLIED DESIGN COLLATERAL LOAD.

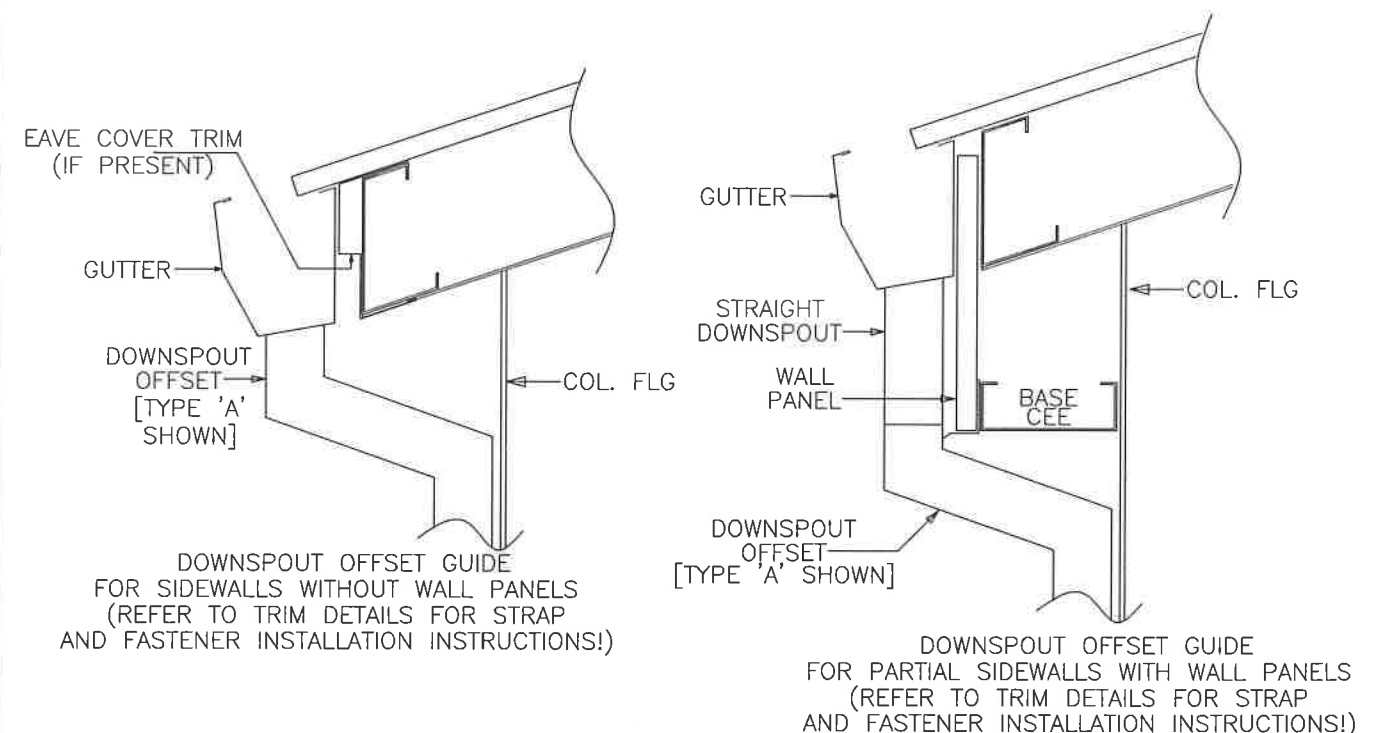
HANGING LOADS SHOULD NOT BE APPLIED TO THE PURLIN LIP. WHERE PERMISSIBLE, THE BEST PRACTICE FOR HANGING LOADS IS TO ATTACH TO THE PURLIN WEB USING A BOLT AND NUT, OR SELF-DRILLING SCREWS.

HANGING UNIFORM LOADS SUCH AS SPRINKLER MAINS OR HVAC EQUIPMENT SHOULD BE DISTRIBUTED OVER SEVERAL PURLINS, AND SHOULD NEVER EXCEED THE COLLATERAL LOAD ALLOWANCE FOR THE ROOF SYSTEM. FOR UNIFORM LOADS THAT RUN PARALLEL TO THE PURLINS, IT MAY BE NECESSARY TO USE TRANSVERSE SUPPORT CHANNELS(A.K.A. TRAPEZE BEAMS) ATTACHED TO THE WEBS OR FLANGES OF ADJACENT PURLINS TO SPREAD THE LOAD BETWEEN TWO OR MORE PURLINS. IN SUCH CASES, CONTACT THE BUILDING MANUFACTURER OR A LOCAL PROFESSIONAL ENGINEER PRIOR TO ATTEMPTING TO HANG LOADS FROM THE PURLINS

DO NOT INSTALL GUTTER WITH OUTSIDE FACE PERPENDICULAR TO THE GROUND.

INSTALL GUTTER WITH OUTSIDE FACE PERPENDICULAR TO THE ROOF.

GUTTER INSTALLATION DETAIL
(ONLY IF PROVIDED)



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235 Sanders Rd.
Hahira, GA 31632

ISSUE	DET	CHK	DATE
RENEGADE STEEL BUILDINGS, INC.			
CUSTOMER:	JUNIOR FAIRCLOTH		
JOB NO:	7871	DATE:	1/31/23
LOCATION:	DUNN, NC 28334		
DRAWING NAME:	SPECIAL DETAILS		
DRAWING NO:	PAGE 9	DRAWN BY:	CHECKED BY:
		DJH	SPW
			SCALE:
			NONE