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***TIRADO TRUCK REPAIR GARAGE***  
US 421 SOUTH, HARNETT COUNTY, NC

**BUILDING DEPARTMENT:**  
HARNETT COUNTY  
PLANNING & INSPECTIONS DEPARTMENT  
420 MCKINNEY PARKWAY  
LILLINGTON, NC 27546  
910-893-7525

LUIS TIRADO  
3577 OLD US 421  
LILLINGTON, NC 27546  
919-648-3999

APPLICABLE CODES INCLUDE BUT ARE NOT LIMITED TO THE FOLLOWING:	
BUILDING	2018 NC BUILDING CODE
PLUMBING	2018 NC PLUMBING CODE
MECHANICAL	2018 NC MECHANICAL CODE
ELECTRICAL	2020 NATIONAL ELECTRICAL CODE (NFPA-70)
FIRE PREVENTION	2018 NC FIRE CODE
ENERGY	2018 NC ENERGY CONSERVATION CODE
ACCESSIBILITY	ICC A117.1-2009 AND THE AMERICANS WITH DISABILITIES ACT (ADAAG) 2018 NC BUILDING CODE CHAPTER 11

JENKINS CONSULTING ENGINEERS, P.A.  
OFFICE in EUREKA SPRINGS, NC  
KELLY J. DODSON  
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1606 MCARTHUR ROAD  
FAYETTEVILLE, NC 28311-1002  
910-822-1724

TBD



DATE:	10 JULY 2025
PROJECT #:	2025-04-02.2
DRAWN BY:	BT
KJD	

PRELIMINARY <input type="checkbox"/> FOR DESIGN DEVELOPMENT ONLY FINAL DRAWING <input checked="" type="checkbox"/> FOR CONSTRUCTION	OWNER/TENANT: LUIS TIrado 3577 OLD US 421 LILLINGTON, NC 27546	CONTRACTOR/BUILDER: TBD TBD
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DISPOSITIONS:	
DATE	DESCRIPTION
07/10/25	FINAL FOR CONS.

**SHEET:**

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**IRADO TRUCK SHOP**

US 421 SOUTH, HARNETT COUNTY, NC

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**COVER & INDEX TO DRAWINGS**

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CS

NOT TO SCALE

US HWY 421 S

HIGHLAND DR

SITE

TIRADO TRUCK

US HWY 27

BRICK MILL RD

AIRPORT RD

US HWY 421 S

NORTH








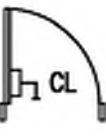

THE PROJECT SCOPE IS TO CONSTRUCT A NEW BUILDING TO BE USED AS A REPAIR GARAGE.









LEGEND	
SYMBOL	DESCRIPTION
F.E. 	ABC FIRE EXTINGUISHER SUGGESTED LOCATION
***** 	GREATEST TRAVEL DISTANCE
33" 	EXIT WIDTH, 36" - 3 = 33" CLEAR WIDTH.
165 CAP. 22 ACT.	EXIT CAPACITY (NUMBER OF PERSONS) ACTUAL OCCUPANT LOAD FOR EXIT DOOR
	EXIT SIGN
	EMERGENCY LIGHT
36" 	AISLE WIDTH WHERE SHOWN
	EXIT SIGN WITH EMERGENCY LIGHTING
	CL- DOOR CLOSER
ROOM LABEL	DESCRIPTION
4 BAY AREA 	OCCUPANT TOTAL ROOM NAME ROOM NUMBER
REPAIR GARAGE 100 SF	FUNCTION TYPE SPACE AREA

# TIRADO TRUCK SHOP

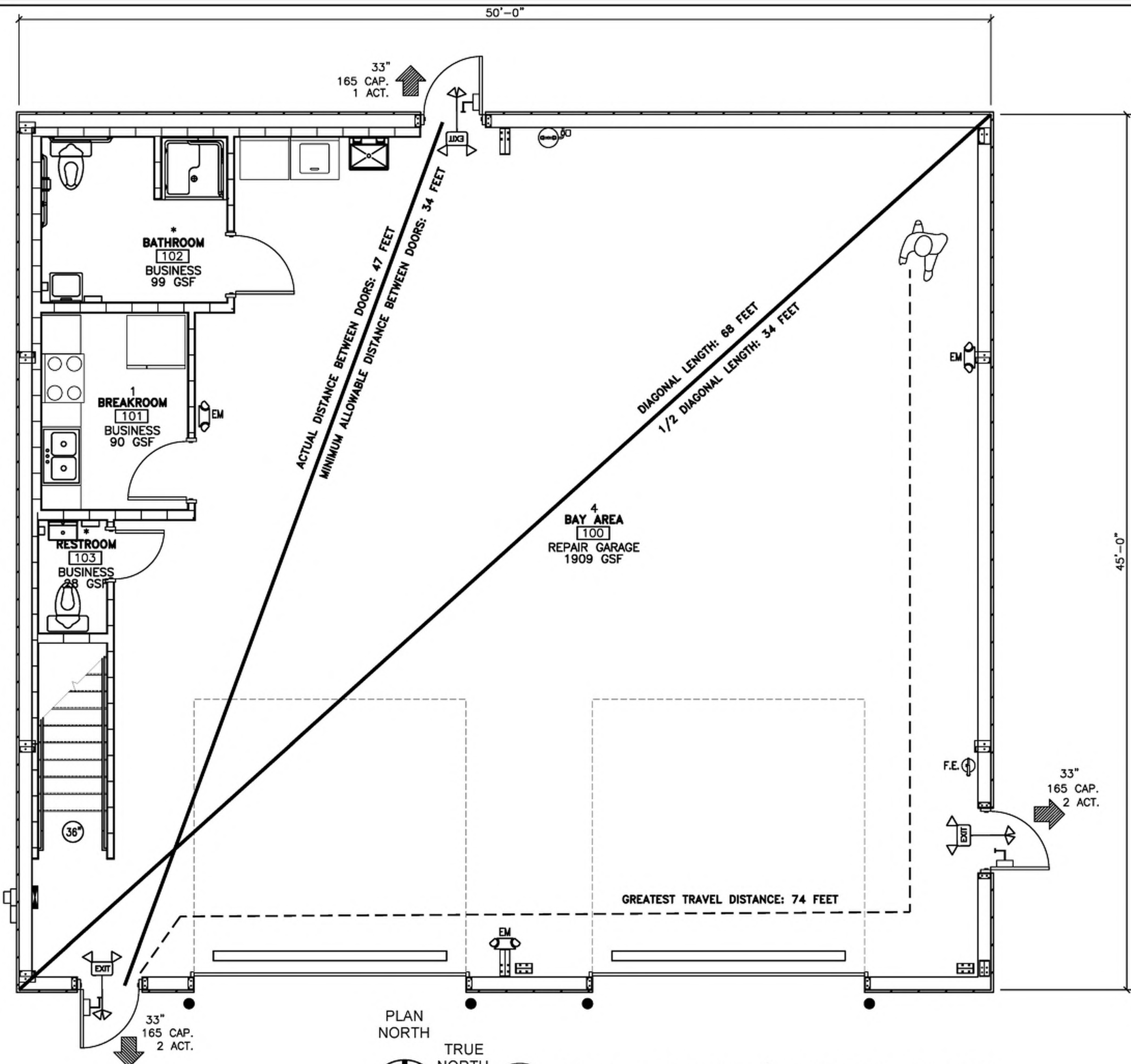
## STORAGE (S-1): REPAIR GARAGE

GROSS SQUARE FOOTAGE OF BUILDING 2,250 SQ. FT.  
TYPE OF CONSTRUCTION: V-B  
BUILDING IS NOT TO BE USED AS A REPAIR GARAGE WITH OFFICE SPACE.  
THIS BUILDING IS NOT PROTECTED BY FIRE SPRINKLERS  
OCCUPANT LOAD FOR CALCULATING EGRESS CAPACITY:  
SPACE OCCUPANCY BY FUNCTION OF SPACE  
SEE TABLE ON THIS SHEET FOR INDIVIDUAL SPACE TOTALS  
TOTAL OCCUPANT LOAD BY AREAS = 5 PERSONS

GREATEST TRAVEL DISTANCE SHOWING: 74 FEET (PER 1017)  
MAXIMUM ALLOWABLE TRAVEL DISTANCE: 200 FEET (PER TABLE 1017.2)  
THE COMMON PATH OF TRAVEL IS LESS THAN 75 FEET. (PER 1006.2.1)  
THERE ARE NO DEAD END CORRIDORS OVER 20 FEET. (PER 1020.4)

BUILDING EXIT WIDTH CALCULATIONS:  
5 PERSON \* 0.2' / OCCUPANCY = 1' REQUIRED, 99' TOTAL PROVIDED (PER 1005.1)  
MIN. NO. OF EXIT REQUIRED: TWO (2) PER TABLES 1006.2.1 AND 1006.3.2(2))  
NUMBER OF EXITS PROVIDED: THREE (3) ACCESSIBLE

EGRESS DOORS DO NOT REQUIRE PANIC HARDWARE. (PER 1010.1.10)  
DOORS DO NOT HAVE DELAYED EGRESS LOCKS (PER 1010.1.9.7)  
DOORS DO NOT HAVE ELECTROMAGNETIC EGRESS LOCKS (PER 1010.1.9.9)  
DOORS DO NOT HAVE HOLD OPEN DEVICES.  
THERE ARE NO EMERGENCY ESCAPE WINOWS (PER 1030)  
NO. OF FIRE EXTINGUISHERS PROVIDED:  
PROVIDE TWO (2) FIRE EXTINGUISHER AT THIS BUILDING  
FIRE EXTINGUISHER FOR CLASS A FIRE HAZARDS REQUIRE NO GREATER THAN 75 FT  
OF MAXIMUM TRAVEL DISTANCE IN LOW, ORDINARY AND EXTRA HAZARD OCCUPANCY.



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DESIGNED / CHECKED BY: <b>KJD</b>	BT	PROJECT #: <b>2025-04-02.2</b>	DATE: <b>10 JULY 2025</b>
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FINAL DRAWING ☐ FOR REVIEW PURPOSES ONLY  
 PRELIMINARY ☐ FOR DESIGN DEVELOPMENT ONLY  
 FINAL DRAWING ☒ FOR CONSTRUCTION

OWNER/TENANT:  
 LUIS TIRADO  
 3577 OLD US 421 LILLINGTON, NC 27146

CONTRACTOR/BUILDER:  
 TBD

REV	DATE	DESCRIPTION
Δ	07/10/25	FINAL FOR CONS

PROJECT: ***TIRADO TRUCK SHOP***  
 US 421 SOUTH, HARNETT COUNTY, NC

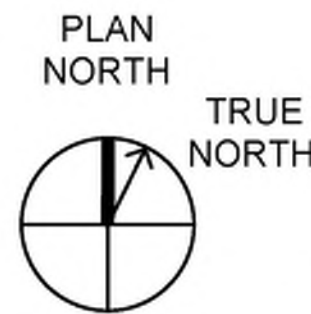
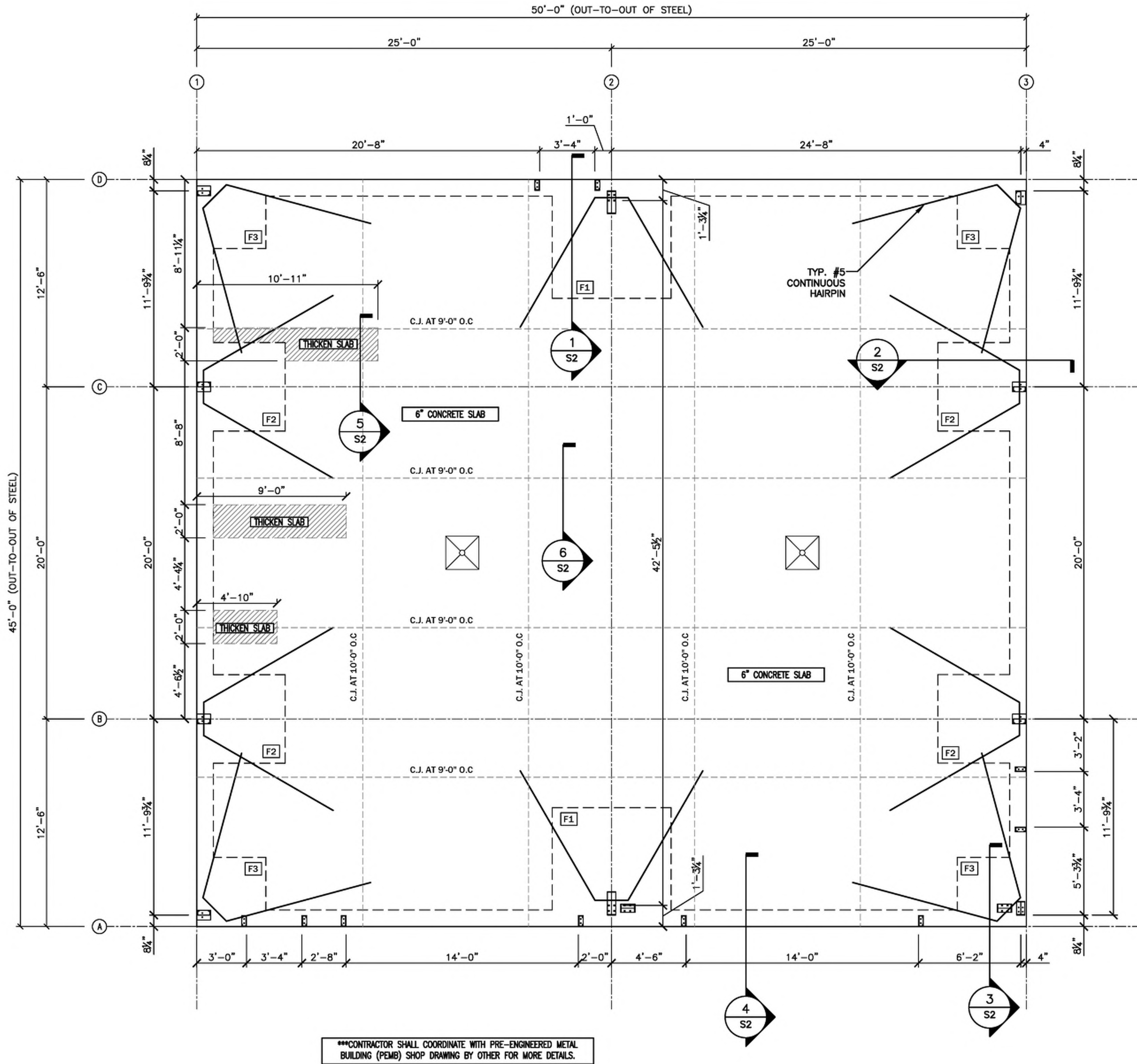
SHEET: **BUILDING LIFE SAFETY -  
EGRESS PLAN**

LS

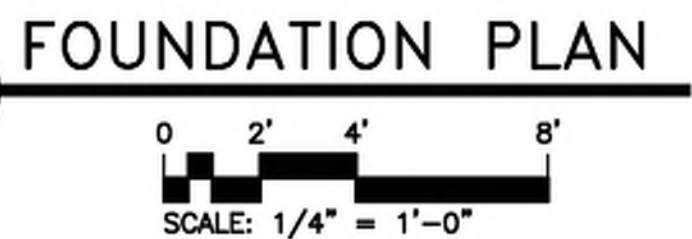




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Printed Date: Jun 10, 2025 - 10:53am



1  
S1



FOOTING SCHEDULE				
TAG	LENGTH	WIDTH	DEPTH	REINFORCING
F1	7'-2"	7'-2"	2'-0"	(8) #6 ALONG LENGTH T & B (8) #6 ALONG WIDTH T & B
F2	5'-4"	5'-4"	2'-0"	(6) #6 ALONG LENGTH T & B (6) #6 ALONG WIDTH T & B
F3	4'-2"	4'-2"	2'-0"	(5) #6 ALONG LENGTH T & B (5) #6 ALONG WIDTH T & B

PROJECT: **TIRADO TRUCK SHOP**  
US 421 SOUTH, HARNETT COUNTY, NC

SHEET: **FOUNDATION PLAN**

S1

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PRELIMINARY ☐ FOR DESIGN DEVELOPMENT ONLY  
FINAL DRAWING ☒ FOR CONSTRUCTION

OWNER/TENANT:  
LUIS TIRADO  
3077 OLD US 421 LUNINGTON, NC 27546  
CONTRACTOR/BUILDER:  
TBD

REVISIONS:		
REV	DATE	DESCRIPTION
1	07/10/25	FINAL FOR CONSTRUCTION

DESIGNED / CHECKED BY: **KJD**  
DRAWN BY: **BT**  
PROJECT #: **2025-04-02.2**  
DATE: **10 JULY 2025**





1'-4"  
8"  
4"  
2'-0" 0  
SCALE: 1'-1/2" = 1'-0"  
2'-8" 0  
SCALE: 1" = 1'-0"  
4"  
SCALE: 3/4" = 1'-0"  
2'-0" 0  
SCALE: 1/2" = 1'-0"  
4"  
SCALE: 1/4" = 1'-0"  
10'-0" 0  
SCALE: 1/8" = 1'-0"  
16'-0" 0  
SCALE: 1/8" = 1'-0"

PRE-ENGINEERED BUILDING:

1. MATERIALS

	ASTM DESIGNATION	
STRUCTURAL STEEL PLATE	A529 OR A572 OR A570 OR A607	GRADE 50
COLD FORMED LIGHT GA. SHAPES	A570 OR A607	GRADE 55
BRACE RODS	A572	GRADE 65, U.O.N.
HOT ROLLED MILL SHAPES	A36 OR A572	Fy=36KSI OR GRADE 50
ROOF AND WALL SHEETS	A653 OR A792	GRADE 50
BOLTS	A307 AND A325	A307, U.O.N.

2. A325 BOLT TIGHTENING REQUIREMENTS:

ALL HIGH STRENGTH BOLTS ARE A325-N UNLESS SPECIFICALLY NOTED OTHERWISE. STRUCTURAL BOLTS SHALL BE TIGHTENED TO THE TURN-OF-THE-NUT METHOD IN ACCORDANCE WITH THE NINTH EDITION, AISC USPECIFICATION FOR STRUCTURAL UNITS USING ASTM A325 OR A490 BOLTS\*. PER SECTION &D.I, A325 BOLTS MAY BE INSTALLED HILTHOUT HASHERS INHEN TIGHTENED BY THE TURN-OF-THE-NUT METHOD. ALL HIGH STRENGTH BOLTS, EXCEPT AS NOTED OTHERWISE, ARE SUBJECT TO DIRECT TENSION AND MAY REQUIRE INSPECTION AS DEFINED BY THE APPLICABLE BUILDING CODE OR STANDARD. IT IS THE RESPONSIBILITY OF THE ERECTOR TO ASSURE PROPER TIGHTNESS.

3. THE BUILDING REACTION DATA REPORTS THE LOADS WHICH THIS BUILDING PLACES ON THE FOUNDATION.

4. ANCHOR BOLTS SHALL BE ACCURATELY SET TO A TOLERANCE OF  $\pm 1/8"$  IN BOTH ELEVATION AND LOCATION.

5. COLUMN BASE PLATES ARE DESIGNED NOT TO EXCEED A BEARING PRESSURE OF 1125 POUNDS PER SQUARE INCH.

6. ALL SHOP DRAWINGS SHALL BE REVIEWED BY THE DESIGN ENGINEER AT JENKINS CONSULTING ENGINEERS. THIS REVIEW IS FOR DESIGN CONTENT ONLY AND DOES NOT REMOVE RESPONSIBILITY FOR THE FABRICATOR OR THE ERECTOR TO PROVIDE MATERIAL IN ACCORDANCE WITH THESE PLANS.

STRUCTURAL DESIGN

DESIGN LOADS:

Importance Factors:	Snow (ls) <u>1.00</u>
	Seismic (ls) <u>1.00</u>
Live Loads:	Roof <u>20</u> psf
	Mezzanine <u>100</u> psf
	Floor <u>N/A</u> psf
Ground Snow Load:	<u>15</u> psf
Wind Load:	Ultimate Wind Speed <u>118</u> mph (ASCE-7)
	Exposure Category <u>C</u>

SEISMIC DESIGN CATEGORY:

<input type="checkbox"/> A	<input checked="" type="checkbox"/> B	<input type="checkbox"/> C	<input type="checkbox"/> D
Provide the following Seismic Design Parameters:			
Risk Category (Table 1604.5)	<input type="checkbox"/> I	<input checked="" type="checkbox"/> II	<input type="checkbox"/> III
Spectral Response Acceleration	SS <u>18.5</u> %g	S1 <u>8.7</u> %g	
Site Classification (ASCE 7)	<input type="checkbox"/> A	<input type="checkbox"/> B	<input checked="" type="checkbox"/> C
Data Source:	<input type="checkbox"/> Field Test	<input checked="" type="checkbox"/> Presumptive	<input type="checkbox"/> Historical Data
Basic structural system	<input type="checkbox"/> Bearing Wall	<input type="checkbox"/> Dual w/Special Moment Frame	<input type="checkbox"/> Dual w/Intermediate R/C or Special Steel
	<input checked="" type="checkbox"/> Building Frame	<input type="checkbox"/> Inverted Pendulum	
Analysis Procedure:	<input checked="" type="checkbox"/> Simplified	<input type="checkbox"/> Equivalent Lateral Force	<input type="checkbox"/> Dynamic
Architectural, Mechanical, Components anchored?	<input type="checkbox"/> Yes	<input checked="" type="checkbox"/> No	

LATERAL DESIGN CONTROL:

Earthquake	<input type="checkbox"/>	Wind	<input checked="" type="checkbox"/>
SOIL BEARING CAPACITIES:			
Field Test (provide copy of test report)	<u>N/A</u>	psf	
Presumptive Bearing capacity	<u>2,000</u>	psf	
Pile size, type, and capacity	<u>N/A</u>		

REINFORCING STEEL

ALL REINFORCING STEEL SHALL BE DEFORMED STEEL BARS CONFORMING TO A.S.T.M. A615, GRADE 60. ALL REINFORCING STEEL SHALL BE MANUFACTURED, DETAILED, FABRICATED AND PLACED IN ACCORDANCE WITH A.C.I. 315R, 318R AND A.C.I. SP 66. WELDED WIRE FABRIC SHALL CONFORM TO A.S.T.M. A185, IN AS LONG A LENGTH AS IS PRACTICAL. WELDED WIRE FABRIC SHALL BE LAPPED AT LEAST ONE GRID WIDTH PLUS 2". REINFORCEMENT SHALL BE BENT COLD AND SHALL NOT BE WELDED.

SPICES: REINFORCEMENT IN CONCRETE AND MASONRY SHALL HAVE LAP LENGTHS AS FOLLOWS, UNLESS OTHERWISE SPECIFIED ON DRAWINGS:

BAR SIZE:	IN CONCRETE:	IN MASONRY:
#3	1'-6"	2'-0"
#4	2'-0"	2'-6"
#5	2'-6"	3'-0"

PLACEMENT:

REINFORCEMENT SHALL BE ACCURATELY PLACED AND SUPPORTED BY CONCRETE, METAL, OR OTHER APPROVED CHAIRS, SPACERS OR TIES, AND SECURED AGAINST DISPLACEMENT DURING CONCRETE OR GROUT PLACEMENT.

EXCEPT WHERE OTHERWISE NOTED, REINFORCEMENT SHALL HAVE CONCRETE COVER AS FOLLOWS:

CONCRETE DEPOSITED AGAINST EARTH	3"
FORMED CONCRETE AGAINST EARTH	2"
EXTERIOR FACES OF WALLS	1"
TO TOP OF SLABS-ON-GRADE	3/4"

ALL SCALES, LOOSE RUST, GREASE OR DIRT SHALL BE REMOVED FROM THE REINFORCING BEFORE IT IS PLACED. PROVIDE #5 "HAIRPIN" X 20' LONG AT EXTERIOR COLUMN LINES. ANCHOR BOLTS SHALL BE (A-3077) HIGH STRENGTH.

SOIL TREATMENT: ADMINISTRATION AS ACCEPTABLE

GENERAL CONDITIONS

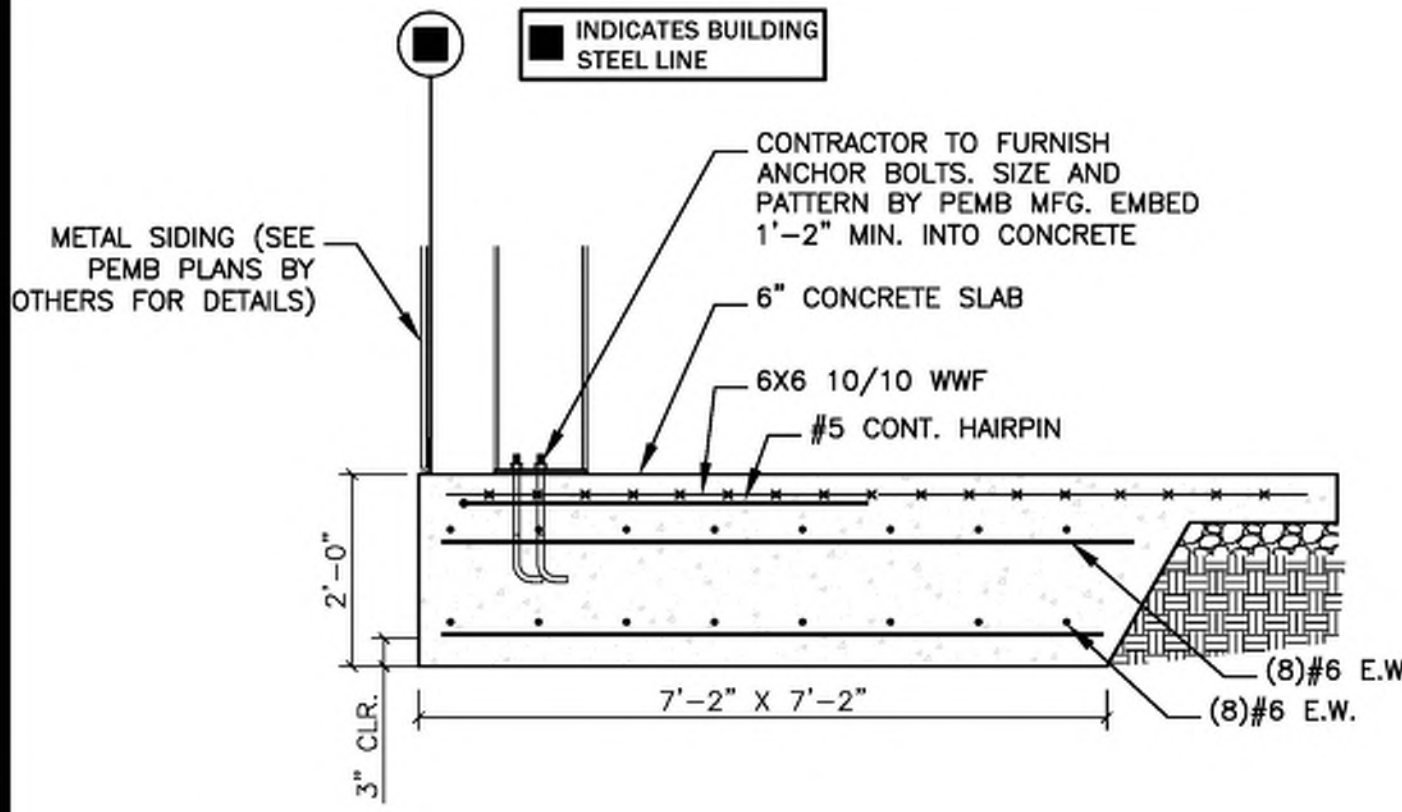
THE GENERAL CONTRACTOR SHALL MAKE ADEQUATE SANITARY PROVISIONS. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR JOB SAFETY AND COMPLIANCE WITH THE REQUIREMENTS OF THE OCCUPATIONAL SAFETY AND HEALTH ACT AS IT MAY REGARD ANY PHASE OF THE WORK ON THIS PROJECT.

SOIL COMPACTION AND TESTING

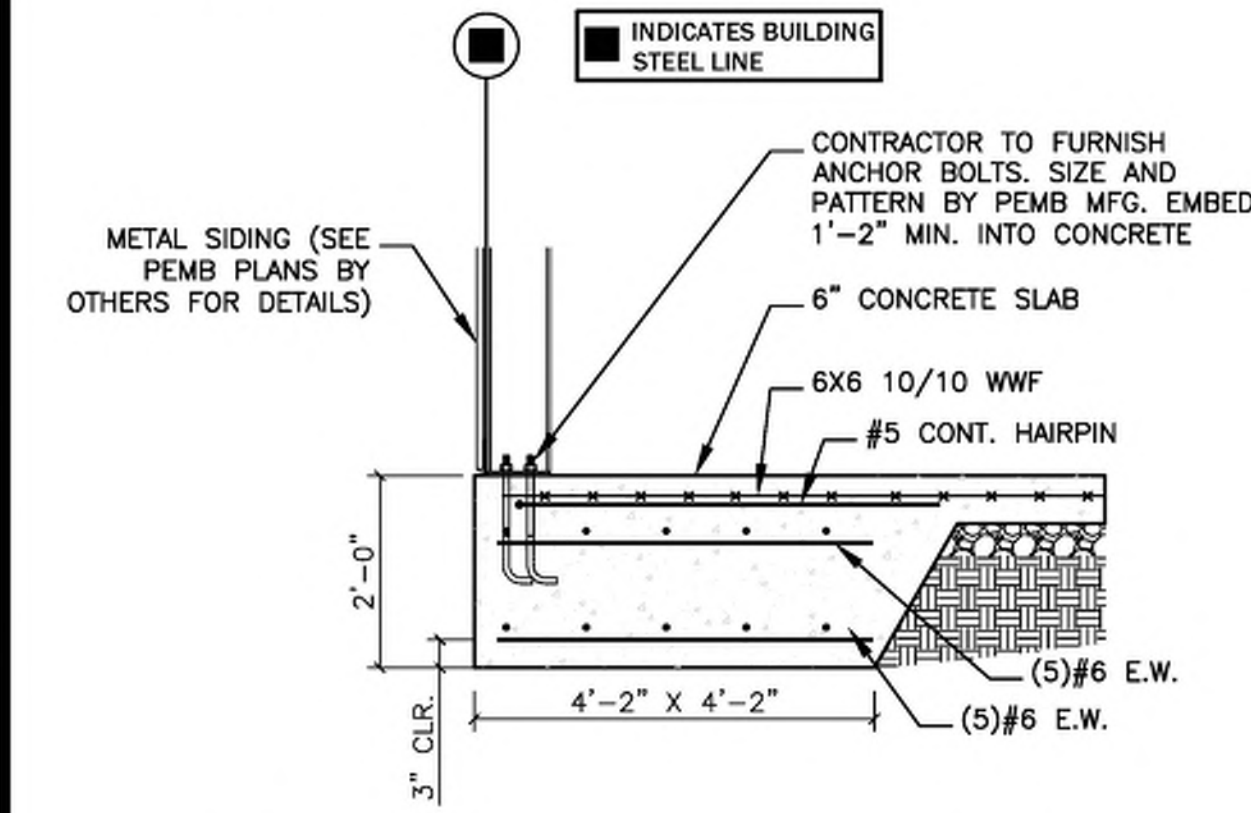
THE GENERAL CONTRACTOR SHALL OBTAIN THE SERVICES OF A TESTING LABORATORY, SUCH AS S&ME OR LAW ENGINEERING FOR THE PURPOSE OF DETERMINING THE SUITABILITY OF THE SUBSURFACE CONDITIONS AND THE BEARING CAPACITIES OF ALL AREAS BELOW CONCRETE (2000psf ASSUMED).. THE SOIL AND BEARING REPORT SHALL BE SUBMITTED PRIOR TO EXCAVATING, WHERE POSSIBLE, BUT PRIOR TO PLACEMENT OF ANY REINFORCING AND CONCRETE.

CONCRETE WORK

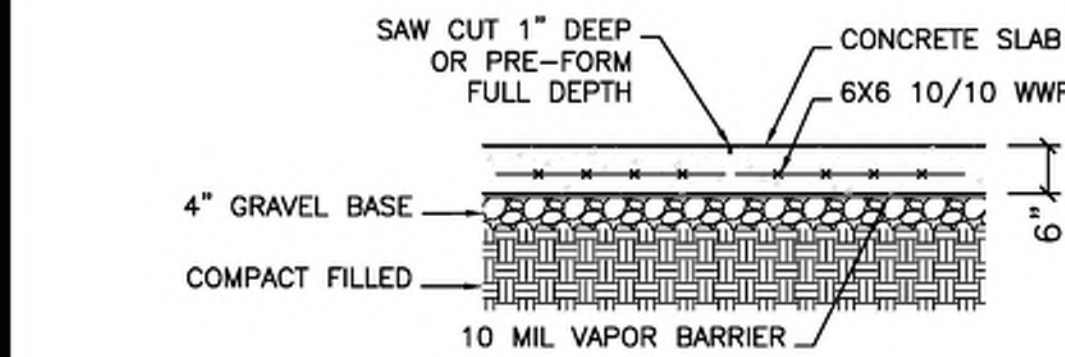
- ALL CONCRETE FOR THE PROJECT SHALL BE "READY MIX" AND SHALL COMPLY WITH ASTM C-94. ALL SECTIONS OF THE CONCRETE WORK SHALL COMPLY WITH ALL A.S.T.M. AND A.C.I. REQUIREMENTS.
- FORM WORK - ALL FORMS TO BE CAREFULLY BUILT AND SECURED IN PLACE IN SUCH A MANNER AS TO HAVE SUFFICIENT STRENGTH TO CARRY THE DEAD WEIGHT OF THE CONSTRUCTION AS A LIQUID, WITHOUT DEFLECTION OR VIBRATION. FORMS TO BE BUILT TIGHT, TRUE TO POSITION AND DIRECTION, THOROUGHLY BRACED, WIRED AND SPIKED OR OTHERWISE FASTENED TOGETHER.
- CONCRETE - MINIMUM OF 3,000 PSI COMPRESSIVE STRENGTH AT 28 DAYS. MINIMUM OF FIVE SACKS OF CEMENT PER CUBIC YARD OF CONCRETE, MAXIMUM OF 4" SLUMP. 3 POUNDS OF FORTA-FERRO FIBER PER CUBIC YARD MAY BE SUBSTITUTED FOR 6X6 10 WWF.
- FINISHING - IN ACCORDANCE WITH THE LATEST A.C.I. CODE, PLUMB, LEVEL, TRUE IN LINE, FREE OF HONEYCOMB. BUILDING SLAB SHALL HAVE A HARD STEEL TROWEL FINISH. WALKS SHALL HAVE BROOMED FINISH AND EXPANSION JOINTS AT APPROXIMATELY 50'-0" O.C. AND DUMMY JOINTS AS SHOWN ON THE SITE PLAN. REMOVAL OF FORMS - FORMS SHALL BE CAREFULLY REMOVED SO AS NOT TO IMPAIR THE FACE OF THE CONCRETE. IMMEDIATELY AFTER THE FORMS ARE REMOVED ALL DAMAGE OF IMPERFECT WORK SHALL BE PATCHED IN A NEAT AND WORKMANLIKE MANNER OR IF BADLY DAMAGED, IN THE OPINION OF THE OWNER, THE WORK SHALL BE REBUILT. THE MINIMUM TIME BEFORE ANY FORMS CAN BE REMOVED IS SEVEN (7) DAYS FOR SUCH MEMBERS AS ARE SUBJECT TO BENDING STRESSES, SUCH AS SLABS.
- CURING - USE MEMBRANE CURING METHOD. USE MFG. RATE, SPRAY IMMEDIATELY FOLLOWING FINISHING. PROTECT FROM FREEZING WEATHER, CURE A TOTAL OF 28 DAYS USING A.C.I. METHODS.



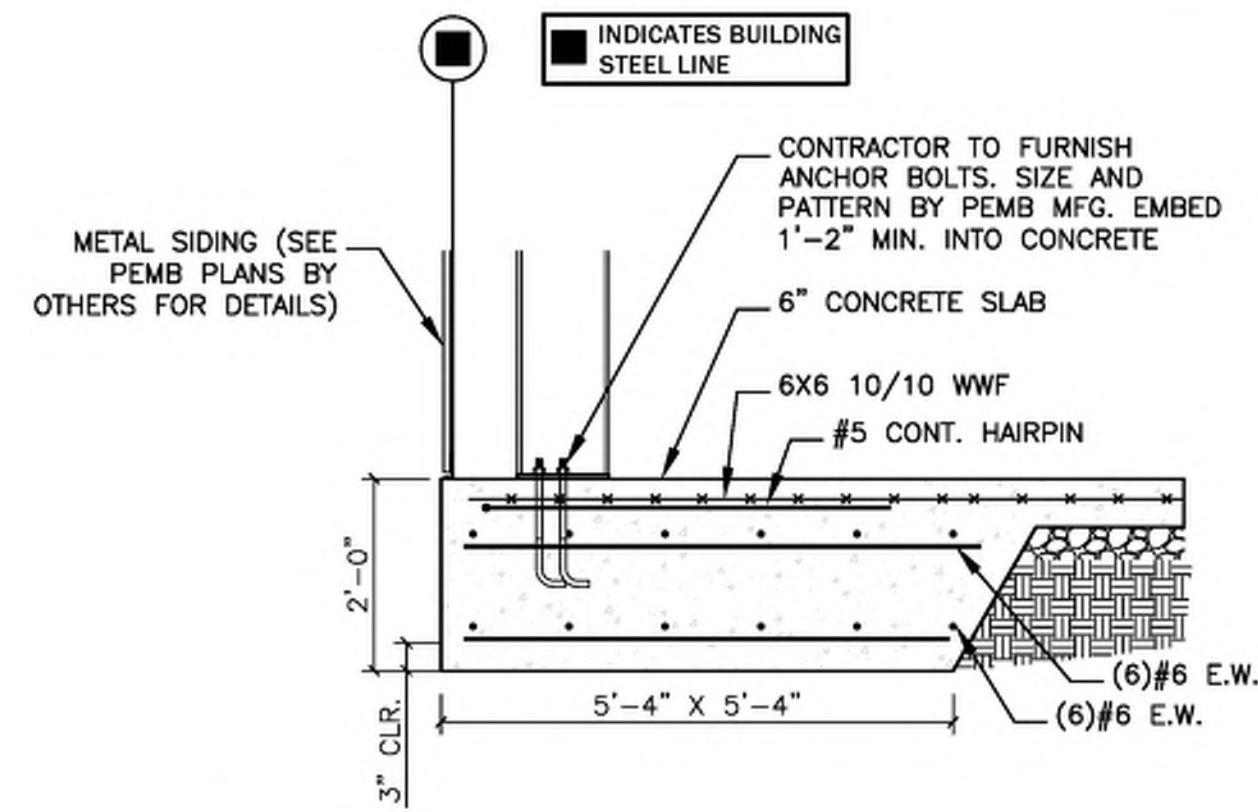
1 FOOTING DETAIL (F1)  
S2 N.T.S.



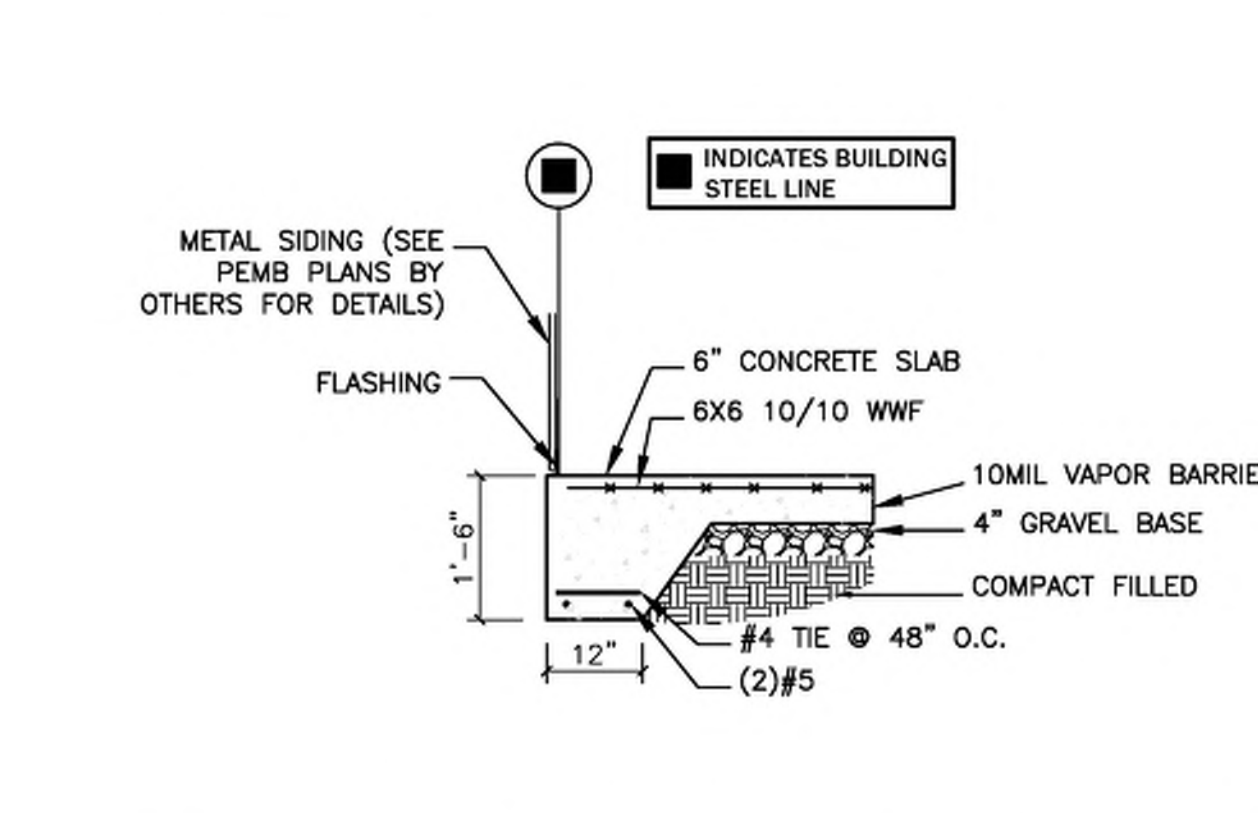
3 FOOTING DETAIL (F3)  
S2 N.T.S.



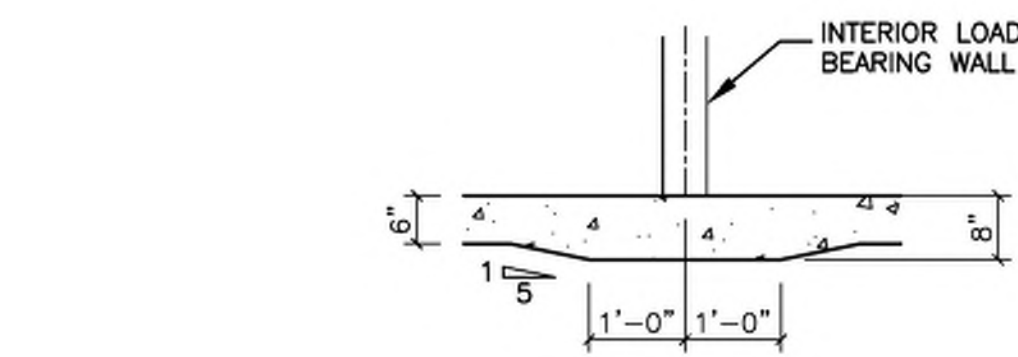
5 CONTROL JOINT DETAIL  
S2 N.T.S.



2 FOOTING DETAIL (F2)  
S2 N.T.S.



4 FOOTING DETAIL (PERIMETER)  
S2 N.T.S.



6 THICKENED SLAB DETAIL  
S2 N.T.S.



DESIGNED / CHECKED BY:	KJD
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FINAL DRAWING	<input checked="" type="checkbox"/>	FOR CONSTRUCTION
OWNER/TENANT:	LUIS TIRADO	
OWNER/CONTRACTOR:	TIRADO TRUCK SHOP	

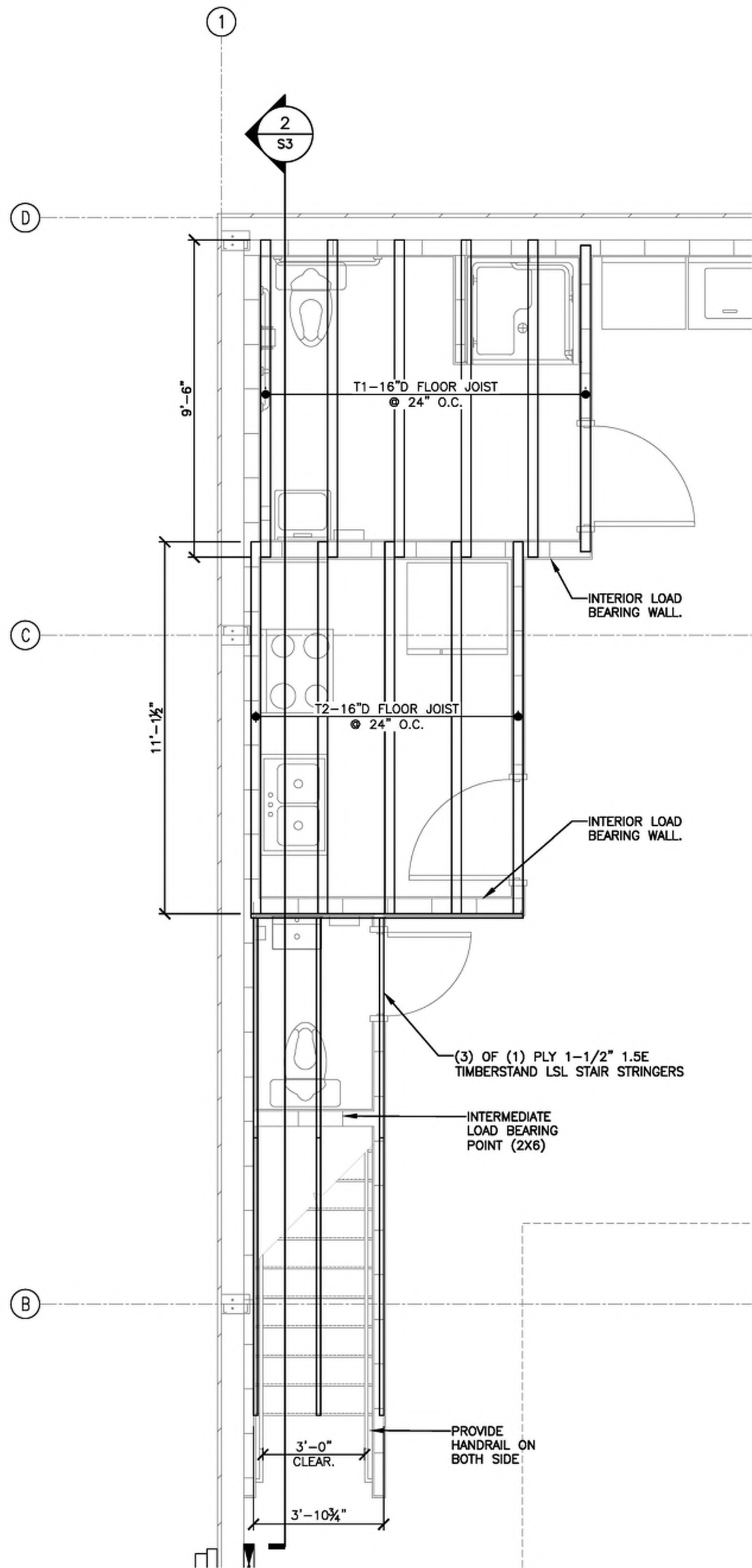
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1	07/10/25	FINAL FOR CON.

PROJECT: TIRADO TRUCK SHOP  
US 421 SOUTH, HARNETT COUNTY, NC  
SHEET: NOTES & FOOTING DETAILS



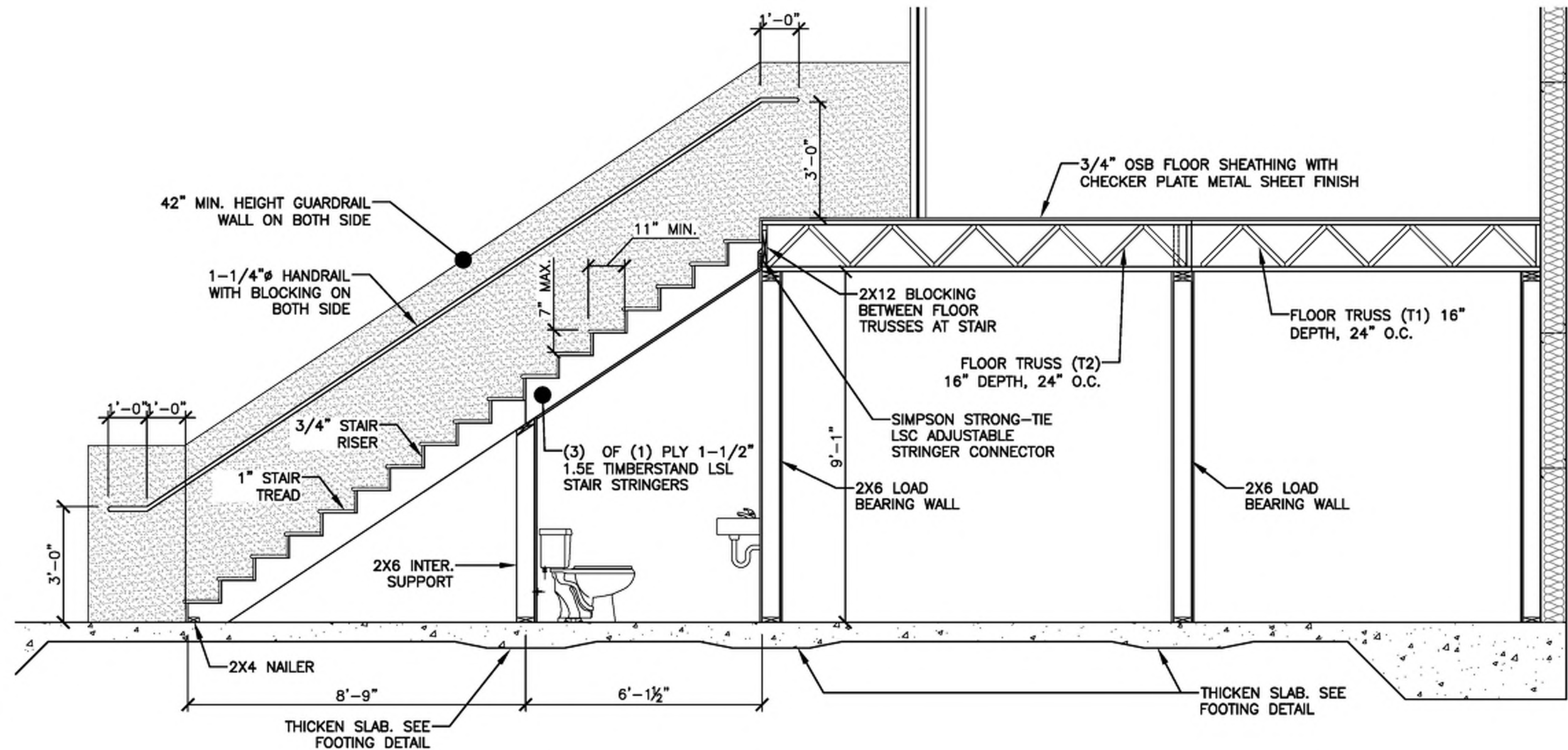


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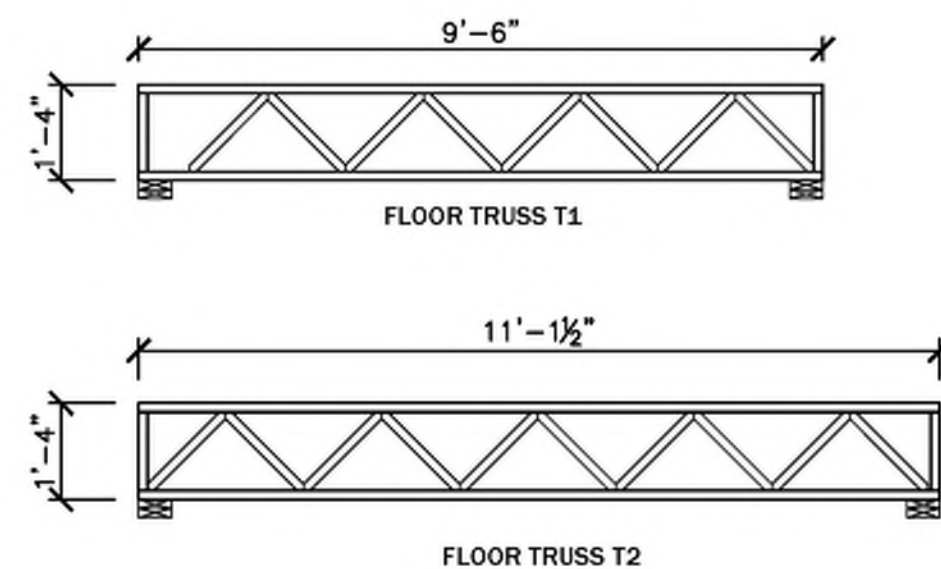


1 FLOOR FRAMING PLAN  
S3  
SCALE: 3/8" = 1'-0"

TRUSS DESIGN LOADS		
DEAD LOADS		
BOTTOM CHORD	10 psf	
TOP CHORD	10 psf	
TOTAL	20 psf	
LIVE LOADS		
BOTTOM CHORD	0 psf	
TOP CHORD	125 psf	
TOTAL	125 psf	

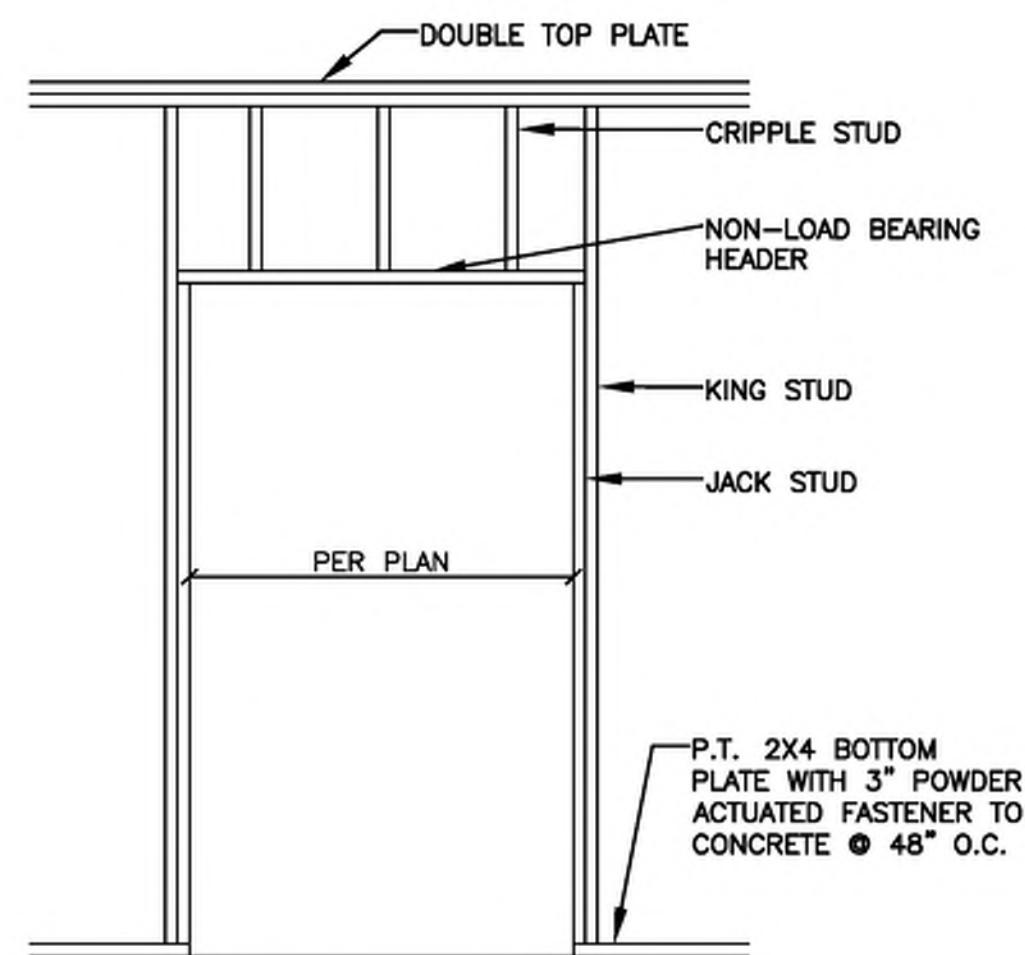


2 FRAMING SECTION  
S3  
SCALE: 3/8" = 1'-0"



\*\*\*NOTE: THESE TRUSS PROFILES ARE ONLY DIAGRAMMATIC, ONLY FOR BASIS OF DESIGN. CONTRACTOR SHALL COORDINATE WITH TRUSS MANUFACTURER SHOP DRAWING FOR SPECIFIC TRUSS PROFILE AND MANUFACTURER'S REQUIREMENT.

3 TRUSS PROFILES  
S3



4 HEADER FRAMING  
NON-LOAD BEARING  
S3  
SCALE: 1/2" = 1'-0"



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10 JULY 2025

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LUIS TIRADO  
3027 OLD US 421 LUNNISTON, NC 27446  
CONTRACTOR/BUILDER:  
TBD

REVISIONS:  
REV DATE DESCRIPTION  
1 07/10/25 FINAL FOR CONSTRUCTION

PROJECT: **TIRADO TRUCK SHOP**  
US 421 SOUTH, HARNETT COUNTY, NC  
SHEET:  
FRAMING PLAN AND DETAILS

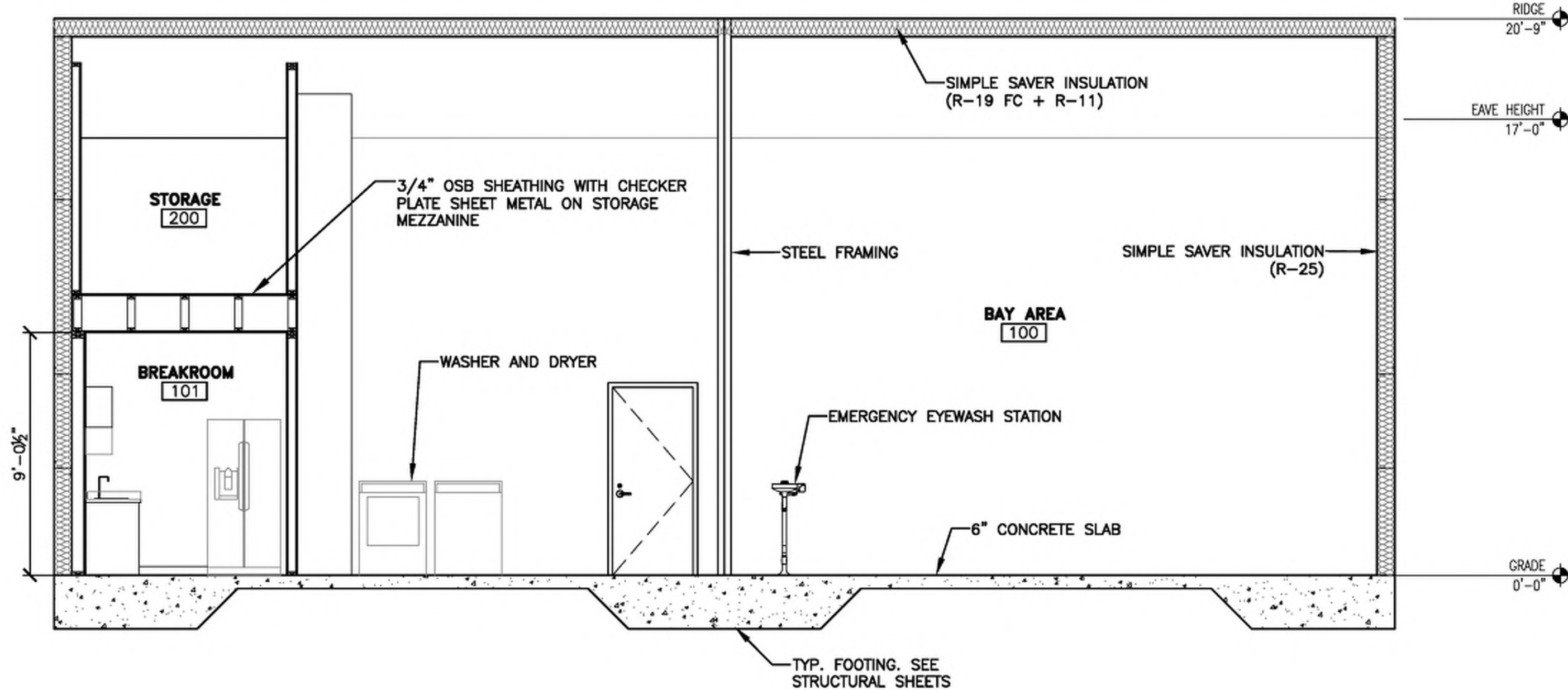
S3



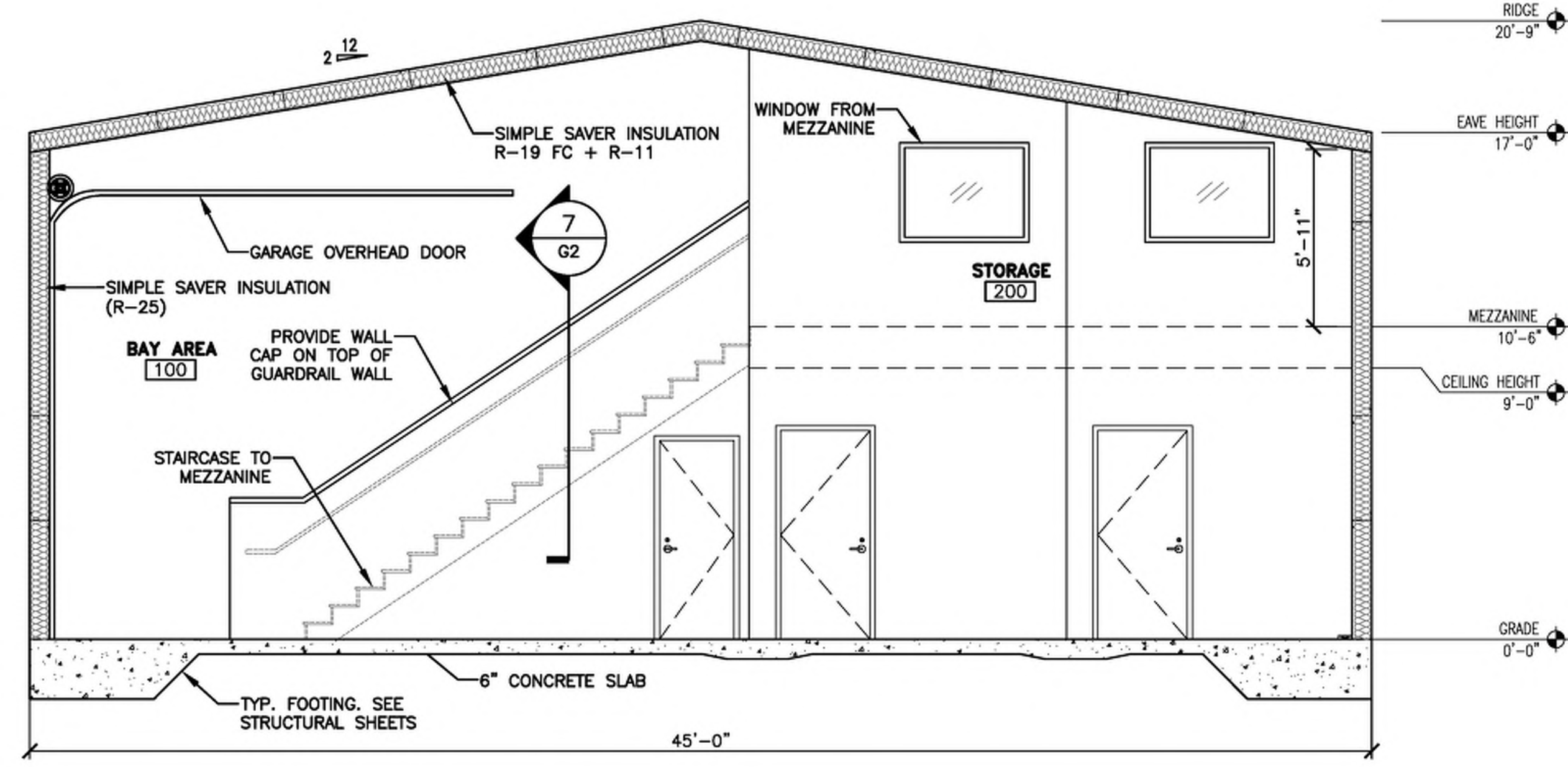




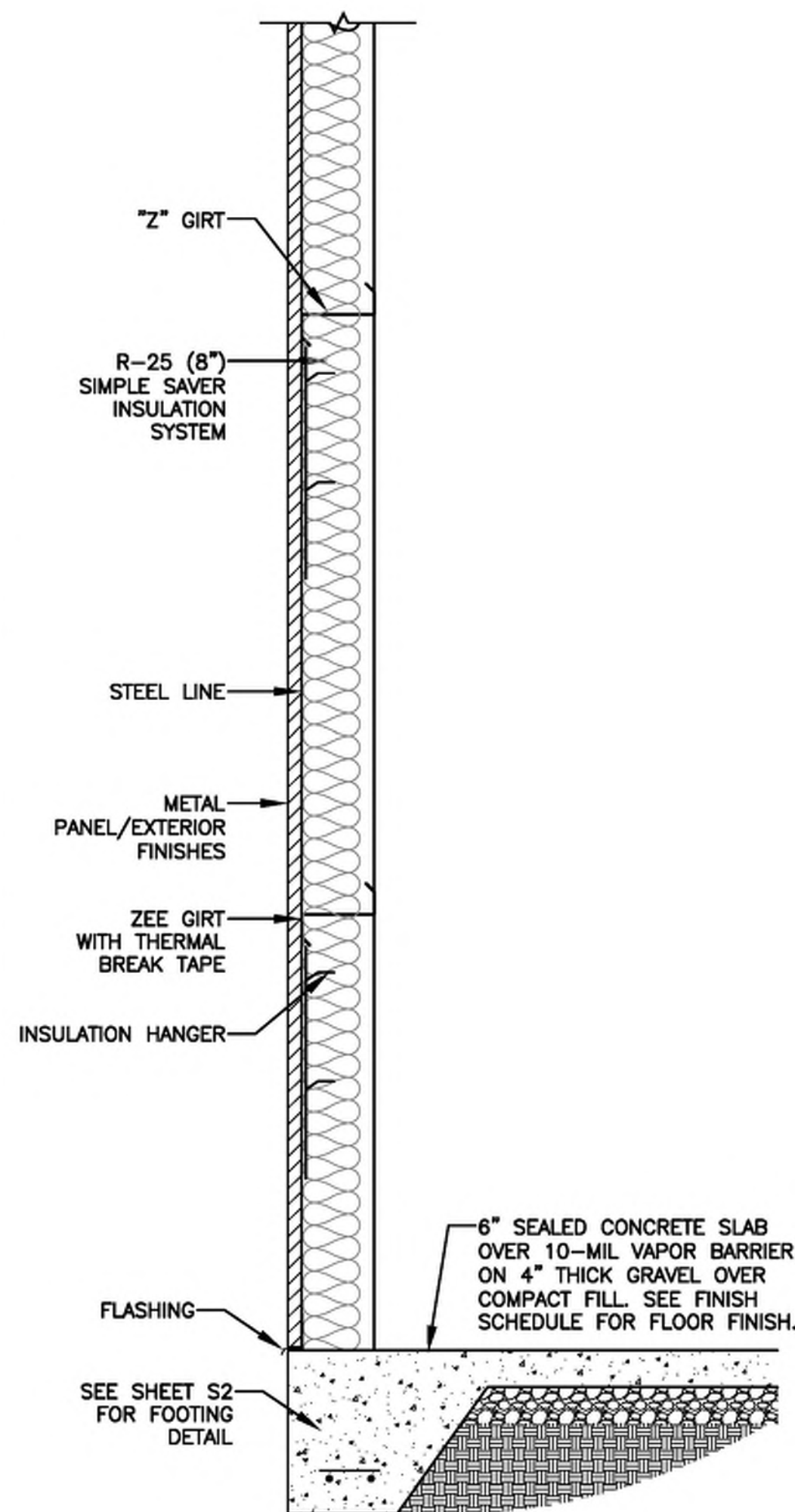
1'-4"  
8"  
2'-0" 0 4" 1'-0"  
SCALE: 1'-1/2" = 1'-0"  
2'-8" 0 6" 1'-0"  
SCALE: 3/4" = 1'-0"  
4" 0 8" 1'-4"  
SCALE: 1/2" = 1'-0"  
2' 0 2' 1'-0"  
SCALE: 1/4" = 1'-0"  
10'-8" 0 2' 4" 1'-0"  
SCALE: 3/16" = 1'-0"  
16' 0 2'-8" 5'-4"  
SCALE: 1/8" = 1'-0"  
0 4" 8" 1'-0"  
SCALE: 1/8" = 1'-0"



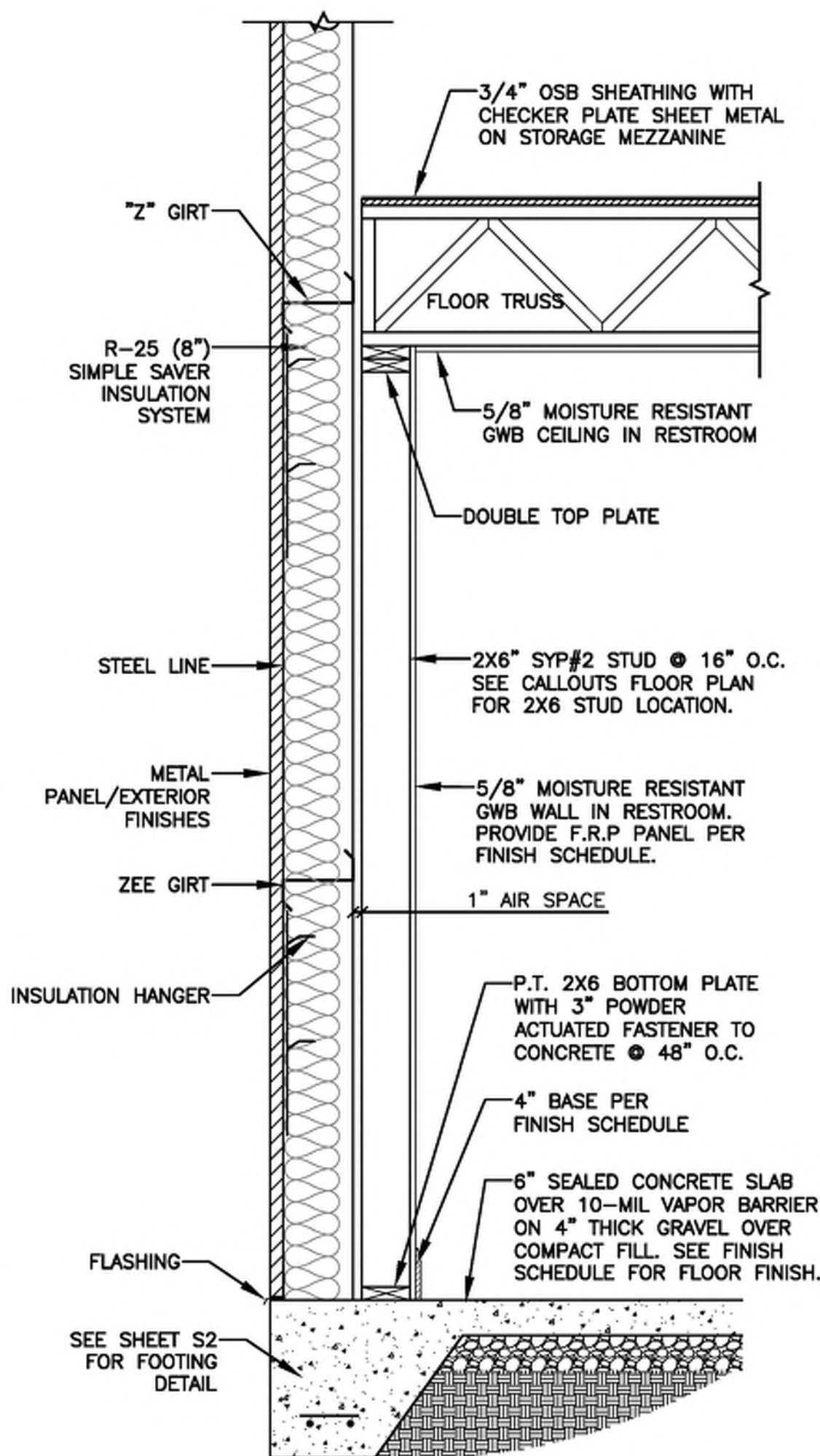
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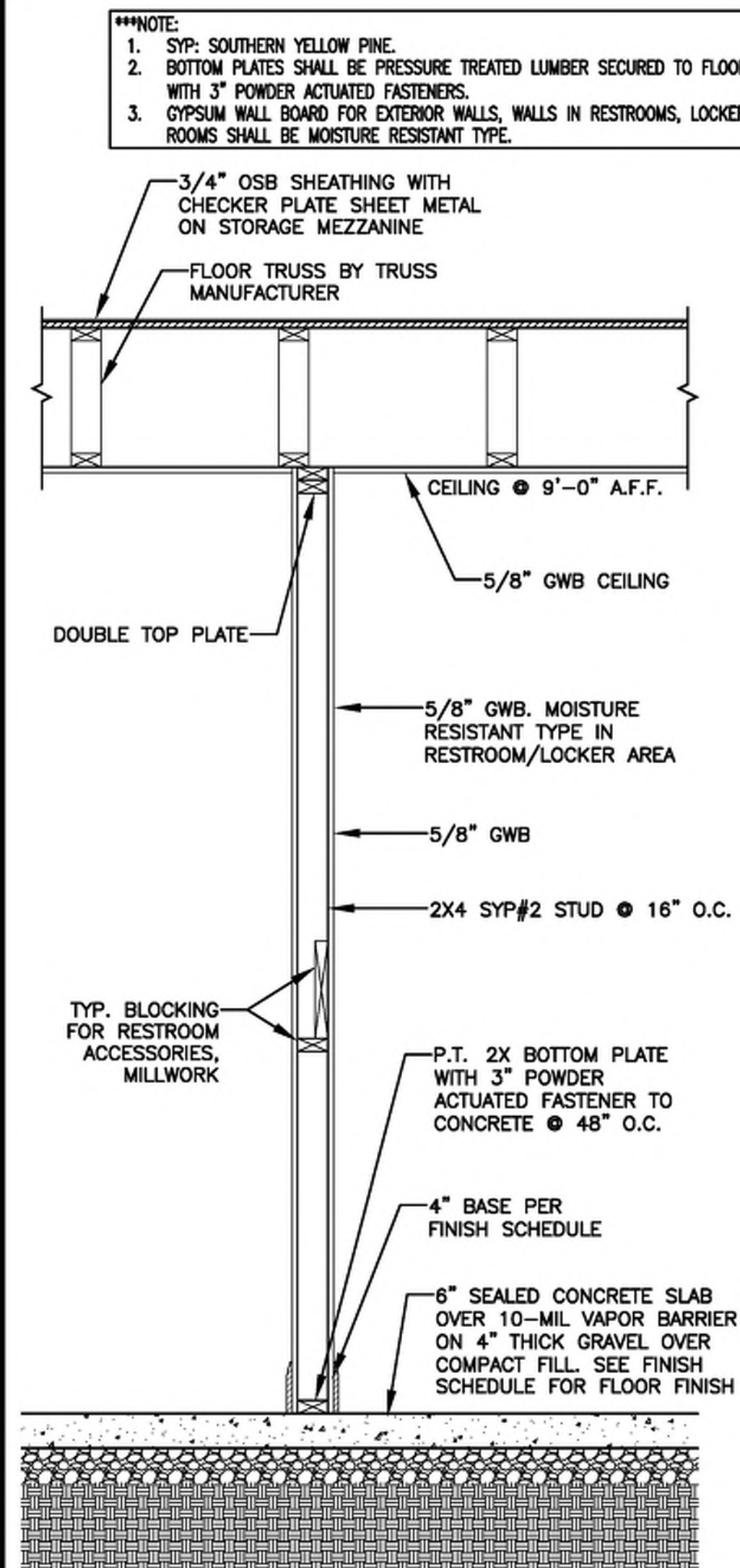
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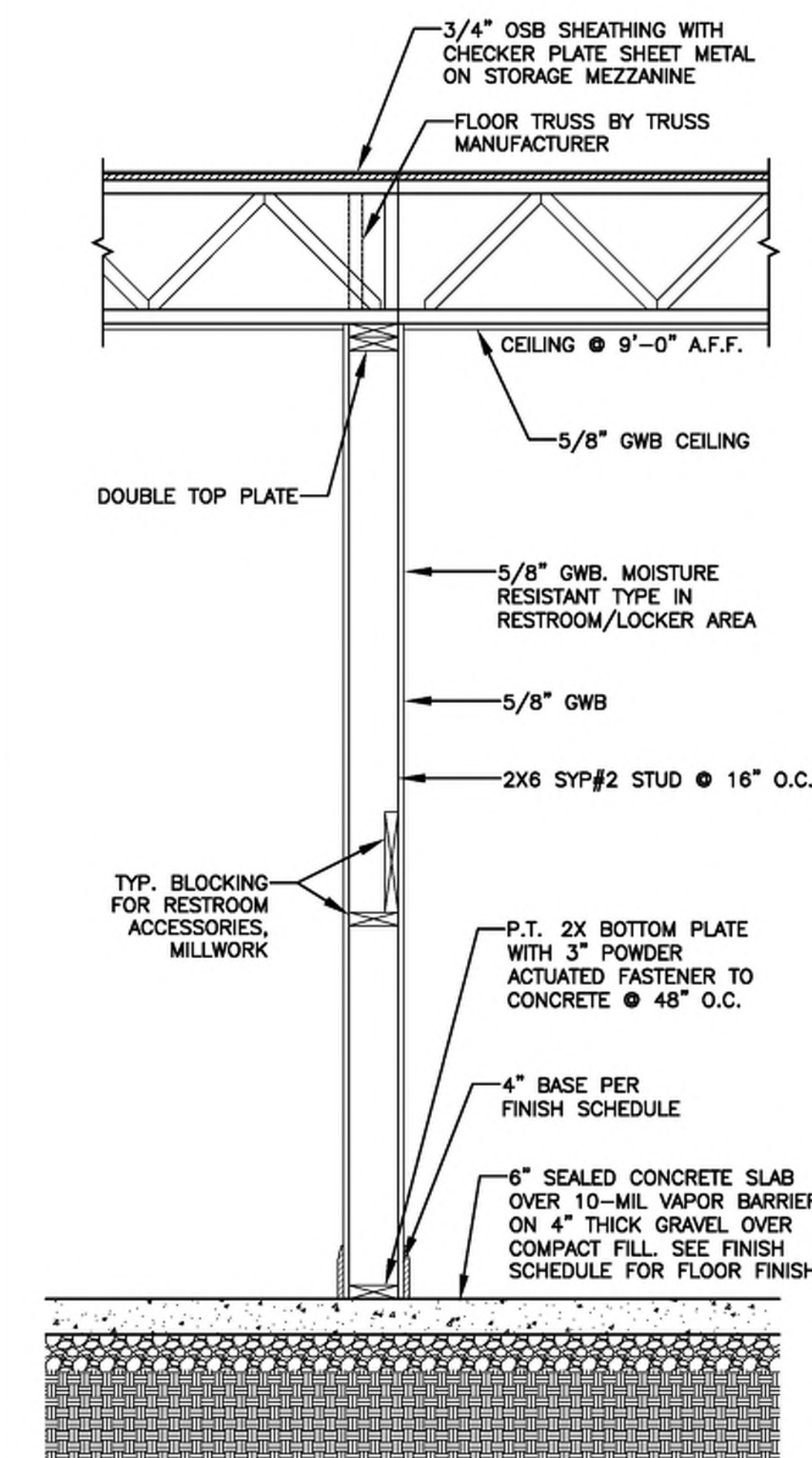
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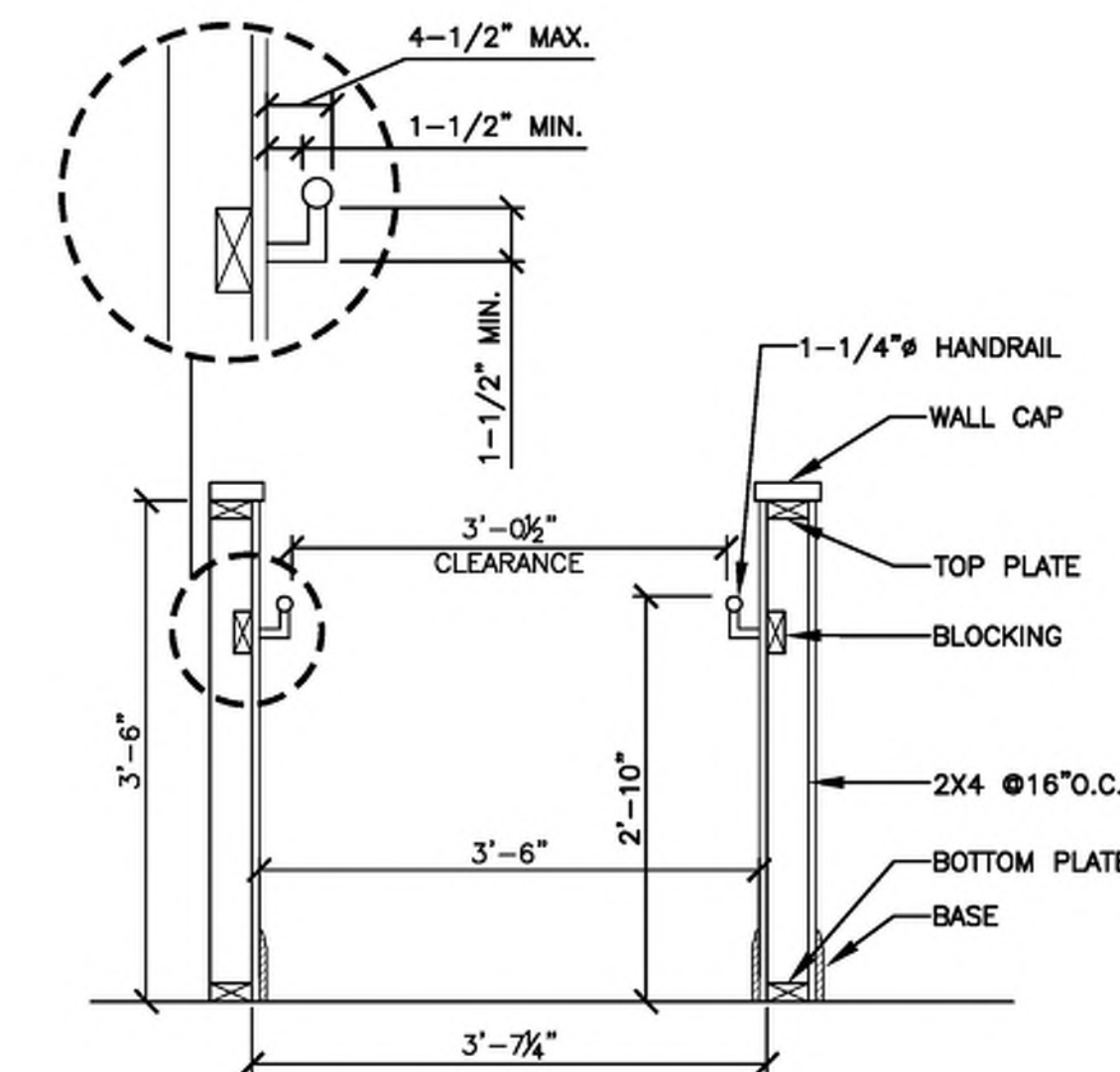
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G2 TYPE 2  
SCALE: 3/4" = 1'-0"



5 WALL SECTION  
G2 TYPE 3  
SCALE: 3/4" = 1'-0"



6 WALL SECTION  
G2 TYPE 4  
SCALE: 3/4" = 1'-0"



7 STAIR GUARDRAIL WALL  
G2  
SCALE: 3/4" = 1'-0"



**J. JENKINS**  
CONSULTING ENGINEERS, PA  
OFFICE IN EUREKA SPRINGS, NORTH CAROLINA  
1608 MARTIN RD. FAYETTEVILLE, NC 28411-1002  
910.822.1724

10 JULY 2025

DESIGNED / CHECKED BY: KJD  
DRAWN BY: BT  
PROJECT #: 2025-04-02.2  
DATE: 10 JULY 2025

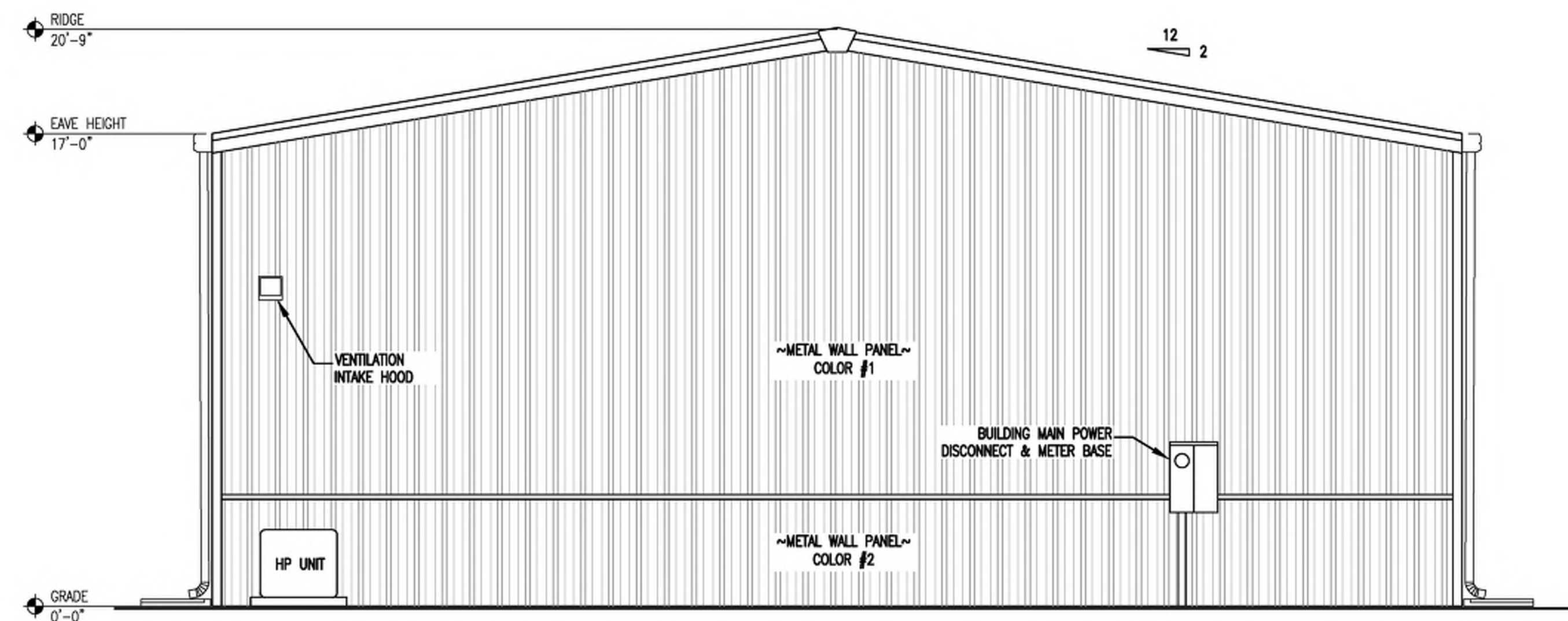
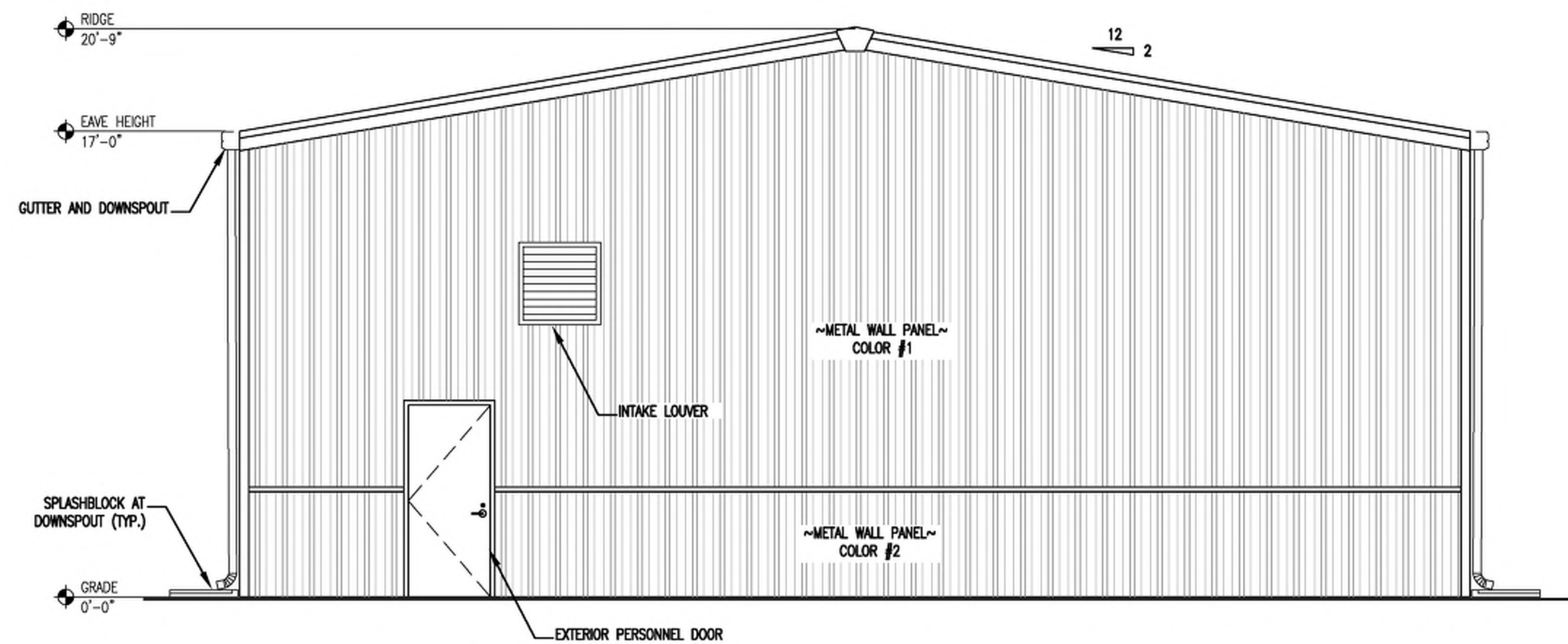
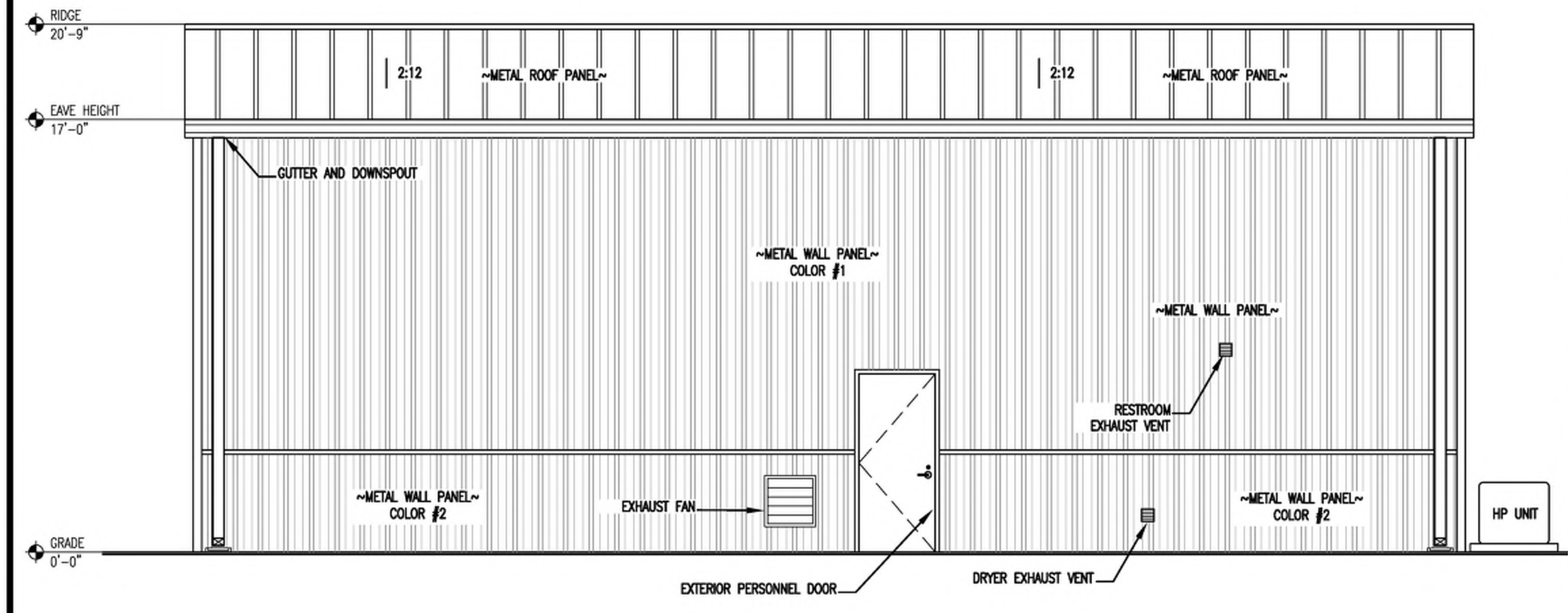
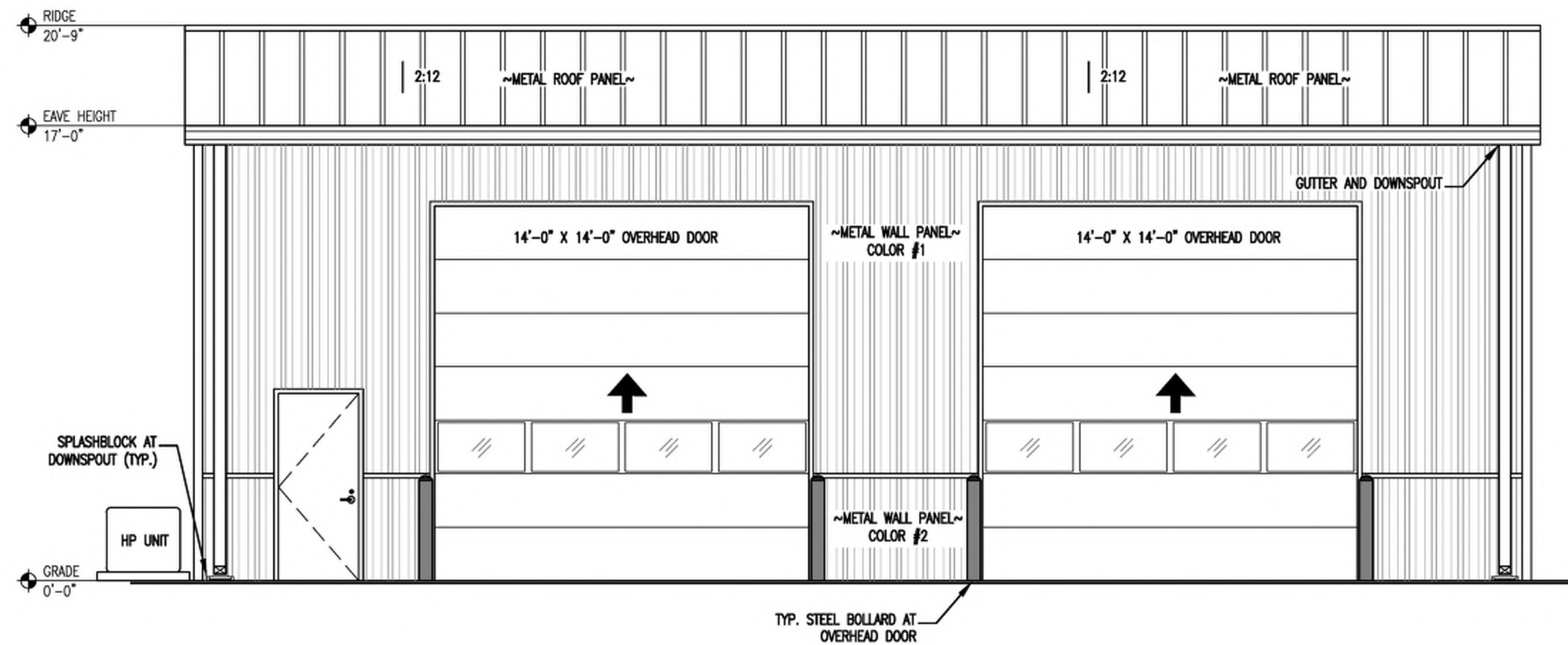
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PRELIMINARY [ ] FOR DESIGN DEVELOPMENT ONLY  
FINAL DRAWING [ ] FOR CONSTRUCTION  
OWNER/TENANT:  
LUIS TIRADO  
3073 OLD US 41 LUNenburg, NC 27446  
CONTRACTOR/BUILDER:  
TBD

REVISIONS:  
REV DATE DESCRIPTION  
1 07/10/25 FINAL FOR CONSTRUCTION

**TIRADO TRUCK SHOP**  
US 421 SOUTH, HARNETT COUNTY, NC  
BUILDING SECTIONS

G2







ROOM FINISH SCHEDULE									
ROOM NAME	ROOM NUMBER	NET SQ.FT.	FLOOR	BASE	WALLS	WALLS HEIGHT	CEILING MATERIAL	CEILING HEIGHT	REMARKS ****NOTE: ALL FINISHES SHALL MATCH WITH EXISTING BUILDING
BAY AREA	100	1862	SEALED CONCRETE	N/A	N/A	N/A	EXPOSED	VARIES	
BREAKROOM	101	76	VCT	WOOD	GYP.BD/PT	9'-0"	GYP.BD/PT	9'-0"	
BATHROOM	102	81	VCT	WOOD	GYP.BD/PT F.R.P. PANEL	9'-0"	GYP.BD/PT	9'-0"	4'-0" F.R.P. PANEL ON WALL
RESTROOM	103	23	VCT	WOOD	GYP.BD/PT F.R.P. PANEL	9'-0"	GYP.BD/PT	9'-0"	4'-0" F.R.P. PANEL ON WALL
STORAGE	200	166	ALUMINUM CHECKER PLATE	WOOD	GYP.BD/PT	VARIES	EXPOSED	VARIES	
FINISH SCHEDULE CODES									
LVT	LUXURY VINYL TILE	PT	PAINTED	VW	VINYL WALLCOVERING	MT	MOSAIC TILE	V.P.	VINYL PLANK
CONC.	CONCRETE	CMU	CONCRETE MASONRY UNIT	SC	SEALED CONCRETE	PNL	PANELING	PAVERS	BRICK PAVERS
COMP.	COMPOSITION	ACOUSTIC	ACOUSTICAL	RU	RUBBER	WD	WOOD	CPT	CARPET
GYP. BD.	GYP.SUM BOARD	SV	SHEET VINYL	CER TILE	CERAMIC TILE	ST	STEEL	1-HOUR	1-305
AA	ANODIZED ALUMINUM	C.G	CORNER GUARD (ACROVYN)	WC	WAINSCOT	FRP	FIRE RESISTANT PANEL		

# DOOR SCHEDULE

DOOR NO	DOOR SIZE	DOOR	FRAME	HARDWARE														REMARKS					
	WIDTH	HEIGHT	THICKNESS	STYLE	MATERIAL	FINISH	TYPE	MATERIAL	FINISH	FIRE RATING	ENTRANCE LOCK	STOREROOM LOCK	PASSAGE SET	PRIVACY SET	PUSH/PULL	PMAC SET	CLOSER	STOP	THRESHOLD	KICK PLATES	WEATHERSTRIPPING	NOTE:	
D100	3'-0"	X 7'-0"	X 1-3/4"	A	HM	P	1	HM	P									X	X	X		X	G.C. TO REVIEW ALL HARDWARE SETS, MATERIAL AND FINISHES WITH OWNER BEFORE ORDER & INSTALLATION
D100A	3'-0"	X 7'-0"	X 1-3/4"	A	HM	P	1	HM	P		X							X	X	X		X	
D100B	3'-0"	X 7'-0"	X 1-3/4"	A	HM	P	1	HM	P		X							X	X	X		X	
D100C	14'-0"	X 14'-0"		B	HM			HM															
D100D	14'-0"	X 14'-0"		B	HM			HM															
D100E	2'-6"	X 6'-8"	X 1-3/4"	C	WD	P	2	WD	P					X						X			
D101	3'-0"	X 7'-0"	X 1-3/4"	C	WD	P	2	WD	P				X							X			
D102	3'-0"	X 7'-0"	X 1-3/4"	C	WD	P	2	WD	P					X						X			
D200	3'-0"	X 7'-0"	X 1-3/4"	C	HM	P	2	WD	P		X												

## NOTES:

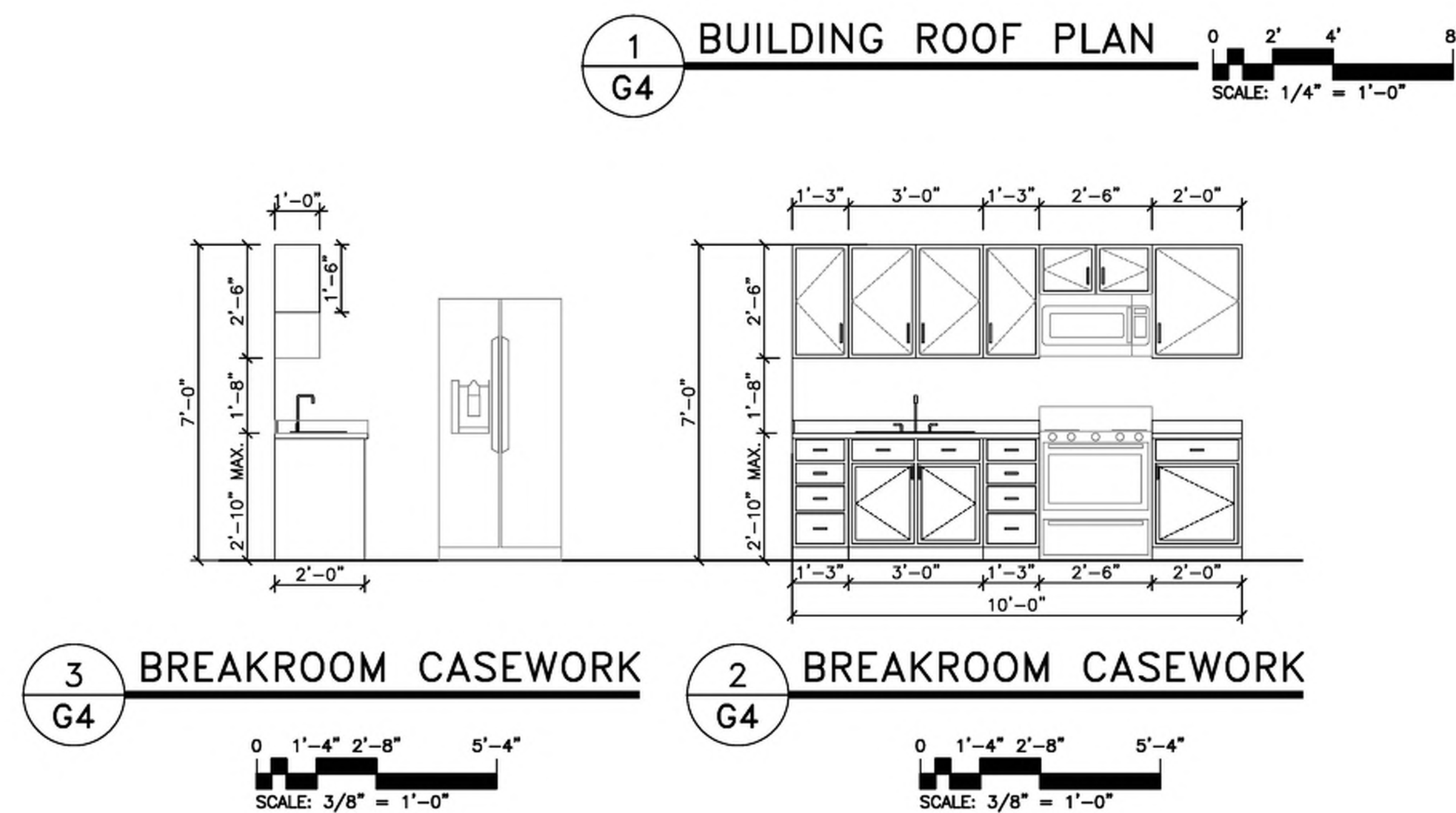
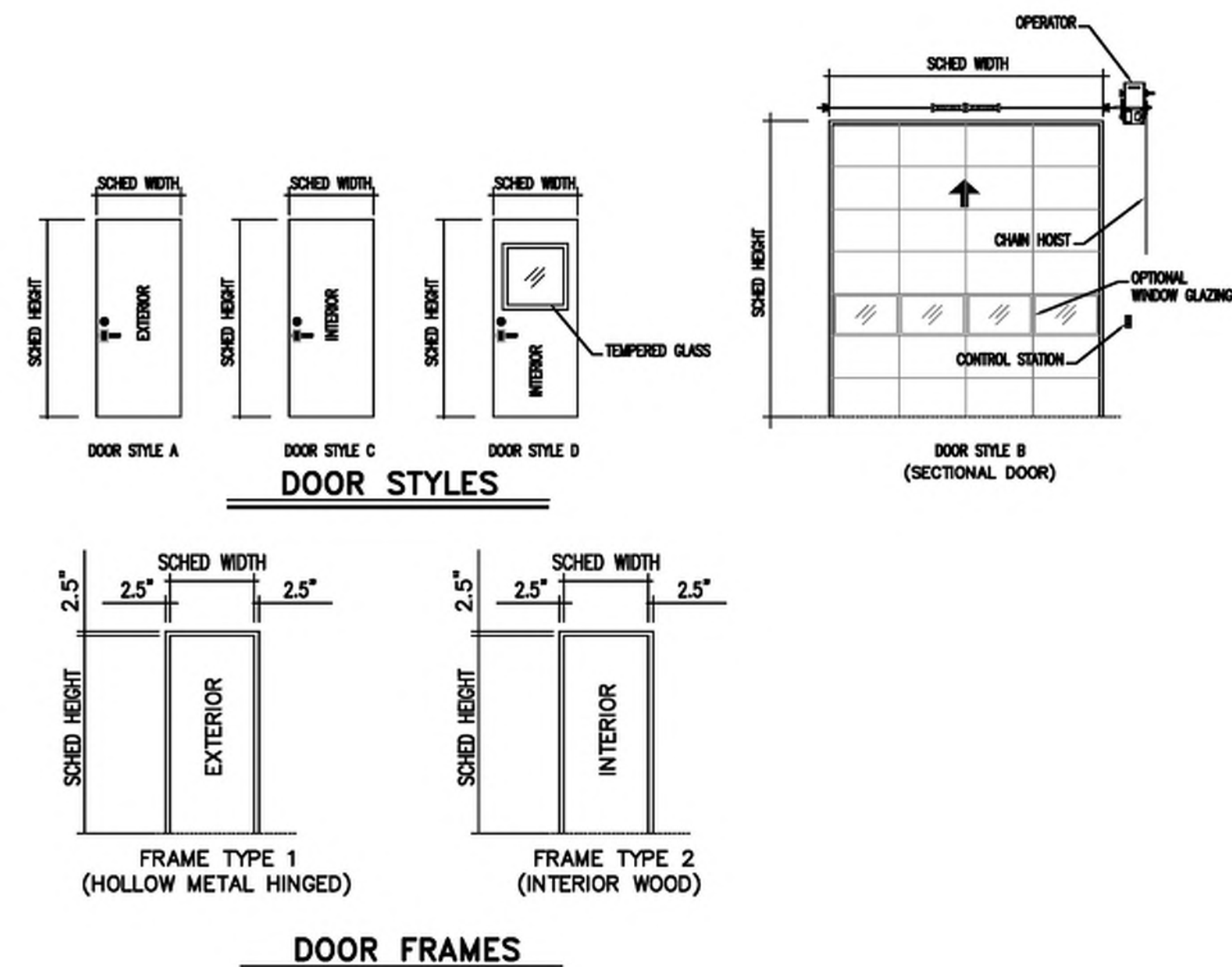
1. APPLY 2 COATS OF SEMI-GLOSS TO ALL WOOD DOORS.
2. ALL EXIT DOORS TO BE OPERABLE FROM THE INSIDE WITHOUT THE USE OF A KEY, TOOL, SPECIAL KNOWLEDGE OF EFFORT. ALL HARDWARE MUST BE DIRECT ACTING REQUIRING NOT MORE THAN ONE OPERATION.
3. DOOR HANDLES, PULLS, LATCHES, LOCKS AND OTHER OPERABLE PARTS ON ACCESSIBLE DOORS SHALL HAVE A SHAPE THAT IS EASY TO GRASP WITH ONE HAND AND DOES NOT REQUIRE THUMB GRASPING, PINCHING OR TWISTING OF THE WRIST TO OPERATE. OPERABLE PARTS OF SUCH HARDWARE SHALL BE 34" MINIMUM AND 48" MAXIMUM ABOVE THE FLOOR PER ICC/ANSI A117.1-2009 SECTIONS 404.2.5& 404.2.7
4. G.C. TO REVIEW ALL HARDWARE SETS WITH OWNER BEFORE INSTALLATION
5. PROVIDE TRANSITION STRIPS AT ALL FLOORING MATERIAL CHANGES

\*\*\*\*NOTE: ALL NEW HARDWARE TO BE LEVER ADA ACCEPTABLE  
ALL THRESHOLDS TO MEET ADA SPECIFICATIONS

**PASSAGE SET:** (CLOSET & HALL) PASSAGE LOCKSETS KEEP DOORS FIRMLY CLOSED, BUT DO NOT ACTUALLY LOCK. BOTH LEVERS ALWAYS TURN FREE WITH NO LOCK CYLINDER OR PROVISION FOR A KEY.

**PRIVACY SET:** (RESTROOM) PRIVACY LOCKSETS ARE LOCKED WITH AN INSIDE PUSH-BUTTON. TURNING THE INSIDE KNOB OR LEVER RELEASES THE LOCK. A SMALL SCREWDRIVER CAN BE USED AS AN EMERGENCY KEY, FROM THE OUTSIDE, IF NECESSARY.

**ENTRANCE LOCK:** (ENTRY) ENTRANCE LOCKED BY PUSHING AND TURNING A BUTTON AND UNLOCKED BY THE KEY UNTIL THE INSIDE BUTTON IS MANUALLY UNLOCKED. THEY ARE ALSO AVAILABLE WITH PUSHBUTTON LOCKING, IN WHICH PUSHING THE BUTTON LOCKS THE OUTSIDE KNOB OR LEVER UNTIL IT IS UNLOCKED BY KEY OR BY TURNING THE OUTSIDE KNOB OR LEVER. THE INSIDE KNOB OR LEVER IS ALWAYS FREE FOR IMMEDIATE EXIT.



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910.622.1724



DESIGNED / CHECKED BY: KJD
DRAWN BY: BT
PROJECT #: 2025-04-02.2
DATE:

FINAL DRAWING <input type="checkbox"/> FOR REVIEW PURPOSES ONLY	OWNER/TENANT:
PRELIMINARY <input type="checkbox"/> FOR DESIGN DEVELOPMENT ONLY	LUIS TIRADO
FINAL DRAWING <input checked="" type="checkbox"/> FOR CONSTRUCTION	3577 OLD US 421 LILLINGTON, NC 27546
	CONTRACTOR/BUILDER:
	TBD

REV	DATE	DESCRIPTION
Δ	07/10/25	FINAL FOR CO

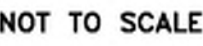
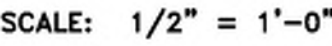
PROJECT: ***TIRADO TRUCK SHOP***  
 US 421 SOUTH, HARNETT COUNTY, NC

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SHEET: **SCHEDULE & ROOF PLAN**

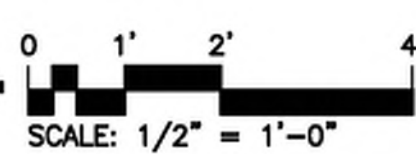
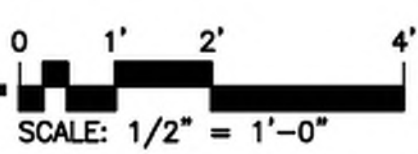
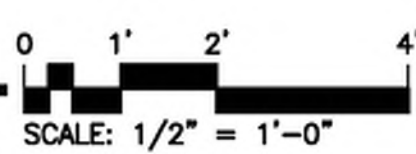
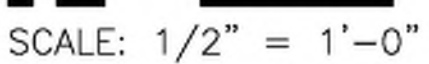
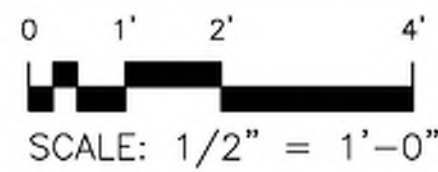
G4





SEE SHEET P1 FOR PLUMBING FIXTURE SCHEDULE.

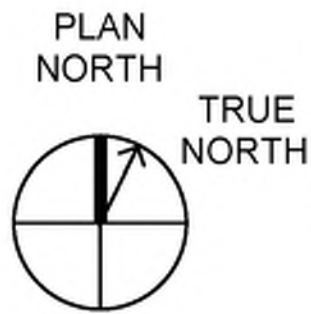
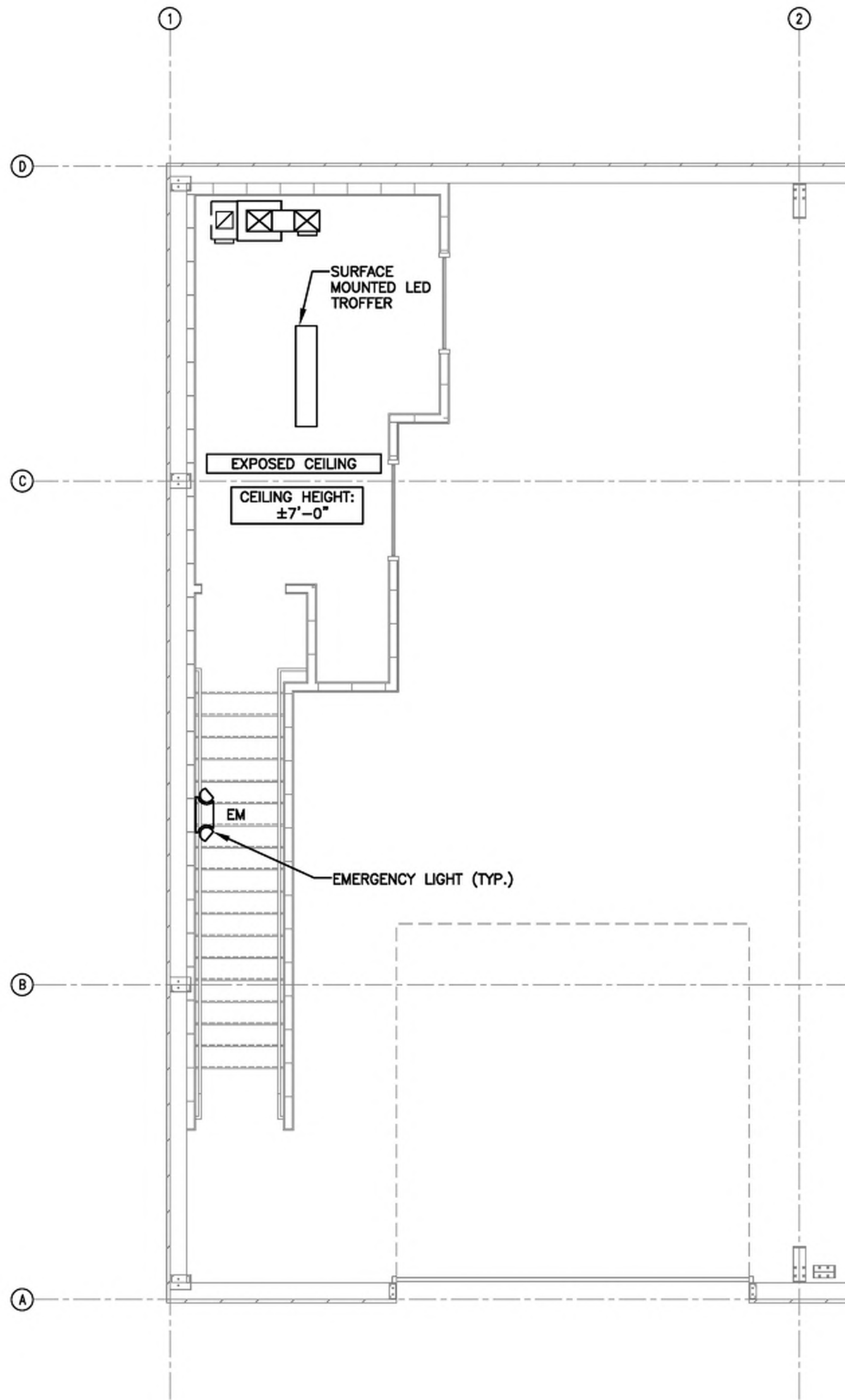
GYPSUM WALL BOARD SHALL BE MOISTURE RESISTANT IN RESTROOM
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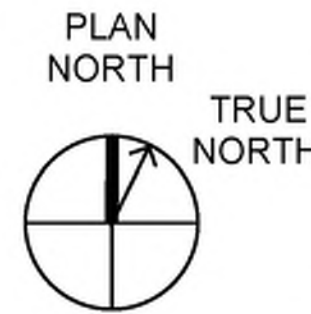
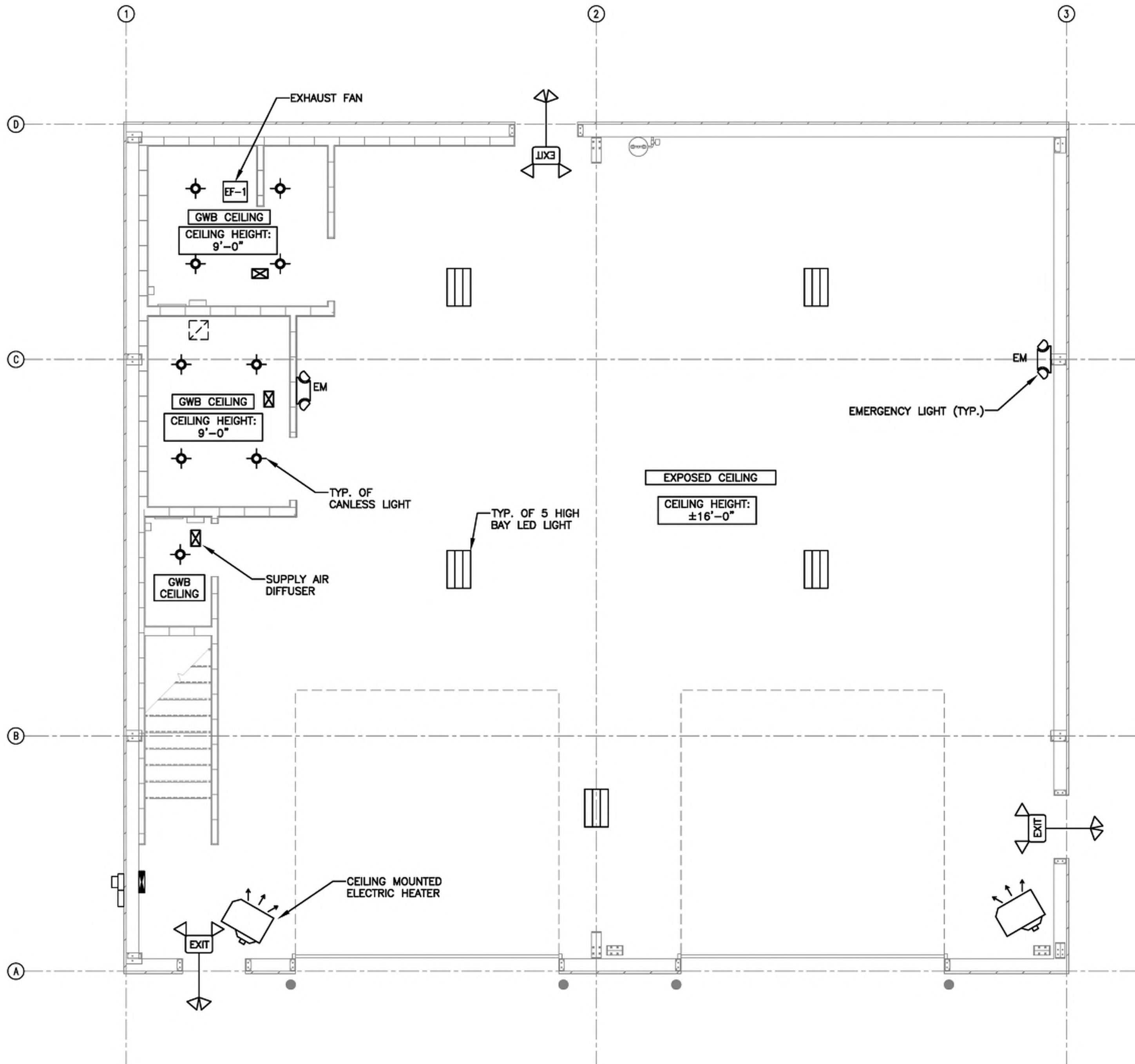
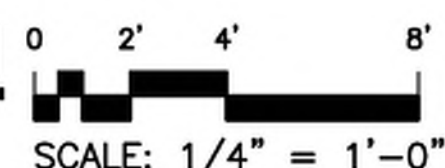




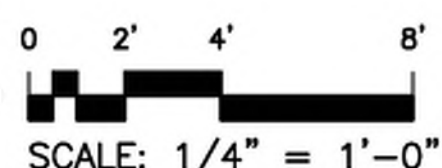
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Printed Date: Jun 10, 2025 - 10:33am



2 REFLECTED CEILING PLAN  
G6 MEZZANINE



1 REFLECTED CEILING PLAN  
G6 FIRST FLOOR



PERMITTING STAMP

PROJECT: **TIRADO TRUCK SHOP**  
US 421 SOUTH, HARNETT COUNTY, NC

SHEET: **REFLECTED CEILING PLAN**

G6

DESIGNED / CHECKED BY: BJ  
DRAWN BY: BT  
PROJECT #: 2025-04-02.2  
DATE: 10 JULY 2025

FINAL DRAWING ☐ FOR REVIEW PURPOSES ONLY  
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FINAL DRAWING ☒ FOR CONSTRUCTION  
OWNER/TENANT:  
LUIS TIRADO  
307 OLD US 421 LILLINGTON, NC 27448  
CONTRACTOR/BUILDER:  
TBD

REV	DATE	DESCRIPTION
1	07/10/25	FINAL FOR CONSTRUCTION

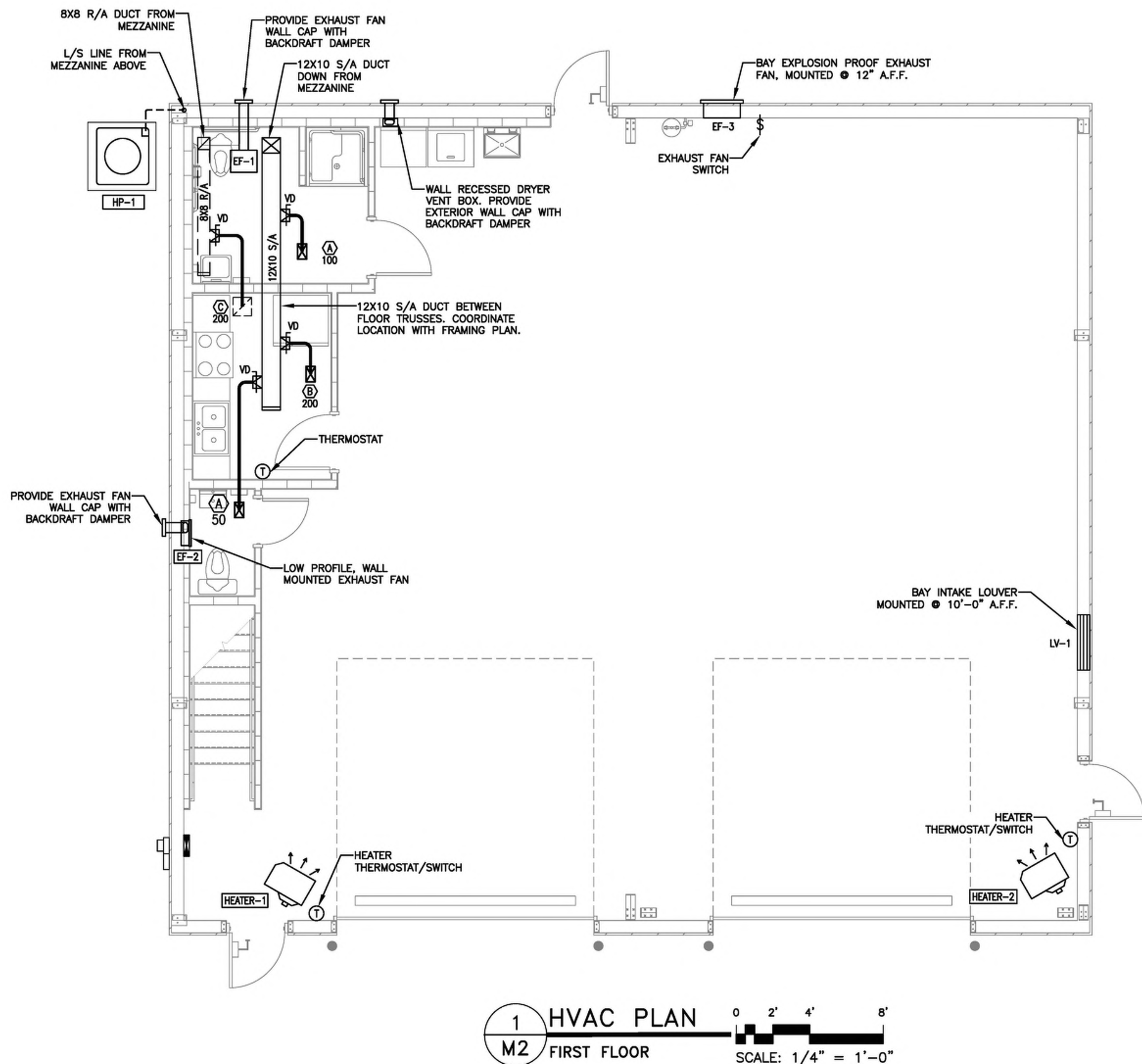
10 JULY 2025





## M1





MECHANICAL CONTRACTOR MAY USE EITHER ROUND OR RECTANGULAR DUCTWORK FOR HVAC SYSTEM, DEPENDING ON THE BUILDING OWNER'S PREFERENCE.

NEW HEAT PUMP/ COOLING UNIT SCHEDULE																							
EQUIPMENT INFO			COOLING CAPACITIES						HEATING CAPACITIES		COMPRESSOR/CONDENSER SECTION						ELECTRICAL INFORMATION				MISCELLANEOUS INFORMATION		
TAG	TYPE	LOCATION	NOM. TONS	TOTAL COOLING	MIN. IEER	MIN. IEER	MIN. SEER2	MIN. COP	UNIT CAPACITY	MIN. HSPF2	NO. OF COMPR.	COMPRESSOR AMPS RLA	CONDENSER FAN AMPS FLA	NO. OF FANS	FAN HP	UNIT VOLTS	UNIT PHASE	MCA	MOCP	WIRE SIZE (CU. 75 C)	MANUFACTURER/MODEL	UNIT SIZE (H)X(W)X(D)	UNIT WEIGHT
HP-1	SPLIT-SYSTEM HEAT PUMP	OUTDOOR PAD	1.5	18,000	N/A	N/A	15.2	N/A	18,000	7.8	1	8.2	0.95	1	1/8	240	1	11.2	20	#12	GOODMAN/GLZS4BA1810	33"x29"x29"	150 LBS

\*\*\*BASIS OF DESIGN: GOODMAN EQUIPMENT. SIMILAR AND EQUAL EQUIPMENT BY TRANE, CARRIER, AND ICP MAY BE SUBSTITUTED FOR THE CARRIER EQUIPMENT. SUBMIT MANUFACTURERS LITERATURE TO ENGINEER FOR APPROVAL BEFORE SUBSTITUTING EQUIPMENT.

\*\*\* RESTROOM EXHAUST FAN SHALL CONTROLLED WITH RESTROOM LIGHT SWITCH.  
\*\*\* BAY AREA EXHAUST FAN SHALL BE OPERATE AT ALL TIME WHEN THE BUILDING IS OCCUPIED.

① FACTORY THERMOSTAT ② FACTORY CEILING SUSPENSION KIT

\*\*\*BASIS OF DESIGN: GOODMAN EQUIPMENT. SIMILAR AND EQUAL EQUIPMENT BY TRANE, CARRIER, AND ICP MAY BE SUBSTITUTED FOR THE CARRIER EQUIPMENT. SUBMIT MANUFACTURERS LITERATURE TO ENGINEER FOR APPROVAL BEFORE SUBSTITUTING EQUIPMENT.

REMARKS FOR AHU AND HP UNITS:PERMITTING STAMP

DESIGNED / CHECKED BY:	BJ
DRAWN BY:	BT
PROJECT #:	2025-04-02.2
DATE:	10 JULY 2025

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PRELIMINARY ☐ FOR DESIGN DEVELOPMENT ONLY  
FINAL DRAWING ☒ FOR CONSTRUCTION

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OWNER/TENANT:  
LUIS TIRADO  
3577 OLD US 421 LILLINGTON, NC 27146

CONTRACTOR/BUILDER:  
TBD

REV	DATE	DESCRIPTION
△	07/10/25	FINAL FOR CON

PROJECT: ***TIRADO TRUCK SHOP***  
 US 421 SOUTH, HARNETT COUNTY, NC

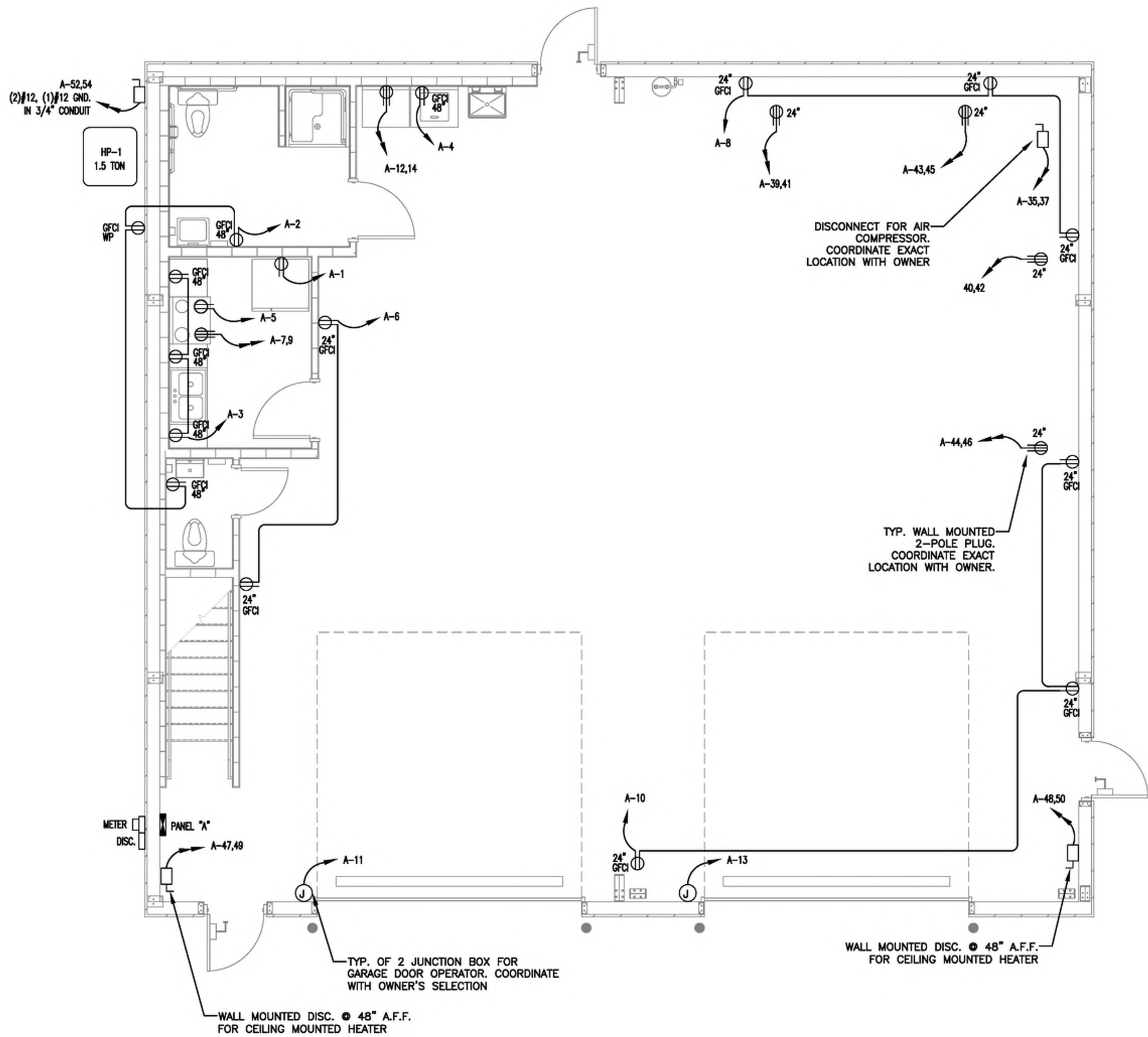
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M2









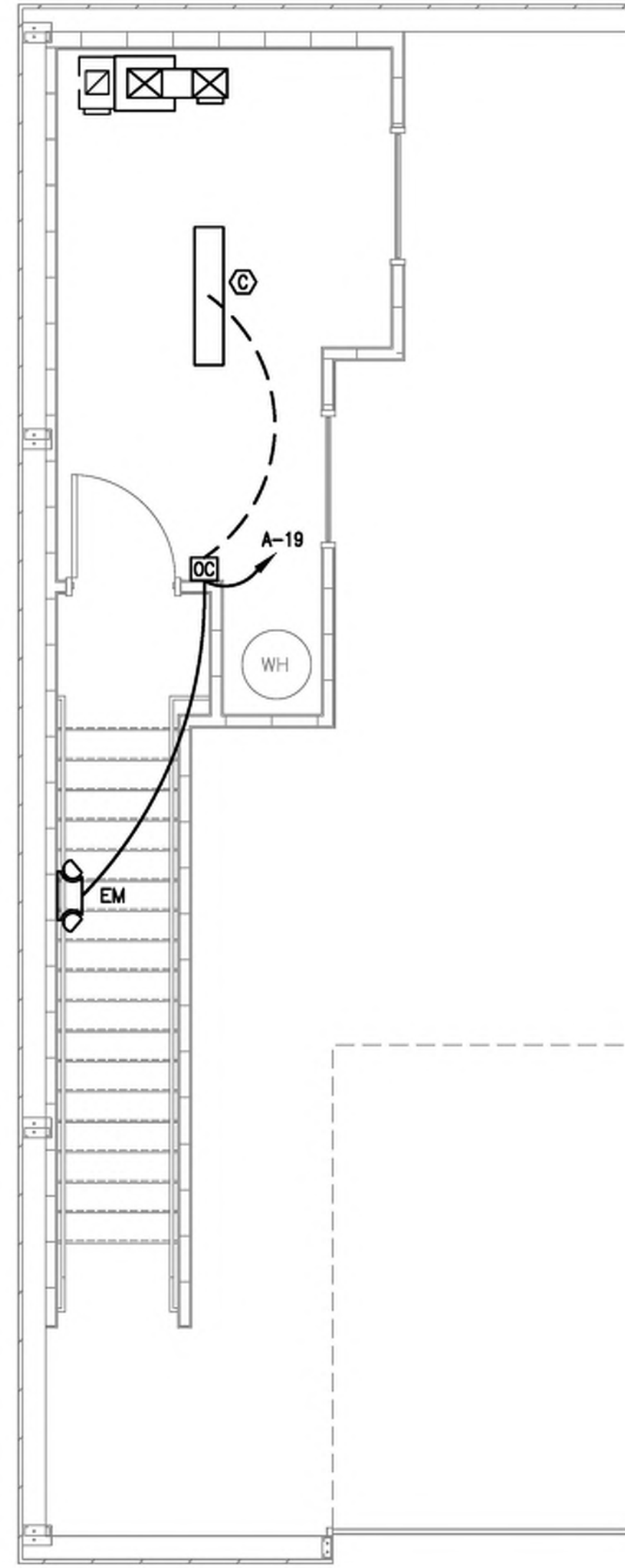
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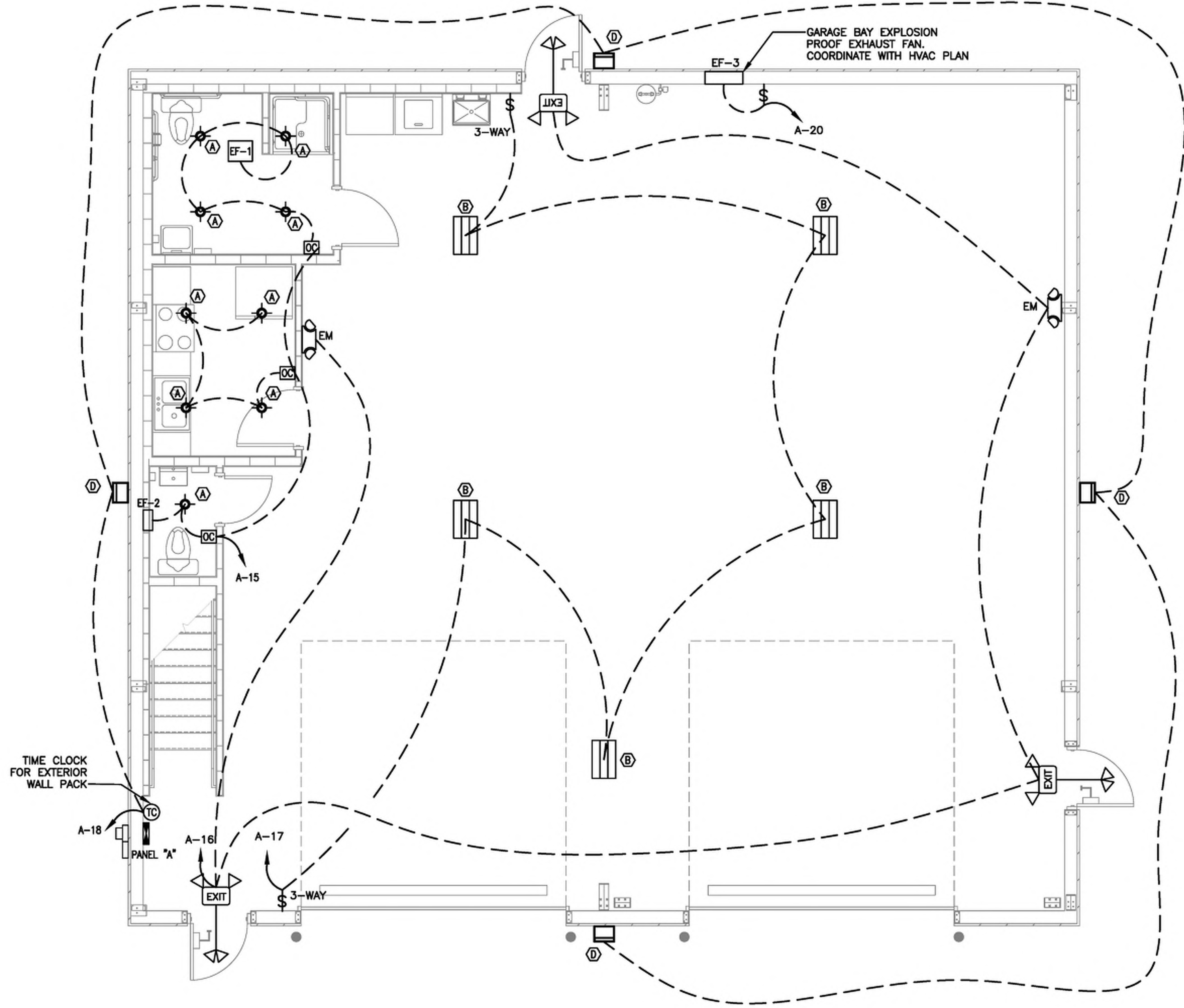


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Printed Date: Jun 10, 2025 - 10:33am

2 LIGHTING PLAN  
E3 MEZZANINE  
SCALE: 1/4" = 1'-0"



1 LIGHTING PLAN  
E3 FIRST FLOOR  
SCALE: 1/4" = 1'-0"



LIGHT FIXTURE SCHEDULE													
TAG	DESCRIPTION	SIZE	MOUNTING	LENS	COLOR	LUMENS	BULB	BALLAST TYPE	HOUSING	VOLTAGE	WATTAGE	MANU/MODEL NUMBER	REMARKS
A	LED RECESSED DOWNLIGHT	6"	RECESSED	N/A	5000 K	1210	LED	LED DRIVER	STEEL	120	14.3	LITHONIA NO. WF8 LED 50K MVOLT 90CRI OR EQUAL	
B	LED HIGH BAY LIGHT	24"x18"	SURFACE	N/A	5000 K	22000	LED	LED DRIVER	STEEL	120	165	LITHONIA NO. IBE L24 22000LM ATC MD 50K 80CRI OR EQUAL	
C	LED TROFFER	48"x12"	SURFACE	N/A	5000 K	4000	LED	LED DRIVER	STEEL	120	165	LITHONIA NO. SBL4 4000LM 80CRI 40K OR EQUAL	
D	EXTERIOR LED WALL PACK	13" X 9"	SURFACE	N/A	4000 K	5300	LED	LED DRIVER	STEEL	120	36	LITHONIA NO. TWR1 LED ALO SHW2 UVOLT PE DDBTXD OR EQUAL	
EM	EMERGENCY	N/A	WALL	N/A	N/A	N/A	(2) LAMPS	ELECTRONIC	POLYCARBONATE	120/240		LITHONIA NO. EU2L M12 OR EQUAL	6 VOLT NICAD BATTERY TEST SWITCH, POWER INDICATOR
EX	EXIT SIGN/EMERGENCY LIGHT COMBO	N/A	WALL	SINGLE	N/A	N/A	LED LIGHT	LED DRIVER	POLYCARBONATE	120/240		LITHONIA NO. LHQM LED R HO M6 OR EQUAL	6 VOLT NICAD BATTERY, (2) REMOTE HEADS

PERMITTING STAMP

PROJECT: **TIRADO TRUCK SHOP**  
US 421 SOUTH, HARNETT COUNTY, NC  
SHEET: **E3**  
ELECTRICAL - LIGHTING PLAN

REVISIONS:		REV	DATE	DESCRIPTION
		A	07/10/25	FINAL FOR CONSTRUCTION

DESIGNED / CHECKED BY: <b>BJ</b>	DRAWN BY: <b>BT</b>	PROJECT #: <b>2025-04-02.2</b>	DATE: <b>10 JULY 2025</b>
OWNER/TENANT: <b>LUIS TIRADO</b> 307 OLD US 421 LILLINGTON, NC 27448		CONTRACTOR/BUILDER: <b>TBD</b>	





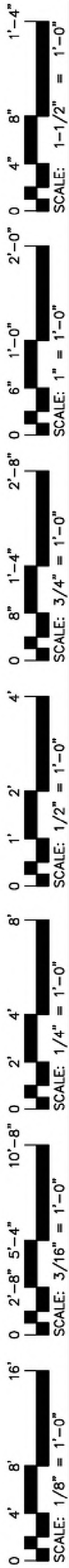
ALL PLUMBING SYSTEMS AND EQUIPMENT SHALL BE GUARANTEED FOR A PERIOD OF ONE YEAR AFTER FINAL ACCEPTANCE OF THE BUILDING FROM THE OWNER.

NOT TO SCALE

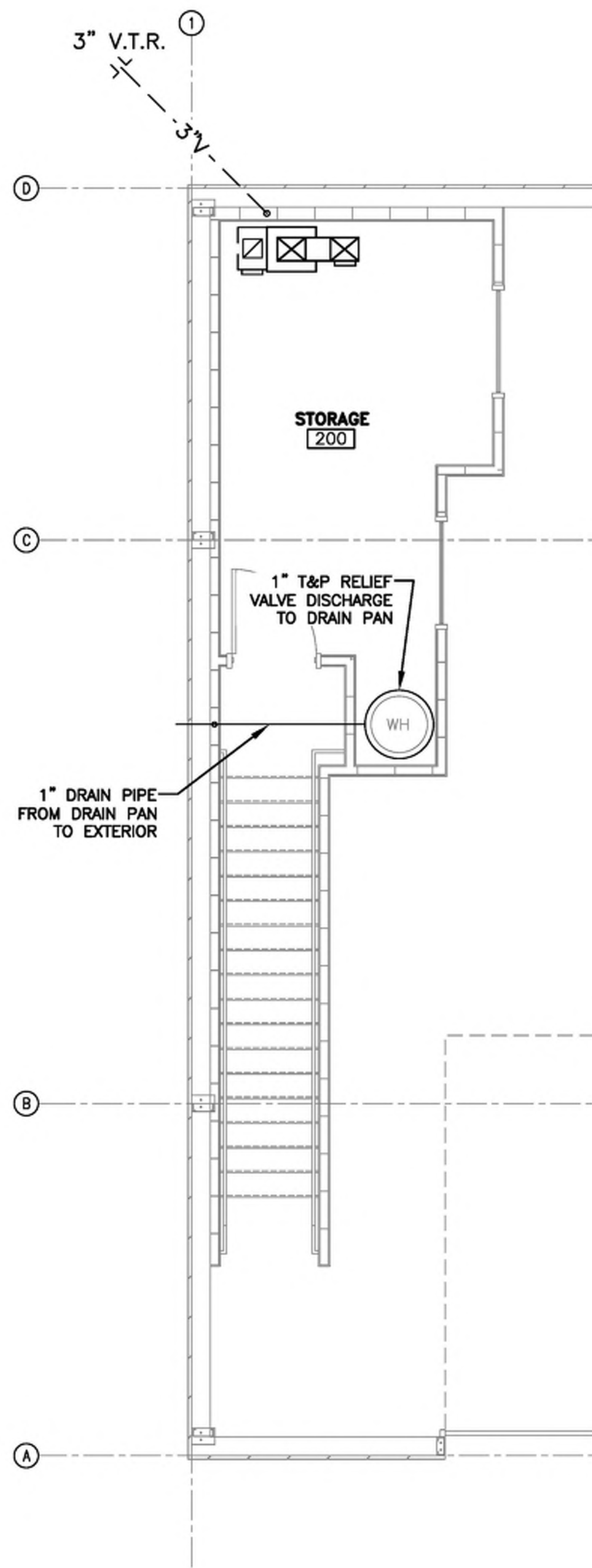


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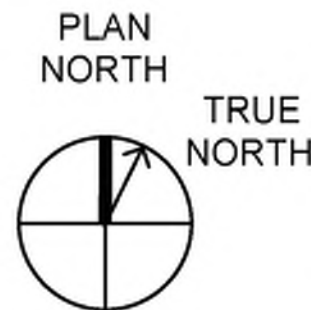
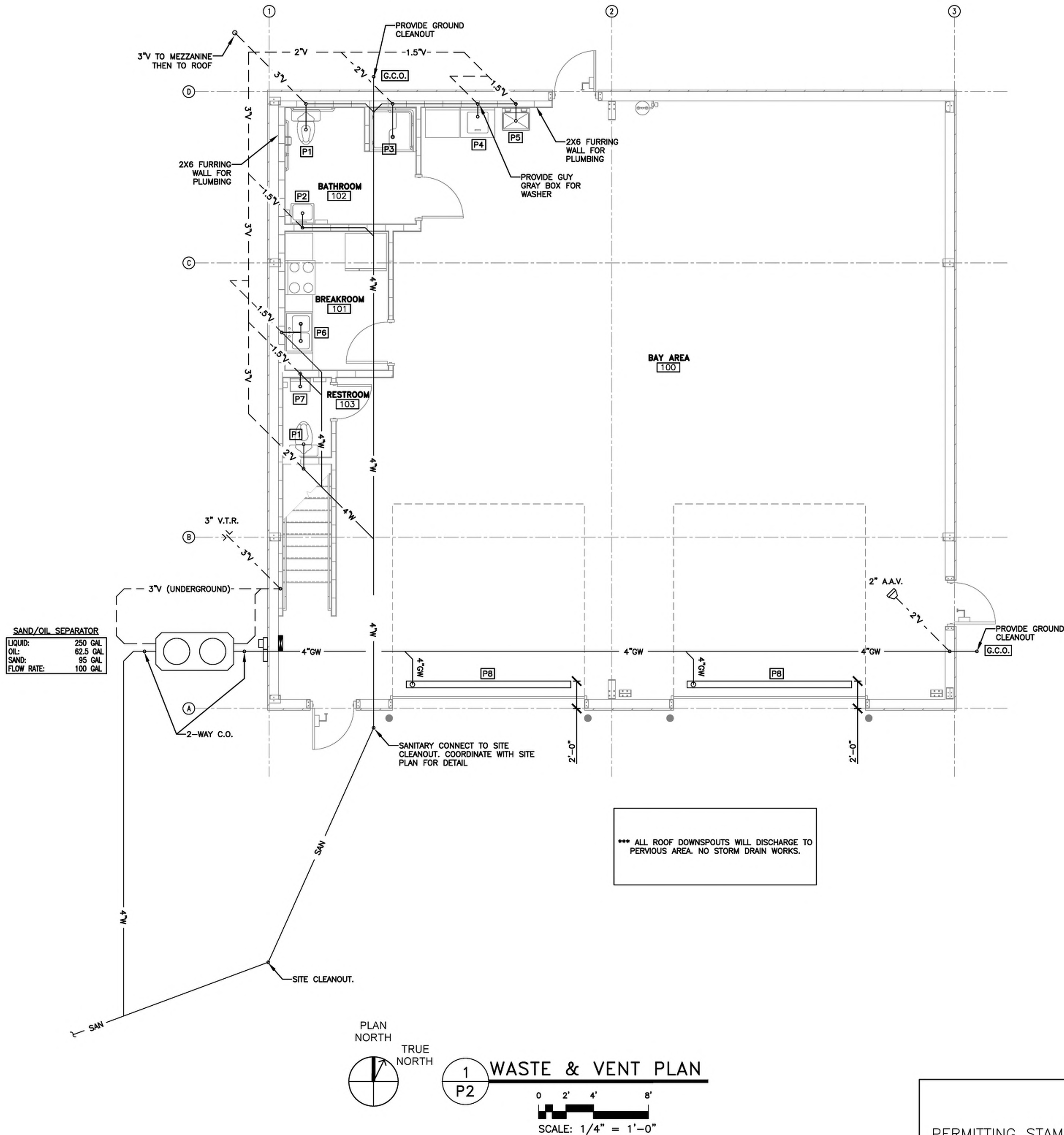




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2 WASTE & VENT PLAN  
MEZZANINE  
SCALE: 1/4" = 1'-0"



1 WASTE & VENT PLAN  
SCALE: 1/4" = 1'-0"

PERMITTING STAMP

PROJECT:

TIRADO TRUCK SHOP

US 421 SOUTH, HARNETT COUNTY, NC

SHEET:

P2

DESIGNED / CHECKED BY:

BJ

DRAWN BY:

BT

PROJECT #:

2025-04-02.2

DATE:

10 JULY 2025

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PRELIMINARY ☐ FOR DESIGN DEVELOPMENT ONLY

FINAL DRAWING ☐ FOR CONSTRUCTION

OWNER/TENANT:

LUIS TIRADO

CONTRACTOR/BUILDER:

TBD

REVISIONS:

REV	DATE	DESCRIPTION
A	07/10/25	FINAL FOR CONSTRUCTION

SEAL

28803

ENGINEER

JENKINS

10 JULY 2025

PROFESSIONAL SEAL

28803

ENGINEER

JENKINS

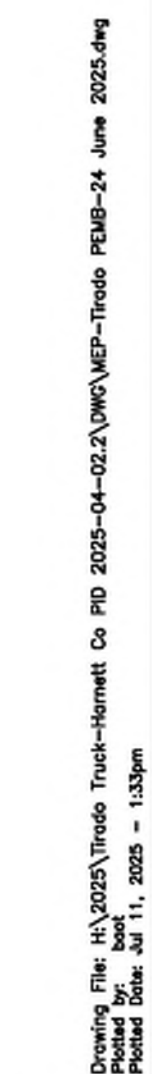
OFFICE IN EUREKA SPRINGS, NORTH CAROLINA

1608 MARTIN RD.

FAVETTEVILLE, NC 28311-1002

910.822.1724





P3



# BUILDER/CONTRACTOR RESPONSIBILITIES

**Drawing Validity** - These drawings, supporting structural calculations and design certification are based on the order documents as of the date of these drawings. These documents describe the material supplied by the manufacturer as of the date of these drawings. Any changes to the order documents after the date on these drawings may void these drawings, supporting structural calculations and design certification. The Builder/Contractor is responsible for notifying the building authority of all changes to the order documents which result in changes to the drawings, supporting structural calculations and design certification.

**Builder Acceptance of Drawings** - Approval of the manufacturer's drawings and design data affirms that the manufacturer has correctly interpreted and applied the requirements of the order documents and constitutes Builder/Contractor acceptance of the manufacturer's interpretations of the order documents and standard product specifications, including its design, fabrication and quality criteria standards and tolerances. (AISC code of standard practice Sept 86 Section 4.2.1) (Mar 05 Section 4.4.1)

**Code Official Approval** - It is the responsibility of the Builder/Contractor to ensure that all project plans and specifications comply with the applicable requirements of any governing building authority. The Builder/Contractor is responsible for securing all required approvals and permits from the appropriate agency as required.

**Builder is responsible for State, Federal and OSHA safety compliance** - The Builder/Contractor is responsible for applying and observing all pertinent safety rules and regulations and OSHA standards as applicable.

**Building Erection** - The Builder/Contractor is responsible for all erection of the steel and associated work in compliance with the Metal Building Manufacturers drawings. Temporary supports, such as temporary guys, braces, false work or other elements required for erection will be determined, furnished and installed by the erector. (AISC Code of Standard Practice Sept 86 Section 7.9.1) (Mar 05 Section 7.10.3)

**Discrepancies** - Where discrepancies exist between the Metal Building plans and plans for other trades, the Metal Building plans will govern. (AISC Code of Standard Practice Sept 86 Section 3.3) (Mar 05 Section 3.3)

**Materials by Others** - All interface and compatibility of any materials not furnished by the manufacturer are the responsibility of and to be coordinated by the Builder/Contractor or A/E firm. Unless specific design criteria concerning any interface between materials if furnished as a part of the order documents, the manufacturers assumptions will govern.

**Modification of the Metal Building from Plans** - The Metal Building supplied by the manufacturer has been designed according to the Building Code and specifications and the loads shown on this drawing. Modification of the building configuration, such as removing wall panels or braces, from that shown on these plans could affect the structural integrity of the building. The Metal Building Manufacturer or a Licensed Structural Engineer should be consulted prior to making any changes to the building configuration shown on these drawings. The Metal Building Manufacturer will assume no responsibility for any loads applied to the building not indicated on these drawings.

**Foundation Design** - The Metal Building Manufacturer is not responsible for the design, materials and workmanship of the foundation. Anchor rod plans prepared by the manufacturer are intended to show only location, diameter and projection of the anchor rods required to attach the Metal Building System to the foundation. It is the responsibility of the end customer to ensure that adequate provisions are made for specifying rod embedment, bearing values, tie rods and or other associated items embedded in the concrete foundation, as well as foundation design for the loads imposed by the Metal Building System, other imposed loads, and the bearing capacity of the soil and other conditions of the building site. (MBMA 05 Sections 3.2.2 and A3)

# PROJECT NOTES

Material properties of steel bar, plate, and sheet used in the fabrication of built-up structural framing members conform to ASTM A529, ASTM A572, ASTM A1011 SS, or ASTM A1011 HSLAS with a minimum yield point of 50 ksi. Material properties of hot rolled structural shapes conform to ASTM A992, ASTM A529, or ASTM A572 with a minimum specified yield point of 50 ksi. Hot rolled angles, or other than flange braces, conform to ASTM 36 minimum. Hollow structural shapes conform to ASTM A500 grade B, minimum yield point is 42 ksi for round HSS and 46 ksi for rectangular HSS. Material properties of cold form light gage steel members conform to the requirements of ASTM A1011 SS Grade 55 or ASTM A1011 HSLAS Class 1 Grade 55, with a minimum yield point of 55 ksi.

The manufacturer does not assume any responsibility for the erection nor field supervision of the structure and or any special inspections that may be required by the local building authority during erection (including inspection of the high strength bolts or field welds) as required during erection. The coordination and the costs associated for setting up and Special Inspections are the responsibility of the Erector, Owner, Architect, or Engineer of Record.

Design is based upon the more severe loading of either the roof snow load or the roof live load.

Loads, as noted, are given within order documents and are applied in general accordance with the applicable provisions of the model code and/or specification indicated. Neither the manufacturer nor the certifying engineer declares or attests that the loads as designated are proper for the local provisions that may apply or for site specific parameters. The manufacturer's Engineer's certification is limited to design loads supplied by an Architect and/or engineer of record for the overall construction project.

This project is designed using manufacturer's standard serviceability standards. Generally this means that all stresses and deflections are within typical performance limits for normal occupancy and standard metal building products. If special requirements for deflections and vibrations must be adhered to, then they must be clearly stated in the contract documents.

This metal building system is designed as enclosed. All exterior components (i.e. doors, windows, vents, etc.) must be designed to withstand the specified wind loading for the design of components and cladding in accordance with the specified building code. Doors are to be closed when a maximum of 50% of design wind velocity is reached.

The design collateral load has been uniformly applied to the design of the building. Hanging loads are to be attached to the purlin web. This may not be appropriate for heavily concentrated loads. Any attached load in excess of 150 pounds shall be accounted for by special design performed by a licensed engineer using concentrated loads and may require separate support members within the roof system.

The metal building manufacturer has not designed the structure for snow accumulation loads at the ground level which may impose snow loads on the wall framing provided by the manufacturer.

# DESIGN LOADING

THIS STRUCTURE IS DESIGNED UTILIZING THE LOADS INDICATED AND APPLIED AS REQUIRED BY:

IBC 15

THE BUILDER IS TO CONFIRM THAT THESE LOADS COMPLY WITH THE REQUIREMENTS OF THE LOCAL BUILDING DEPARTMENT.

ROOF DEAD LOAD 1.89 PSF  
SUPERIMPOSED 1.00 PSF  
COLLATERAL (LIGHTS)

ROOF LIVE LOAD 20.00 PSF (NOT REDUCIBLE)

RISK CATEGORY II - Normal

SNOW LOAD  
GROUND SNOW LOAD (Pg) 15.00 PSF  
SNOW LOAD IMPORTANCE FACTOR (I<sub>s</sub>) 1.0000  
FLAT ROOF SNOW LOAD (P<sub>f</sub>) 10 PSF (AS PER ASCE 7-10 SECTION 7.3)  
MIN. ROOF SNOW LOAD (P<sub>r</sub>) 15 PSF (USED IN DESIGN)  
SNOW EXPOSURE FACTOR (C<sub>e</sub>) 1.0  
THERMAL FACTOR (C<sub>t</sub>) 1.00

WIND LOAD  
ULTIMATE WIND SPEED 118 MPH  
NOMINAL WIND SPEED (V<sub>ref</sub>) 91 MPH (IBC SECTION 1609.3.1)  
SERVICEABILITY WIND SPEED 76 MPH  
WIND EXPOSURE CATEGORY C  
TOPOGRAPHICAL FACTOR 1.0

INTERNAL PRESSURE COEFFICIENT (C<sub>pi</sub>) 0.18 / -0.18  
ZONE 4, COMPONENT WIND LOAD ≤ 10 FT<sup>2</sup>  
28.47 PSF PRESSURE -37.89 PSF SUCTION  
ZONE 5, COMPONENT WIND LOAD < 10 FT<sup>2</sup>  
28.47 PSF PRESSURE -30.84 PSF SUCTION

ZONES PER ASCE 7-10: FIG. 30.4-1  
ZONES PRESSURES SHOWN ARE UN-FACTORED

RAIN INTENSITY  
5-MINUTE DURATION, 5-YEAR  
RECURRENT (I<sub>r</sub>) 7.0 IN/HOUR

SEISMIC LOAD  
SEISMIC IMPORTANCE FACTOR (I<sub>e</sub>) 1.00  
S<sub>s</sub> 0.185 S<sub>ce</sub> 0.197  
S<sub>1</sub> 0.087 S<sub>ce1</sub> 0.139  
SITE CLASS D STIFF SOIL  
SEISMIC DESIGN CATEGORY B

ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE

COLUMN LINE	TRANSVERSE	LONGITUDINAL
	FRONT	BACK
BASIC FORCE RESISTING SYSTEM*	H	H
RESPONSE MODIFICATION COEFFICIENT(R)	3	3
SYSTEM OVER-STRENGTH FACTOR(O <sub>2</sub> )	2.5000	2.5000
SEISMIC RESPONSE COEFFICIENT(C <sub>s</sub> )	0.066	0.066
BLDG DESIGN BASE SHEAR (V) TRANSVERSE 1.18 (K) LONGITUDINAL 1.08 (K)		

THE TRANSVERSE DIRECTION IS PARALLEL TO THE ROOF FRAMES  
THE LONGITUDINAL DIRECTION IS PERPENDICULAR TO THE ROOF FRAMES  
BASIC STRUCTURAL SYSTEM (FROM ASCE 7-10 TABLE 12.2-1)

BASIC FORCE RESISTING SYSTEM\*  
H. STRUCTURAL STEEL SYSTEMS NOT SPECIFICALLY  
DETAILED FOR SEISMIC RESISTANCE

# DRAWING INDEX

PAGE	DESCRIPTION
C1	COVER SHEET
F1	ANCHOR BOLT PLAN
F2	ANCHOR BOLT REACTIONS
F3	ANCHOR BOLT DETAILS
E1	ROOF FRAMING PLAN
E2	ROOF SHEETING PLAN
E3	FRONT SIDEWALL
E4	BACK SIDEWALL
E5	LEFT ENDWALL
E6	RIGHT ENDWALL
E7	FRAME CROSS SECTION
E8	WIND BENT ELEVATION
DET1-18	STANDARD DETAILS
R1-R3	INSTALLATION SHEETS

# DRAWING STATUS

☐ **FOR APPROVAL**  
THESE DRAWINGS, BEING FOR APPROVAL, ARE BY DEFINITION NOT FINAL, AND ARE FOR CONCEPTUAL REPRESENTATION ONLY. THEIR PURPOSE IS TO CONFIRM PROPER INTERPRETATION OF THE PROJECT DOCUMENTS. ONLY DRAWINGS ISSUED "FOR ERECTOR INSTALLATION" CAN BE CONSIDERED AS COMPLETE.

☐ **FOR CONSTRUCTION PERMIT**  
THESE DRAWINGS, BEING FOR PERMIT, ARE BY DEFINITION NOT FINAL. ONLY DRAWINGS ISSUED "FOR ERECTOR INSTALLATION" CAN BE CONSIDERED AS COMPLETE.

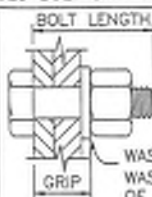
☒ **FOR ERECTOR INSTALLATION**  
FINAL DRAWINGS FOR CONSTRUCTION.

FOR QUESTIONS OR ASSISTANCE  
CONCERNING ERECTION CALL:

905-477-1894

MONDAY - FRIDAY 7:30AM TO 5:00PM

1/2" A325 BOLT GRIP TABLE		
GRIP	LENGTH	BOLT LENGTH
0 TO 9/16"	1 1/4" F.T.	
Over 9/16" TO 1 1/16"	1 3/4" F.T.	
Over 1 1/16" TO 1 5/16"	2"	
Over 1 5/16" TO 1 9/16"	2 1/4"	
Over 1 9/16" TO 1 13/16"	2 1/2"	
Over 1 13/16" TO 2 1/16"	2 3/4"	
F.T. DENOTES FULLY THREADED		



NOTE:  
FULL THREAD ENGAGEMENT IS  
DEEMED TO HAVE BEEN MET  
WHEN THE END OF THE BOLT  
IS FLUSH WITH THE FACE OF  
THE NUT.

WASHER REQUIRED ONLY WHEN SPECIFIED.  
WASHER MAY BE LOCATED UNDER HEAD  
OF BOLT, UNDER NUT, OR AT BOTH AT  
LOCATIONS NOTED ON ERECTION DRAWINGS.  
ADD 5/32" FOR EACH WASHER TO MATERIAL  
THICKNESS TO DETERMINE GRIP.

BUILDING SIZE: 45'-0" x 50'-0" x 17'-0" 2.0:12

ISSUE	DATE	DESCRIPTION	BY	CHK'D	DSN
0	10/25/22	FOR ERECTOR INSTALLATION	MOB	HPD	CM

BUILDING SOLD BY HERITAGE BUILDING SYSTEMS

MICHAEL W. CUSTER, P.E.  
642 OAKBEND DRIVE  
COPPEL TX. 75019  
PH. 972-571-7082

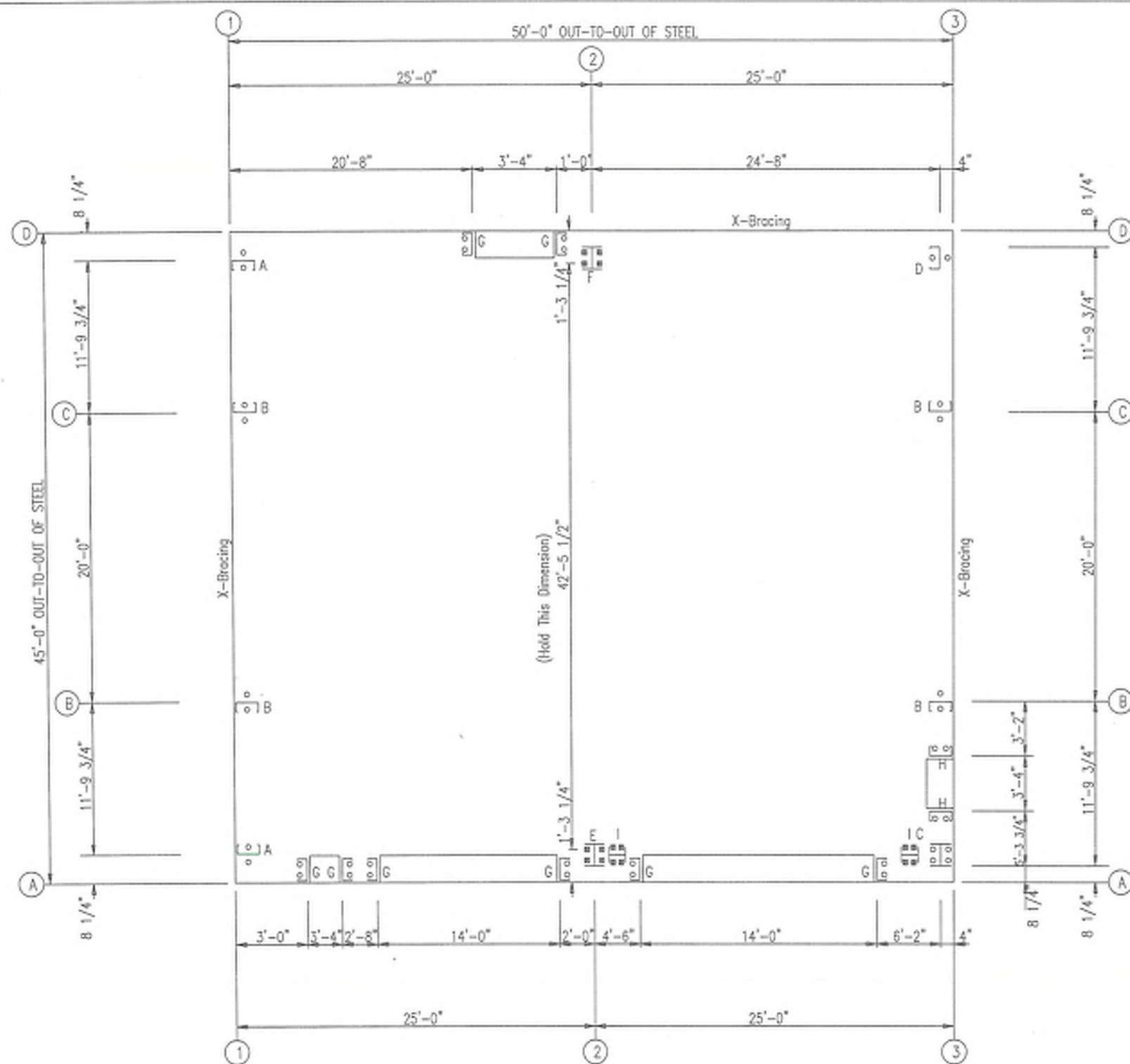
MICHAEL W. CUSTER

Digitally signed by  
MICHAEL W. CUSTER  
Date: 2022.11.02  
09:03:11 -0500

SEAL  
023748

PROFESSIONAL  
ENGINEER  
UNDER MY DIRECT SUPERVISION





○ Dia= 5/8"

⊗ Dia= 3/4"

ANCHOR BOLT PLAN

BUILDING SIZE: 45'-0" x 50'-0" x 17'-0" 2.0:12

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	10/25/22	FOR ERECTOR INSTALLATION	MOB	HPD	CM

BUILDING SOLD BY HERITAGE BUILDING SYSTEMS

MICHAEL W. CUSTER, P.E.  
642 OAKBEND DRIVE  
COPPEL TX. 75019  
PH. 972-571-7082

**MICHAEL W CUSTER**

Digitally signed by  
MICHAEL W. CUSTER  
Date: 2022.10.25  
09:03:50 -0500

SEAL  
023748

PROJECT: LUIS TIRADO - 45X50X17

CUSTOMER: LUIS TIRADO

LOCATION: LILLINGTON, NC 27546

W CUSTER

Date 09:03

OWNER: LUIS TIRADO

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	5/24/19	N.T.S.	1	A	19-B-19564	F1	0

PROFESSIONAL ENGINEER  
MICHAEL W. CUSTER  
UNDER MY DIRECT SUPERVISION



# GENERAL NOTES

- THE REACTIONS PROVIDED ARE BASED ON THE ORDER DOCUMENTS AT THE TIME OF MAILING. ANY CHANGES TO BUILDING LOADS OR DIMENSIONS MAY CHANGE THE REACTIONS. THE REACTIONS WILL BE SUPERSEDED AND VOIDED BY ANY FUTURE MAILING.
- THE REACTIONS PROVIDED HAVE BEEN CREATED WITH THE FOLLOWING LAYOUT (UNLESS NOTED OTHERWISE):
  - A REACTION TABLE IS PROVIDED WITH REACTIONS FOR EACH LOAD GROUP
  - RIGID FRAME
    - SEE NOTE 3.
  - ENDWALLS
    - SEE NOTE 3.
  - X-BRACING
    - X-BRACING REACTIONS ARE INCLUDED IN VALUES SHOWN IN THE REACTION TABLES AS NOTED IN THE BRACING REACTIONS TABLE.
    - FOR IBC AND UBC BASED BUILDING CODES, WHEN X-BRACING IS PRESENT IN THE SIDEWALL, INDIVIDUAL LONGITUDINAL SEISMIC LOADS DO NOT INCLUDE THE AMPLIFICATION FACTOR,  $\Omega$ .
    - FOR IBC AND UBC BASED BUILDING CODES, WHEN X-BRACING IS PRESENT IN THE ENDWALL, INDIVIDUAL TRANSVERSE SEISMIC LOADS DO NOT INCLUDE THE AMPLIFICATION FACTOR,  $\Omega$ .
- THE METAL BUILDING MANUFACTURER IS RESPONSIBLE ONLY FOR THE PORTION OF THE ANCHOR ROD DESIGN PERTAINING TO THE TRANSFER OF FORCES BETWEEN THE BASE PLATE BEARING AND THE ANCHOR RODS. SHEAR AND TENSION, THE METAL BUILDING MANUFACTURER IS NOT RESPONSIBLE FOR THE ANCHOR ROD EMBEDMENT FOR TRANSFER OF FORCES TO THE FOUNDATION. THE METAL BUILDING MANUFACTURER DOES NOT DESIGN AND IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL AND CONSTRUCTION OF THE FOUNDATION EMBEDMENT. THE END USER CUSTOMER SHALL ASSURE THAT ADEQUATE PROVISIONS ARE MADE TO THE FOUNDATION DESIGN FOR LOADS IMPOSED BY COLUMN REACTIONS OF THE BUILDING, OTHER IMPOSED LOADS, AND BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE. IT IS RECOMMENDED THAT THE ANCHORAGE AND FOUNDATION OF THE BUILDING BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER COMPETENT IN THE DESIGN OF SUCH STRUCTURES.
  - (REF. APPENDIX A3 OF THE MBMA METAL BUILDING BUILDING SYSTEMS MANUAL)
- ANCHOR RODS ARE ASTM F1554 GR. 36 MATERIAL UNLESS NOTED OTHERWISE ON THE ANCHOR ROD LAYOUT DRAWING.
- REACTIONS ARE PROVIDED AS UN-FACTORED FOR EACH LOAD GROUP APPLIED TO THE COLUMN. THE FACTORS APPLIED TO LOAD GROUPS FOR THE STEEL COLUMN DESIGN MAY BE DIFFERENT THAN THE FACTORS USED IN THE FOUNDATION DESIGN. THE FOUNDATION ENGINEER SHALL APPLY THE APPROPRIATE LOAD FACTORS AND COMBINE THE REACTIONS IN ACCORDANCE WITH THE BUILDING CODE AND DESIGN SPECIFICATIONS FOR PROPER FOUNDATION DESIGN.
  - FOR PROJECTS USING ULTIMATE DESIGN WIND SPEEDS SUCH AS 2012 IBC, 2015 IBC, OR FLORIDA BUILDING CODE, THE WIND LOAD REACTIONS ARE AT A STRENGTH VALUE WITH A LOAD FACTOR OF 1.0.
  - FOR IBC CODES, THE SEISMIC REACTIONS PROVIDED ARE AT A STRENGTH LEVEL WITH A LOAD FACTOR OF 1.0, AND DO NOT CONTAIN THE RHO FACTOR.

THE MANUFACTURER DOES NOT PROVIDE "MAXIMUM" LOAD COMBINATION REACTIONS. HOWEVER, THE INDIVIDUAL LOAD REACTIONS PROVIDED MAY BE USED BY THE FOUNDATION ENGINEER TO DETERMINE THE APPLICABLE LOAD COMBINATIONS FOR HIS/HER DESIGN PROCEDURES AND ALLOW FOR AN ECONOMIC FOUNDATION DESIGN.

## WIND BENT REACTIONS

Loc	Line	Col	Line	Wind(k)	Reactions	Seismic(k)	Bolt(k)	Base Plate(n)	Thick
					Horz	Vert	Horz	Length	
F_SW	A	2	2.1	2.9	0.3	0.4	4	0.750	8.000
F_SW	A	3	2.1	2.9	0.3	0.4	4	0.750	8.000

## ENDWALL COLUMN:

Frm	Col	Dead	Collat	Live	Snow	Wind_Left1	Wind_Right1	Wind_Left2	Wind_Right2	Wind Press
Line	Line	Vert	Vert	Vert	Vert	Horz	Horz	Horz	Horz	Horz
1	D	0.2	0.1	1.0	0.5	0.0	-1.2	0.0	-0.7	0.0
1	C	0.7	0.2	4.7	2.5	-2.3	-6.5	-2.3	-6.5	-3.2
1	B	0.7	0.2	4.7	2.5	0.0	-2.2	0.0	-2.2	-3.2
1	A	0.2	0.1	1.0	0.5	0.0	-1.2	0.0	-0.4	0.0

Frm	Col	Wind	Wind_Long1	Wind_Long2	Seis_Left	Seis_Right	Seis_Long	-MIN_SNOW--
Line	Line	Horz	Horz	Horz	Horz	Horz	Horz	Horz
1	D	0.0	0.0	0.0	0.0	0.0	0.0	0.0
1	C	3.5	0.0	-0.4	-0.6	-0.3	0.0	0.0
1	B	3.5	0.0	-0.4	-0.6	0.0	0.0	0.0
1	A	0.0	0.0	-1.2	0.0	0.0	0.0	0.0

Frm	Col	E1UNB_SL_L--	E1UNB_SL_R--
Line	Line	Horz	Horz
1	D	0.0	0.0
1	C	0.0	0.0
1	B	0.0	0.0
1	A	0.0	0.0

Frm	Col	Dead	Collat	Live	Snow	Wind_Left1	Wind_Right1	Wind_Left2	Wind_Right2
Line	Line	Vert	Vert	Vert	Vert	Horz	Horz	Horz	Horz
3	A	0.3	0.1	1.0	0.5	0.0	-1.2	0.0	-0.4
3	B	0.7	0.2	4.7	2.5	-2.3	-6.5	-2.3	-6.5
3	C	0.7	0.2	4.7	2.5	0.0	-2.2	0.0	-2.2
3	D	0.2	0.1	1.0	0.5	0.0	-1.2	0.0	-0.4

Frm	Col	Wind_Press	Wind_Suct	Wind_Long1	Wind_Long2	Seis_Left	Seis_Right	Seis_Long
Line	Line	Horz	Horz	Horz	Horz	Horz	Horz	Horz
3	A	0.0	0.0	0.0	0.0	0.0	0.0	0.0
3	B	-3.2	0.0	3.5	0.0	-0.6	-0.3	0.0
3	C	-3.2	0.0	3.5	0.0	-0.6	0.0	0.0
3	D	-4.2	-2.6	0.0	2.6	0.0	-1.2	0.0

Frm	Col	-MIN_SNOW--	E2UNB_SL_L--	E2UNB_SL_R--
Line	Line	Horz	Horz	Horz
3	A	0.0	0.0	0.0
3	B	0.0	0.0	0.0
3	C	0.0	0.0	0.0
3	D	0.0	0.0	0.0

## BUILDING BRACING REACTIONS

Loc	Line	Col	Line	Reactions in plane of wall	Panel Shear	Note
				Wind	Seismic	
L_EW	1	C.B		Bracing, see EW reactions		(c)
R_SW	3	A	2.3	Bracing, see EW reactions		
R_SW	3	B	0.0	Bracing, see EW reactions		
R_SW	3	C	0.0	Bracing, see EW reactions		

(a) Wind bent in bay

\*See RF reactions table for vertical and horizontal reactions in plane of the rigid frame.

## NOTES FOR REACTIONS

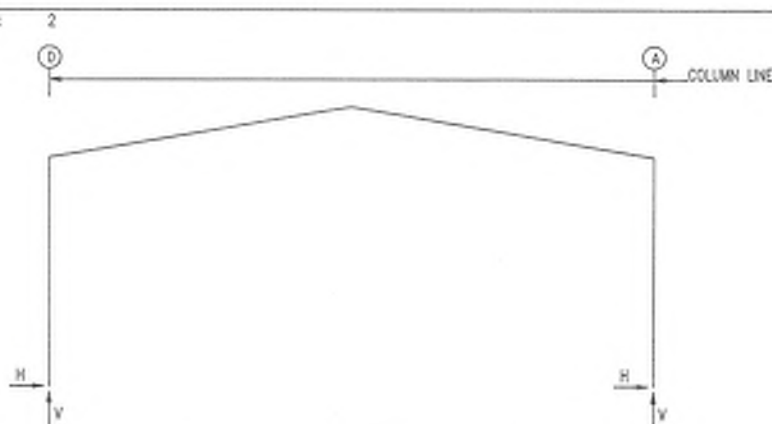
BUILDING REACTIONS ARE BASED ON THE FOLLOWING BUILDING DATA:

WIDTH (FT)	= 45
LENGTH (FT)	= 50
EAVE HEIGHT (FT)	= 17 / 17
ROOF SLOPE (rise/12)	= 2.0/12 / 2.0/12
DEAD LOAD (psf)	= 1.990
COLLATERAL LOAD (psf)	= 1
ROOF LIVE LOAD (psf)	= 20.00
FRAME LIVE LOAD (psf)	= 20
ROOF SNOW LOAD (psf)	= 10.5
GROUND SNOW LOAD (psf)	= 15.00
MINIMUM ROOF SNOW LOAD (psf)	= 15.00
WIND SPEED (MPH)	= 118
WIND CODE	= NCBC 18
EXPOSURE	= C
CLOSED/OPEN	= Closed
IMPORTANCE - WIND	= 1.00
IMPORTANCE - SEISMIC	= 1.00
SEISMIC ZONE	= C

## REACTION KEY:

WIND Left/Right 1 = (with +GCP Internal Pressure)  
 WIND Left/Right 2 = (with -GCP Internal Pressure)  
 Wind\_Long 1 = Wind Load Case B at Left EW  
 Wind\_Long 2 = Wind Load Case B at Right EW  
 MIN\_SNOW = Minimum Snow (Psf) per code  
 E1UNB\_SL\_L = Endwall Unbalanced Snow Left  
 E1UNB\_SL\_R = Endwall Unbalanced Snow Right  
 F1UNB\_SL\_L = Rigid Frame Unbalanced Snow Left  
 F1UNB\_SL\_R = Rigid Frame Unbalanced Snow Right

## FRAME LINES:



## RIGID FRAME:

### ANCHOR BOLTS & BASE PLATES

Frm	Col	Line	Line	Anchor Bolt Qty	Base Plate Width	Base Plate Length	Base Plate Thick	Grout (in)
2	D			4	0.750	6.000	16.50	0.625
2	A			4	0.750	6.000	17.00	0.375

## RIGID FRAME:

### BASIC COLUMN REACTIONS (k)

Frame	Column	Dead	Collateral	Live	Snow	Wind_Left1	Wind_Right1	Wind_Left2	Wind_Right2
Line	Line	Horz	Horz	Horz	Horz	Horz	Horz	Horz	Horz
2	D	0.7	2.0	0.3	0.7	5.7	14.0	3.0	7.4
2	A	-0.7	-0.3	0.7	-5.7	-14.1	-3.0	-7.4	-10.2

Frame	Column	Wind_Left2	Wind_Right2	Wind_Long1	Wind_Long2	Seismic_Left	Seismic_Right
Line	Line	Horz	Horz	Horz	Horz	Horz	Horz
2	D	-9.3	-9.9	0.5	-3.5	-0.2	-16.8
2	A	-0.5	-3.5	9.4	-10.0	1.2	-14.3

Frame	Column	Seismic_Long	-MIN_SNOW--	F1UNB_SL_L--	F1UNB_SL_R--
Line	Line	Horz	Horz	Horz	Horz
2	D	0.0	-0.3	4.3	10.5
2	A	0.0	0.0	-4.3	10.5

## ENDWALL COLUMN:

### ANCHOR BOLTS & BASE PLATES

Frm	Col	Line	Line	Anchor Bolt Qty	Base Plate Width	Base Plate Length	Base Plate Thick	Grout (in)
1	D			2	0.625	7.000	10.00	0.250
1	C			2	0.625	7.000	10.00	0.250
1	B			2	0.625	7.000	10.00	0.250
1	A			2	0.625	7.000	10.00	0.250
3	A			4	0.625	6.000	10.00	0.375
3	B			2	0.625	7.000	10.00	0.250
3	C			2	0.625	7.000	10.00	0.250
3	D			2	0.625	7.000	10.00	0.250

## ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type	Proj (in)
20	Jamb	5/8"	F1554	2.00
18	Endwall	5/8"	F1554	2.00
8	Frame	3/4"	F1554	2.50
8	WindCol	3/4"	F1554	2.50

BUILDING SIZE: 45'-0" x 50'-0" x 17'-0" 2.0:12

ISSUE	DATE	DESCRIPTION	BY	CHK'D	DSN
0	10/25/22	FOR ERECTOR INSTALLATION	MOB	HPD	CM

BUILDING SOLD BY HERITAGE BUILDING SYSTEMS

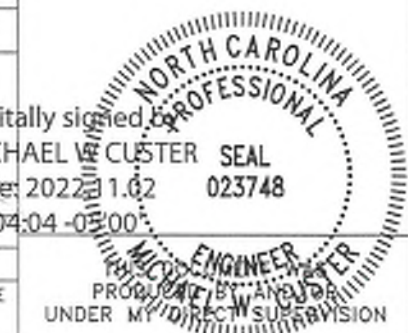
MICHAEL W. CUSTER, P.E.

642 OAKBEND DRIVE  
 COPPEL TX. 75019  
 PH. 972-571-7082

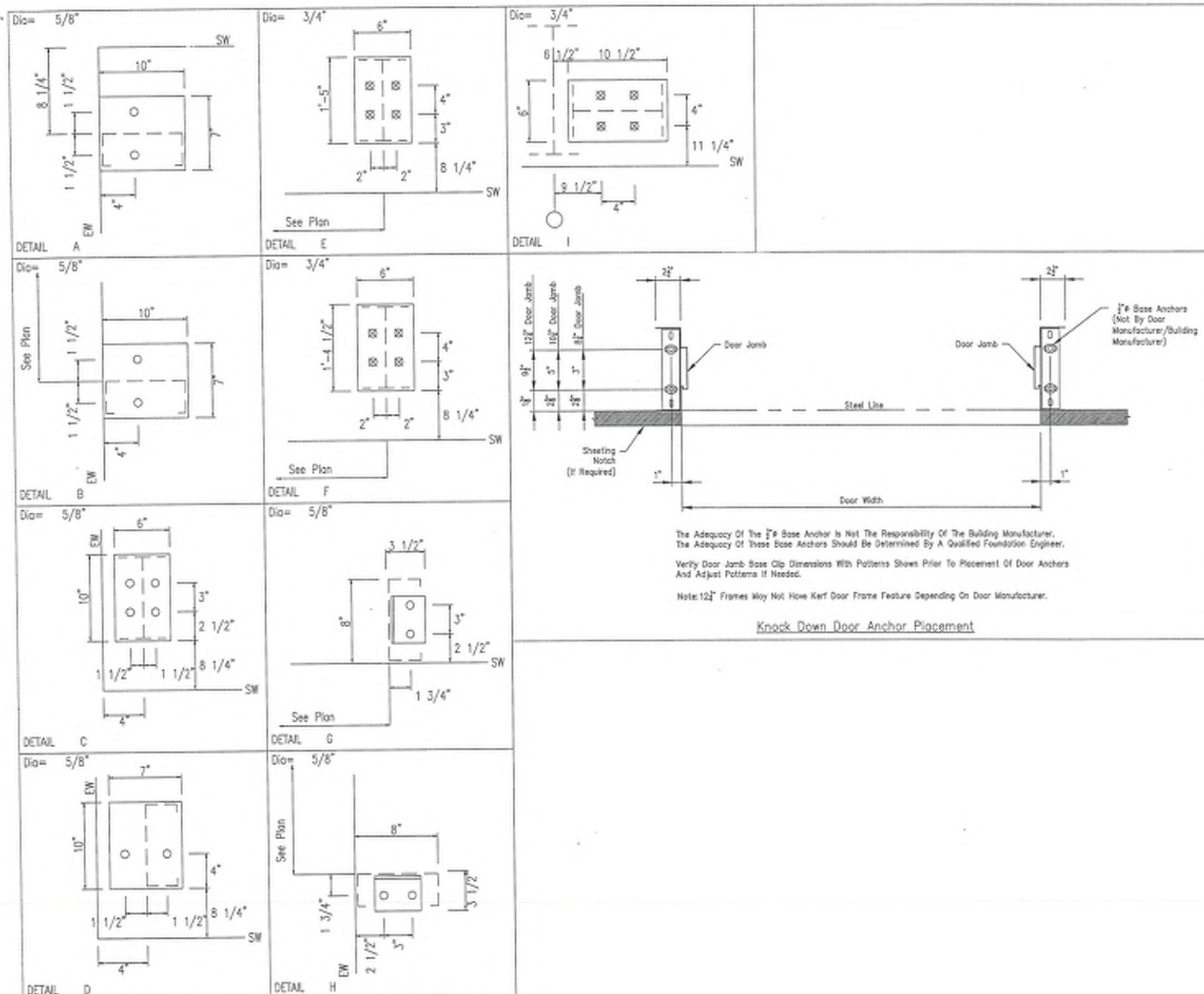
**MICHAEL W CUSTER**

Digitally signed by MICHAEL W CUSTER  
 Date: 2022.11.02 09:04:04 -0500  
 023748

PROJECT: LUIS TRADO - 45X50X17		OWNER: LUIS TRADO		W CUSTER		Date: 09/01/19	
CUSTOMER: LUIS TRADO							
LOCATION: ULLINGTON, NC 27548							
CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	5/24/19	N.T.S.	1	A	19-B-19564	F2	0

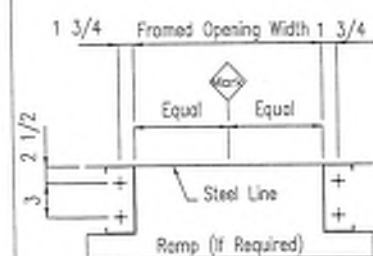






The Adequacy Of The 1" Base Anchor Is Not The Responsibility Of The Building Manufacturer.  
The Adequacy Of These Base Anchors Should Be Determined By A Qualified Foundation Engineer.  
Verify Door Jamb Base Clip Dimensions With Patterns Shown Prior To Placement Of Door Anchors  
And Adjust Patterns If Needed.  
Note: 12" Frames May Not Have Kerf Door Frame Feature Depending On Door Manufacturer.

#### Knock Down Door Anchor Placement



ARDia Framed Opening AR Layout  
5/8"

BUILDING SIZE: 45'-0" x 50'-0" x 17'-0" 2.0:12

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	10/25/22	FOR ERECTOR INSTALLATION	MDR	HPD	CM

BUILDING SOLD BY HERITAGE BUILDING SYSTEMS

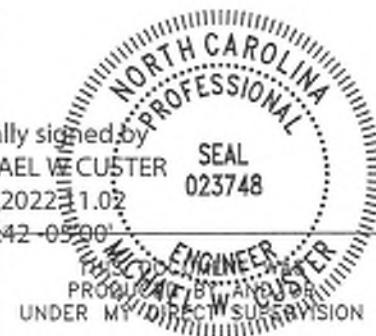
MICHAEL W. CUSTER, P.E.  
642 OAKBEND DRIVE  
COPPEL TX. 75019  
PH. 972-571-7082

MICHAEL W. CUSTER

Digitally signed by  
MICHAEL W. CUSTER  
Date: 2022.11.02  
09:04:42 -0500

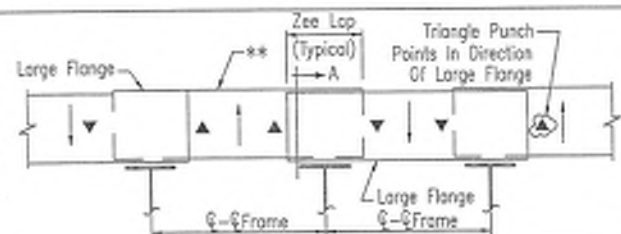
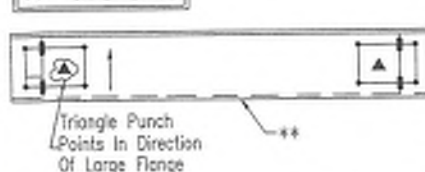
SEAL  
023748

PROJECT: LUIS TIAGO - 45X50X17		OWNER: LUIS TIAGO		W CUSTER		Date	
CUSTOMER: LUIS TIAGO						09:0	
LOCATION: ULLINGTON, NC 27546							
CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	5/24/19	N.T.S.	1	A	19-8-19564	F3	0





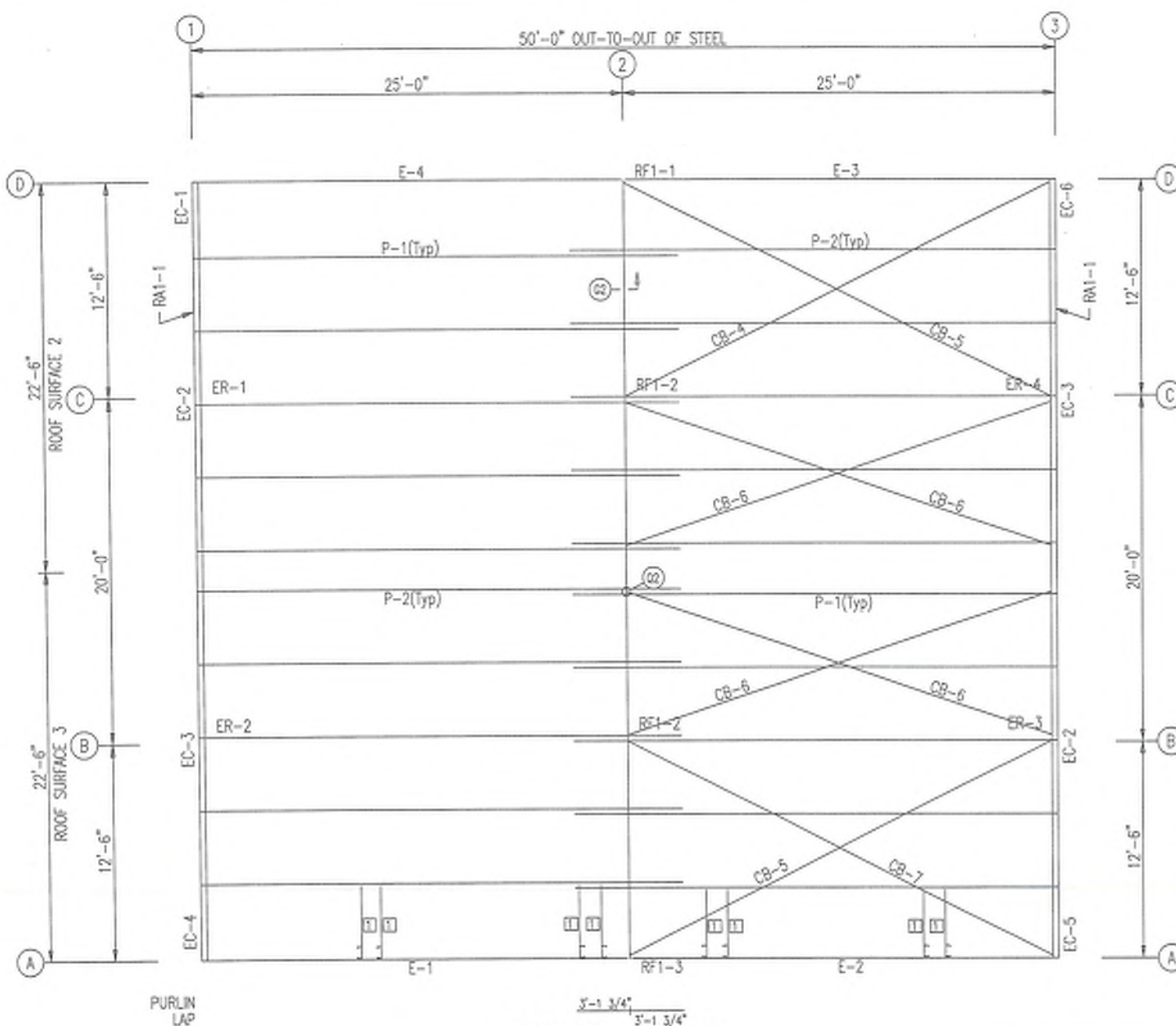
\*\*\* SAME FLANGE



The large leg of the Zee must be alternated from top to bottom in order to nest the member correctly. A triangle has been added to the end of the Zee near the connection holes, that will point to the large leg of the member.

MEMBER TABLE		
MARK	PART	LENGTH
P-1	8X25Z14	28'-1 1/2"
P-2	8X25Z14	28'-1 1/2"
E-1	8ES2L14	24'-11 1/2"
E-2	8ES2L14	24'-11 1/2"
E-3	8ES2L14	24'-11 1/2"
E-4	8ES2L14	24'-11 1/2"
CB-4	1/4" CABLE	28'-2"
CB-5	1/4" CABLE	27'-9"
CB-6	1/4" CABLE	26'-7"
CB-7	1/4" CABLE	27'-5"

CONNECTION PLATES	
MARK	PART
11081	

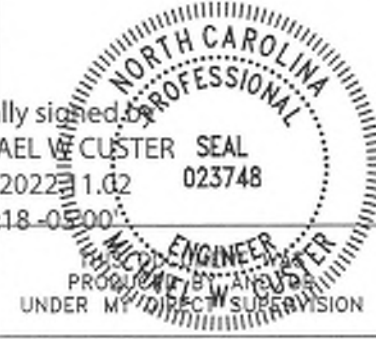


**GENERAL NOTES:**

1. INSTALL ALL PURLIN AND FLANGE BRACES (FB) AS SHOWN.
2. ROOF PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.
3. STRUT PURLINS, IF PROVIDED, MUST BE INSTALLED AND FASTENED TO ROOF SHEETING PER "PBR" PANEL ROOF DETAIL.
4. DO NOT ADD ANY ADDITIONAL ROOF OPENINGS WITHOUT BUILDING MANUFACTURER APPROVAL OR PROFESSIONAL ENGINEER APPROVAL.
5. DO NOT STACK SHEET BUNDLES ON ROOF. ONLY RAISE INDIVIDUAL SHEETS AS NEEDED.
6. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHAVINGS CAUSED BY DRILLING.

ROOF FRAMING PLAN					
BUILDING SIZE: 45'-0" x 50'-0" x 17'-0" 2.0:12					
ISSUE	DATE	DESCRIPTION	BY	CHK'D	DSN
0	10/25/22	FOR ERECTOR INSTALLATION	MOB	HPD	CM

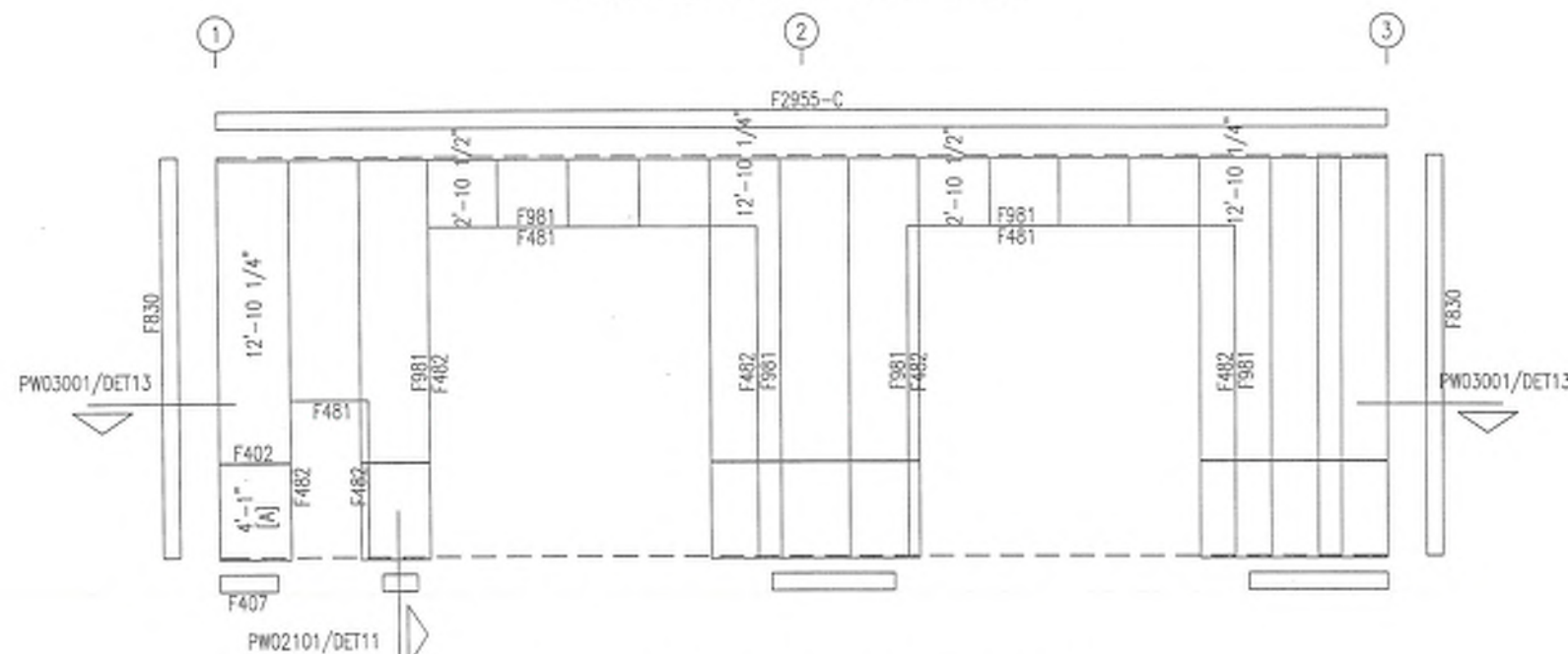
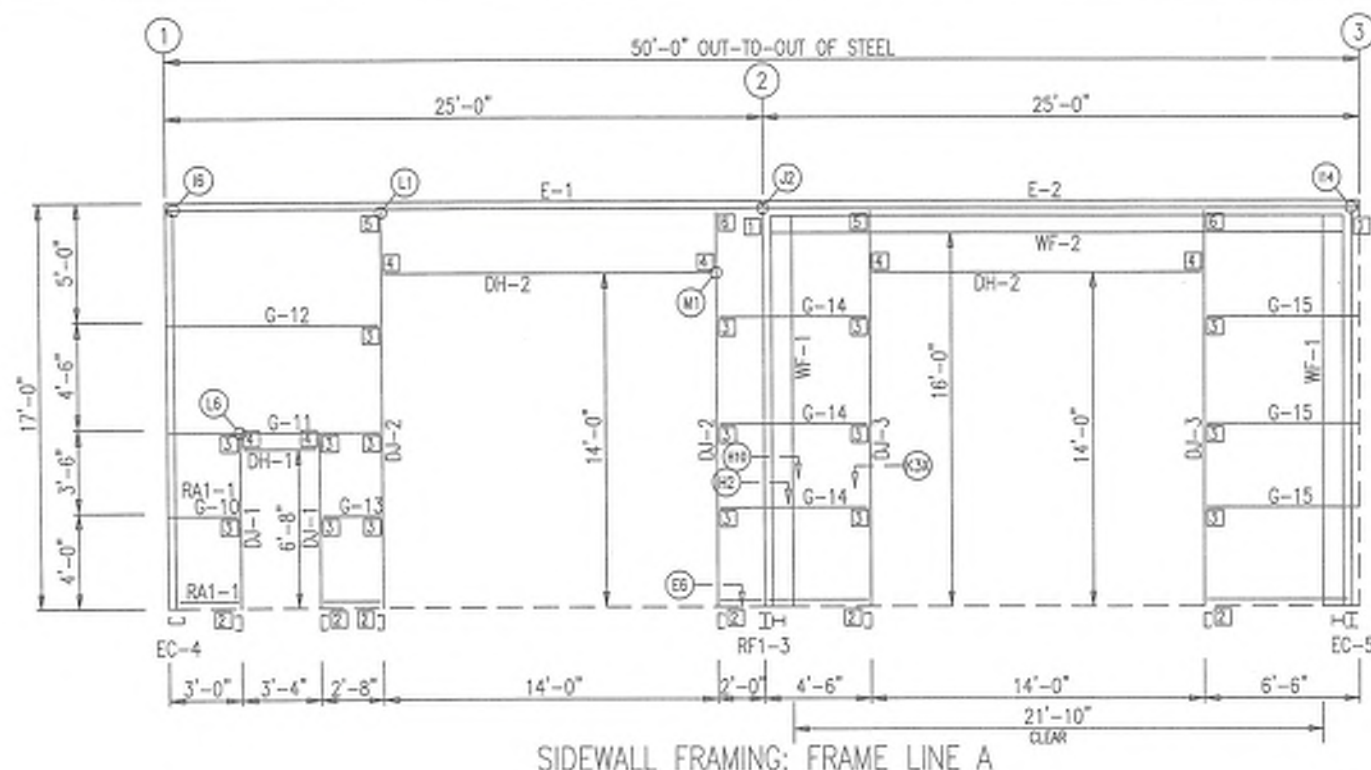
BUILDING SOLD BY HERITAGE BUILDING SYSTEMS					
MICHAEL W. CUSTER, P.E.					
642 OAKBEND DRIVE					
COPPEL TX. 75019					
PH. 972-571-7082					
PROJECT: LUIS TRADO - 45X50X17					
CUSTOMER: LUIS TRADO					
LOCATION: ULLINGTON, NC 27548					
CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER
	5/24/19	N.T.S.	1	A	19-B-19564
				SHEET NUMBER	ISSUE
				E1	0











### SIDEWALL SHEETING & TRIM: FRAME LINE A

PANELS: 26 Gauge PBR - Polar White

[A] PANELS: 26 Gauge PBR - Coal Black

BUILDING SIZE: 45'-0" x 50'-0" x 17'-0" 2.0:12

ISSUE	DATE	DESCRIPTION	BY	CHK'D	DSN
0	10/25/22	FOR ERECTOR INSTALLATION	MDR	HPD	CM

BUILDING SOLD BY HERITAGE BUILDING SYSTEMS

MICHAEL W. CUSTER, P.E.

642 OAKBEND DRIVE

COPPEL TX. 75019

PH. 972-571-7082

PROJECT: LUIS TRADO - 45X50X17

CUSTOMER: LUIS TRADO

OWNER: LUIS TRADO

LOCATION: ULLINGTON, NC 27546

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	5/24/19	N.T.S.	1	A	19-B-19564	E3	0

### GENERAL NOTES:

1. INSTALL ALL GIRTS AND FLANGE BRACES (FB) AS SHOWN.
2. WALL PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.
3. OTHER THAN FOR WALK DOORS AND WINDOWS SHOWN ON THE CONTRACT, DO NOT ADD ADDITIONAL WALL OPENINGS WITHOUT APPROVAL OF BUILDING MANUFACTURER OR PROFESSIONAL ENGINEER.
4. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHAVINGS CAUSED BY DRILLING.

BOLT TABLE				
FRAME LINE A				
LOCATION	QUAN	TYPE	DIA	LENGTH
WF-1 - WF-2	8	A325	3/4"	2"
WF-1 - RF1-3	8	A325	3/4"	1 1/2"
WF-1 - EC-5	8	A325	3/4"	1 1/2"

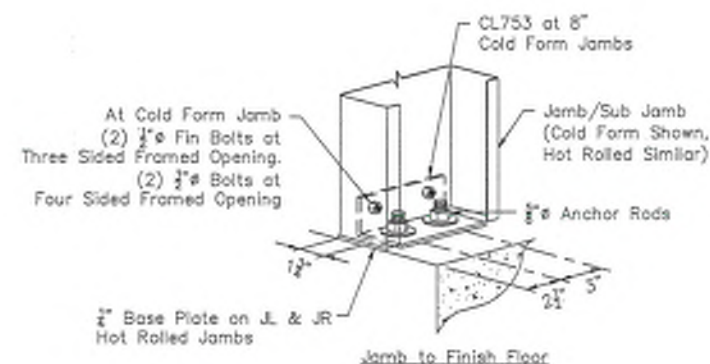
MEMBER TABLE		
FRAME LINE A		
MARK	PART	LENGTH
WF-1	W10x42	16'-8 1/2"
WF-2	W8x42	21'-9 1/2"
DJ-1	8X35C14	7'-1 3/4"
DJ-2	8X35C13	16'-2 11/16"
DJ-3	8X35C14	16'-2 11/16"
DH-1	8X35C14	3'-3 3/4"
DH-2	8X35C14	13'-11 3/4"
E-1	8ES2L14	24'-11 1/2"
E-2	8ES2L14	24'-11 1/2"
G-10	8X25Z16	2'-7 3/4"
G-11	8X25Z16	8'-7 3/4"
G-12	8X25Z16	8'-7 3/4"
G-13	8X25Z16	2'-0"
G-14	8X25Z16	5'-10"
G-15	8X25Z16	6'-1 3/4"

### CONNECTION PLATES

FRAME LINE A

ID MARK/PART

1	SC479
2	CL753
3	CL751
4	SC425
5	SC585L
6	SC585R









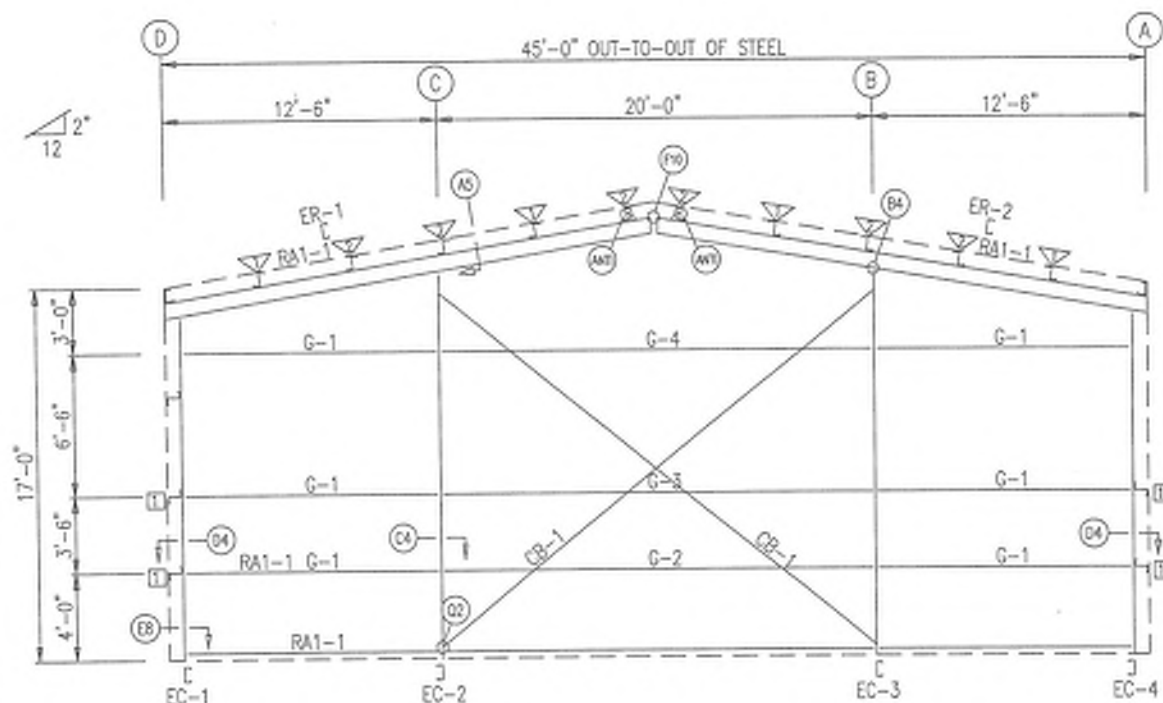
BEARING FRAME ONLY  
WASHER TO BE USED AT ENDWALL COLUMN TO ENDWALL  
RAFTER CONNECTION. USE ONE WASHER ON COLUMN SIDE.  
WASHER NOT NEEDED ON CLIP SIDE.

BOLT TABLE				
FRAME LINE 1				
LOCATION	QUAN	TYPE	ØA	LENGTH
ER-1/ER-2	4	A325	5/8"	1 3/4"
Columns/Raft	4	A325	1/2"	1 1/4"

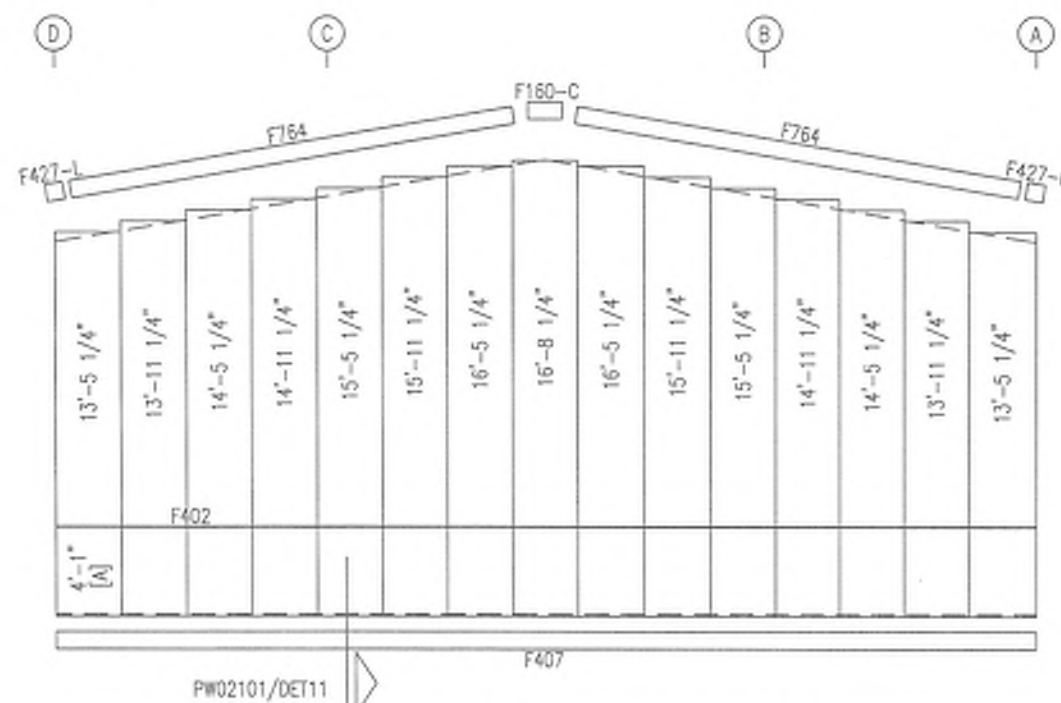
MEMBER TABLE		
FRAME LINE 1		
MARK	PART	LENGTH
EC-1	10F25C14	15'-5 7/8"
EC-2	10F35C13	17'-5 1/2"
EC-3	10F35C13	17'-5 1/2"
EC-4	10F25C14	15'-5 7/8"
ER-1	10F35C13	22'-11 1/8"
ER-2	10F35C13	22'-11 1/8"
G-1	8X25Z16	11'-1 3/4"
G-2	8X25Z16	19'-4"
G-3	8X25Z14	19'-4"
G-4	8X35Z13	19'-4"
CB-1	1/4" CABLE	76'-4"

FLANGE BRACE TABLE		
FRAME LINE 1		
ID	PART	LENGTH
1	FB30	L2X2X14G 2'-6"
2	FB7-1	L2.5X2.5X3/16 2'-6"

CONNECTION PLATES		
FRAME LINE D		
ID	MARK/PART	SCS
1	SC5	

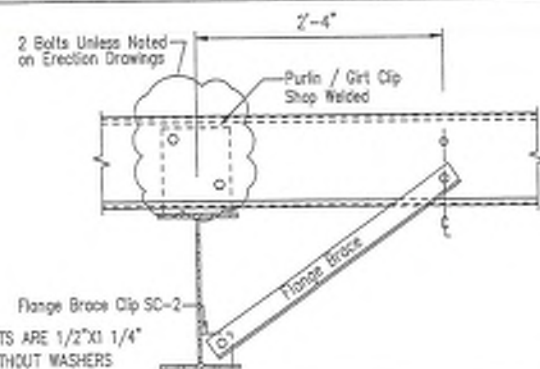


ENDWALL FRAMING: FRAME LINE 1



ENDWALL SHEETING & TRIM: FRAME LINE 1

PANELS: 26 Gauge PBR - Polar White  
[A] PANELS: 26 Gauge PBR - Cool Black



#### GENERAL NOTES:

1. INSTALL ALL GIRTS AND FLANGE BRACES (FB) AS SHOWN.
2. WALL PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.
3. OTHER THAN FOR WALK DOORS AND WINDOWS SHOWN ON THE CONTRACT, DO NOT ADD ADDITIONAL WALL OPENINGS WITHOUT APPROVAL OF BUILDING MANUFACTURER OR PROFESSIONAL ENGINEER.
4. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHAVINGS CAUSED BY DRILLING.

BUILDING SIZE: 45'-0" x 50'-0" x 17'-0" 2.0:12

ISSUE	DATE	DESCRIPTION	BY	CHK'D	DSN
0	10/25/22	FOR ERECTOR INSTALLATION	MDR	HPD	CM

BUILDING SOLD BY HERITAGE BUILDING SYSTEMS

MICHAEL W. CUSTER, P.E.  
642 OAKBEND DRIVE  
COPPEL TX. 75019  
PH. 972-571-7082

MICHAEL W. CUSTER

Digitally signed by MICHAEL W. CUSTER  
Date: 2022.10.25 09:06:45 -0500  
SEAL 023748

PROJECT:	LUIS TIRADO - 45X50X17		OWNER:	LUIS TIRADO	
CUSTOMER:	LUIS TIRADO		LOCATION:	LILLINGTON, NC 27546	
CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER
	5/24/19	N.T.S.	1	A	19-B-19564
					SHEET NUMBER
					ES
					ISSUE
					0

NORTH CAROLINA  
PROFESSIONAL  
ENGINEER  
MICHAEL W. CUSTER  
UNDER MY DIRECT SUPERVISION



## BEARING FRAME ONLY:

WASHER TO BE USED AT ENDWALL COLUMN TO ENDWALL  
RAFTER CONNECTION. USE ONE WASHER ON COLUMN SIDE.  
WASHER NOT NEEDED ON CLIP SIDE.

## BOLT TABLE

LOCATION	QUAN	TYPE	DIA	LENGTH
EC-5/ER-3	4	A325	5/8"	1 1/4"
ER-3/ER-4	4	A325	5/8"	1 3/4"
Int. Column/Ref	4	A325	1/2"	1 1/4"
EC-5/ER-4	4	A325	5/8"	1 1/2"

## MEMBER TABLE

MARK	PART	LENGTH
EC-2	10F35C13	17'-5 1/2"
EC-3	10F35C13	17'-5 1/2"
EC-5	W10X12	16'-6 5/8"
EC-6	10F35C14	15'-8 9/16"
ER-3	10F35C13	21'-5"
ER-4	10F35C13	22'-11 1/8"
DJ-1	8X35C14	7'-1 3/4"
DH-1	8X35C14	3'-3 3/4"
G-2	8X25Z16	19'-4"
G-3	8X25Z14	19'-4"
G-4	8X35Z13	19'-4"
G-5	8X25Z16	4'-1 5/8"
G-6	8X25Z16	10'-7 5/8"
G-7	8X25Z16	10'-7 5/8"
G-8	8X25Z16	2'-6"
G-9	8X25Z16	10'-7 1/2"
CB-1	1/4" CABLE	28'-4"

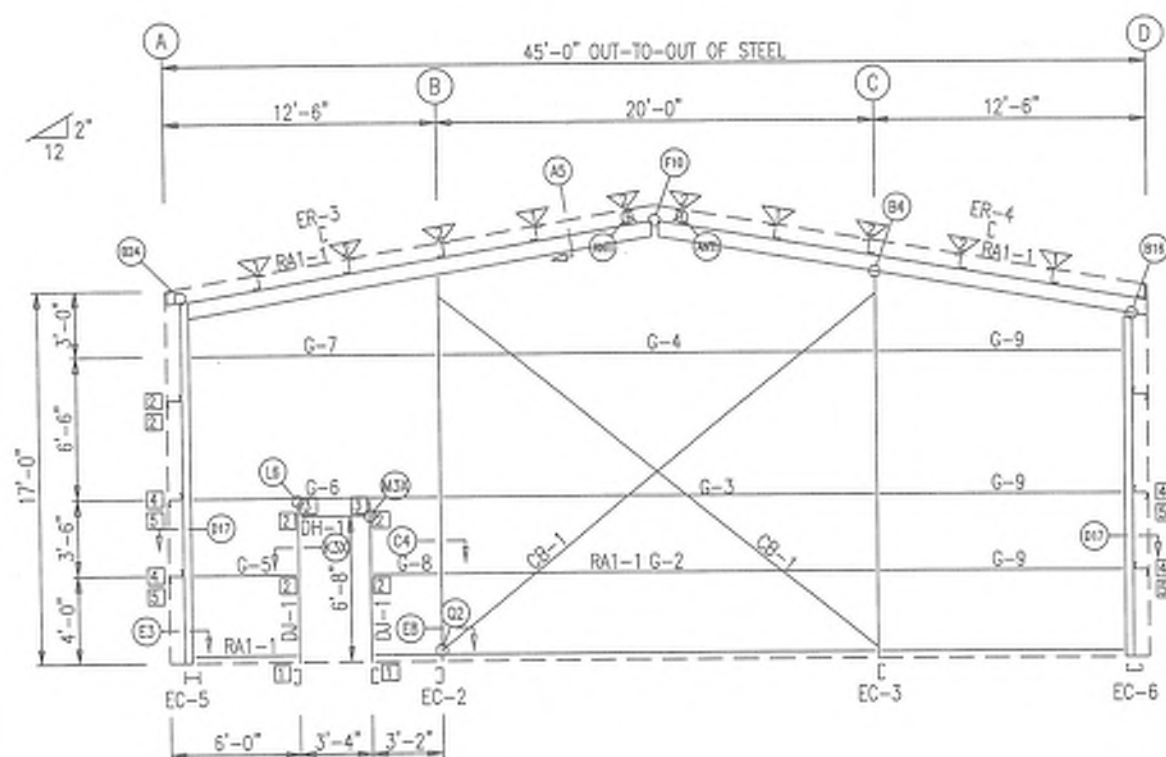
## FLANGE BRACE TABLE

ID	PART	LENGTH
1	FB30	L2X2X140
2	FB7-1	L2.5X2.5X3/16

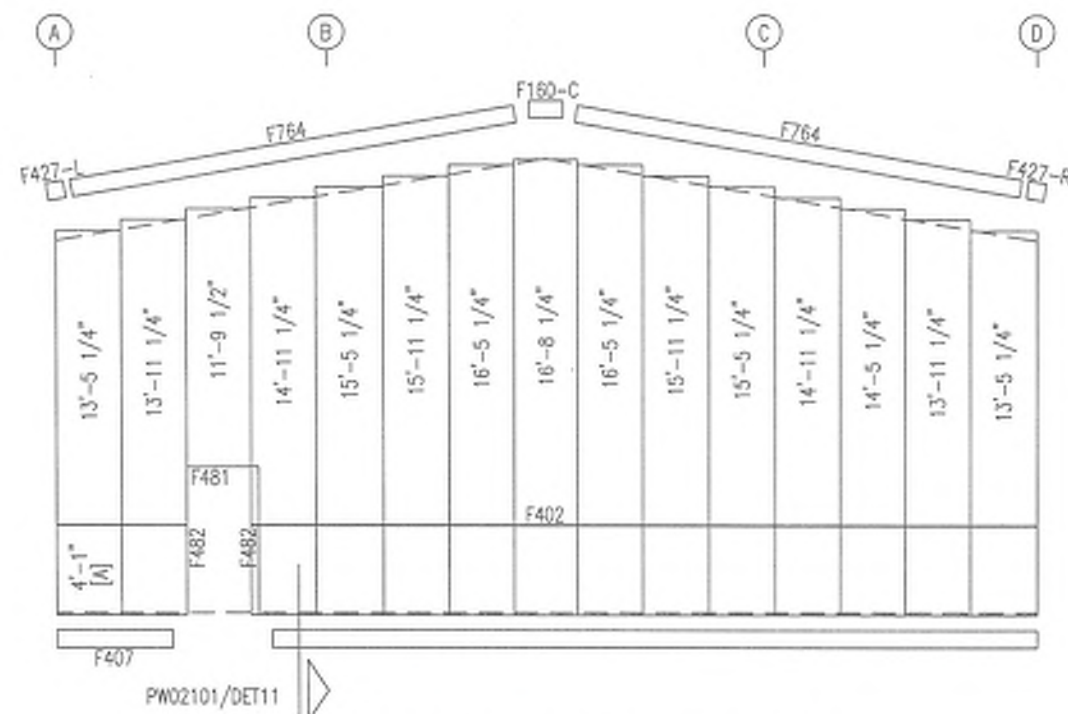
## CONNECTION PLATES

## FRAME LINE 3

ID	MARK/PART
1	CL753
2	CL751
3	SC425
4	SCS
5	PC30



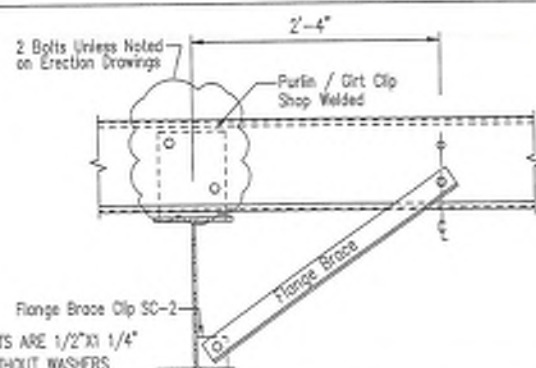
ENDWALL FRAMING: FRAME LINE 3



ENDWALL SHEETING &amp; TRIM: FRAME LINE 3

PANELS: 26 Gauge PBR - Polar White

[A] PANELS: 26 Gauge PBR - Cool Black



ALL BOLTS ARE 1/2"X1 1/4"  
A325 WITHOUT WASHERS

## GENERAL NOTES:

1. INSTALL ALL GIRTS AND FLANGE BRACES (FB) AS SHOWN.
2. WALL PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.
3. OTHER THAN FOR WALK DOORS AND WINDOWS SHOWN ON THE CONTRACT, DO NOT ADD ADDITIONAL WALL OPENINGS WITHOUT APPROVAL OF BUILDING MANUFACTURER OR PROFESSIONAL ENGINEER.
4. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHAVINGS CAUSED BY DRILLING.

BUILDING SIZE: 45'-0" x 50'-0" x 17'-0" 2.0:12

BUILDING SOLD BY HERITAGE BUILDING SYSTEMS

MICHAEL W. CUSTER, P.E.

642 OAKBEND DRIVE  
COPPEL TX. 75019  
PH. 972-571-7082

**MICHAEL W. CUSTER**  
P.E.

Digitally signed by  
MICHAEL W. CUSTER SEAL  
Date: 2022.11.02 023748

09:06:58 -05'00'

**NORTH CAROLINA**  
PROFESSIONAL  
ENGINEER  
MICHAEL W. CUSTER  
UNDER MY DIRECT SUPERVISION

ISSUE	DATE	DESCRIPTION	BY	CHK'D	DSN
0	10/25/22	FOR ERECTOR INSTALLATION	MDR	HPD	CM

PROJECT: LUIS TIRODO - 45X50X17

CUSTOMER: LUIS TIRODO

OWNER: LUIS TIRODO

LOCATION: LILLINGTON, NC 27546

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	5/24/19	N.T.S.	1	A	19-B-19564	EB	0



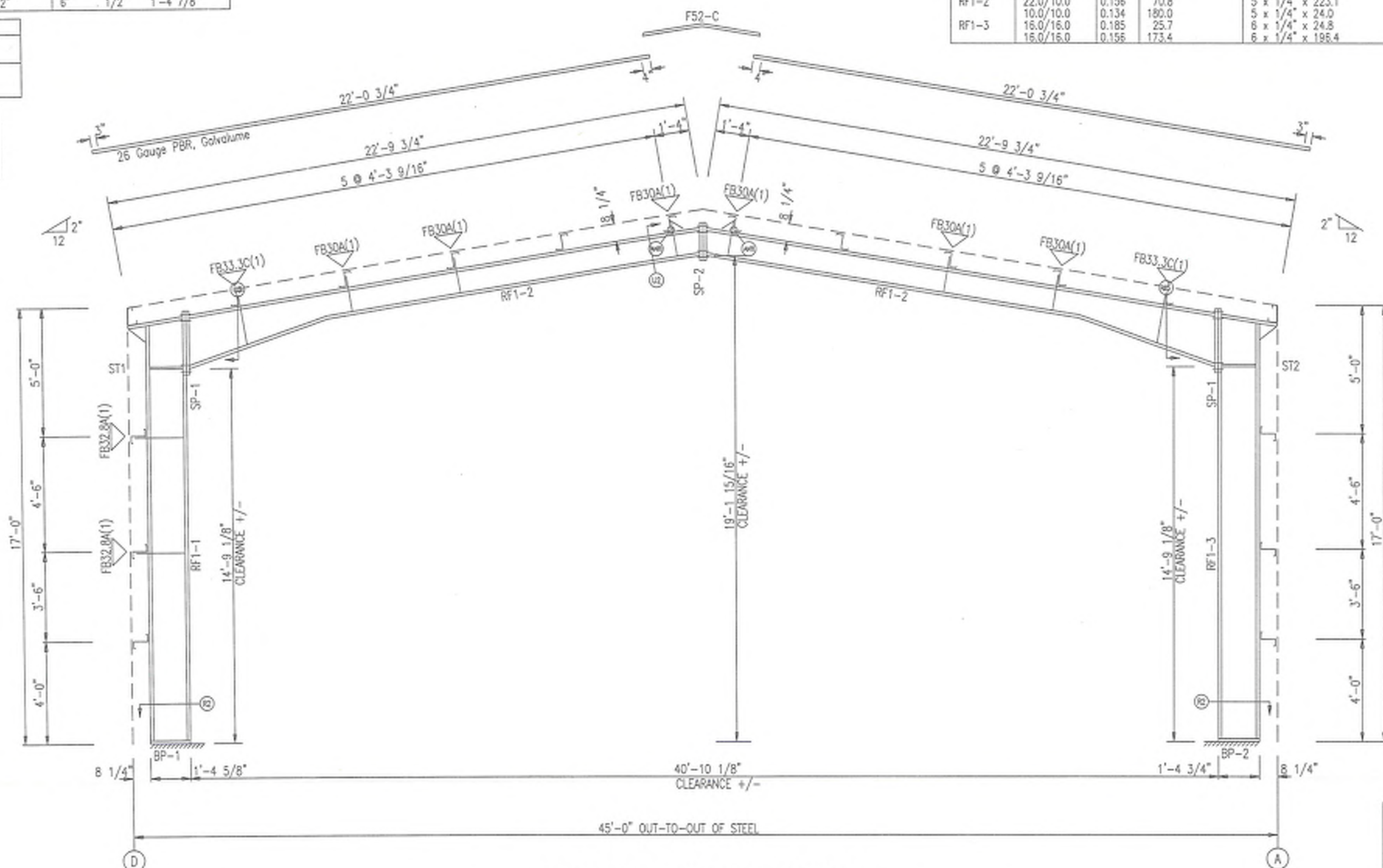
SPLICE PLATE & BOLT TABLE									
Mark	Qty	Top	Bot	Int	Type	Dia	Length	Width	Thick
SP-1	4	4	0	0	A325	3/4"	2' 1/4"	6"	5/8"
SP-2	4	4	0	0	A325	3/4"	2"	6"	1/2"

STIFFENER TABLE				
Mark	Stiff	Mark	Plate Size	Length
RF1-1	ST1		2' 1/2" x 1/4"	16"
RF1-3	ST2		2' 1/2" x 1/4"	16"

BASE PLATE TABLE			
Col	Mark	Plate Size	Length
BP-1		6" x 5/8"	1'-4 1/2"
BP-2		6" x 3/8"	1'-5"

FLANGE BRACES: FBxx (1 or 2)  
 xx=length(in)  
 (1) One Side; (2) Two Sides  
 A - L2X2X1/4  
 C - L2X2X1/8

MEMBER TABLE									
Mark	Web Depth	Start/End	Web Plate	Thick	Length	Outside Flange	W x Thk x Length	Inside Flange	W x Thk x Length
RF1-1	16.0/16.0	0.156	173.1	6	1/4" x 196.1	6 x 1/4" x 120.0		6 x 1/4" x 120.0	
RF1-2	16.0/16.0	0.185	25.7	6	1/4" x 24.8	6 x 3/8" x 53.1		6 x 3/8" x 53.1	
RF1-3	10.0/10.0	0.134	180.0	5	1/4" x 24.0	5 x 1/4" x 71.8		5 x 1/4" x 71.8	
	16.0/16.0	0.185	25.7	6	1/4" x 24.8	5 x 1/4" x 178.3		5 x 1/4" x 178.3	
	16.0/16.0	0.156	173.4	6	1/4" x 196.4	6 x 1/2" x 53.4		6 x 1/2" x 53.4	



RIGID FRAME ELEVATION: FRAME LINE 2

BUILDING SIZE: 45'-0" x 50'-0" x 17'-0" 2.0:12

BUILDING SOLD BY HERITAGE BUILDING SYSTEMS

# GENERAL NOTES:

- BOLTED JOINTS WITH A325 TYPE 1 BOLTS GREATER THAN 1/2" DIAMETER ARE SPECIFIED AS PRE-TENSIONED JOINTS IN ACCORDANCE WITH THE MOST RECENT EDITION OF THE RSC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS. PRE-TENSIONING CAN BE ACCOMPLISHED BY USING THE TURN-OF-NUT METHOD OF TIGHTENING, CALIBRATED WRENCH, TWIST-OFF-TYPE TENSION-CONTROL BOLTS OR DIRECT-TENSION-INDICATOR AS ACCEPTABLE TO THE INSPECTING AGENCY AND BUILDING OFFICIAL. INSTALLATION INSPECTION REQUIREMENTS FOR PRE-TENSIONED JOINTS (SPECIFICATION FOR STRUCTURAL JOINTS SECTION 9.2) USING TURN-OF-NUT/CALIBRATED WRENCH/TWIST OFF TYPE TENSION CONTROL BOLTS/DIRECT TENSION INDICATOR METHOD IS SUGGESTED. THE CONNECTIONS ON THIS PROJECT ARE NOT SLIP CRITICAL.
- ALL FIELD CONNECTIONS OF SECONDARY FRAMING SHALL BE BOLTED WITH A325 BOLTS.
- INSTALL ALL FLANGE BRACES ON COLUMN AND RAFTER AS SHOWN.

ISSUE	DATE	DESCRIPTION	BY	CHK'D	DSN
0	10/25/22	FOR ERECTOR INSTALLATION	MDR	HPD	DN

MICHAEL W. CUSTER, P.E.

642 OAKBEND DRIVE  
 COPPEL TX. 75019  
 PH. 972-571-7082

**MICHAEL W CUSTER**

Digitally signed by  
 MICHAEL W. CUSTER

Date: 2022.10.02  
 09:07:11 -05'00'



PROJECT: LUIS TRADO - 45X50X17

Date: 09:07

CUSTOMER: LUIS TRADO

OWNER: LUIS TRADO

LOCATION: ULLINGTON, NC 27546

W CUSTER

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	5/24/19	N.T.S.	1	A	19-B-19564	E7	0

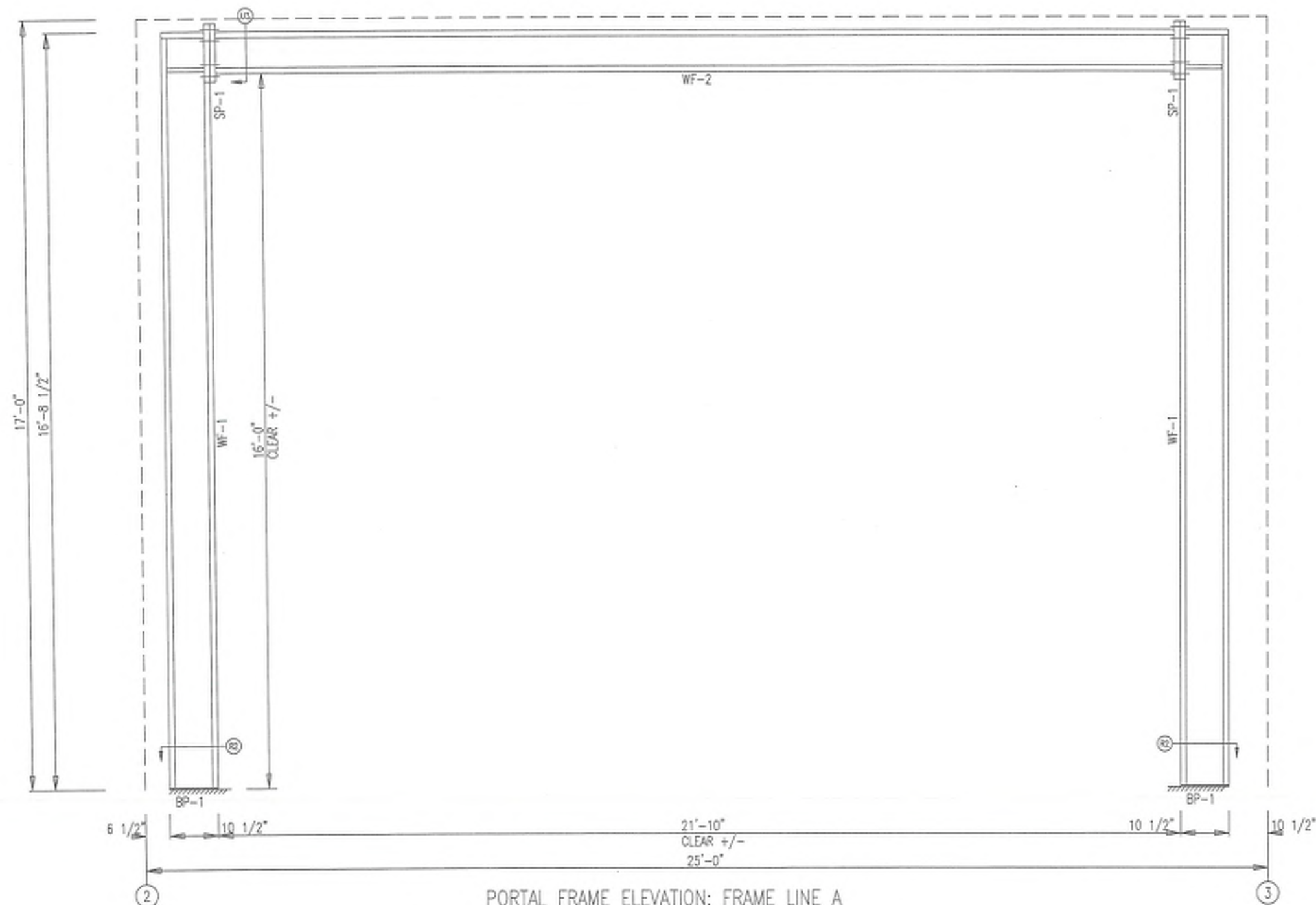


SPICE PLATES & BOLTS									
Splice Mark	Quan	Top/ Bot	Type	Size	Length	Plate Size	Width	Thick	Length
SP-1	4	4	A325	3/4"	2"	6"	1 1/2"	1'-4 1/4"	

STIFFENER TABLE				
Mark	Stiff Mark	Width	Plate Size	Length
WF-1	St-1	2"	5/16	10"

BASE PLATES			
Col Mark	Width	Plate Size	Length
BP-1	6"	3/8"	10 1/2"

MEMBER SIZE TABLE (in)						
MARK	LENGTH	WEB DEPTH	WEB PLATE	OUTSIDE FLANGE	INSIDE FLANGE	
		START/END	THICK	LENGTH	W x T x LENGTH	W x T x LENGTH
WF-1	200.5	10.0/10.0	0.156	18'-8 1/2"	5 x 1/4" x 18'-8 1/2"	5 x 1/4" x 18'-8 1/2"
WF-2	261.5	8.0/ 8.0	0.156	21'-9 1/2"	6 x 1/4" x 21'-9 1/2"	6 x 1/4" x 21'-9 1/2"



BUILDING SIZE: 45'-0" x 50'-0" x 17'-0" 2.0:12

BUILDING SOLD BY HERITAGE BUILDING SYSTEMS

#### GENERAL NOTES:

- BOLTED JOINTS WITH A325 TYPE 1 BOLTS GREATER THAN 1/2" DIAMETER ARE SPECIFIED AS PRE-TENSIONED JOINTS IN ACCORDANCE WITH THE MOST RECENT EDITION OF THE RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS. PRE-TENSIONING CAN BE ACCOMPLISHED BY USING THE TURN-OF-NUT METHOD OF TIGHTENING, CALIBRATED WRENCH, TWIST-OFF-TYPE TENSION-CONTROL BOLTS OR DIRECT-TENSION-INDICATOR AS ACCEPTABLE TO THE INSPECTING AGENCY AND BUILDING OFFICIAL. INSTALLATION INSPECTION REQUIREMENTS FOR PRE-TENSIONED JOINTS (SPECIFICATION FOR STRUCTURAL JOINTS SECTION 9.2) USING TURN-OF-NUT/CALIBRATED WRENCH/TWIST OFF TYPE TENSION CONTROL BOLTS/DIRECT TENSION INDICATOR METHOD IS SUGGESTED. THE CONNECTIONS ON THIS PROJECT ARE NOT SLIP CRITICAL.
- ALL FIELD CONNECTIONS OF SECONDARY FRAMING SHALL BE BOLTED WITH A325 BOLTS.
- INSTALL ALL FLANGE BRACES ON COLUMN AND RAFTER AS SHOWN.

ISSUE	DATE	DESCRIPTION	BY	CHK'D	DSN
0	10/25/22	FOR ERECTOR INSTALLATION	MDR	HPD	CM

MICHAEL W. CUSTER, P.E.

642 OAKBEND DRIVE  
COPPEL TX. 75019  
PH. 972-571-7082

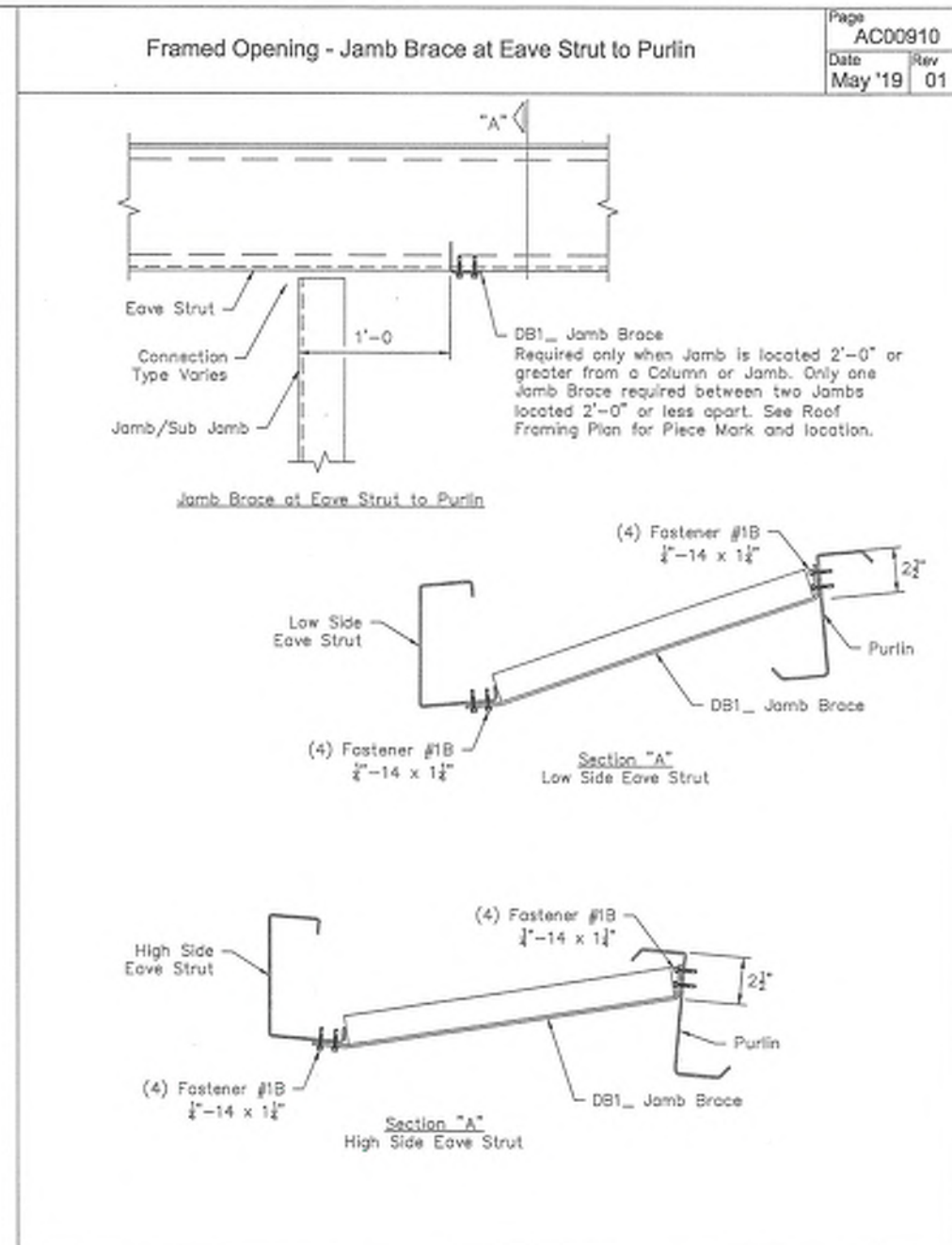
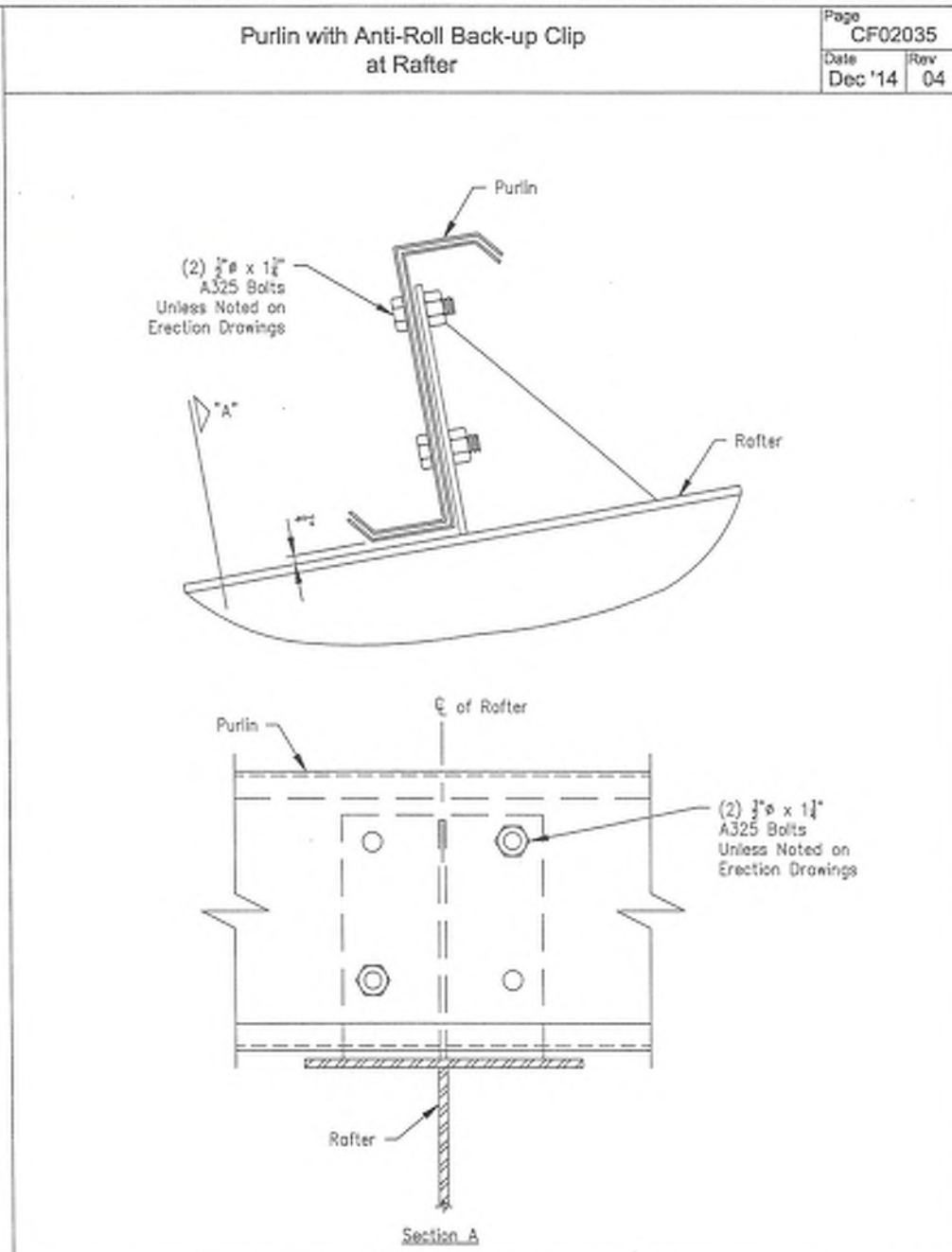
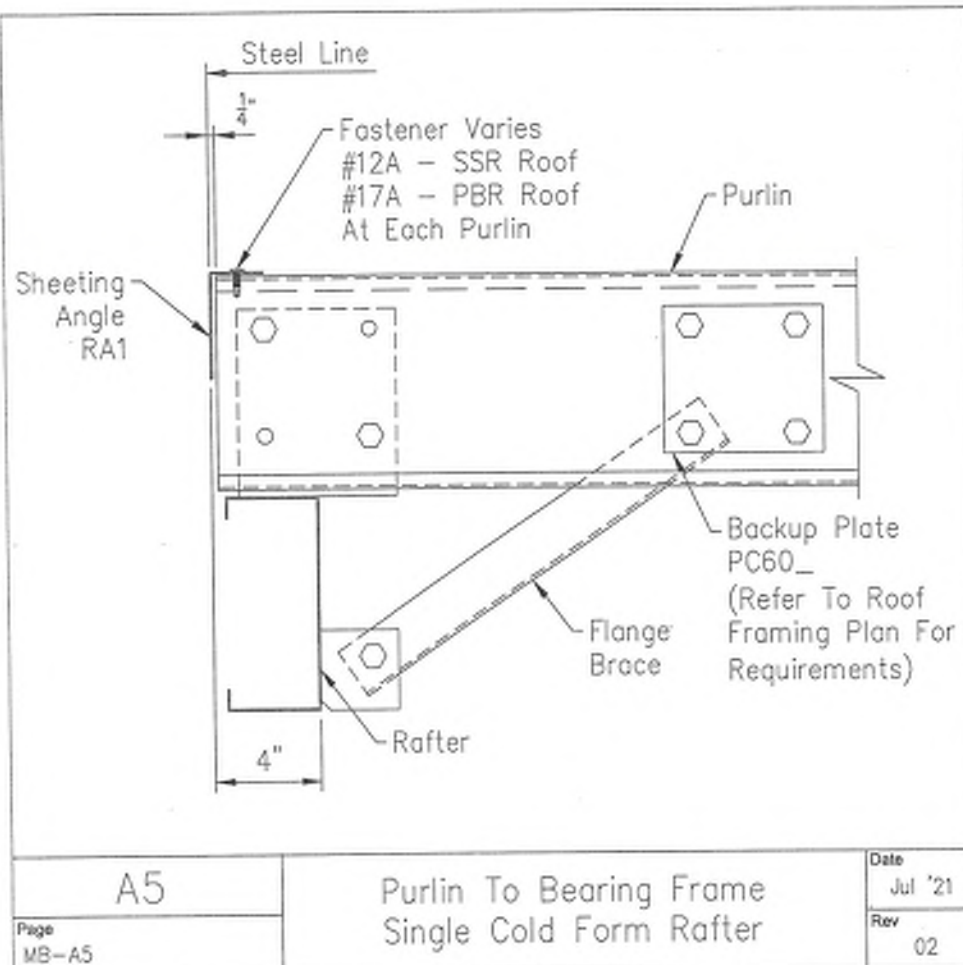
**MICHAEL W CUSTER**

Digitally signed by  
MICHAEL W CUSTER  
Date: 2022.10.24 09:07:24 -0500  
SEAL  
023748

PROJECT: LUIS TIRODO - 45X50X17		W CUSTER		Date			
CUSTOMER: LUIS TIRODO				09:0			
LOCATION: LELINGTON, NC 27546		OWNER: LUIS TIRODO					
CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	5/24/19	N.T.S.	1	A	19-B-19564	EB	0







BUILDING SIZE: 45'-0" x 50'-0" x 17'-0" 2.0:12

ISSUE	DATE	DESCRIPTION	BY	CHK'D	DSN
0	10/25/22	FOR ERECTOR INSTALLATION	MOB	HPD	CM

BUILDING SOLD BY HERITAGE BUILDING SYSTEMS

MICHAEL W. CUSTER, P.E.  
642 OAKBEND DRIVE  
COPPEL TX. 75019  
PH. 972-571-7082

PROJECT: LUIS TIRADO - 45X50X17

CUSTOMER: LUIS TIRADO

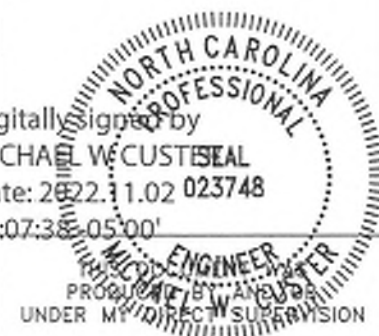
OWNER: LUIS TIRADO

LOCATION: LILLINGTON, NC 27546

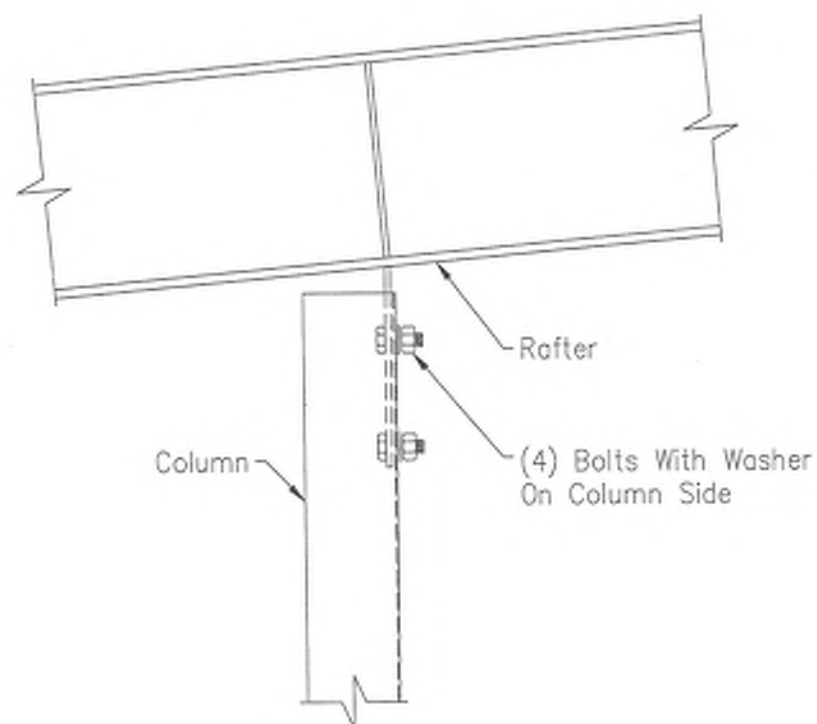
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	5/24/19	N.T.S.	1	A	19-B-19564	DET1	0

MICHAEL W. CUSTER

Digitally signed by  
MICHAEL W. CUSTER  
Date: 2022.11.02 02:37:48  
09:07:38 -05'00'







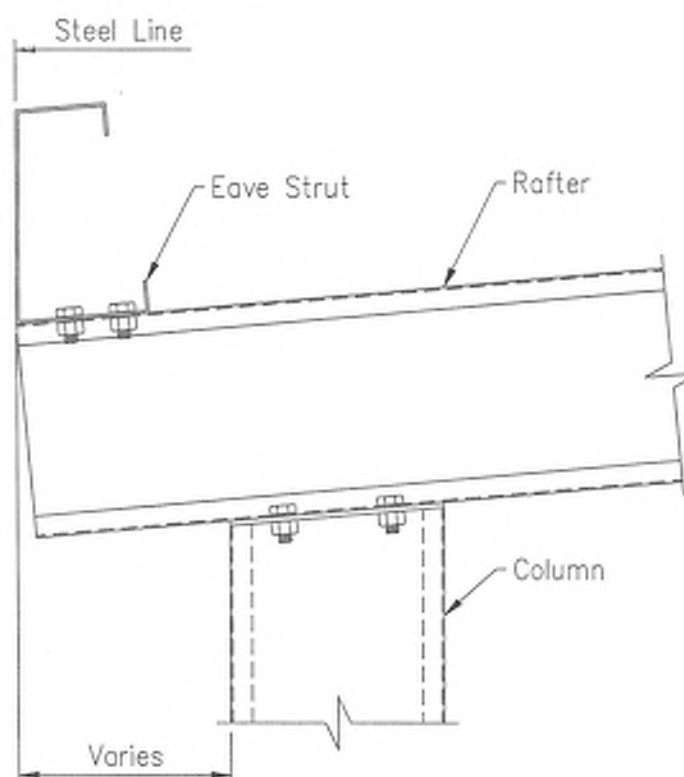
B4

Cold Form Endwall  
Column To Rafter

Date  
Aug '20

Rev  
01

Page  
MB-B4



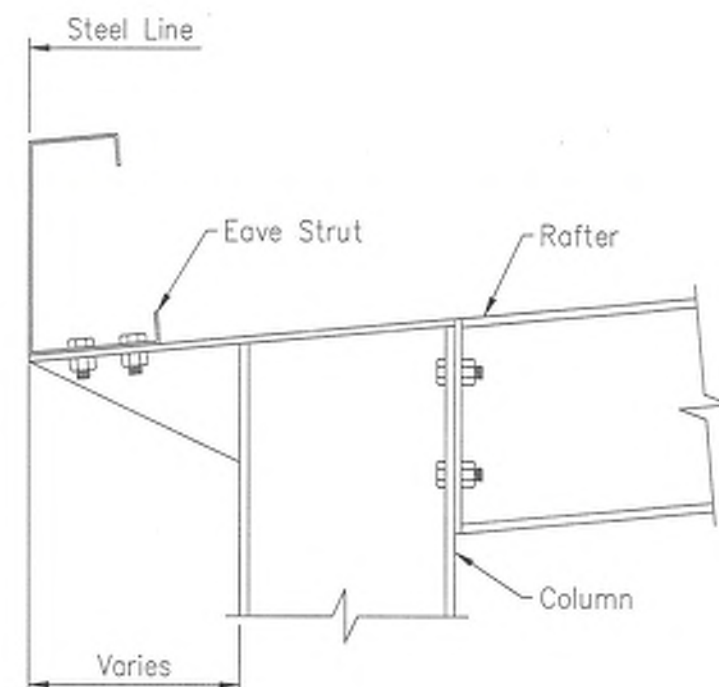
B16

Corner Column To Bearing Frame  
Single Cold Form Rafter

Date  
Jun '17

Rev  
00

Page  
MB-B16



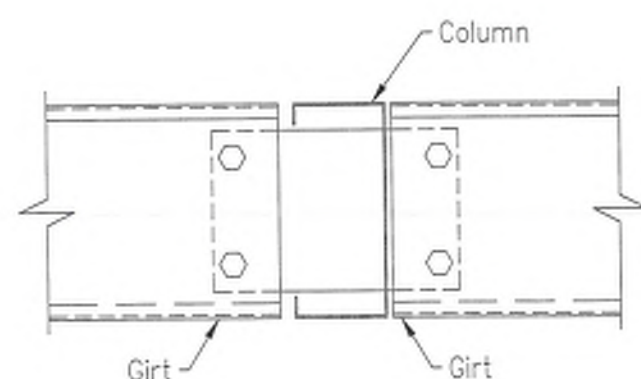
B24

Corner Column To  
Bearing Frame Rafter

Date  
Jun '17

Rev  
00

Page  
MB-B24



C4

Girt To Cold Form Column

Date  
Jun '17

Rev  
00

Page  
MB-C4

BUILDING SIZE: 45'-0" x 50'-0" x 17'-0" 2.0:12

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	10/25/22	FOR ERECTOR INSTALLATION	MDG	HPD	CM

BUILDING SOLD BY HERITAGE BUILDING SYSTEMS

MICHAEL W. CUSTER, P.E.  
642 OAKBEND DRIVE  
COPPEL TX. 75019  
PH. 972-571-7082

**MICHAEL  
W CUSTER**

Digitally signed by  
MICHAEL W CUSTER SEAL  
Date: 2022.11.02 02:3748  
09:07:51 -05'00'

PROJECT: LUIS TIRADO - 45X50X17

CUSTOMER: LUIS TIRADO

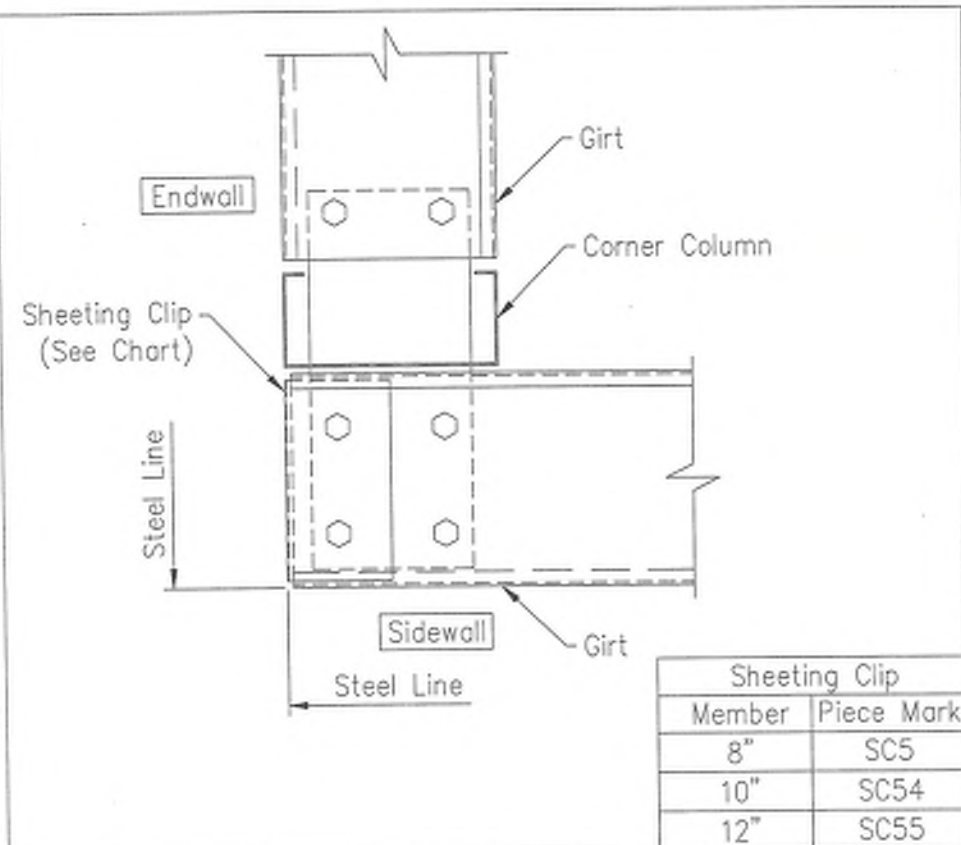
OWNER: LUIS TIRADO

LOCATION: ULLINGTON, NC 27548

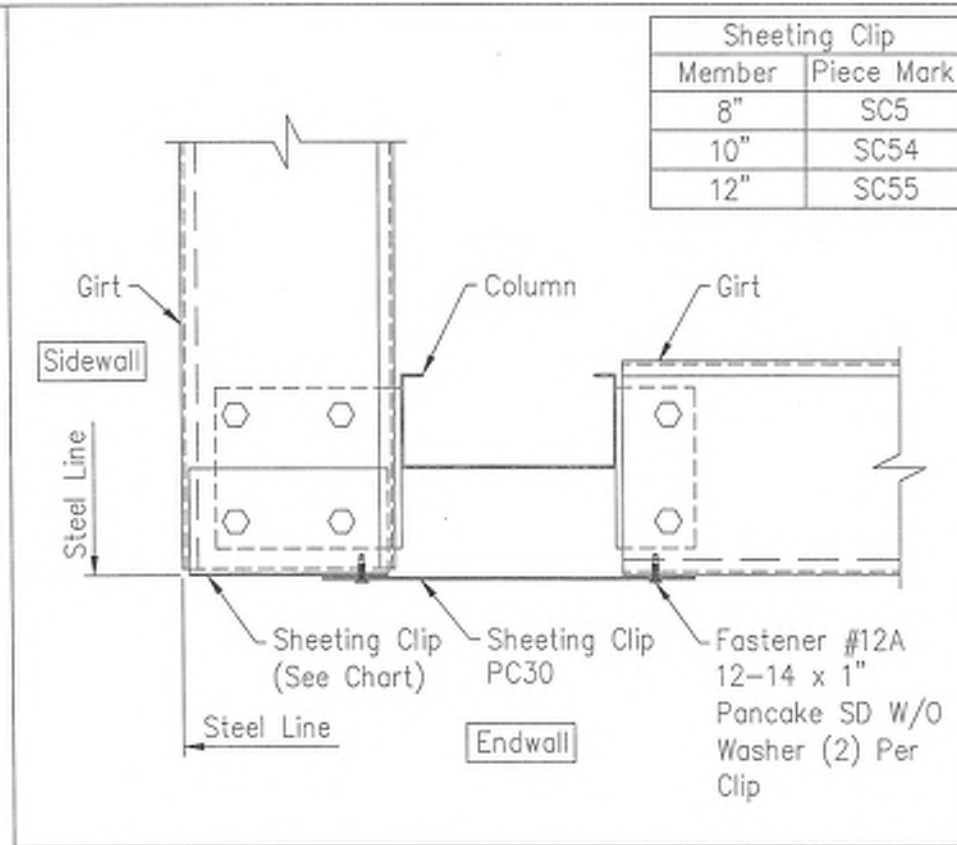
CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	5/24/19	N.T.S.	1	A	19-B-19564	DET2	0



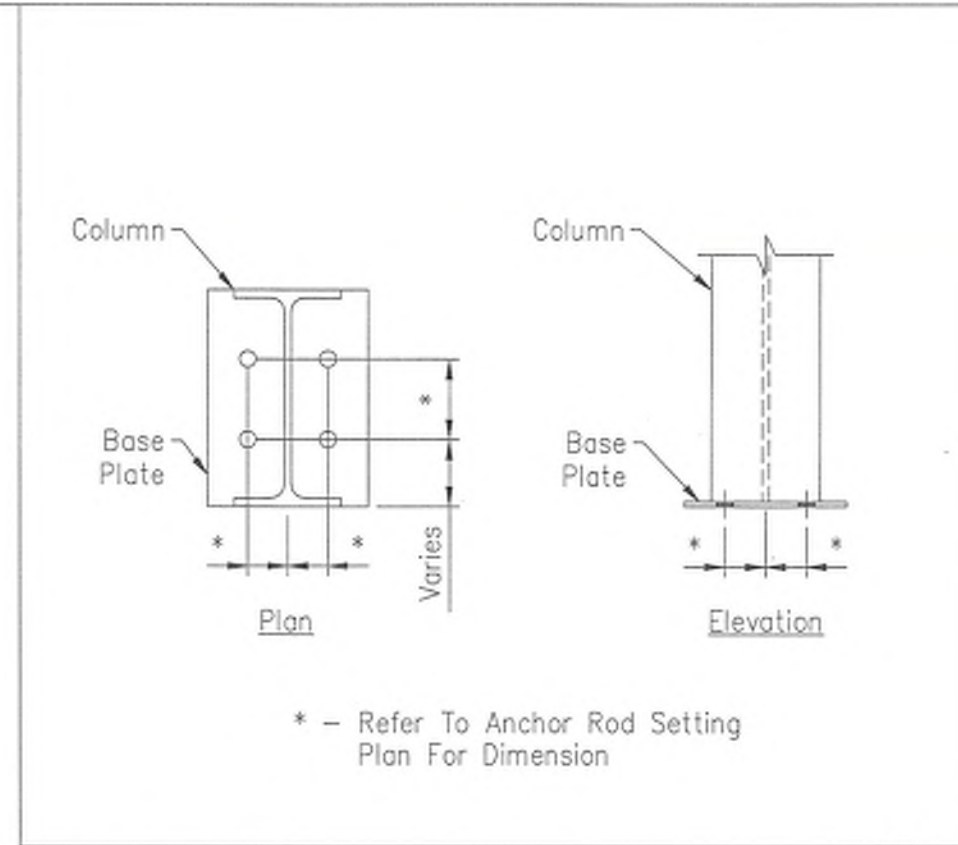




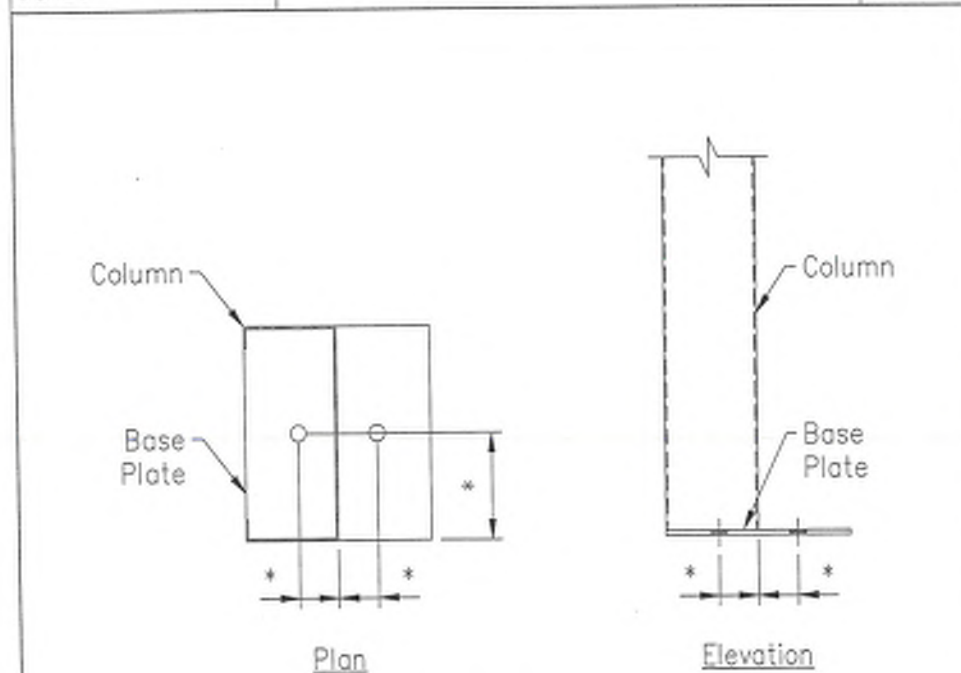
D4	Girt To Cold Form Corner Column	Date Oct '19
Page MB-D4		Rev 01



D17	Girt To Cold Form Rotated Corner Column	Date Jun '17
Page MB-D17		Rev 00



E3	Endwall Column Base Plate	Date Dec '18
Page MB-E3		Rev 01



E8	Cold Form Endwall Column Base Plate	Date Dec '18
Page MB-E8		Rev 01

BUILDING SIZE: 45'-0" x 50'-0" x 17'-0" 2.0:12

ISSUE	DATE	DESCRIPTION	BY	CHK'D	DSN
0	10/25/22	FOR ERECTOR INSTALLATION	MOB	HPD	CM

BUILDING SOLD BY HERITAGE BUILDING SYSTEMS							
MICHAEL W. CUSTER, P.E.							
642 OAKBEND DRIVE COPPEL TX. 75019 PH. 972-571-7082							
PROJECT: LUIS TIRADO - 45X50X17				OWNER: LUIS TIRADO			
CUSTOMER: LUIS TIRADO				LOCATION: ULLINGTON, NC 27546			
CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	5/24/19	N.T.S.	1	A	19-8-19564	DET3	0

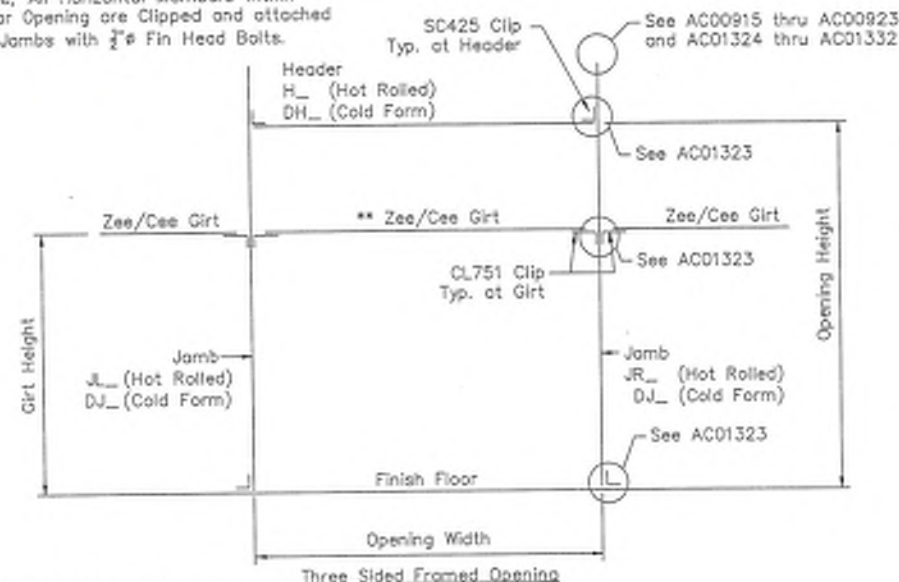




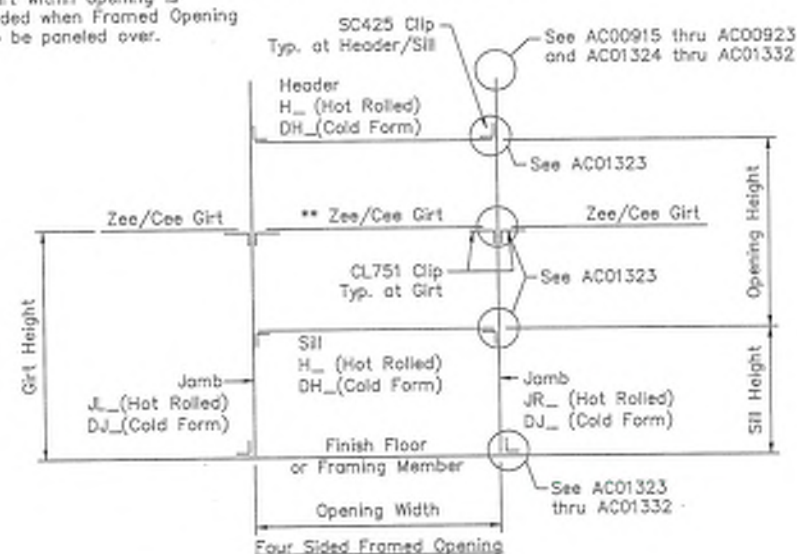
# Bolted Clips - Framed Opening Connections - Cold Form and Hot Rolled Cee - Three and Four Sided Openings

Page  
AC01320  
Date  
May '19  
Rev  
03

Note: All Horizontal Members within clear Opening are Clipped and attached to Jamb with 1/2" Fin Head Bolts.



\*\* Girt within opening is provided when Framed Opening is to be paneled over.



# Bolted Clips - Framed Opening Connections - Three and Four Sided Openings - Girt Header

Page  
AC01322  
Date  
Apr '20  
Rev  
05

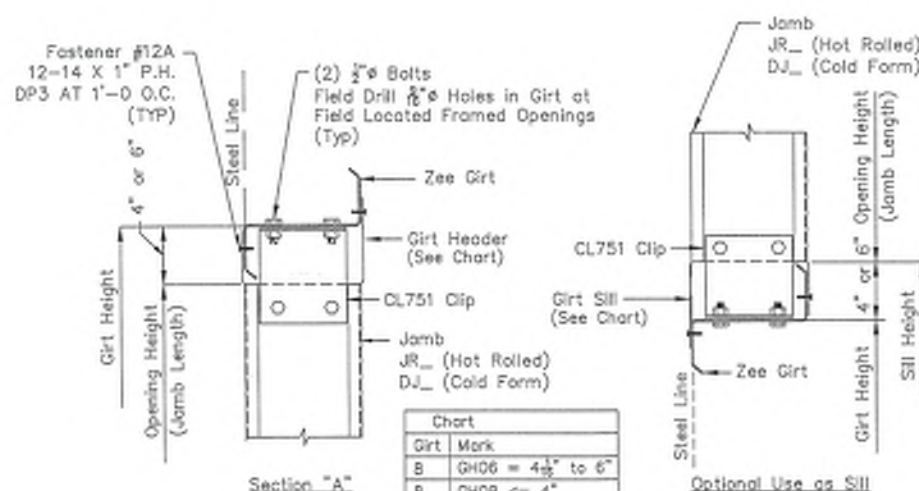
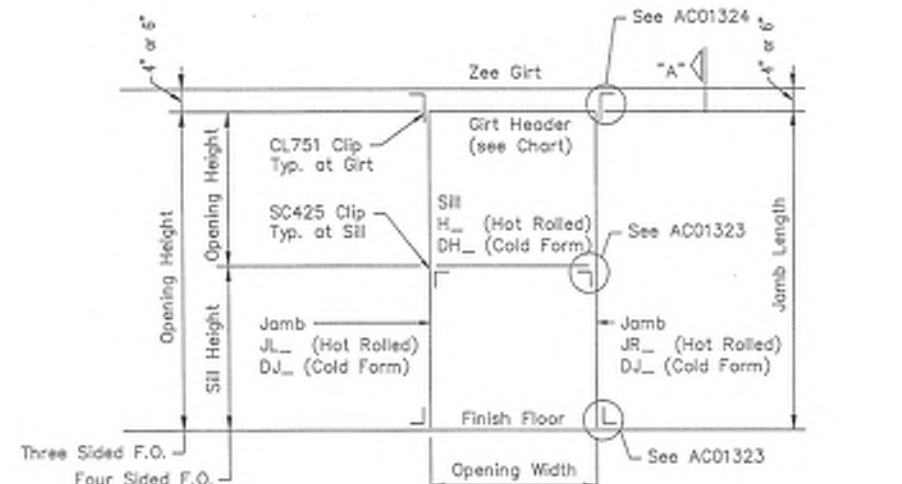
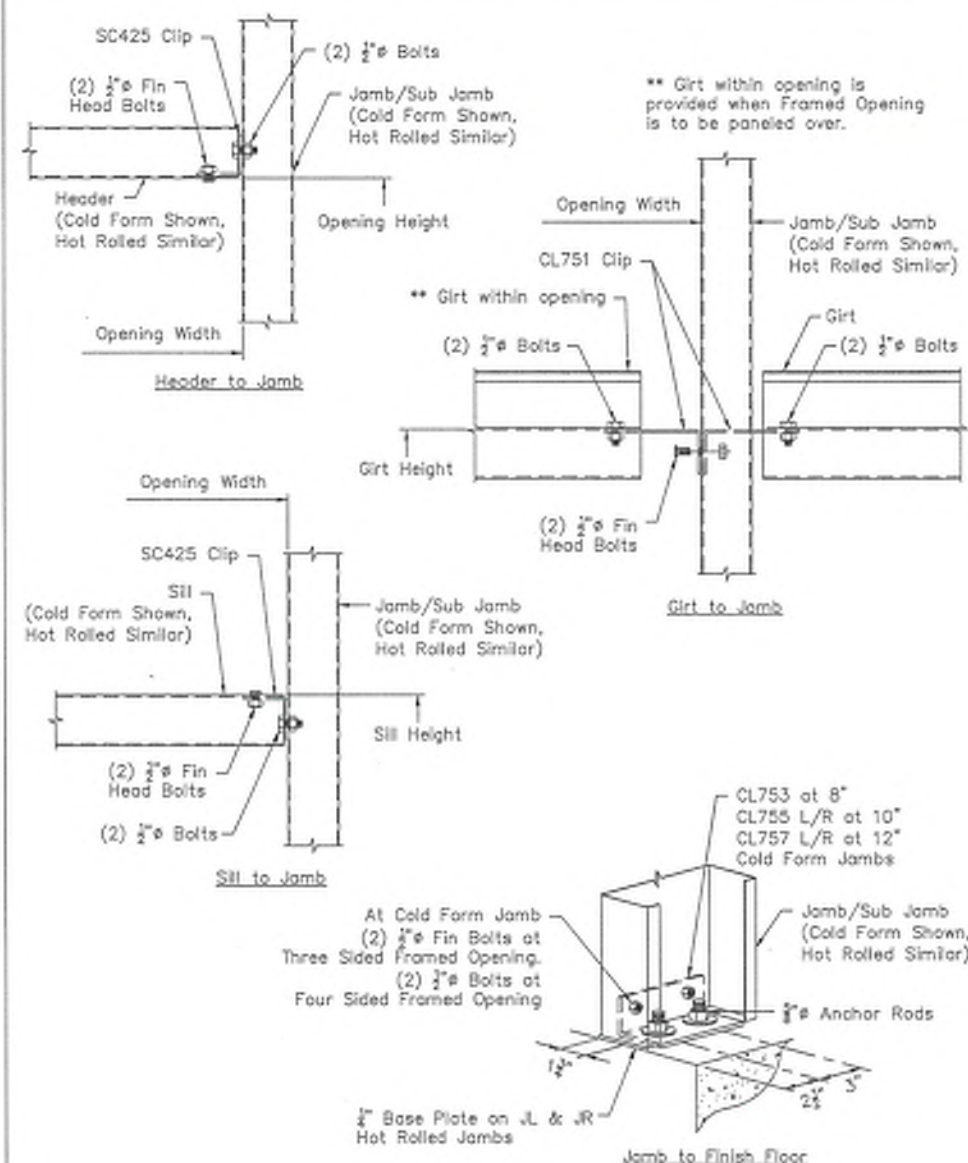


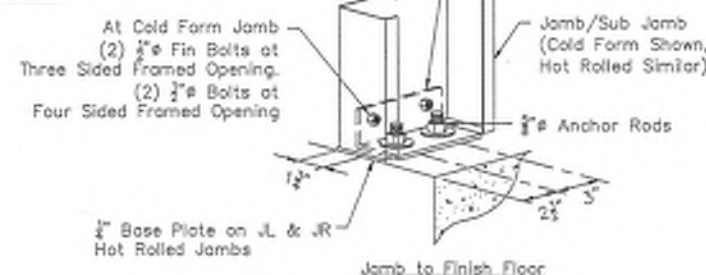
Chart	Girt Mark
8	GH08 = 4 1/2" to 6"
8	GH08 <= 4"
10	GH10
12	GH12

# Bolted Clips - Framed Opening Connections - Cold Form and Hot Rolled Base, Girt, Header, and Sill to Jamb

Page  
AC01323  
Date  
May '19  
Rev  
03



\*\* Girt within opening is provided when Framed Opening is to be paneled over.



BUILDING SIZE: 45'-0" x 50'-0" x 17'-0" 2.0:12

ISSUE	DATE	DESCRIPTION	BY	CHK'D	DSN
0	10/25/22	FOR ERECTOR INSTALLATION	MOB	HPD	DN

BUILDING SOLD BY HERITAGE BUILDING SYSTEMS

MICHAEL W. CUSTER, P.E.  
642 OAKBEND DRIVE  
COPPEL TX. 75019  
PH. 972-571-7082

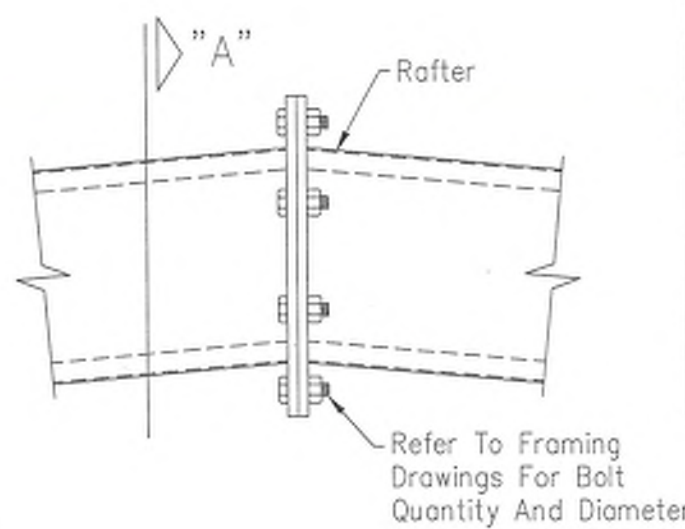
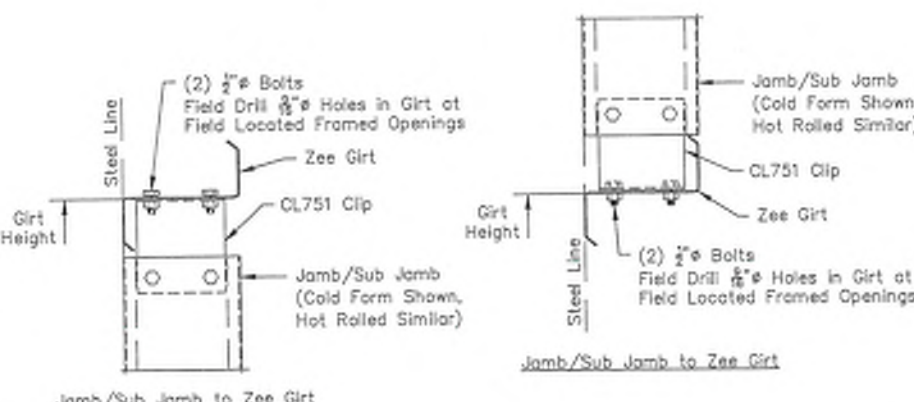
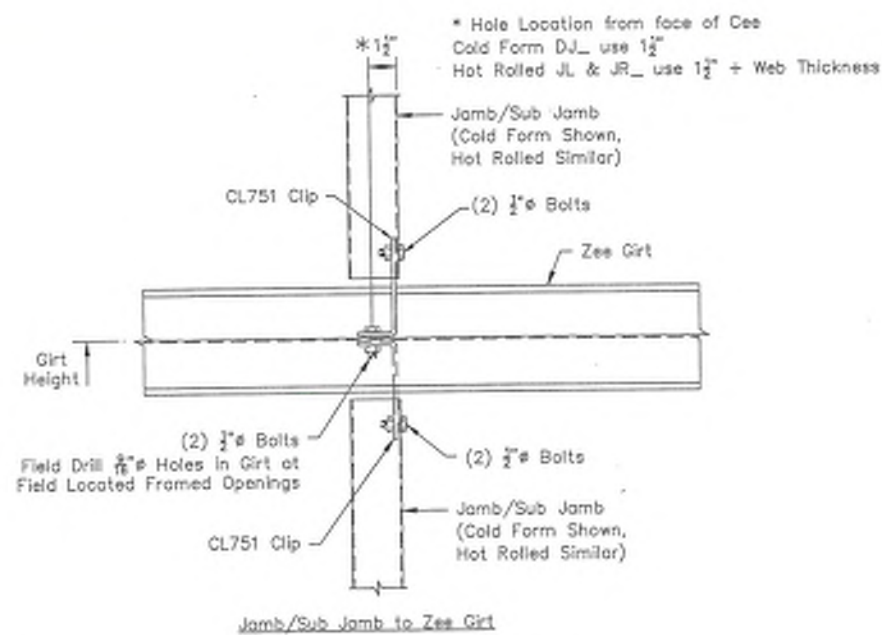
MICHAEL W. CUSTER

Digitally signed by MICHAEL W. CUSTER SEAL  
Date: 2022.11.02 09:08:20 -05'00'

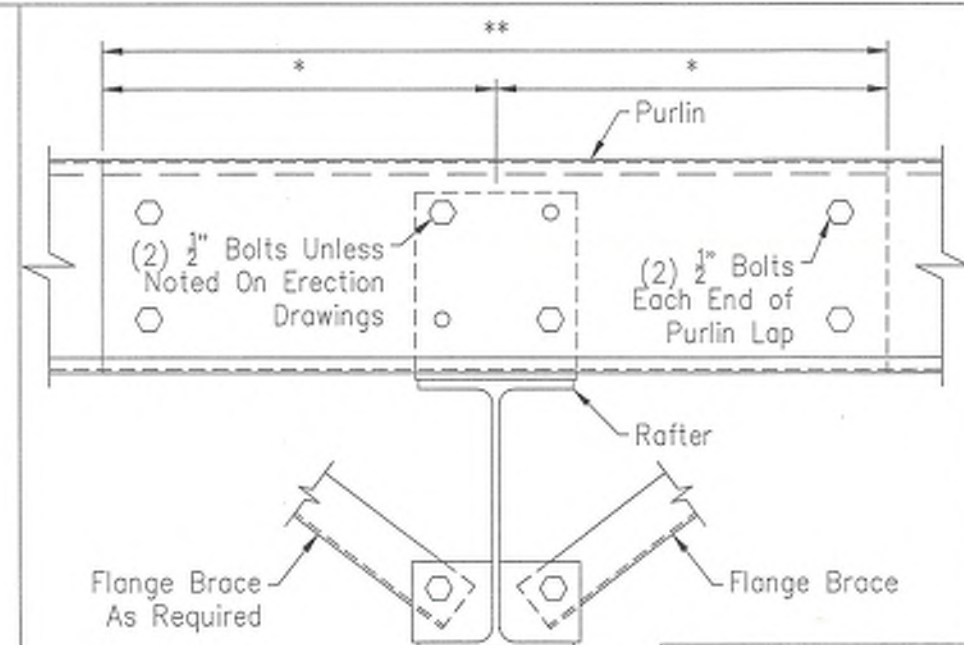


PROJECT: LUIS TIAGO - 45X50X17		W C USTER		Da			
CUSTOMER: LUIS TIAGO		OWNER: LUIS TIAGO		09			
LOCATION: ULLINGTON, NC 27546							
CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	5/24/19	N.T.S.	1	A	19-8-19564	DET4	0





F10	Endwall Bearing Frame - Cold Form Rafter Splice At Ridge	Date Jun '17
Page MB-F10		Rev 00



Purlin Lap Dimensions	
*	**
1'-5 1/2"	2'-11 1/2"
2'-5 1/2"	4'-11 1/2"
3'-1 1/2"	6'-3 1/2"

G2	Purlin To Rigid Frame	Date Sep '19
Page MB-G2		Rev 01

BUILDING SIZE: 45'-0" x 50'-0" x 17'-0" 2.0:12

ISSUE	DATE	DESCRIPTION	BY	CHK'D	DSN
0	10/25/22	FOR DIRECTOR INSTALLATION	MDG	HPD	CM

BUILDING SOLD BY HERITAGE BUILDING SYSTEMS							
MICHAEL W. CUSTER, P.E. 642 OAKBEND DRIVE COPPEL TX. 75019 PH. 972-571-7082							
PROJECT: LUIS TIRADO - 45X50X17							
CUSTOMER: LUIS TIRADO				OWNER: LUIS TIRADO			
LOCATION: ULLINGTON, NC 27546							
CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	5/24/19	N.T.S.	1	A	19-B-19564	DETS	0

