BUILDER/CONTRACTOR RESPONSIBILITIES

Drawing Validity – These drawings, supporting structural calculations and design certification are based on the order documents as of the date of these drawings. These documents describe the material supplied by the manufacturer as of the date of these drawings. Any changes to the order documents after the date on these drawings may void these drawings, supporting structural calculations and design certification. The Builder/Contractor is responsible for notifying the building authority of all changes to the order documents which result in changes to the drawings, supporting structural calculations and design certification.

Builder Acceptance of Drawings – Approval of the manufacturer's drawings and design data affirms that the manufacturer has correctly interpreted and applied the requirements of the order documents and constitutes Builder/Contractor acceptance of the manufacturer's interpretations of the order documents and standard product specifications, including its design, fabrication and quality criteria standards and tolerances. (AISC code of standard practice Sept 86 Section 4.2.1) (Mar 05 Section 4.4.1)

Code Official Approval – It is the responsibility of the Builder/Contractor to ensure that all project plans and specifications comply with the applicable requirements of any governing building authority. The Builder/Contractor is responsible for securing all required approvals and permits from the appropriate agency as required.

Builder is responsible for State, Federal and OSHA safety compliance – The Builder/Contractor is responsible for applying and observing all pertinent safety rules and regulations and OSHA standards as applicable.

Building Erection – The Builder/Contractor is responsible for all erection of the steel and associated work in compliance with the Metal Building Manufacturers drawings. Temporary supports, such as temporary guys, braces, false work or other elements required for erection will be determined, furnished and installed by the erector. (AISC Code of Standard Practice Sept 86 Section 7.9.1) (Mar 05 Section 7.10.3)

Discrepancies – Where discrepancies exist between the Metal Building plans and plans for other trades, the Metal Building plans will govern. (AISC Code of Standard Practice Sept 86 Section 3.3) (Mar 05 Section 3.3)

Materials by Others – All interface and compatibility of any materials not furnished by the manufacturer are the responsibility of and to be coordinated by the Builder/Contractor or A/E firm. Unless specific design criteria concerning any interface between materials if furnished as a part of the order documents, the manufacturers assumptions will govern.

Modification of the Metal Building from Plans – The Metal Building supplied by the manufacturer has been designed according to the Building Code and specifications and the loads shown on this drawing. Modification of the building configuration, such as removing wall panels or braces, from that shown on these plans could affect the structural integrity of the building. The Metal Building Manufacturer or a Licensed Structural Engineer should be consulted prior to making any changes to the building configuration shown on these drawings. The Metal Building Manufacturer will assume no responsibility for any loads applied to the building not indicated on these drawings.

Foundation Design – The Metal Building Manufacturer is not responsible for the design, materials and workmanship of the foundation. Anchor rod plans prepared by the manufacturer are intended to show only location, diameter and projection of the anchor rods required to attach the Metal Building System to the foundation. It is the responsibility of the end customer to ensure that adequate provisions are made for specifying rod embedment, bearing values, tie rods and or other associated items embedded in the concrete foundation, as well as foundation design for the loads imposed by the Metal Building System, other imposed loads, and the bearing capacity of the soil and other conditions of the building site. (MBMA 06 Sections 3.2.2 and A3)

PROJECT NOTES

Material properties of steel bar, plate, and sheet used in the fabrication of built-up structural framing members conform to ASTM A529, ASTM A572, ASTM A1011 SS, or ASTM A1011 HSLAS with a minimum yield point of 50 ksi. Material properties of hot rolled structural shapes conform to ASTM A992, ASTM A529, or ASTM A572 with a minimum specified yield point of 50 ksi. Hot rolled angles, or other than flange braces, conform to ASTM 36 minimum. Hollow structural shaped conform to ASTM A500 grade b, minimum yield point is 42 ksi for round HSS and 46 ksi for rectangular HSS. Material properties of cold form light gage steel members conform to the requirements of ASTM A1011 SS Grade 55 or ASTM A1011 HSLAS Class 1 Grade 55, with a minimum yield point of 55 ksi.

The manufacturer does not assume any responsibility for the erection nor field supervision of the structure and or any special inspections that may be required by the local building authority during erection (including inspection of the high strength bolts or field welds) as required during erection. The coordination and the costs associated for setting up and Special Inspections are the responsibility of the Erector, Owner, Architect, or Engineer of Record.

Design is based upon the more severe loading of either the roof snow load or the roof live load.

Loads, as noted, are given within order documents and are applied in general accordance with the applicable provisions of the model code and/or specification indicated. Neither the manufacture nor the certifying engineer declares or attests that the loads as designated are proper for the local provisions that may apply or for site specific parameters. The manufacturer's Engineer's certification is limited to design loads supplied by an Architect and/or engineer of record for the overall construction project.

This project is designed using manufacture's standard serviceability standards. Generally this means that all stresses and deflections are within typical performance limits for normal occupancy and standard metal building products. If special requirements for deflections and vibrations must be adhered to, then they must be clearly stated in the contract documents.

This metal building system is designed as enclosed. All exterior components (i.e. doors, windows, vents, etc.) must be designed to withstand the specified wind loading for the design of components and cladding in accordance with the specified building code. Doors are to be closed when a maximum of 50% of design wind velocity is reached.

The design collateral load has been uniformly applied to the design of the building. Hanging loads are to be attached to the purlin web. This may not be appropriate for heavily concentrated loads. Any attached load in excess of 150 pounds shall be accounted for by special design performed by a licensed engineer using concentrated loads and may require separate support members within the roof system.

The metal building manufacturer has not designed the structure for snow accumulation loads at the ground level which may impose snow loads on the wall framing provided by the manufacturer.

DESIGN LOADING

THIS STRUCTURE IS DESIGNED UTILIZING THE LOADS INDICATED AND APPLIED AS REQUIRED BY:

IBC 15

THE BUILDER IS TO CONFIRM THAT THESE LOADS COMPLY WITH THE REQUIREMENTS OF THE LOCAL BUILDING DEPARTMENT.

ROOF DEAD LOAD		
SUPERIMPOSED	1.99	PSF
COLLATERAL (LIGHTS)	1.00	PSF
ROOF LIVE LOAD	20.00	PSF(NOT REDUCIBLE)
RISK CATEGORY	II – Normal	
SNOW LOAD		
GROUND SNOW LOAD (Pg)	15.00	PSF
SNOW LOAD IMPORTANCE FACTOR (Is)	1.0000	
FLAT ROOF SNOW LOAD (Pf)	10	PSF(AS PER ASCE 7–10 SECTION 7.3)
MIN. ROOF SNOW LOAD (Pf)	15	PSF(USED IN DESIGN)
SNOW EXPOSURE FACTOR (Ce)	1.0	
THERMAL FACTOR (Ct)	1.00	
WIND LOAD		
ULTIMATE WIND SPEED	118	MPH
NOMINAL WIND SPEED(Vasd)	91	MPH(IBC SECTION 1609.3.1)
SERVICEABILITY WIND SPEED	76	MPH
WIND EXPOSURE CATEGORY	C	
TOPOGRAPHICAL FACTOR	1.0	
INTERNAL PRESSURE COEFFICIENT (GCpi)	0.18 /–0.18	

ZONE 4, COMPONENT WIND LOAD ≤ 10FT²

28.47 PSF PRESSURE –37.89 PSF SUCTION

ZONE 5, COMPONENT WIND LOAD < 10FT²

28.47 PSF PRESSURE –30.84 PSF SUCTION

ZONES PER ASCE 7–10; FIG. 30.4–1
ZONES PRESSURES SHOWN ARE UN–FACTORED

RAIN INTENSITY		
5–MINUTE DURATION, 5–YEAR RECURRENCE (I1)	7.0	IN/HOUR
SEISMIC LOAD		
SEISMIC IMPORTANCE FACTOR (Ie)	1.00	
S_s 0.185	S_{Ds} 0.197	
S1 0.087	S_{D1} 0.139	
SITE CLASS	D STIFF SOIL	
SEISMIC DESIGN CATEGORY	B	

ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE

COLUMN LINE	TRANSVERSE	LONGITUDINAL FRONT	LONGITUDINAL BACK
BASIC FORCE RESISTING SYSTEM*	H	H	H
RESPONSE MODIFICATION COEFFICIENT(R)	3	3	3
SYSTEM OVER–STRENGTH FACTOR(Ω_0)	2.5000	2.5000	2.5000
SEISMIC RESPONSE COEFFICIENT(C_s)	0.066	0.066	0.066

BLDG DESIGN BASE SHEAR (V) TRANSVERSE 1.18 (k) LONGITUDINAL 1.08 (k)

THE TRANSVERSE DIRECTION IS PARALLEL TO THE RIGID FRAMES
THE LONGITUDINAL DIRECTION IS PERPENDICULAR TO THE RIGID FRAMES
BASIC STRUCTURAL SYSTEM (FROM ASCE 7–10 TABLE 12.2–1)

BASIC FORCE RESISTING SYSTEM*
H. STRUCTURAL STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE

DRAWING INDEX

DRAWING STATUS

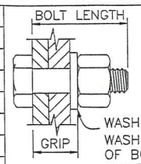
☐ FOR APPROVAL
THESE DRAWINGS, BEING FOR APPROVAL, ARE BY DEFINITION NOT FINAL, AND ARE FOR CONCEPTUAL REPRESENTATION ONLY. THEIR PURPOSE IS TO CONFIRM PROPER INTERPRETATION OF THE PROJECT DOCUMENTS. ONLY DRAWINGS ISSUED "FOR ERECTOR INSTALLATION" CAN BE CONSIDERED AS COMPLETE.

☐ FOR CONSTRUCTION PERMIT
THESE DRAWINGS, BEING FOR PERMIT, ARE BY DEFINITION NOT FINAL. ONLY DRAWINGS ISSUED "FOR ERECTOR INSTALLATION" CAN BE CONSIDERED AS COMPLETE.

☒ FOR ERECTOR INSTALLATION
FINAL DRAWINGS FOR CONSTRUCTION.

FOR QUESTIONS OR ASSISTANCE CONCERNING ERECTION CALL:
905–477–1894
MONDAY – FRIDAY 7:30AM TO 5:00PM

1/2"ø A325 BOLT GRIP TABLE	
GRIP	LENGTH
0 TO 9/16"	1 1/4" F.T.
Over 9/16" TO 1 1/16"	1 3/4" F.T.
Over 1 1/16" TO 1 5/16"	2"
Over 1 5/16" TO 1 9/16"	2 1/4"
Over 1 9/16" TO 1 13/16"	2 1/2"
Over 1 13/16" TO 2 1/16"	2 3/4"
LOCATIONS OF BOLTS LONGER THAN 2 3/4" NOTED ON ERECTION DRAWINGS	
F.T. DENOTES FULLY THREADED	



NOTE:
FULL THREAD ENGAGEMENT IS DEEMED TO HAVE BEEN MET WHEN THE END OF THE BOLT IS FLUSH WITH THE FACE OF THE NUT.

WASHER REQUIRED ONLY WHEN SPECIFIED. WASHER MAY BE LOCATED UNDER HEAD OF BOLT, UNDER NUT, OR AT BOTH AT LOCATIONS NOTED ON ERECTION DRAWINGS. ADD 5/32" FOR EACH WASHER TO MATERIAL THICKNESS TO DETERMINE GRIP.

Rev. 4/4/2017

BUILDING SIZE: 45'–0" x 50'–0" x 17'–0" 2.0:12

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	10/25/22	FOR ERECTOR INSTALLATION	MDB	HPD	CM

BUILDING SOLD BY HERITAGE BUILDING SYSTEMS

MICHAEL W. CUSTER, P.E.
642 OAKBEND DRIVE
COPPEL TX. 75019
PH. 972–571–7082

MICHAEL W CUSTER
Digitally signed by MICHAEL W CUSTER
Date: 2022.11.02 09:03:11 -05'00'

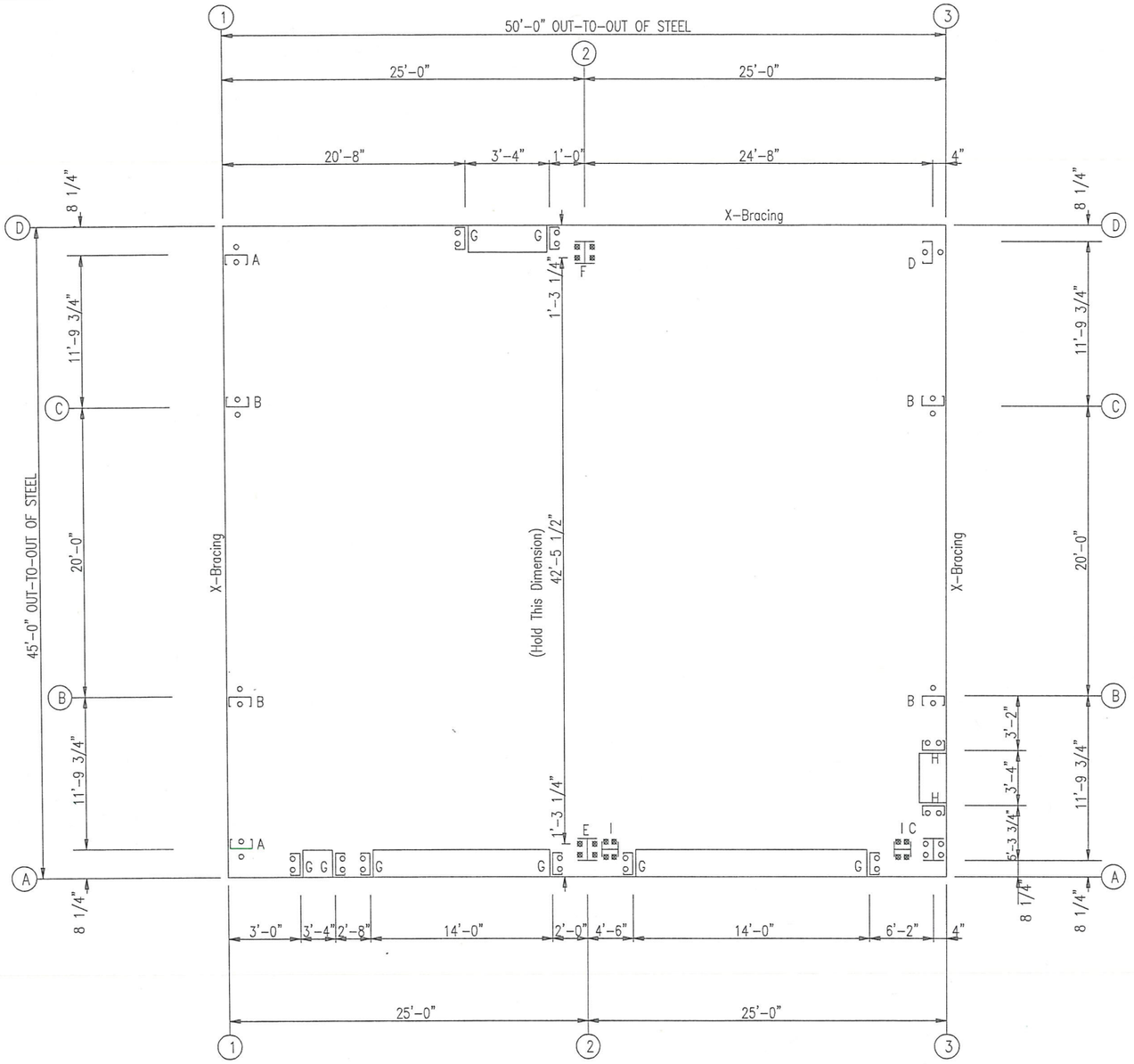
PROJECT: LUIS TIRADO – 45X50X17
CUSTOMER: LUIS TIRADO OWNER: LUIS TIRADO
LOCATION: LILLINGTON, NC 27546

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	5/24/19	N.T.S.	1	A	19–B–19564	C1	0

NOV 02 2022

NORTH CAROLINA PROFESSIONAL SEAL 023748 ENGINEER MICHAEL W CUSTER

THIS DOCUMENT WAS PREPARED BY AND/OR UNDER MY DIRECT SUPERVISION



ANCHOR BOLT PLAN

○ Dia= 5/8"

⊠ Dia= 3/4"

BUILDING SIZE: 45'-0" x 50'-0" x 17'-0" 2.0:12

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	10/25/22	FOR ERECTOR INSTALLATION	MDB	HPD	CM

BUILDING SOLD BY HERITAGE BUILDING SYSTEMS

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NORTH CAROLINA PROFESSIONAL SEAL 023748 ENGINEER MICHAEL W. CUSTER

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PROJECT:	LUIS TIRADO - 45X50X17
CUSTOMER:	LUIS TIRADO
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CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	5/24/19	N.T.S.	1	A	19-B-19564	F1	0

GENERAL NOTES

1) THE REACTIONS PROVIDED ARE BASED ON THE ORDER DOCUMENTS AT THE TIME OF MAILING. ANY CHANGES TO BUILDING LOADS OR DIMENSIONS MAY CHANGE THE REACTIONS. THE REACTIONS WILL BE SUPERSEDED AND VOIDED BY ANY FUTURE MAILING.
2) THE REACTIONS PROVIDED HAVE BEEN CREATED WITH THE FOLLOWING LAYOUT (UNLESS NOTHED OTHERWISE)
   A) A REACTION TABLE IS PROVIDED WITH REACTIONS FOR EACH LOAD GROUP
   B) RIGID FRAMES
      (1) SEE NOTE 3.
   C) ENDWALLS
      (1) SEE NOTE 3.
   D) X-BRACING
      1) X-BRACING REACTIONS ARE INCLUDED IN VALUES SHOWN IN THE REACTION TABLES AS NOTED IN THE BRACING REACTIONS TABLE.
      2) FOR IBC AND UBC BASED BUILDING CODES, WHEN X-BRACING IS PRESENT IN THE SIDEWALL, INDIVIDUAL LONGITUDINAL SEISMIC LOADS DO NOT INCLUDE THE AMPLIFICTION FACTOR, OMEGAo.
      3) FOR IBC AND UBC BASED BUILDING CODES, WHEN X-BRACING IS PRESENT IN THE ENDWALL, INDIVIDUAL TRANSVERSE SEISMIC LOADS DO NOT INCLUDE THE AMPLIFICATION FACTOR, OMEGAo.
   E) THE METAL BUILDING MANUFACTURER IS RESPONSIBLE ONLY FOR THE PORTION OF THE ANCHOR ROD DESIGN PERTAINING TO THE TRANSFER OF FORCES BETWEEN THE BASE PLATE BEARING AND THE ANCHOR ROD?S SHEAR AND TENSION. THE METAL BUILDING MANUFACTURER IS NOT RESPONSIBLE FOR THE ANCHOR ROD EMBEDMENT FOR TRANSFER OF FORCES TO THE FOUNDATION. THE METAL BUILDING MANUFACTURER DOES NOT DESIGN AND IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL, AND CONSTRUCTION OF THE FOUNDATION EMBEDMENT. THE END USE CUSTOMER SHALL ASSURE THAT ADEQUATE PROVISIONS ARE MADE TO THE FOUNDATION DESIGN FOR LOADS IMPOSED BY COLUMN REACTIONS OF THE BUILDING, OTHER IMPOSED LOADS, AND BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE. IT IS RECOMMENDED THAT THE ANCHORAGE AND FOUNDATION OF THE BUILDING BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER COMPETENT IN THE DESIGN OF SUCH STRUCTURES.
      i) (REF. APPENDIX A3 OF THE MBMA METAL BUILDING BUILDING SYSTEMS MANUAL)
   F) ANCHOR RODS ARE ASTM F1554 GR. 36 MATERIAL UNLESS NOTED OTHERWISE ON THE ANCHOR ROD LAYOUT DRAWING.
3) REACTIONS ARE PROVIDED AS UN-FACTORED FOR EACH LOAD GROUP APPLIED TO THE COLUMN. THE FACTORS APPLIED TO LOAD GROUPS FOR THE STEEL COLUMN DESIGN MAY BE DIFFERENT THAN THE FACTORS USED IN THE FOUNDATION DESIGN. THE FOUNDATION ENGINEER SHALL APPLY THE APPROPRIATE LOAD FACTORS AND COMBINE THE REACTIONS IN ACCORDANCE WITH THE BUILDING CODE AND DESIGN SPECIFICATIONS FOR PROPER FOUNDATION DESIGN.
   A) FOR PROJECTS USING ULTIMATE DESIGN WIND SPEEDS SUCH AS 2012 IBC, 2015 IBC, OR FLORIDA BUILDING CODE, THE WIND LOAD REACTIONS ARE AT A STRENGTH VALUE WITH A LOAD FACTOR OF 1.0.
   B) FOR IBC CODES, THE SEISMIC REACTIONS PROVIDED ARE AT A STRENGTH LEVEL WITH A LOAD FACTOR OF 1.0, AND DO NOT CONTAIN THE RHO FACTOR.

THE MANUFACTURER DOES NOT PROVIDE "MAXIMUM" LOAD COMBINATION REACTIONS. HOWEVER, THE INDIVIDUAL LOAD REACTIONS PROVIDED MAY BE USED BY THE FOUNDATION ENGINEER TO DETERMINE THE APPLICABLE LOAD COMBINATIONS FOR HIS/HER DESIGN PROCEDURES AND ALLOW FOR AN ECONOMICAL FOUNDATION DESIGN.

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Snow Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert	Wind_Left2 Horz	Wind_Left2 Vert	Wind_Right2 Horz	Wind_Right2 Vert	Wind Press Horz
1	D	0.2	0.1	1.0	0.5	0.0	−1.4	0.0	−1.2	0.0	−0.7	0.0	−0.4	0.0
1	C	0.7	0.2	4.7	2.5	−2.3	−8.5	0.0	−2.2	−2.3	−6.6	0.0	−0.2	−3.2
1	B	0.7	0.2	4.7	2.5	0.0	−2.2	2.3	−8.5	0.0	−0.2	2.3	−6.6	−3.2
1	A	0.2	0.1	1.0	0.5	0.0	−1.2	0.0	−1.4	0.0	−0.4	0.0	−0.7	0.0

Frm Line	Col Line	Wind Suct Horz	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert	Seis_Left Horz	Seis_Left Vert	Seis_Right Horz	Seis_Right Vert	Seis Long Vert	−MIN_SNOW− Horz	−MIN_SNOW− Vert
1	D	0.0	0.0	−2.0	0.0	−1.2	0.0	0.0	0.0	0.0	0.0	0.0	0.8
1	C	3.5	0.0	−5.4	−0.6	−4.3	−0.3	−0.3	0.0	0.3	0.0	0.0	3.5
1	B	3.5	0.6	−4.3	0.0	−5.4	0.0	0.3	0.3	−0.3	0.0	0.0	3.5
1	A	0.0	0.0	−1.2	0.0	−2.0	0.0	0.0	0.0	0.0	0.0	0.0	0.8

Frm Line	Col Line	E1UNB_SL_L− Horz	E1UNB_SL_L− Vert	E1UNB_SL_R− Horz	E1UNB_SL_R− Vert
1	D	0.0	0.5	0.0	0.0
1	C	0.0	2.9	0.0	1.2
1	B	0.0	1.2	0.0	2.9
1	A	0.0	0.0	0.0	0.5

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Snow Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert	Wind_Left2 Horz	Wind_Left2 Vert	Wind_Right2 Horz	Wind_Right2 Vert
3	A	0.3	0.1	1.0	0.5	0.0	−1.4	0.0	−1.2	0.0	−0.7	0.0	−0.4
3	B	0.7	0.2	4.7	2.5	−2.3	−8.5	0.0	−2.2	−2.3	−6.6	0.0	−0.2
3	C	0.7	0.2	4.7	2.5	0.0	−2.2	2.3	−8.5	0.0	−0.2	2.3	−6.6
3	D	0.2	0.1	1.0	0.5	0.0	−1.2	0.0	−1.4	0.0	−0.4	0.0	−0.7

Frm Line	Col Line	Wind_Press Horz	Wind_Press Vert	Wind_Suct Horz	Wind_Suct Vert	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert	Seis_Left Horz	Seis_Left Vert	Seis_Right Horz	Seis_Right Vert	Seis_Long Horz	Seis_Long Vert
3	A	0.0	0.0	0.0	0.0	0.0	−2.0	0.0	−1.2	0.0	0.0	0.0	0.0	0.0	0.0
3	B	−3.2	0.0	3.5	0.0	0.0	−5.4	−0.6	−4.3	−0.3	−0.3	0.0	0.3	0.0	0.0
3	C	−3.2	0.0	3.5	0.0	0.6	−4.3	0.0	−5.4	0.0	0.3	0.3	−0.3	0.0	0.0
3	D	−4.2	−2.6	0.0	2.6	0.0	−1.2	0.0	−2.0	0.0	0.0	0.0	0.0	−0.5	−0.3

Frm Line	Col Line	−MIN_SNOW− Horz	−MIN_SNOW− Vert	E2UNB_SL_L− Horz	E2UNB_SL_L− Vert	E2UNB_SL_R− Horz	E2UNB_SL_R− Vert
3	A	0.0	0.8	0.0	0.5	0.0	0.0
3	B	0.0	3.5	0.0	2.9	0.0	1.2
3	C	0.0	3.5	0.0	1.2	0.0	2.9
3	D	0.0	0.8	0.0	0.0	0.0	0.5

BUILDING BRACING REACTIONS

Reactions in plane of wall ± Reactions(k) — Panel_Shear (lb/ft)

Wall Loc	Wall Line	Col Line	Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Wind	Seis	Note
L_EW	1	C,B	Bracing, see EW reactions						
F_SW	A	2,3							(a)
R_EW	3	B,C	Bracing, see EW reactions						
B_SW	D	3,2	4.2	*	0.5	*			

(a)Wind bent in bay

*See RF reactions table for vertical and horizontal reactions in plane of the rigid frame.

NOTES FOR REACTIONS

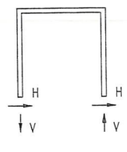
BUILDING REACTIONS ARE BASED ON THE FOLLOWING BUILDING DATA:

WIDTH (FT)	= 45
LENGTH (FT)	= 50
EAVE HEIGHT (FT)	= 17 / 17
ROOF SLOPE (rise/12)	= 2.0:12 / 2.0:12
DEAD LOAD (psf)	= 1.990
COLLATERAL LOAD (psf)	= 1
ROOF LIVE LOAD (psf)	= 20.00
FRAME LIVE LOAD (psf)	= 20
ROOF SNOW LOAD (psf)	= 10.5
GROUND SNOW LOAD (psf)	= 15.00
MINIMUM ROOF SNOW LOAD (psf)	= 15.00
WIND SPEED (MPH)	= 118
WIND CODE	= NCBC 18
EXPOSURE	= C
CLOSED/OPEN	= Closed
IMPORTANCE - WIND	= 1.00
IMPORTANCE - SEISMIC	= 1.00
SEISMIC ZONE	= C

REACTION KEY:

WIND Left/Right 1 = (with +GCpi Internal Pressure)
WIND Left/Right 2 = (with −GCpi Internal Pressure)
Wind_Long 1 = Wind Load Case B at Left EW
Wind_Long 2 = Wind Load Case B at Right EW
MIN_SNOW = Minimum Snow (Pm) per code
E1UNB_SL_L = Endwall Unbalanced Snow Left
E1UNB_SL_R = Endwall Unbalanced Snow Right
F1UNB_SL_L = Rigid Frame Unbalanced Snow Left
F1UNB_SL_R = Rigid Frame Unbalanced Snow Right

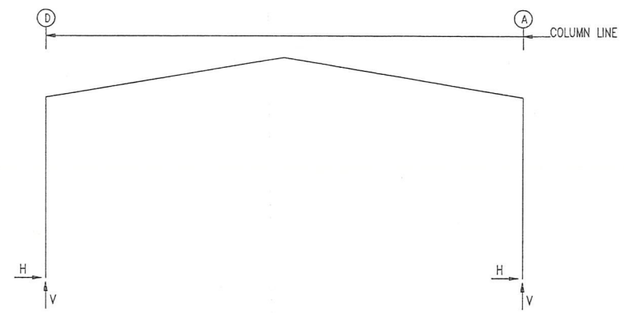
WIND BENT REACTIONS



± Reactions

Wall Loc	Wall Line	Col Line	Wind(k) Horz	Wind(k) Vert	Seismic(k) Horz	Seismic(k) Vert	Bolt(in) Qty	Bolt(in) Dia	Base_Plate(in) Width	Base_Plate(in) Length	Thick
F_SW	A	2	2.1	2.9	0.3	0.4	4	0.750	6.000	10.500	0.375
F_SW	A	3	2.1	2.9	0.3	0.4	4	0.750	6.000	10.500	0.375

FRAME LINES: 2



RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc._Bolt Qty	Anc._Bolt Dia	Base_Plate (in) Width	Base_Plate (in) Length	Thick	Grout (in)
2	D	4	0.750	6.000	16.50	0.625	0.0
2	A	4	0.750	6.000	17.00	0.375	0.0

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Snow Horiz	Snow Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert
2	D	0.7	2.0	0.3	0.7	5.7	14.0	3.0	7.4	−9.4	−16.6	0.5	−10.2
2	A	−0.7	2.1	−0.3	0.7	−5.7	14.1	−3.0	7.4	−0.5	−10.2	9.4	−16.6

Frame Line	Column Line	Wind_Left2 Horiz	Wind_Left2 Vert	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	Seismic_Left Horiz	Seismic_Left Vert	Seismic_Right Horiz	Seismic_Right Vert
2	D	−9.3	−9.9	0.5	−3.5	−0.2	−16.8	−1.1	−14.6	−0.3	−0.2	0.3	0.2
2	A	−0.5	−3.5	9.4	−10.0	1.2	−12.0	0.2	−14.3	−0.3	0.2	0.3	−0.2

Frame Line	Column Line	Seismic_Long Horiz	Seismic_Long Vert	MIN_SNOW Horiz	MIN_SNOW Vert	F1UNB_SL_L Horiz	F1UNB_SL_L Vert	F1UNB_SL_R Horiz	F1UNB_SL_R Vert
2	D	0.0	−0.3	4.3	10.5	2.6	7.3	2.6	4.2
2	A	0.0	0.0	−4.3	10.6	−2.6	4.2	−2.6	7.3

ENDWALL COLUMN: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc._Bolt Qty	Anc._Bolt Dia	Base_Plate (in) Width	Base_Plate (in) Length	Thick	Grout (in)
1	D	2	0.625	7.000	10.00	0.250	0.0
1	C	2	0.625	7.000	10.00	0.250	0.0
1	B	2	0.625	7.000	10.00	0.250	0.0
1	A	2	0.625	7.000	10.00	0.250	0.0
3	A	4	0.625	6.000	10.00	0.375	0.0
3	B	2	0.625	7.000	10.00	0.250	0.0
3	C	2	0.625	7.000	10.00	0.250	0.0
3	D	2	0.625	7.000	10.00	0.250	0.0

ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type	Proj (in)
○ 20	Jamb	5/8"	F1554	2.00
○ 18	Endwall	5/8"	F1554	2.00
⊠ 8	Frame	3/4"	F1554	2.50
⊠ 8	WindCol	3/4"	F1554	2.50

BUILDING SIZE: 45'−0" x 50'−0" x 17'−0" 2.0:12

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	10/25/22	FOR ERECTOR INSTALLATION	MDB	HPD	CM

BUILDING SOLD BY HERITAGE BUILDING SYSTEMS

MICHAEL W. CUSTER, P.E.
642 OAKBEND DRIVE
COPPEL TX. 75019
PH. 972-571-7082

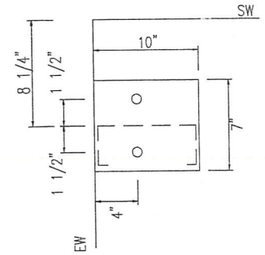
MICHAEL W CUSTER
Digitally signed by MICHAEL W CUSTER
Date: 2022.11.02 09:04:04 -05'00'

NORTH CAROLINA PROFESSIONAL SEAL 023748 ENGINEER MICHAEL W CUSTER

THIS DOCUMENT WAS PREPARED BY AND/OR UNDER MY DIRECT SUPERVISION

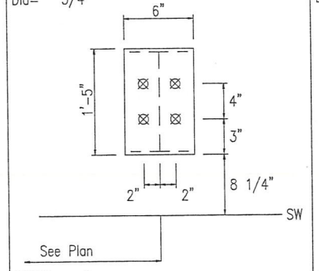
PROJECT:	LUIS TIRADO - 45X50X17						
CUSTOMER:	LUIS TIRADO	OWNER:	LUIS TIRADO				
LOCATION:	LILLINGTON, NC 27546						
CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	5/24/19	N.T.S.	1	A	19-B-19564	F2	0

Dia= 5/8"



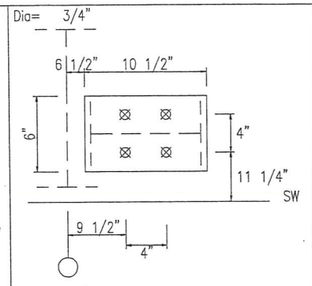
DETAIL A

Dia= 3/4"



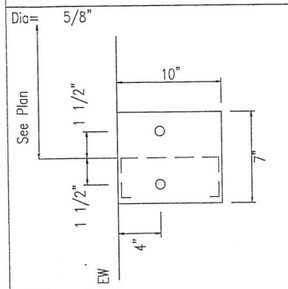
DETAIL E

Dia= 3/4"



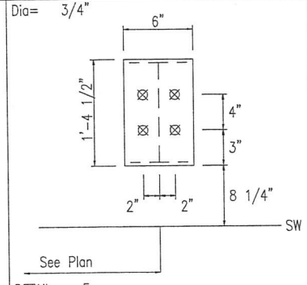
DETAIL I

Dia= 5/8"

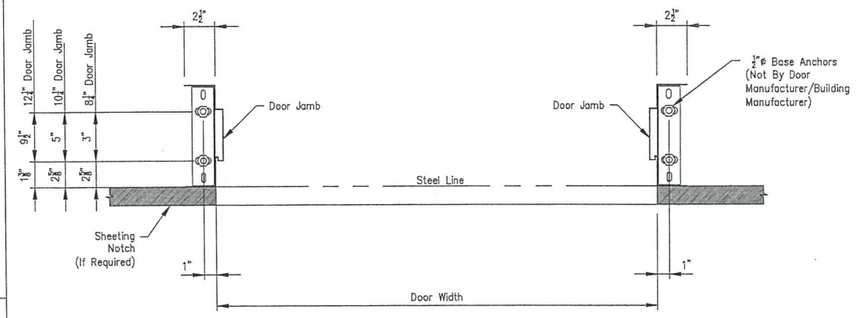


DETAIL B

Dia= 3/4"



DETAIL F



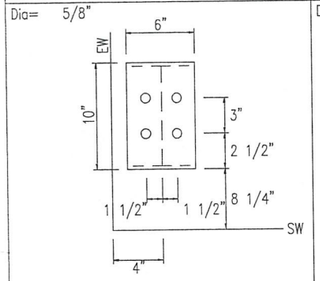
The Adequacy Of The ½"6 Base Anchor Is Not The Responsibility Of The Building Manufacturer.
The Adequacy Of These Base Anchors Should Be Determined By A Qualified Foundation Engineer.

Verify Door Jamb Base Clip Dimensions With Patterns Shown Prior To Placement Of Door Anchors And Adjust Patterns If Needed.

Note: 12½" Frames May Not Have Kerf Door Frame Feature Depending On Door Manufacturer.

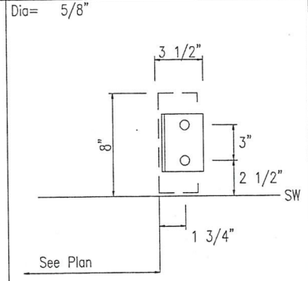
Knock Down Door Anchor Placement

Dia= 5/8"



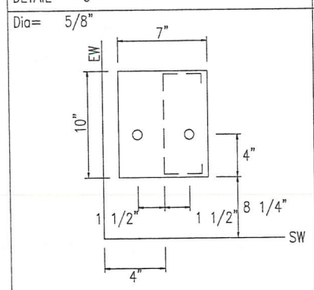
DETAIL C

Dia= 5/8"



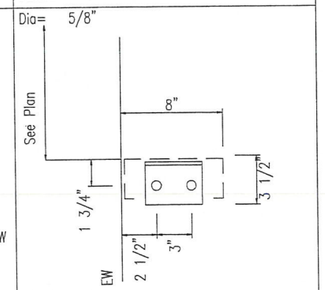
DETAIL G

Dia= 5/8"

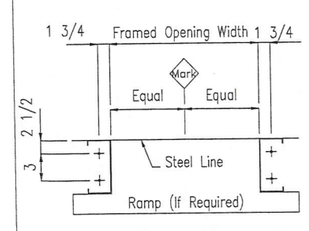


DETAIL D

Dia= 5/8"



DETAIL H



ARDia	Framed Opening AR Layout
5/8"	

BUILDING SIZE: 45'–0" x 50'–0" x 17'–0" 2.0:12

BUILDING SOLD BY HERITAGE BUILDING SYSTEMS

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	10/25/22	FOR ERECTOR INSTALLATION	MDB	HPD	CM

MICHAEL W. CUSTER, P.E.
642 OAKBEND DRIVE
COPPEL TX. 75019
PH. 972–571–7082

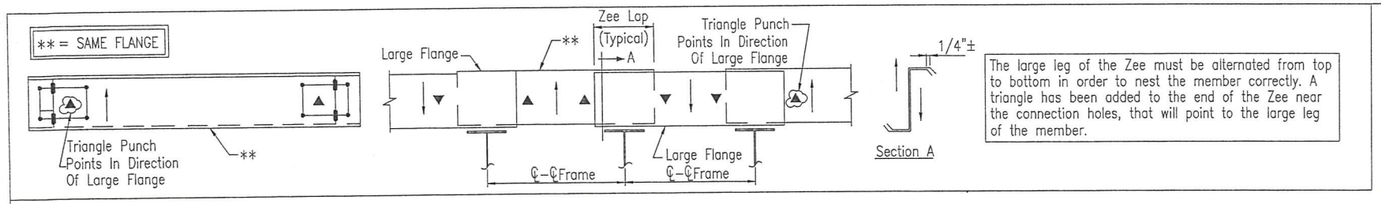
MICHAEL W CUSTER Digitally signed by MICHAEL W CUSTER Date: 2022.11.02 09:04:42 -05'00'

NORTH CAROLINA PROFESSIONAL SEAL 023748 ENGINEER MICHAEL W. CUSTER

THIS DOCUMENT WAS PREPARED BY AND/OR UNDER MY DIRECT SUPERVISION

PROJECT:	LUIS TIRADO – 45X50X17		
CUSTOMER:	LUIS TIRADO	OWNER:	LUIS TIRADO
LOCATION:	LILLINGTON, NC 27546		

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	5/24/19	N.T.S.	1	A	19–B–19564	F3	0



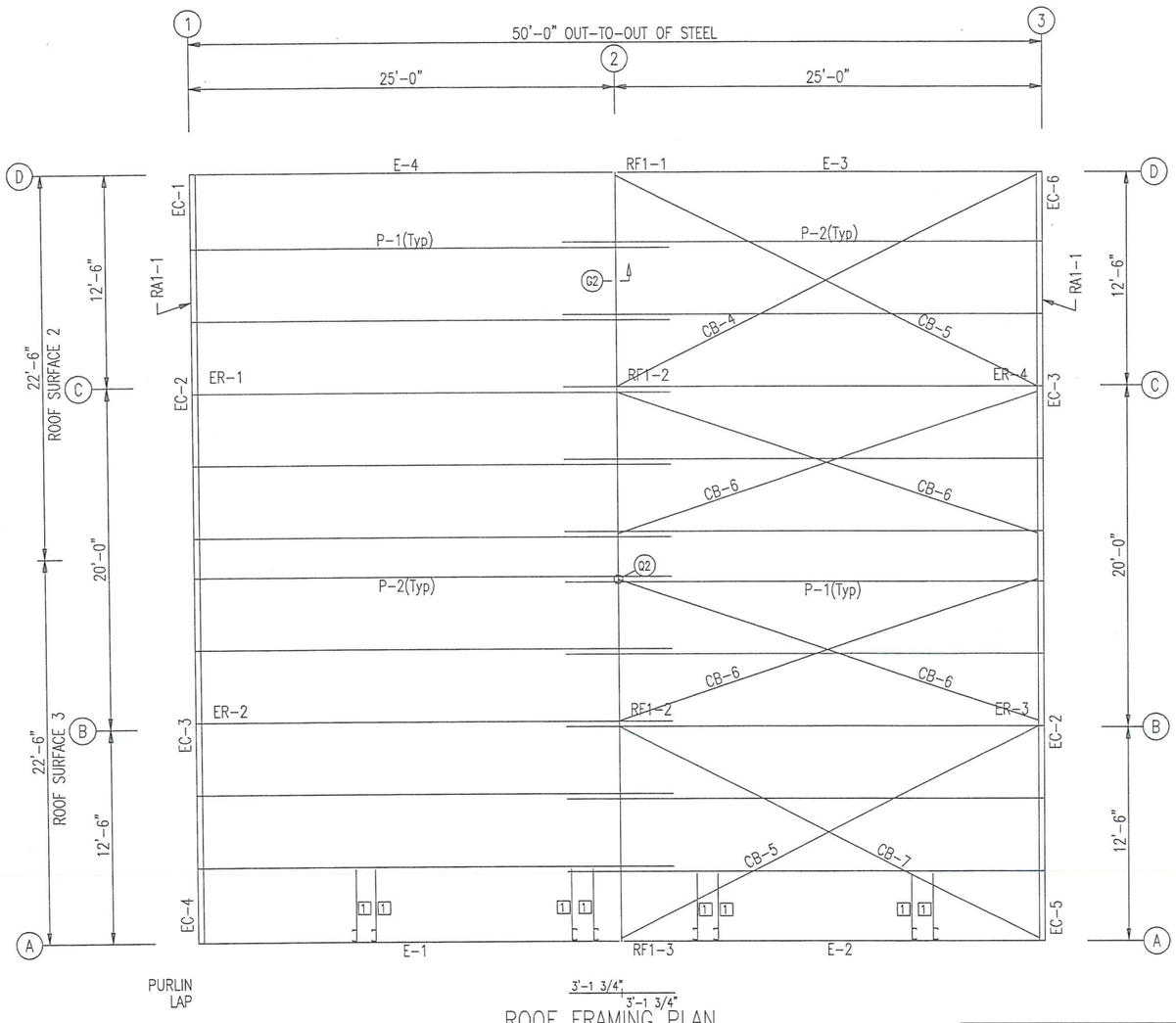
The large leg of the Zee must be alternated from top to bottom in order to nest the member correctly. A triangle has been added to the end of the Zee near the connection holes, that will point to the large leg of the member.

MEMBER TABLE
ROOF PLAN

MARK	PART	LENGTH
P–1	8X25Z14	28'–1 1/2"
P–2	8X25Z14	28'–1 1/2"
E–1	8ES2L14	24'–11 1/2"
E–2	8ES2L14	24'–11 1/2"
E–3	8ES2L14	24'–11 1/2"
E–4	8ES2L14	24'–11 1/2"
CB–4	1/4" CABLE	28'–2"
CB–5	1/4" CABLE	27'–9"
CB–6	1/4" CABLE	26'–7"
CB–7	1/4" CABLE	27'–6"

CONNECTION PLATES
ROOF PLAN

ID	MARK/PART
1	DB1



ROOF FRAMING PLAN

BUILDING SIZE: 45'–0" x 50'–0" x 17'–0" 2.0:12

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	10/25/22	FOR ERECTOR INSTALLATION	MDB	HPD	CM

BUILDING SOLD BY HERITAGE BUILDING SYSTEMS

MICHAEL W. CUSTER, P.E.
642 OAKBEND DRIVE
COPPEL TX. 75019
PH. 972–571–7082

MICHAEL W CUSTER
Digitally signed by MICHAEL W CUSTER
Date: 2022.11.02 09:05:18 -05'00'

NORTH CAROLINA PROFESSIONAL SEAL 023748 ENGINEER MICHAEL W. CUSTER
THIS DOCUMENT WAS PREPARED BY AND/OR UNDER MY DIRECT SUPERVISION

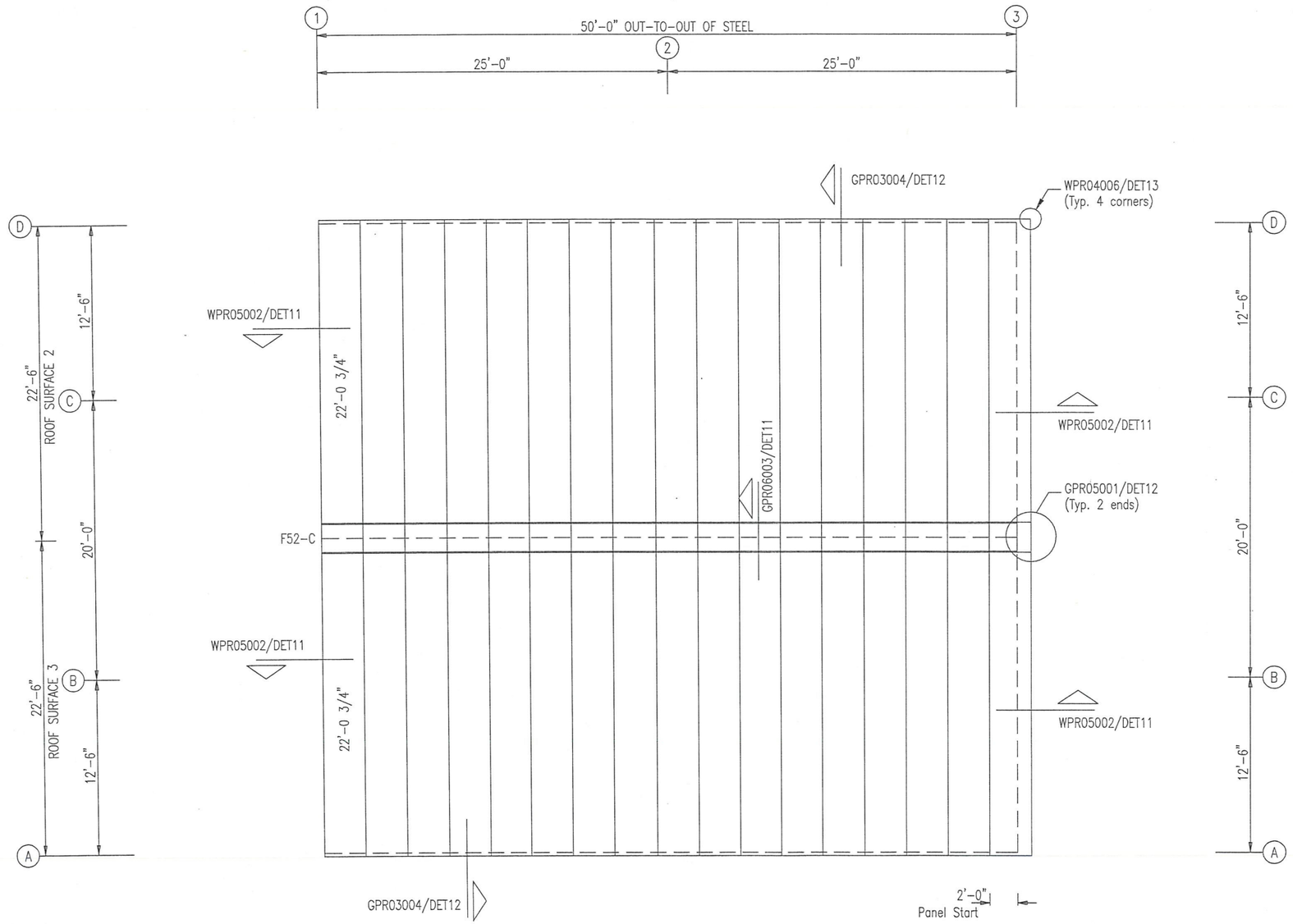
PROJECT:	LUIS TIRADO – 45X50X17		
CUSTOMER:	LUIS TIRADO	OWNER:	LUIS TIRADO
LOCATION:	LILLINGTON, NC 27546		

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	5/24/19	N.T.S.	1	A	19–B–19564	E1	0

GENERAL NOTES:

1. INSTALL ALL PURLIN AND FLANGE BRACES (FB) AS SHOWN.
2. ROOF PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.
3. STRUT PURLINS, IF PROVIDED, MUST BE INSTALLED AND FASTENED TO ROOF SHEETING PER "PBR" PANEL ROOF DETAIL.
4. DO NOT ADD ANY ADDITIONAL ROOF OPENINGS WITHOUT BUILDING MANUFACTURER APPROVAL OR PROFESSIONAL ENGINEER APPROVAL.
5. DO NOT STACK SHEET BUNDLES ON ROOF. ONLY RAISE INDIVIDUAL SHEETS AS NEEDED.
6. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHAVINGS CAUSED BY DRILLING.

PBR ROOF SHEETING NOTE:
PBR ROOF PANELS ARE TO BE FIELD CUT IF THE PANELS EXTEND OUTSIDE OF THE ROOF PLANE, PANELS ARE NOT TO BE BACK LAPPED.



ROOF SHEETING PLAN

PANELS: 26 Gauge PBR – Galvalume
BUILDING SIZE: 45'–0" x 50'–0" x 17'–0" 2.0:12

BUILDING SOLD BY HERITAGE BUILDING SYSTEMS

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	10/25/22	FOR ERECTOR INSTALLATION	MDB	HPD	CM

MICHAEL W. CUSTER, P.E.
642 OAKBEND DRIVE
COPPEL TX. 75019
PH. 972-571-7082

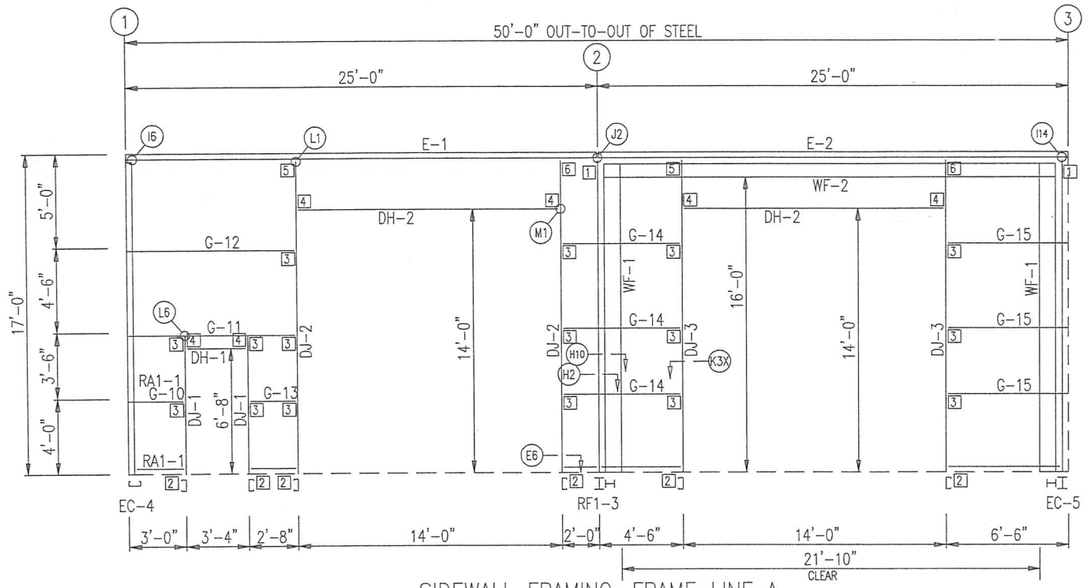
MICHAEL W CUSTER
Digitally signed by MICHAEL W CUSTER
Date: 2022.11.02 09:05:56 -05'00'

PROJECT: LUIS TIRADO – 45X50X17
CUSTOMER: LUIS TIRADO
OWNER: LUIS TIRADO
LOCATION: LILLINGTON, NC 27546

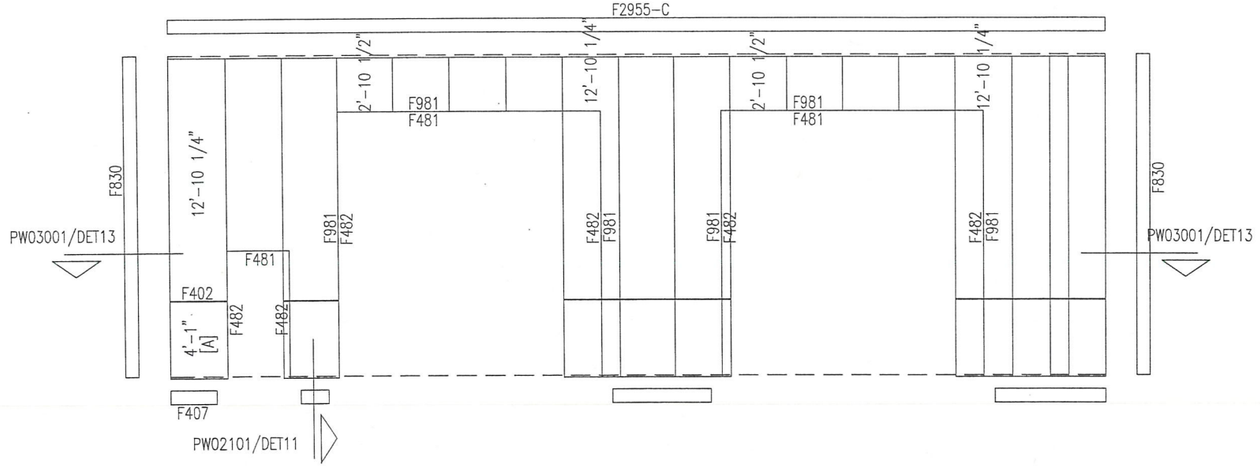
CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	5/24/19	N.T.S.	1	A	19-B-19564	E2	0

NORTH CAROLINA PROFESSIONAL ENGINEER SEAL 023748 MICHAEL W. CUSTER
THIS DOCUMENT WAS PREPARED BY AND/OR UNDER MY DIRECT SUPERVISION

GENERAL NOTES:
1. INSTALL ALL PURLIN AND FLANGE BRACES (FB) AS SHOWN.
2. ROOF PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.
3. STRUT PURLINS, IF PROVIDED, MUST BE INSTALLED AND FASTENED TO ROOF SHEETING PER "PBR" PANEL ROOF DETAIL.
4. DO NOT ADD ANY ADDITIONAL ROOF OPENINGS WITHOUT BUILDING MANUFACTURER APPROVAL OR PROFESSIONAL ENGINEER APPROVAL.
5. DO NOT STACK SHEET BUNDLES ON ROOF. ONLY RAISE INDIVIDUAL SHEETS AS NEEDED.
6. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHAVINGS CAUSED BY DRILLING.



SIDEWALL FRAMING: FRAME LINE A



SIDEWALL SHEETING & TRIM: FRAME LINE A

PANELS: 26 Gauge PBR – Polar White

[A] PANELS: 26 Gauge PBR – Coal Black

BUILDING SIZE: 45'–0" x 50'–0" x 17'–0" 2.0:12

BOLT TABLE
FRAME LINE A

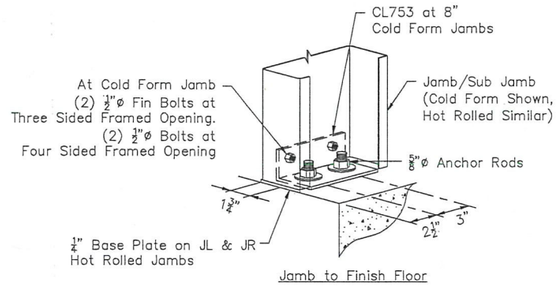
LOCATION	QUAN	TYPE	DIA	LENGTH
WF-1 – WF-2	8	A325	3/4"	2"
WF-1 – RF1-3	8	A325	3/4"	1 1/2"
WF-1 – EC-5	8	A325	3/4"	1 1/2"

MEMBER TABLE
FRAME LINE A

MARK	PART	LENGTH
WF-1	W10542	16'-8 1/2"
WF-2	W08642	21'-9 1/2"
DJ-1	8X35C14	7'-1 3/4"
DJ-2	8X35C13	16'-2 11/16"
DJ-3	8X35C14	16'-2 11/16"
DH-1	8X35C14	3'-3 3/4"
DH-2	8X35C14	13'-11 3/4"
E-1	8ES2L14	24'-11 1/2"
E-2	8ES2L14	24'-11 1/2"
G-10	8X25Z16	2'-7 3/4"
G-11	8X25Z16	8'-7 3/4"
G-12	8X25Z16	8'-7 3/4"
G-13	8X25Z16	2'-0"
G-14	8X25Z16	5'-10"
G-15	8X25Z16	6'-1 3/4"

CONNECTION PLATES
FRAME LINE A

ID	MARK/PART
1	SC479
2	CL753
3	CL751
4	SC425
5	SC585L
6	SC585R



GENERAL NOTES:

1. INSTALL ALL GIRTS AND FLANGE BRACES (FB) AS SHOWN.
2. WALL PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.
3. OTHER THAN FOR WALK DOORS AND WINDOWS SHOWN ON THE CONTRACT, DO NOT ADD ADDITIONAL WALL OPENINGS WITHOUT APPROVAL OF BUILDING MANUFACTURER OR PROFESSIONAL ENGINEER.
4. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHAVINGS CAUSED BY DRILLING.

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	10/25/22	FOR ERECTOR INSTALLATION	MDB	HPD	CM

BUILDING SOLD BY HERITAGE BUILDING SYSTEMS

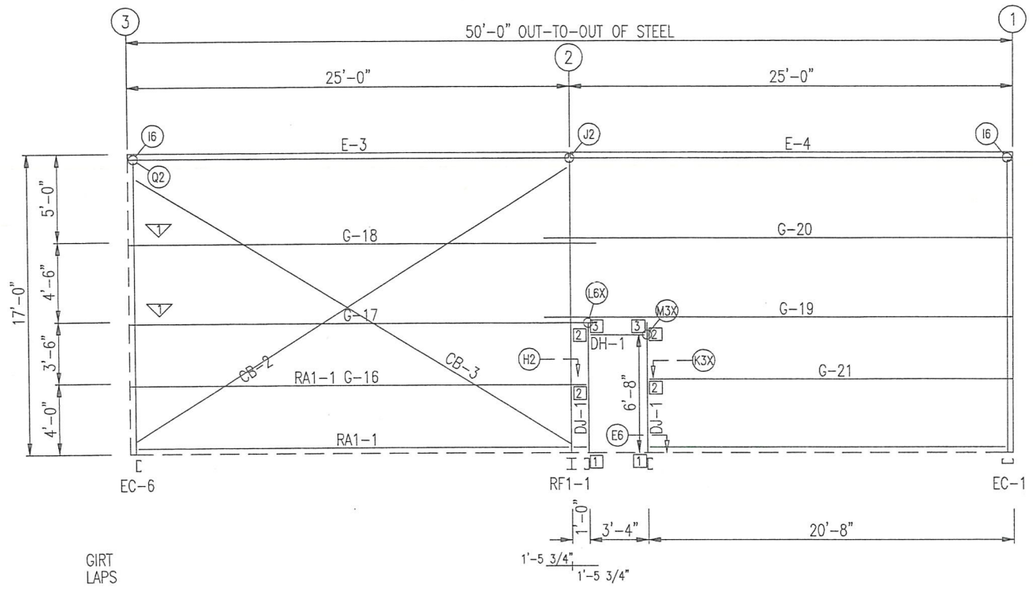
MICHAEL W. CUSTER, P.E.
642 OAKBEND DRIVE
COPPEL TX. 75019
PH. 972-571-7082

MICHAEL W CUSTER
Digitally signed by MICHAEL W CUSTER
Date: 2022.11.02 09:06:18 -05'00'

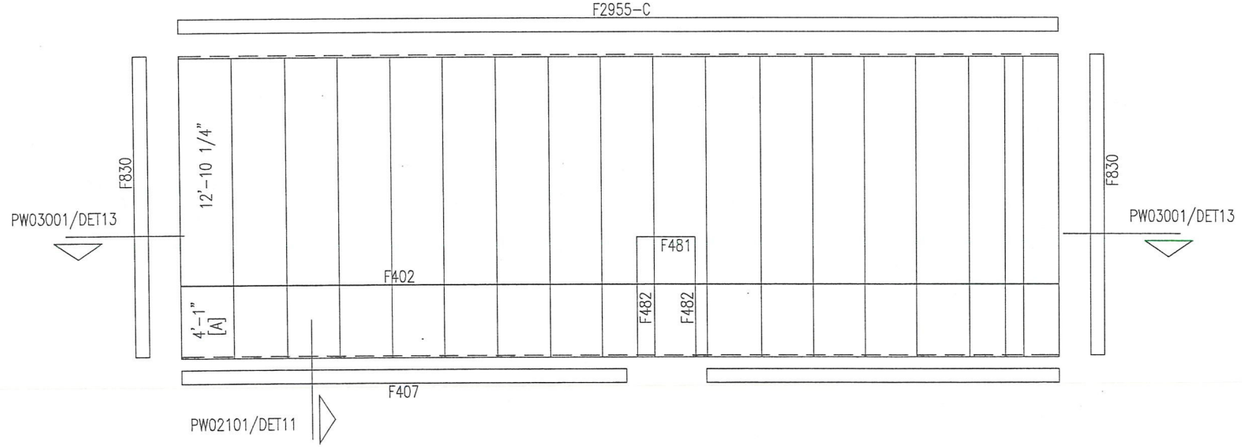
NORTH CAROLINA PROFESSIONAL SEAL 023748 ENGINEER MICHAEL W. CUSTER

THIS DRAWING WAS PREPARED BY AND/OR UNDER MY DIRECT SUPERVISION

PROJECT:	LUIS TIRADO – 45X50X17						
CUSTOMER:	LUIS TIRADO				OWNER:	LUIS TIRADO	
LOCATION:	LILLINGTON, NC 27546						
CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	5/24/19	N.T.S.	1	A	19-B-19564	E3	0



SIDEWALL FRAMING: FRAME LINE D



SIDEWALL SHEETING & TRIM: FRAME LINE D

PANELS: 26 Gauge PBR – Polar White

[A] PANELS: 26 Gauge PBR – Coal Black

BUILDING SIZE: 45'–0" x 50'–0" x 17'–0" 2.0:12

MEMBER TABLE
FRAME LINE D

MARK	PART	LENGTH
DJ–1	8X35C14	7'–1 3/4"
DH–1	8X35C14	3'–3 3/4"
E–3	8ES2L14	24'–11 1/2"
E–4	8ES2L14	24'–11 1/2"
G–16	8X25Z14	25'–7 3/4"
G–17	8X25Z16	26'–5 1/2"
G–18	8X25Z16	26'–5 1/2"
G–19	8X25Z14	26'–5 1/2"
G–20	8X25Z16	26'–5 1/2"
G–21	8X25Z16	20'–3 3/4"
CB–2	1/4" CABLE	29'–8"
CB–3	1/4" CABLE	29'–10"

FLANGE BRACE TABLE
FRAME LINE 3

▽ ID	PART	LENGTH
FB30	L2X2X14G	2'–6"

CONNECTION PLATES
FRAME LINE D

□ID	MARK/PART
1	CL753
2	CL751
3	SC425

GENERAL NOTES:

1. INSTALL ALL GIRTS AND FLANGE BRACES (FB) AS SHOWN.
2. WALL PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.
3. OTHER THAN FOR WALK DOORS AND WINDOWS SHOWN ON THE CONTRACT, DO NOT ADD ADDITIONAL WALL OPENINGS WITHOUT APPROVAL OF BUILDING MANUFACTURER OR PROFESSIONAL ENGINEER.
4. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHAVINGS CAUSED BY DRILLING.

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	10/25/22	FOR ERECTOR INSTALLATION	MDB	HPD	CM

BUILDING SOLD BY HERITAGE BUILDING SYSTEMS

MICHAEL W. CUSTER, P.E.
642 OAKBEND DRIVE
COPPEL TX. 75019
PH. 972–571–7082

MICHAEL W CUSTER

Digitally signed by MICHAEL W CUSTER
Date: 2022.11.02 09:06:32 -05'00'

NORTH CAROLINA PROFESSIONAL SEAL 023748 ENGINEER MICHAEL W CUSTER

THIS DOCUMENT WAS PREPARED BY AND/OR UNDER MY DIRECT SUPERVISION

PROJECT:	LUIS TIRADO – 45X50X17						
CUSTOMER:	LUIS TIRADO				OWNER:	LUIS TIRADO	
LOCATION:	LILLINGTON, NC 27546						
CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	5/24/19	N.T.S.	1	A	19–B–19564	E4	0

BEARING FRAME ONLY!

WASHER TO BE USED AT ENDWALL COLUMN TO ENDWALL RAFTER CONNECTION. USE ONE WASHER ON COLUMN SIDE. WASHER NOT NEEDED ON CLIP SIDE.

BOLT TABLE
FRAME LINE 1

LOCATION	QUAN	TYPE	DIA	LENGTH
ER-1/ER-2	4	A325	5/8"	1 3/4"
Columns/Raf	4	A325	1/2"	1 1/4"

MEMBER TABLE
FRAME LINE 1

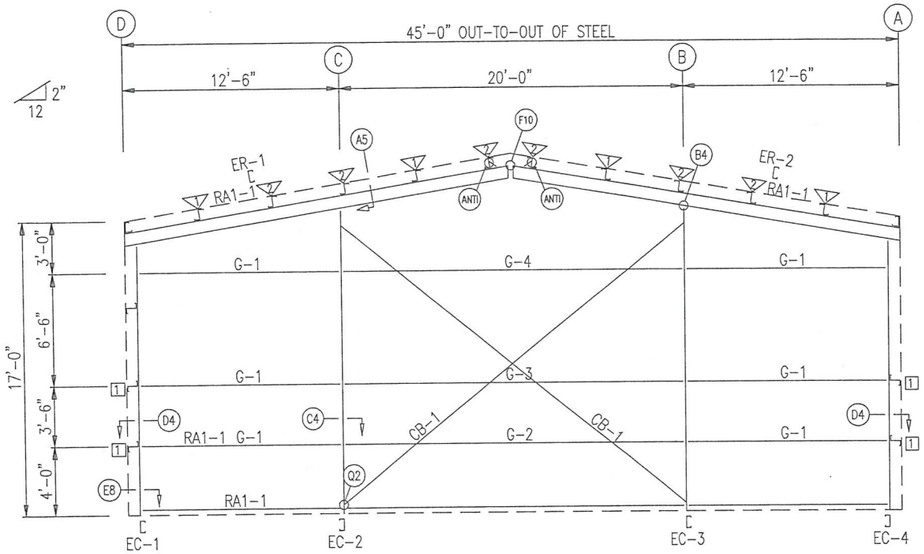
MARK	PART	LENGTH
EC-1	10F25C14	15'-5 7/8"
EC-2	10F35C13	17'-5 1/2"
EC-3	10F35C13	17'-5 1/2"
EC-4	10F25C14	15'-5 7/8"
ER-1	10F35C13	22'-11 1/8"
ER-2	10F35C13	22'-11 1/8"
G-1	8X25Z16	11'-1 3/4"
G-2	8X25Z16	19'-4"
G-3	8X25Z14	19'-4"
G-4	8X35Z13	19'-4"
CB-1	1/4" CABLE	26'-4"

FLANGE BRACE TABLE
FRAME LINE 1

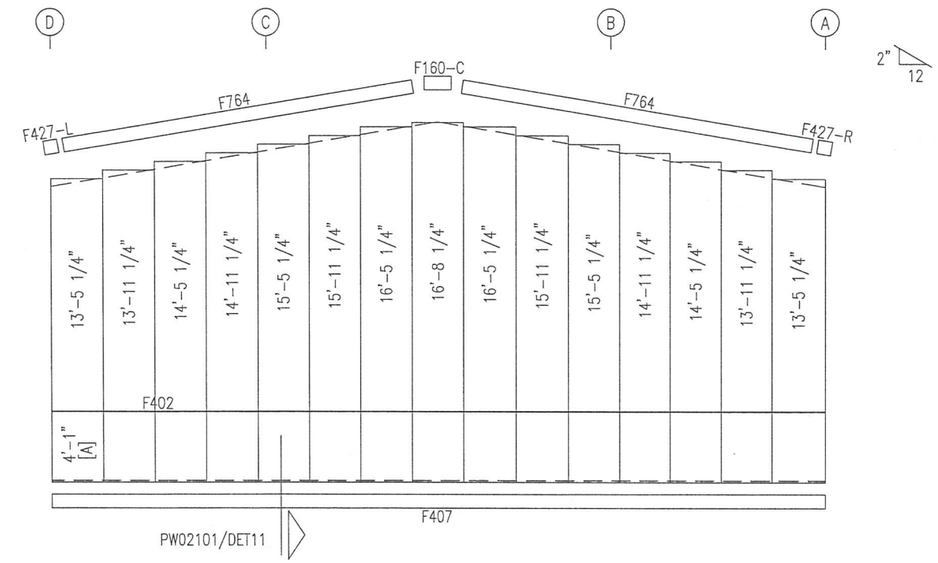
▽ ID	PART	LENGTH
1 FB30	L2X2X14G	2'-6"
2 FB7-1	L2.5X2.5X3/16	2'-6"

CONNECTION PLATES
FRAME LINE D

□ID	MARK/PART
1	SC5



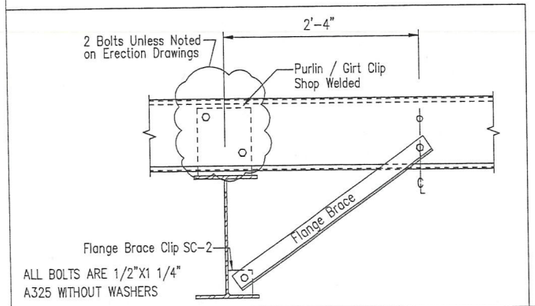
ENDWALL FRAMING: FRAME LINE 1



ENDWALL SHEETING & TRIM: FRAME LINE 1

PANELS: 26 Gauge PBR – Polar White

[A] PANELS: 26 Gauge PBR – Coal Black



GENERAL NOTES:

1. INSTALL ALL GIRTS AND FLANGE BRACES (FB) AS SHOWN.
2. WALL PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.
3. OTHER THAN FOR WALK DOORS AND WINDOWS SHOWN ON THE CONTRACT, DO NOT ADD ADDITIONAL WALL OPENINGS WITHOUT APPROVAL OF BUILDING MANUFACTURER OR PROFESSIONAL ENGINEER.
4. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHAVINGS CAUSED BY DRILLING.

BUILDING SIZE: 45'-0" x 50'-0" x 17'-0" 2.0:12

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	10/25/22	FOR ERECTOR INSTALLATION	MDB	HPD	CM

BUILDING SOLD BY HERITAGE BUILDING SYSTEMS

MICHAEL W. CUSTER, P.E.
642 OAKBEND DRIVE
COPPEL TX. 75019
PH. 972-571-7082

MICHAEL W CUSTER
Digitally signed by MICHAEL W CUSTER
Date: 2022.11.02 09:06:45 -05'00'

NORTH CAROLINA PROFESSIONAL SEAL 023748 ENGINEER MICHAEL W. CUSTER

THIS DOCUMENT WAS PREPARED BY, AND/OR UNDER MY DIRECT SUPERVISION

PROJECT:	LUIS TIRADO – 45X50X17						
CUSTOMER:	LUIS TIRADO				OWNER:	LUIS TIRADO	
LOCATION:	LILLINGTON, NC 27546						
CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	5/24/19	N.T.S.	1	A	19-B-19564	E5	0

BEARING FRAME ONLY!

WASHER TO BE USED AT ENDWALL COLUMN TO ENDWALL RAFTER CONNECTION. USE ONE WASHER ON COLUMN SIDE. WASHER NOT NEEDED ON CLIP SIDE.

BOLT TABLE
FRAME LINE 3

LOCATION	QUAN	TYPE	DIA	LENGTH
EC-5/ER-3	4	A325	5/8"	1 1/4"
ER-3/ER-4	4	A325	5/8"	1 3/4"
Int_Column/Raf	4	A325	1/2"	1 1/4"
EC-6/ER-4	4	A325	5/8"	1 1/2"

MEMBER TABLE
FRAME LINE 3

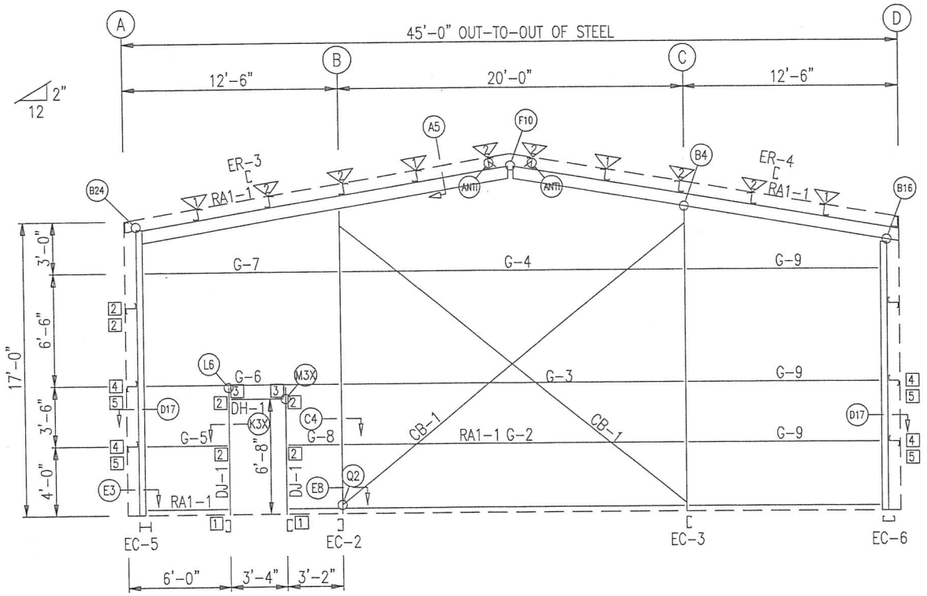
MARK	PART	LENGTH
EC-2	10F35C13	17'-5 1/2"
EC-3	10F35C13	17'-5 1/2"
EC-5	W10X12	16'-6 5/8"
EC-6	10F35C14	15'-8 9/16"
ER-3	10F35C13	21'-5"
ER-4	10F35C13	22'-11 1/8"
DJ-1	8X35C14	7'-1 3/4"
DH-1	8X35C14	3'-3 3/4"
G-2	8X25Z16	19'-4"
G-3	8X25Z14	19'-4"
G-4	8X35Z13	19'-4"
G-5	8X25Z16	4'-1 5/8"
G-6	8X25Z16	10'-7 5/8"
G-7	8X25Z16	10'-7 5/8"
G-8	8X25Z16	2'-6"
G-9	8X25Z16	10'-7 1/2"
CB-1	1/4" CABLE	26'-4"

FLANGE BRACE TABLE
FRAME LINE 1

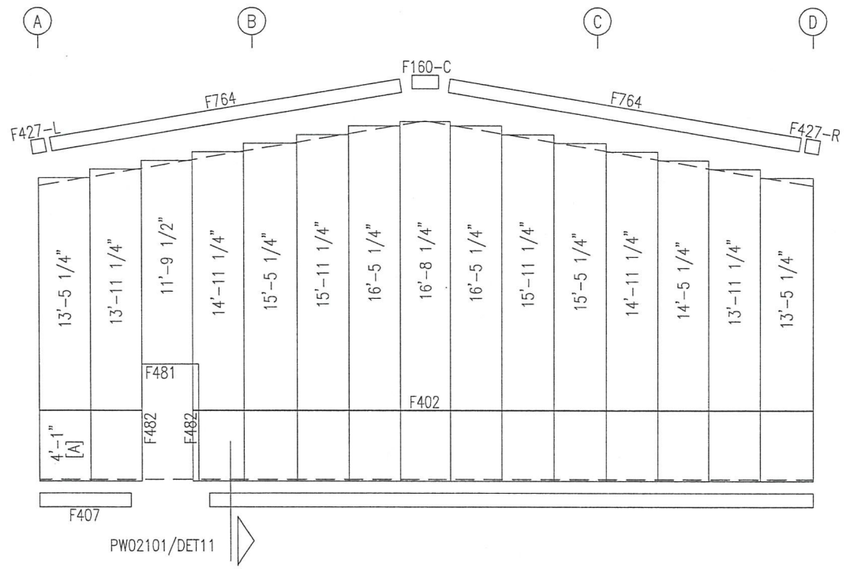
▽	ID	PART	LENGTH
1	FB30	L2X2X14G	2'-6"
2	FB7-1	L2.5X2.5X3/16	2'-6"

CONNECTION PLATES
FRAME LINE 3

□	ID	MARK/PART
	1	CL753
	2	CL751
	3	SC425
	4	SC5
	5	PC30



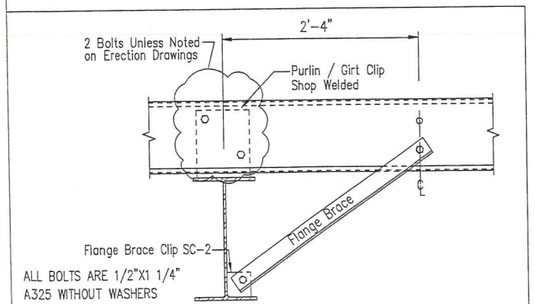
ENDWALL FRAMING: FRAME LINE 3



ENDWALL SHEETING & TRIM: FRAME LINE 3

PANELS: 26 Gauge PBR – Polar White

[A] PANELS: 26 Gauge PBR – Coal Black



GENERAL NOTES:

1. INSTALL ALL GIRTS AND FLANGE BRACES (FB) AS SHOWN.
2. WALL PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.
3. OTHER THAN FOR WALK DOORS AND WINDOWS SHOWN ON THE CONTRACT, DO NOT ADD ADDITIONAL WALL OPENINGS WITHOUT APPROVAL OF BUILDING MANUFACTURER OR PROFESSIONAL ENGINEER.
4. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHAVINGS CAUSED BY DRILLING.

BUILDING SIZE: 45'-0" x 50'-0" x 17'-0" 2.0:12

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	10/25/22	FOR ERECTOR INSTALLATION	MDB	HPD	CM

BUILDING SOLD BY HERITAGE BUILDING SYSTEMS

MICHAEL W. CUSTER, P.E.
642 OAKBEND DRIVE
COPPEL TX. 75019
PH. 972-571-7082

MICHAEL W CUSTER Digitally signed by MICHAEL W CUSTER Date: 2022.11.02 09:06:58 -05'00'

NORTH CAROLINA PROFESSIONAL SEAL 023748 ENGINEER MICHAEL W. CUSTER

THIS DOCUMENT WAS PREPARED BY AND/OR UNDER MY DIRECT SUPERVISION

PROJECT: LUIS TIRADO – 45X50X17

CUSTOMER: LUIS TIRADO OWNER: LUIS TIRADO

LOCATION: LILLINGTON, NC 27546

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	5/24/19	N.T.S.	1	A	19-B-19564	E6	0

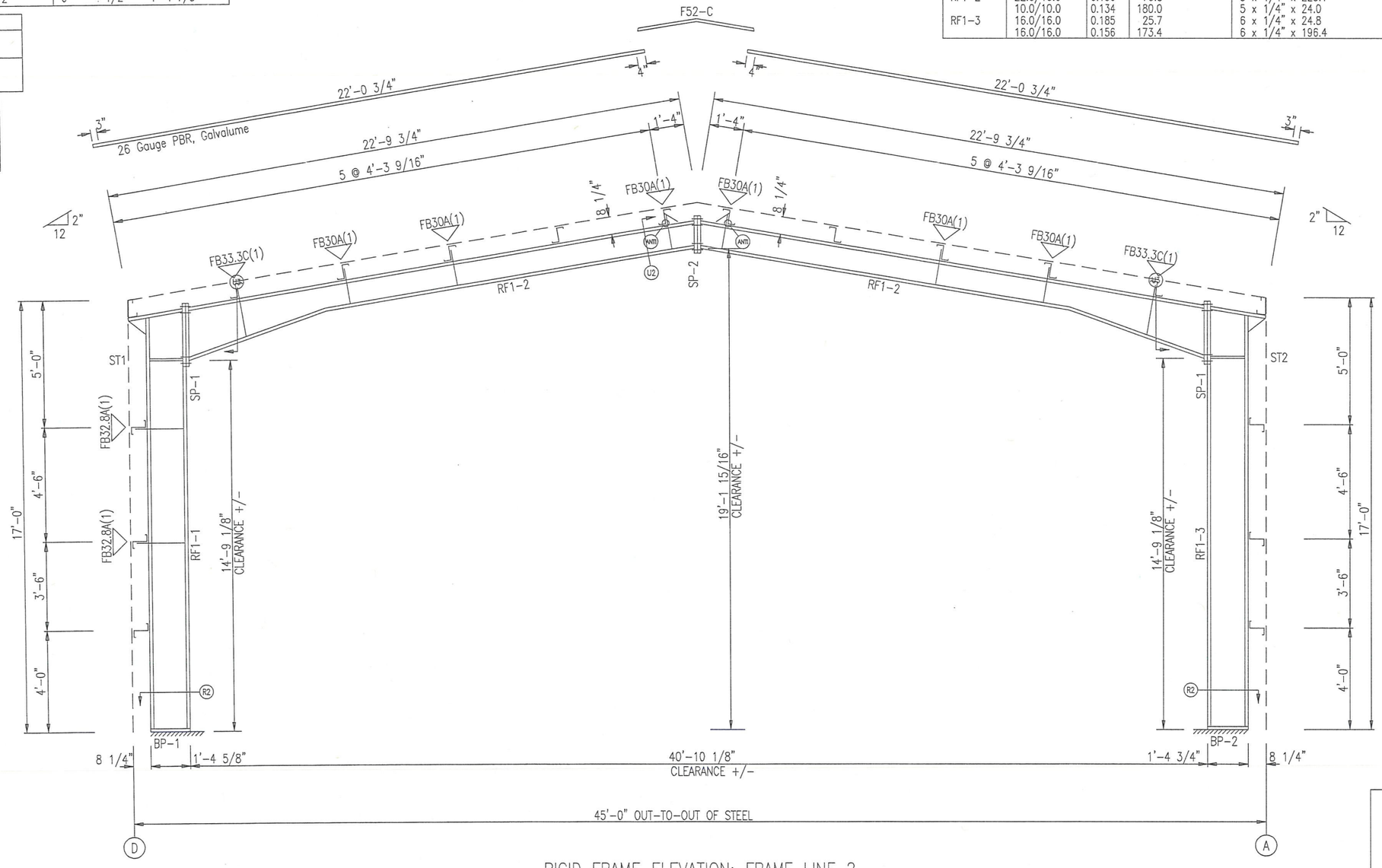
SPLICE PLATE & BOLT TABLE									
Mark	Qty Top	Bot	Int	Type	Dia	Length	Width	Thick	Length
SP-1	4	4	0	A325	3/4"	2 1/4"	6"	5/8"	2'-5 1/4"
SP-2	4	4	0	A325	3/4"	2"	6"	1/2"	1'-4 7/8"

STIFFENER TABLE				
Mark	Stiff Mark	Plate Size Width	Thick	Length
RF1-1	ST1	2 1/2	1/4"	16"
RF1-3	ST2	2 1/2	1/4"	16"

BASE PLATE TABLE			
Col Mark	Plate Size Width	Thick	Length
BP-1	6"	5/8"	1'-4 1/2"
BP-2	6"	3/8"	1'-5"

FLANGE BRACES: FBxx (1 or 2)
xx=length(in)
(1) One Side; (2) Two Sides
A - L2X2X14G
C - L2X2X1/8

MEMBER TABLE					
Mark	Web Depth Start/End	Web Plate Thick	Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RF1-1	16.0/16.0	0.156	173.1	6 x 1/4" x 196.1	6 x 1/4" x 120.0
	16.0/16.0	0.185	25.7	6 x 1/4" x 24.8	6 x 3/8" x 53.1
RF1-2	22.0/10.0	0.156	70.8	5 x 1/4" x 223.1	5 x 1/4" x 71.8
	10.0/10.0	0.134	180.0	5 x 1/4" x 24.0	5 x 1/4" x 178.3
RF1-3	16.0/16.0	0.185	25.7	6 x 1/4" x 24.8	6 x 1/2" x 53.4
	16.0/16.0	0.156	173.4	6 x 1/4" x 196.4	6 x 3/8" x 120.0



RIGID FRAME ELEVATION: FRAME LINE 2

BUILDING SIZE: 45'-0" x 50'-0" x 17'-0" 2.0:12

GENERAL NOTES:

1. BOLTED JOINTS WITH A325 TYPE 1 BOLTS GREATER THAN 1/2" DIAMETER ARE SPECIFIED AS PRE-TENSIONED JOINTS IN ACCORDANCE WITH THE MOST RECENT EDITION OF THE RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS. PRE-TENSIONING CAN BE ACCOMPLISHED BY USING THE TURN-OF-NUT METHOD OF TIGHTENING, CALIBRATED WRENCH, TWIST-OFF-TYPE TENSION-CONTROL BOLTS OR DIRECT-TENSION-INDICATOR AS ACCEPTABLE TO THE INSPECTING AGENCY AND BUILDING OFFICIAL. INSTALLATION INSPECTION REQUIREMENTS FOR PRE-TENSIONED JOINTS (SPECIFICATION FOR STRUCTURAL JOINTS SECTION 9.2) USING TURN-OF-NUT/CALIBRATED WRENCH/TWIST OFF TYPE TENSION CONTROL BOLTS/DIRECT TENSION INDICATOR] METHOD IS SUGGESTED. THE CONNECTIONS ON THIS PROJECT ARE NOT SLIP CRITICAL.
2. ALL FIELD CONNECTIONS OF SECONDARY FRAMING SHALL BE BOLTED WITH A325 BOLTS.
3. INSTALL ALL FLANGE BRACES ON COLUMN AND RAFTER AS SHOWN.

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	10/25/22	FOR ERECTOR INSTALLATION	MDB	HPD	CM

BUILDING SOLD BY HERITAGE BUILDING SYSTEMS

MICHAEL W. CUSTER, P.E.
642 OAKBEND DRIVE
COPPEL TX. 75019
PH. 972-571-7082

MICHAEL W CUSTER
Digitally signed by MICHAEL W CUSTER
Date: 2022.11.02 09:07:11 -05'00'

NORTH CAROLINA PROFESSIONAL SEAL 023748 ENGINEER MICHAEL W. CUSTER

PROJECT: LUIS TIRADO - 45X50X17							
CUSTOMER: LUIS TIRADO					OWNER: LUIS TIRADO		
LOCATION: LILLINGTON, NC 27546							
CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	5/24/19	N.T.S.	1	A	19-B-19564	E7	0

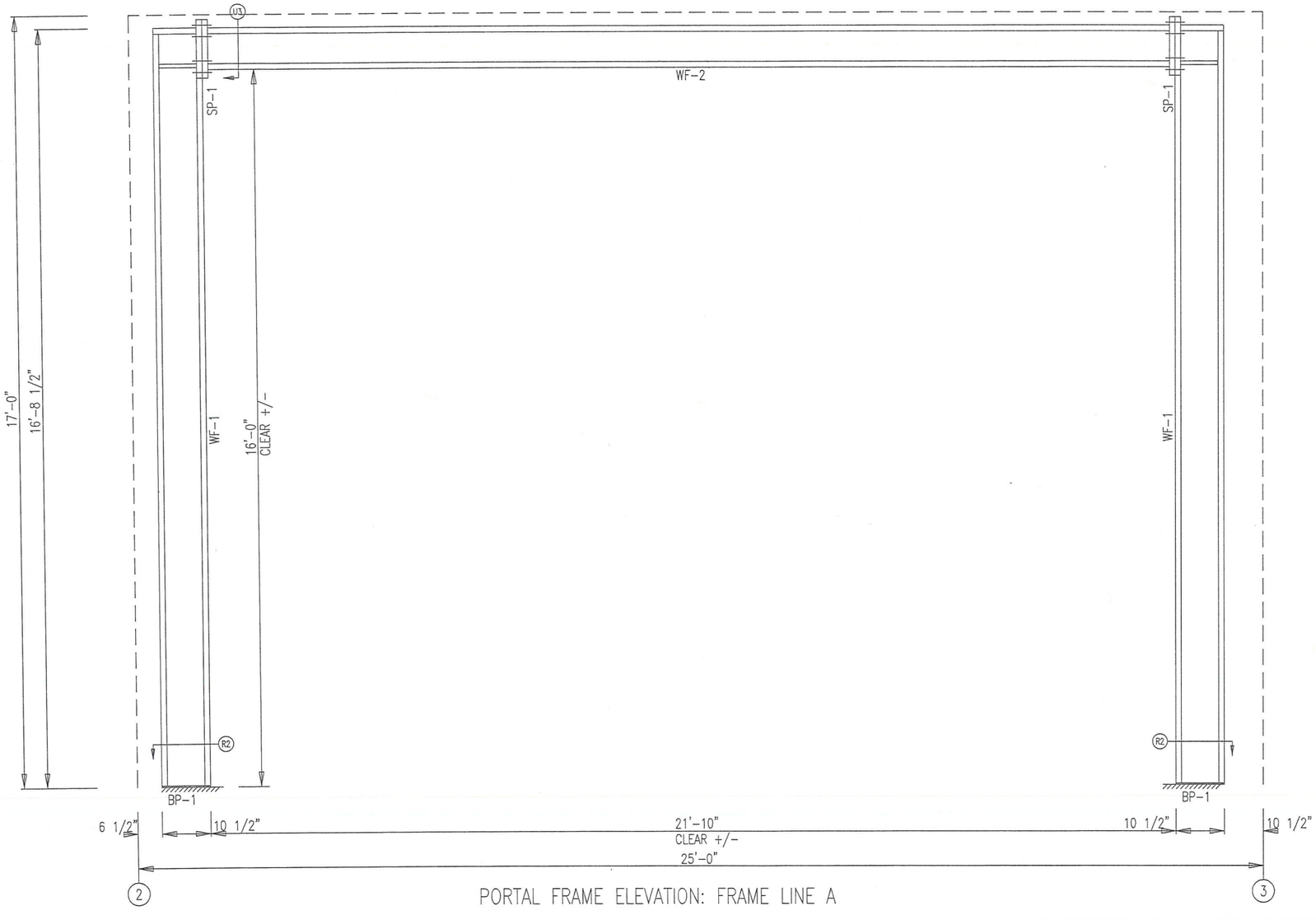
THIS DOCUMENT WAS PREPARED BY AND/OR UNDER MY DIRECT SUPERVISION

SPLICE PLATES & BOLTS								
Splice Mark	Quan Top/	Bot	Bolt Type	Bolt Dia	Bolt Length	Plate Size Width	Thick	Length
SP– 1	4	4	A325	3/4"	2"	6"	1/2"	1'–4 1/4"

STIFFENER TABLE				
Mark	Stiff Mark	Plate Size Width	Thick	Length
WF–1	St– 1	2"	5/16	10"

BASE PLATES			
Col Mark	Plate Size Width	Thick	Length
BP– 1	6"	3/8"	10 1/2"

MEMBER SIZE TABLE (in)						
MARK	LENGTH	WEB DEPTH START/END	WEB PLATE THICK	WEB PLATE LENGTH	OUTSIDE FLANGE W x T x LENGTH	INSIDE FLANGE W x T x LENGTH
WF–1	200.5	10.0/10.0	0.156	16'–8 1/2"	5 x 1/4" x 16'–8 1/2"	5 x 1/4" x 16'–8 1/2"
WF–2	261.5	8.0/ 8.0	0.156	21'–9 1/2"	6 x 1/4" x 21'–9 1/2"	6 x 1/4" x 21'–9 1/2"



PORTAL FRAME ELEVATION: FRAME LINE A

BUILDING SIZE: 45'–0" x 50'–0" x 17'–0" 2.0:12

BUILDING SOLD BY HERITAGE BUILDING SYSTEMS

GENERAL NOTES:

1. BOLTED JOINTS WITH A325 TYPE 1 BOLTS GREATER THAN 1/2" DIAMETER ARE SPECIFIED AS PRE–TENSIONED JOINTS IN ACCORDANCE WITH THE MOST RECENT EDITION OF THE RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS. PRE–TENSIONING CAN BE ACCOMPLISHED BY USING THE TURN–OF–NUT METHOD OF TIGHTENING, CALIBRATED WRENCH, TWIST–OFF–TYPE TENSION–CONTROL BOLTS OR DIRECT–TENSION–INDICATOR AS ACCEPTABLE TO THE INSPECTING AGENCY AND BUILDING OFFICIAL. INSTALLATION INSPECTION REQUIREMENTS FOR PRE–TENSIONED JOINTS (SPECIFICATION FOR STRUCTURAL JOINTS SECTION 9.2) USING TURN–OF–NUT/CALIBRATED WRENCH/TWIST OFF TYPE TENSION CONTROL BOLTS/DIRECT TENSION INDICATOR] METHOD IS SUGGESTED. THE CONNECTIONS ON THIS PROJECT ARE NOT SLIP CRITICAL.
2. ALL FIELD CONNECTIONS OF SECONDARY FRAMING SHALL BE BOLTED WITH A325 BOLTS.
3. INSTALL ALL FLANGE BRACES ON COLUMN AND RAFTER AS SHOWN.

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	10/25/22	FOR ERECTOR INSTALLATION	MDB	HPD	CM

MICHAEL W. CUSTER, P.E.
642 OAKBEND DRIVE
COPPEL TX. 75019
PH. 972–571–7082

MICHAEL W CUSTER

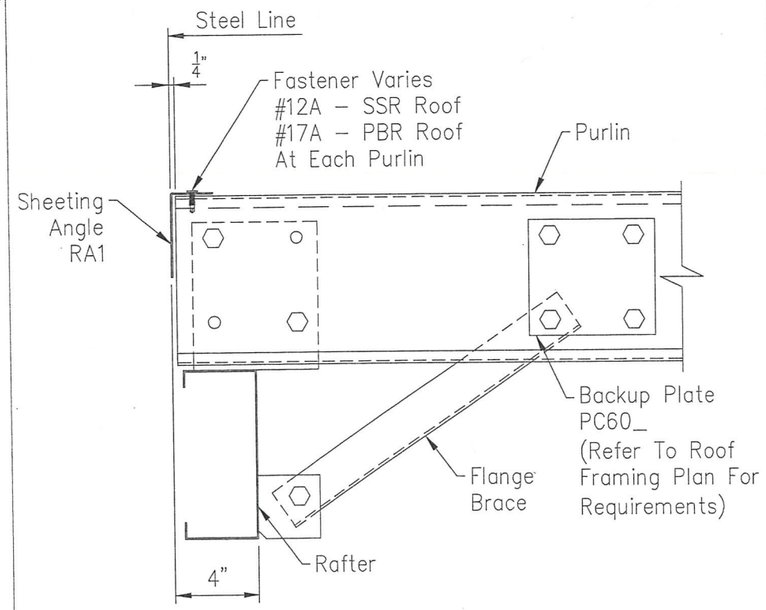
Digitally signed by MICHAEL W CUSTER
Date: 2022.11.02 09:07:24 -05'00'

NORTH CAROLINA PROFESSIONAL SEAL 023748 ENGINEER MICHAEL W. CUSTER

THIS DOCUMENT WAS PREPARED BY AND/OR UNDER MY DIRECT SUPERVISION

PROJECT:	LUIS TIRADO – 45X50X17		
CUSTOMER:	LUIS TIRADO	OWNER:	LUIS TIRADO
LOCATION:	LILLINGTON, NC 27546		

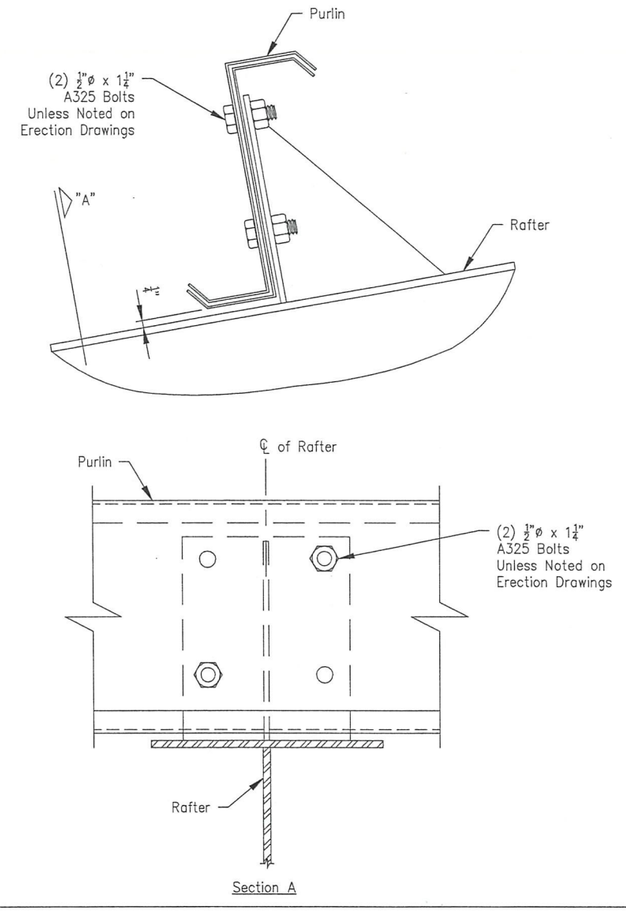
CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	5/24/19	N.T.S.	1	A	19–B–19564	E8	0



A5	Purlin To Bearing Frame Single Cold Form Rafter	Date Jul '21
Page MB–A5		Rev 02

Purlin with Anti-Roll Back-up Clip at Rafter

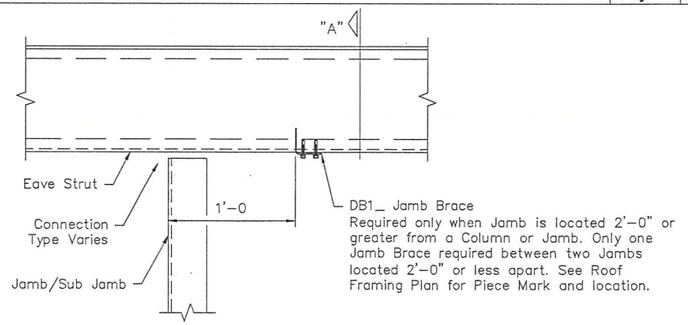
Page CF02035	
Date Dec '14	Rev 04



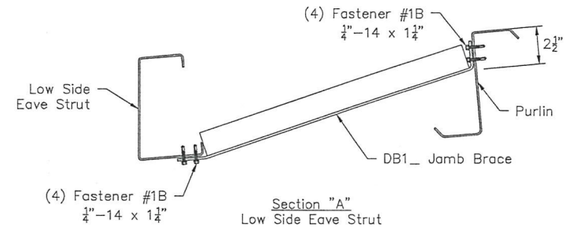
Section A

Framed Opening - Jamb Brace at Eave Strut to Purlin

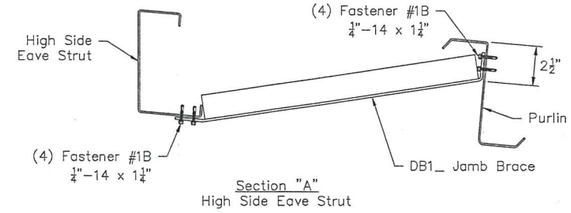
Page AC00910	
Date May '19	Rev 01



Jamb Brace at Eave Strut to Purlin



Section "A"
Low Side Eave Strut



Section "A"
High Side Eave Strut

BUILDING SIZE: 45'–0" x 50'–0" x 17'–0" 2.0:12

BUILDING SOLD BY HERITAGE BUILDING SYSTEMS

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	10/25/22	FOR ERECTOR INSTALLATION	MDB	HPD	CM

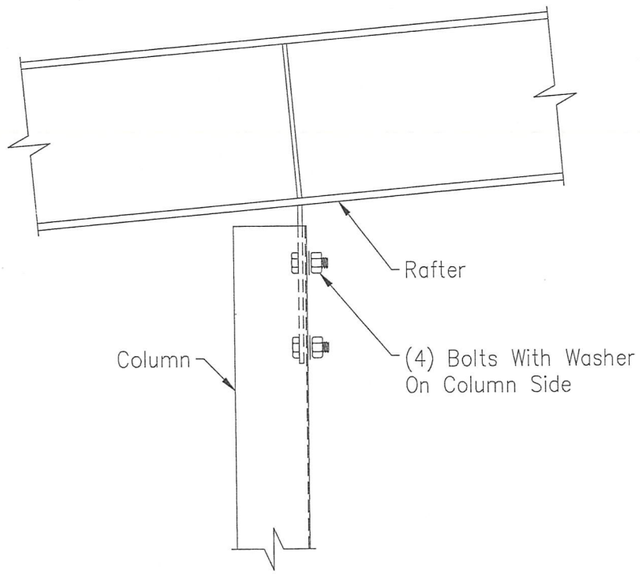
MICHAEL W. CUSTER, P.E.
642 OAKBEND DRIVE
COPPEL TX. 75019
PH. 972–571–7082

MICHAEL W CUSTER Digitally signed by MICHAEL W CUSTER Date: 2022.11.02 09:07:38 -05'00'

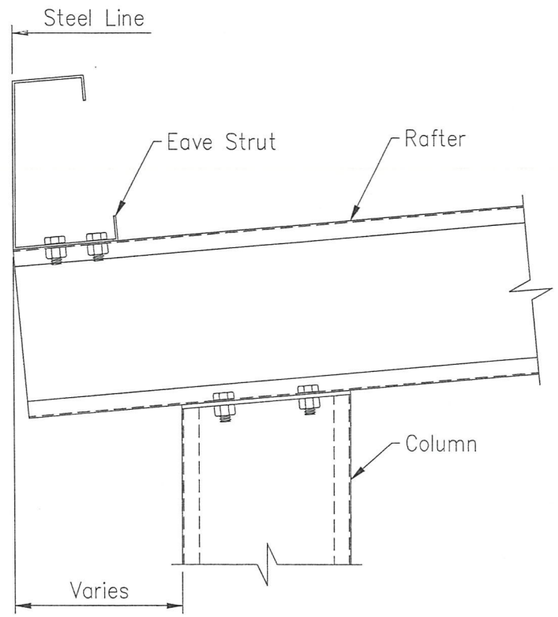
NORTH CAROLINA PROFESSIONAL SEAL 023748 ENGINEER MICHAEL W. CUSTER

THIS DOCUMENT WAS PREPARED BY AND/OR UNDER MY DIRECT SUPERVISION

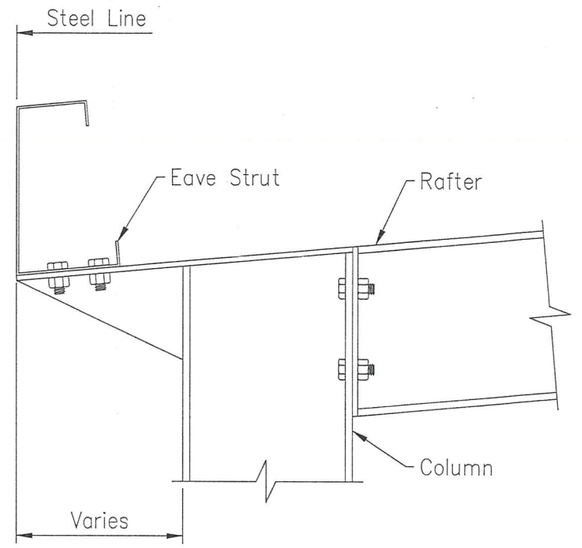
PROJECT:	LUIS TIRADO – 45X50X17						
CUSTOMER:	LUIS TIRADO				OWNER:	LUIS TIRADO	
LOCATION:	LILLINGTON, NC 27546						
CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	5/24/19	N.T.S.	1	A	19–B–19564	DET1	0



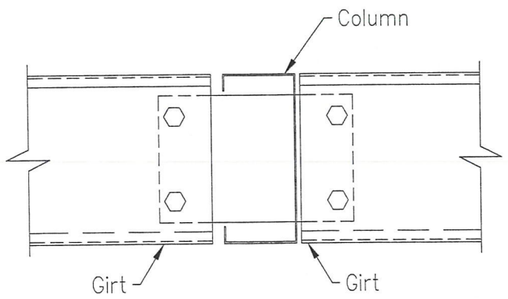
B4	Cold Form Endwall Column To Rafter	Date Aug '20
Page MB-B4		Rev 01



B16	Corner Column To Bearing Frame Single Cold Form Rafter	Date Jun '17
Page MB-B16		Rev 00



B24	Corner Column To Bearing Frame Rafter	Date Jun '17
Page MB-B24		Rev 00



C4	Girt To Cold Form Column	Date Jun '17
Page MB-C4		Rev 00

BUILDING SIZE: 45'-0" x 50'-0" x 17'-0" 2.0:12

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	10/25/22	FOR ERECTOR INSTALLATION	MDB	HPD	CM

BUILDING SOLD BY HERITAGE BUILDING SYSTEMS

MICHAEL W. CUSTER, P.E.
642 OAKBEND DRIVE
COPPEL TX. 75019
PH. 972-571-7082

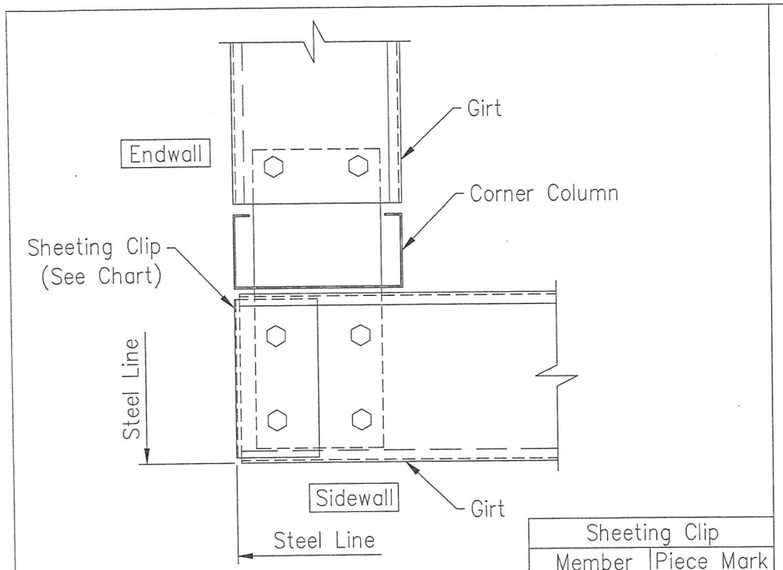
MICHAEL W CUSTER Digitally signed by MICHAEL W CUSTER Date: 2022.11.02 09:07:51 -05'00'

NORTH CAROLINA PROFESSIONAL SEAL 023748 ENGINEER MICHAEL W. CUSTER

THIS DOCUMENT WAS PREPARED BY AND/OR UNDER MY DIRECT SUPERVISION

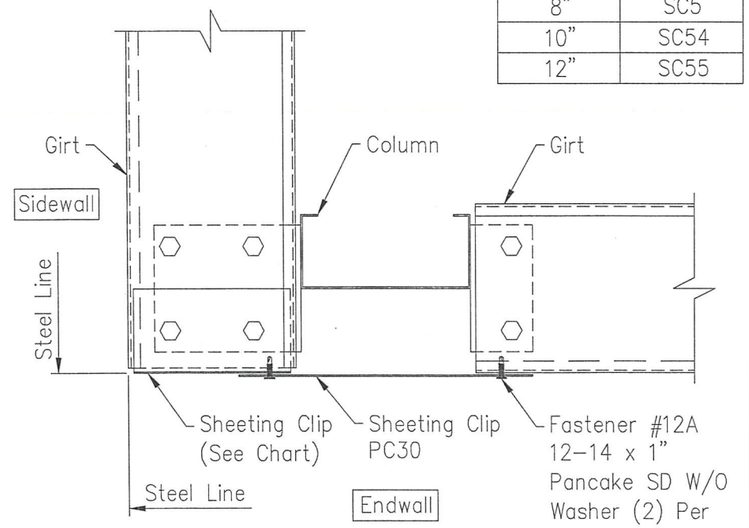
PROJECT: LUIS TIRADO - 45X50X17
CUSTOMER: LUIS TIRADO
OWNER: LUIS TIRADO
LOCATION: LILLINGTON, NC 27546

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	5/24/19	N.T.S.	1	A	19-B-19564	DET2	0



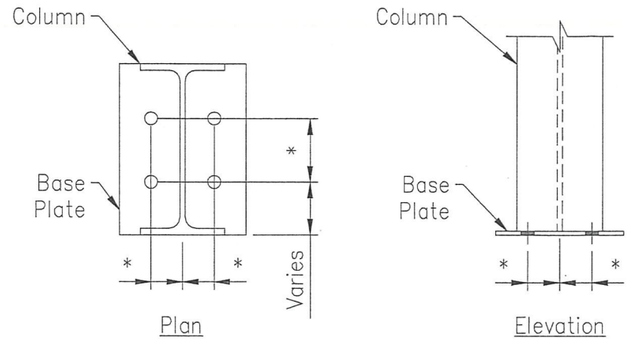
Sheeting Clip	
Member	Piece Mark
8"	SC5
10"	SC54
12"	SC55

D4	Girt To Cold Form Corner Column	Date Oct '19
Page MB-D4		Rev 01



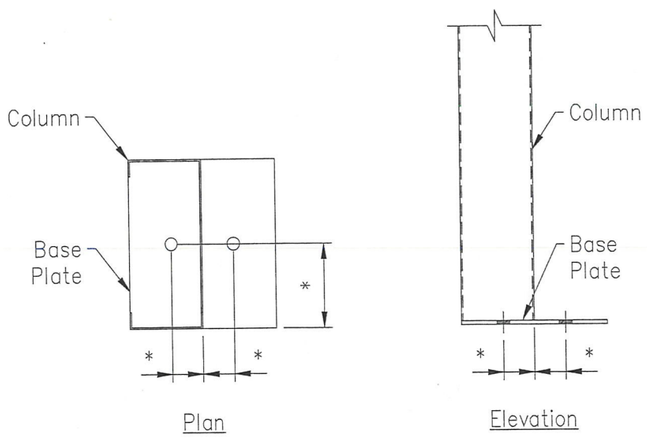
Sheeting Clip	
Member	Piece Mark
8"	SC5
10"	SC54
12"	SC55

D17	Girt To Cold Form Rotated Corner Column	Date Jun '17
Page MB-D17		Rev 00



* – Refer To Anchor Rod Setting Plan For Dimension

E3	Endwall Column Base Plate	Date Dec '18
Page MB-E3		Rev 01



* – Refer To Anchor Rod Setting Plan For Dimension

E8	Cold Form Endwall Column Base Plate	Date Dec '18
Page MB-E8		Rev 01

BUILDING SIZE: 45'–0" x 50'–0" x 17'–0" 2.0:12

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	10/25/22	FOR ERECTOR INSTALLATION	MDB	HPD	CM

BUILDING SOLD BY HERITAGE BUILDING SYSTEMS

MICHAEL W. CUSTER, P.E.
642 OAKBEND DRIVE
COPPEL TX. 75019
PH. 972-571-7082

MICHAEL W CUSTER
Digitally signed by MICHAEL W CUSTER
Date: 2022.11.02 09:08:06 -05'00'

NORTH CAROLINA PROFESSIONAL SEAL 023748 ENGINEER MICHAEL W. CUSTER
THIS DOCUMENT WAS PREPARED BY AND/OR UNDER MY DIRECT SUPERVISION

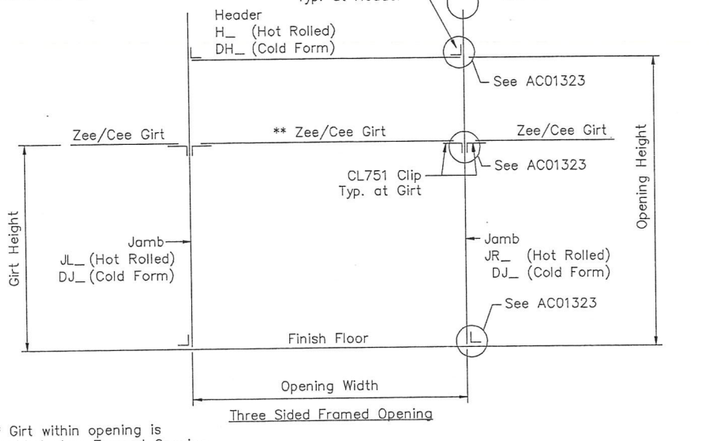
PROJECT: LUIS TIRADO – 45X50X17
CUSTOMER: LUIS TIRADO
OWNER: LUIS TIRADO
LOCATION: LILLINGTON, NC 27546

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	5/24/19	N.T.S.	1	A	19-B-19564	DET3	0

Bolted Clips - Framed Opening Connections - Cold Form and Hot Rolled Cee - Three and Four Sided Openings

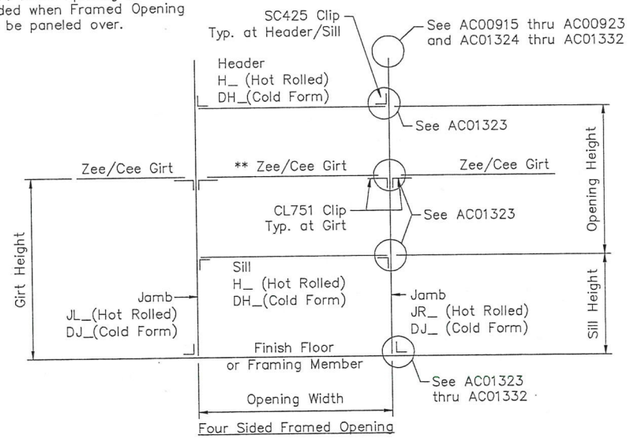
Page AC01320 | Date May '19 | Rev 03

Note; All Horizontal Members within clear Opening are Clipped and attached to Jambs with ½"ø Fin Head Bolts.



Three Sided Framed Opening

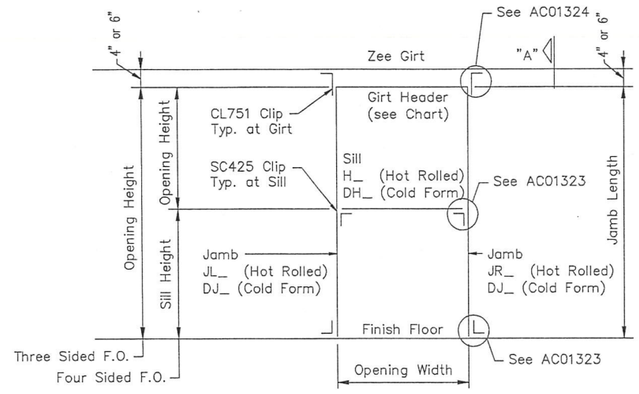
** Girt within opening is provided when Framed Opening is to be paneled over.

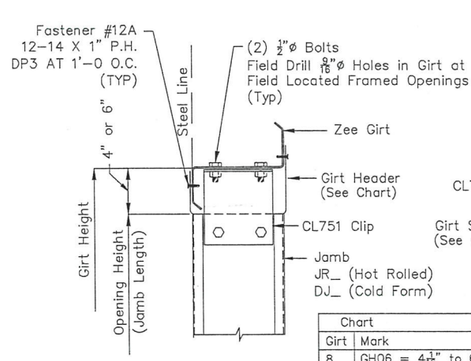


Four Sided Framed Opening

Bolted Clips - Framed Opening Connections - Three and Four Sided Openings - Girt Header

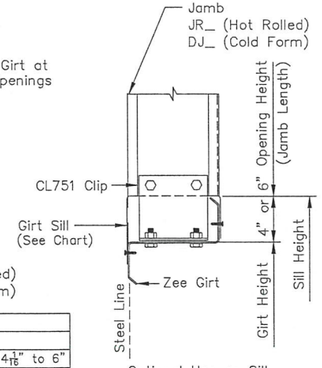
Page AC01322 | Date Apr '20 | Rev 05





Section "A"

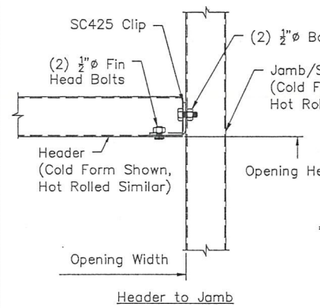
Chart	
Girt	Mark
8	GH06 = 4 1/16" to 6"
8	GH08 <= 4"
10	GH10
12	GH12



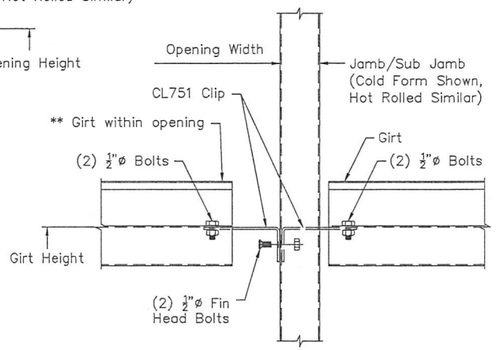
Optional Use as Sill

Bolted Clips - Framed Opening Connections - Cold Form and Hot Rolled Base, Girt, Header, and Sill to Jamb

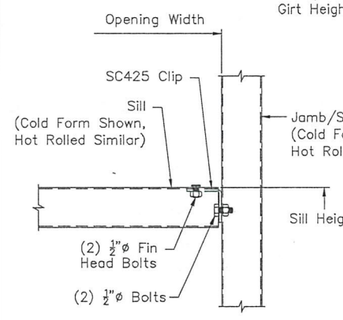
Page AC01323 | Date May '19 | Rev 03



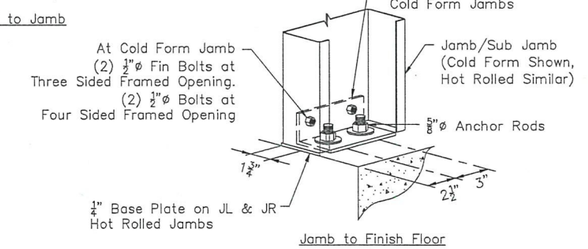
Header to Jamb



Girt to Jamb



Sill to Jamb



Jamb to Finish Floor

BUILDING SIZE: 45'-0" x 50'-0" x 17'-0" 2.0:12

BUILDING SOLD BY HERITAGE BUILDING SYSTEMS

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	10/25/22	FOR ERECTOR INSTALLATION	MDB	HPD	CM

MICHAEL W. CUSTER, P.E.
642 OAKBEND DRIVE
COPPEL TX. 75019
PH. 972-571-7082

MICHAEL W CUSTER Digitally signed by MICHAEL W CUSTER Date: 2022.11.02 09:08:20 -05'00'

NORTH CAROLINA PROFESSIONAL ENGINEER SEAL 023748

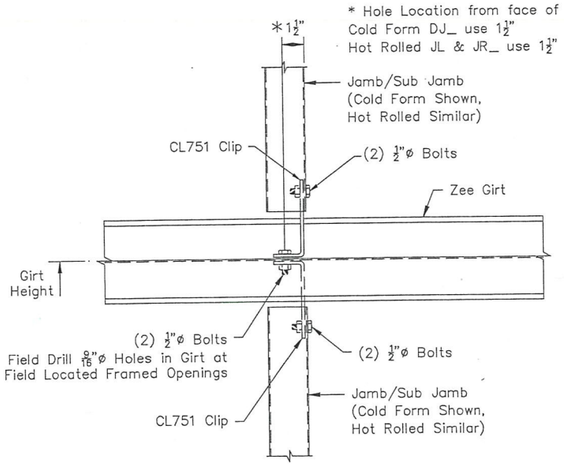
THIS DOCUMENT WAS PREPARED BY, AND/OR UNDER MY DIRECT SUPERVISION

PROJECT:	LUIS TIRADO - 45X50X17		
CUSTOMER:	LUIS TIRADO	OWNER:	LUIS TIRADO
LOCATION:	LILLINGTON, NC 27546		

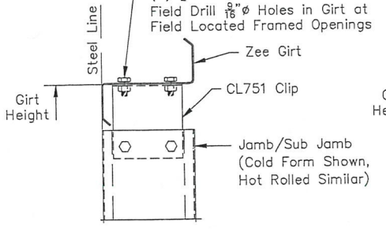
CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	5/24/19	N.T.S.	1	A	19-B-19564	DET4	0

Bolted Clips - Framed Opening Connections - Cold Form and Hot Rolled Jamb/Sub Jamb to Zee Girt

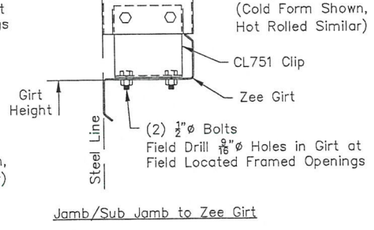
Page AC01324 | Date May '19 | Rev 01



Jamb/Sub Jamb to Zee Girt



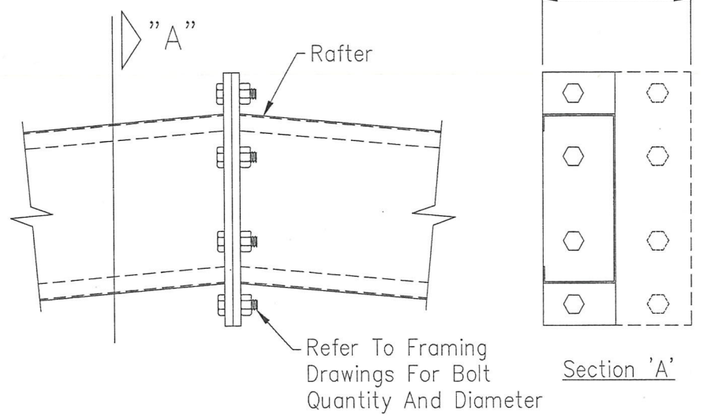
Jamb/Sub Jamb to Zee Girt



Jamb/Sub Jamb to Zee Girt

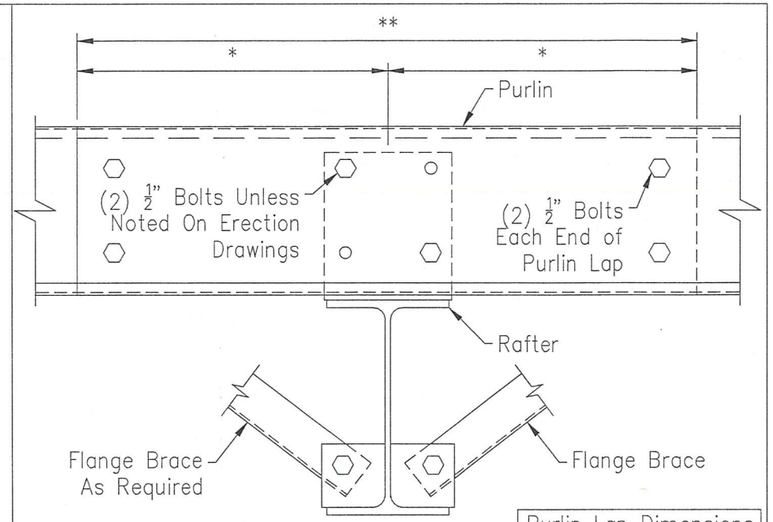
F10 — Endwall Bearing Frame – Cold Form Rafter Splice At Ridge

Page MB-F10 | Date Jun '17 | Rev 00



G2 — Purlin To Rigid Frame

Page MB-G2 | Date Sep '19 | Rev 01



Purlin Lap Dimensions	
*	**
1'-5¾"	2'-11½"
2'-5¾"	4'-11½"
3'-1¾"	6'-3½"

BUILDING SIZE: 45'-0" x 50'-0" x 17'-0" 2.0:12

BUILDING SOLD BY HERITAGE BUILDING SYSTEMS

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	10/25/22	FOR ERECTOR INSTALLATION	MDB	HPD	CM

MICHAEL W. CUSTER, P.E.
642 OAKBEND DRIVE
COPPEL TX. 75019
PH. 972-571-7082

MICHAEL W CUSTER

Digitally signed by MICHAEL W CUSTER
Date: 2022.11.02 09:08:34 -05'00'

NORTH CAROLINA PROFESSIONAL SEAL 023748 ENGINEER MICHAEL W. CUSTER

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PROJECT: LUIS TIRADO – 45X50X17

CUSTOMER: LUIS TIRADO | OWNER: LUIS TIRADO

LOCATION: LILLINGTON, NC 27546

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	5/24/19	N.T.S.	1	A	19-B-19564	DET5	0