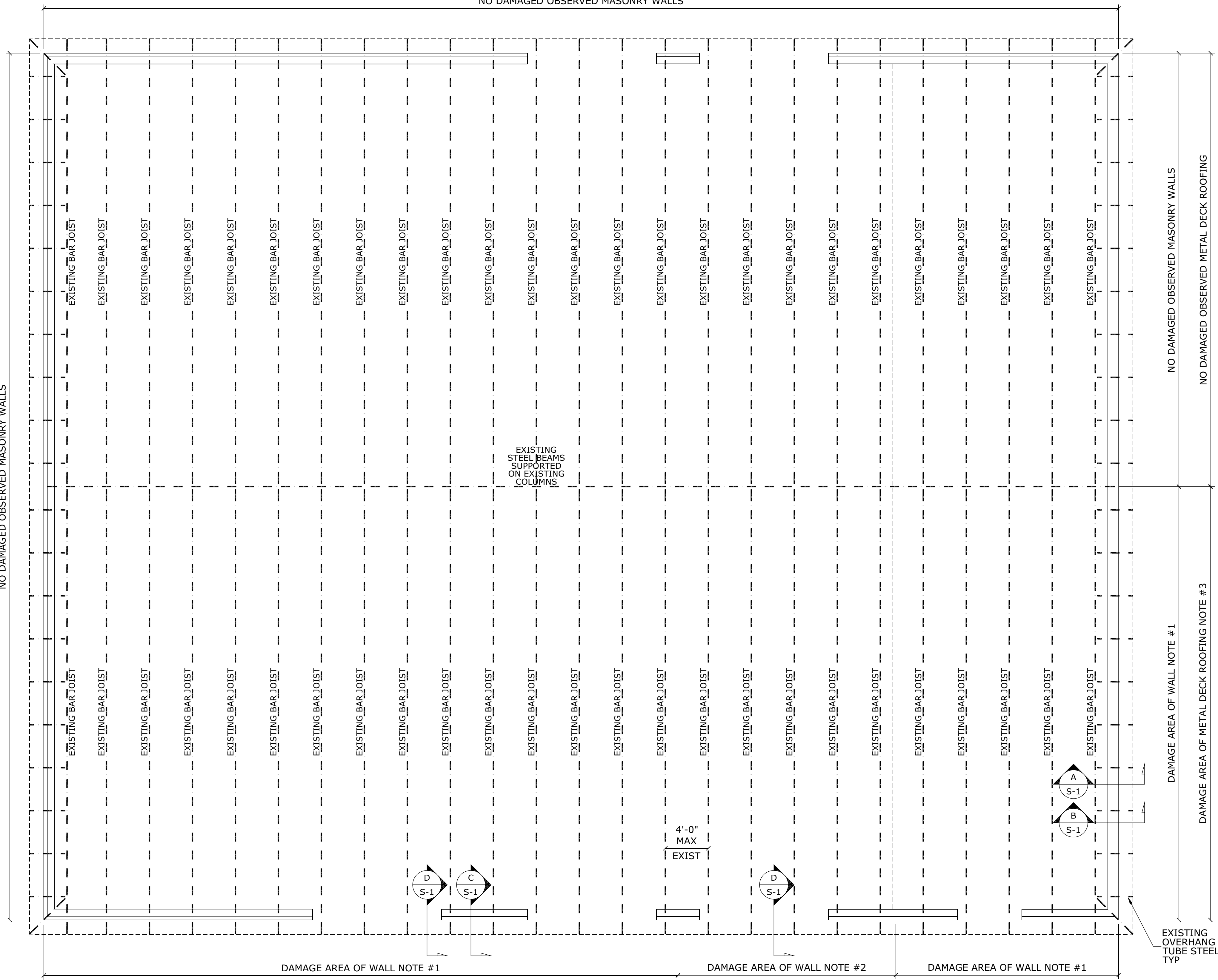


NO DAMAGED OBSERVED MASONRY WALLS



NOTES:  
1) THE TOP SINGLE OR DOUBLE MASONRY COURSES SHIFTED FROM WIND EVENT. THESE CAN BE REPAIRED WITH EITHER OPTION A OR OPTION B FOR OVERHANG AND BAR JOISTS.  
2) THE TOP SINGLE OR DOUBLE MASONRY COURSES SHIFTED OR LIFTED VERTICALLY FROM WIND EVENT. THESE BAR JOIST NEED TO BE REINFORCED USING OPTION B FOR BAR JOISTS.  
3) EXISTING METAL DECKING OF ROOF WAS SIGNIFICANTLY DAMAGED AND PULLED FROM THE BAR JOISTS FRONT 50% ONLY. RECOMMENDATION OF COMPLETE REPLACEMENT OF METAL DECKING TO THE FRONT OF THE CENTER STEEL BEAM.

ROOF METAL DECKING (REPLACEMENT):  
- 1.5A22 METAL DECKING TO BE USED WHICH MATCHES EXISTING 1.5A METAL DECKING THAT REMAINS.  
- METAL ROOF DECKING TO BE ATTACHED 12" ON CENTER INTERMEDIATELY AND 6" ON CENTER AT EDGES, OVERHANGS AND LAPS JOINTS W/ #12 X 1" METAL DECK SCREWS.  
- ANY AND ALL METAL DECKING SHEETS THAT BECAME LOOSE DURING THE WIND EVENT ARE TO BE REPLACED AS REQUIRED.

- GENERAL NOTES
- THESE PLANS ARE DESIGNED TO BE USED BY A LICENSED GENERAL CONTRACTOR.
  - IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO ENSURE THAT ALL PHASES OF CONSTRUCTION COMPLY WITH ALL BUILDING CODE REQUIREMENTS.
  - PRIOR TO CONSTRUCTION, THE GENERAL CONTRACTOR IS TO REVIEW ALL PLANS AND BE RESPONSIBLE FOR VERIFYING ALL DIMENSIONS AND CONDITIONS.
  - ANY DISCREPANCY IN THE PLANS IS TO BE BROUGHT TO THE ATTENTION OF THE DESIGNER PRIOR TO THE BEGINNING OF CONSTRUCTION.
  - DO NOT SCALE DRAWINGS. WRITTEN DIMENSIONS WILL HAVE PRECEDENCE OVER SCALED DIMENSIONS.
  - PLUMBING AND HVAC PLANS ARE TO BE HANDLED BY THE GENERAL CONTRACTOR UNLESS SPECIFIED OTHERWISE. EACH MUST COMPLY WITH ALL BUILDING CODE REQUIREMENTS.

## ROOF/MASONRY WALL PLAN

SCALE: 1/4" = 1'-0"  
REPLACEMENT AND/OR REINFORCEMENT OF ROOF METAL DECKING AND TOP OF MASONRY WALLS

A SITE INSPECTION WAS PERFORMED BY WOODARD SEASE & ASSOCIATES PC TO INSPECT THE EXISTING STRUCTURE AFTER THE WIND EVENT OCCURRED FOR THE DAMAGED THAT OCCURRED. HOWEVER SOMETIMES EXISTING CONDITIONS MAY NOT BE VISIBLE OR MAY NOT BECOME VISIBLE UNTIL UNDER CONSTRUCTION AND DURING DEMO. CONTRACTOR/INSTALLER SHOULD CONTACT WOODARD SEASE AND ASSOCIATES PC IMMEDIATELY IF ANY DISCREPANCIES BETWEEN THESE PLANS AND ACTUAL CONDITIONS BECOME EVIDENT DURING CONSTRUCTION.

PURPOSE:  
A WIND EVENT OCCURRED ON 3607 HODGES CHAPEL ROAD WHERE THE FRONT ROOF METAL DECKING WAS LIFTED FROM THE BAR JOIST AND THE TOP SINGLE OR DOUBLE COURSE OF THE EXISTING MASONRY WALL WAS EITHER LIFTED OR SHIFTED DURING THE EVENT. THE ABOVE IS THE STRUCTURAL RECOMMENDATIONS TO BE IMPLEMENTED TO REPAIR/REINFORCE THE EXISTING BUILDING BASED ON STRUCTURAL LOADING REQUIREMENTS.

**WOODARD SEASE & ASSOCIATES, PC**  
STRUCTURAL ENGINEERS  
4915 WATERS EDGE DRIVE, SUITE 225 RALEIGH, NC 27606  
OFFICE (919) 307-3995  
LICENSE # C-3041

PRELIMINARY  
DO NOT USE  
FOR  
CONSTRUCTION

SEAL DATE: 06/24/2025  
WOODARD SEASE & ASSOCIATES, PC  
C-3041  
NORTH CAROLINA PROFESSIONAL ENGINEERING BOARD

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**3607 HODGES CHAPEL ROAD**  
DUNN, NORTH CAROLINA  
CONTRACTOR: SUNDAGW RENOVATIONS INC.  
PURPOSE: REPAIR/REINFORCEMENT PLAN PLAN: ROOF/MASONRY REPAIR

PROJECT	25-20-106	DATE	06/24/25
ENGINEER	WPS	REV #	DATE
OWNER	WPS	1	06/24/25
DESIGN BY	WPS	2	06/24/25
CHD BY	BEW	4	06/24/25

SCALE: 1/4" = 1'-0"  
FOUNDATION PLAN  
1ST FLOOR FRAMING

SHEET:  
**S-1**

1 OF 2

GENERAL STRUCTURAL NOTES

1. ALL CONSTRUCTION, WORKMANSHIP, AND MATERIALS SHALL CONFORM TO THE LATEST REQUIREMENTS OF "2018 NORTH CAROLINA BUILDING CODE" AND LOCAL REGULATIONS.
2. THE ENGINEER WHOSE SEAL APPEARS ON THESE DRAWINGS IS THE STRUCTURAL ENGINEER OF RECORD FOR THIS PROJECT. NO OTHER PARTY MAY MODIFY OR REUSE THESE CONSTRUCTION DOCUMENTS WITHOUT WRITTEN PERMISSION FROM WOODARD SEASE & ASSOCIATES, PC OR STRUCTURAL ENGINEER OF RECORD. ENGINEERS SEAL ONLY APPLIES TO STRUCTURAL COMPONENTS AND SYSTEMS AND DOES NOT CERTIFY DIMENSIONAL ACCURACY OF THE ARCHITECTURAL LAYOUT.
3. THE ENGINEER SHALL HAVE NO LIABILITY TO OTHERS FOR ACTS OR OMISSIONS OF THE CONTRACTOR/BUILDER OR ANY OTHERS PERFORMING WORK ON THIS PROJECT. THE ENGINEER IS NOT RESPONSIBLE FOR CONSTRUCTION SEQUENCES, METHODS, OR TECHNIQUES AND/OR SAFETY REQUIREMENTS IN CONNECTION WITH THE CONSTRUCTION OF THIS STRUCTURE.
4. THE CONTRACTOR ASSUMES ALL RESPONSIBILITY FROM DEPICTED OR IMPLIED STRUCTURAL INFORMATION. SHOULD ANY DISCREPANCIES BECOME APPARENT, THE STRUCTURAL ENGINEER OF RECORD MUST BE NOTIFIED IMMEDIATELY BEFORE CONSTRUCTION BEGINS.
5. ONLY SEALED DRAWINGS W/LATEST REVISIONS ARE APPLICABLE FOR CONSTRUCTION.
6. DEFLECTION: FLOOR: L/360, ATTIC W/ CEILING: L/240, ROOF: L/180 - MORE STRINGENT CRITERIA MAY BE USED AT ENGINEER'S DISCRETION OR AS REQUESTED.
7. DO NOT SCALE DRAWINGS. CONTRACTOR SHALL CONTACT ARCHITECT FOR ITEMS NOT DIMENSIONED.
8. THE CONTRACTOR SHALL PROVIDE TEMPORARY ERECTION BRACING AND SHORING OF ALL STRUCTURAL MEMBERS AS REQUIRED FOR THE STABILITY OF THE STRUCTURE DURING ALL PHASES OF CONSTRUCTION. THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ARCHITECT / ENGINEER OF ANY CONDITION WHICH, IN HIS OPINION, MIGHT ENDANGER THE STABILITY OF THE STRUCTURE OR CAUSE DISTRESS WITHIN THE STRUCTURE.
9. CONSTRUCTION MATERIALS SHALL NOT BE STACKED ON FLOORS OR ROOFS IN EXCESS OF THE DESIGN LIVE LOADS WHICH ARE INDICATED IN THE DESIGN LOADS. IT IS THE GENERAL CONTRACTOR'S RESPONSIBILITY TO ENSURE THAT THE SUBCONTRACTORS ARE INFORMED AND DO NOT VIOLATE THIS IMPORTANT REQUIREMENT. IMPACT SHALL BE AVOIDED WHEN PLACING MATERIALS ON FLOOR AND ROOFS.
10. SEE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS FOR MISCELLANEOUS STEEL ITEMS NOT SHOWN.
11. COORDINATE SIZES AND LOCATIONS OF OPENINGS IN FLOORS AND ROOF WITH ARCHITECTURAL, MECHANICAL, AND ELECTRICAL REQUIREMENTS.
12. FOR ACTUAL ELEVATION OF FIRST FLOOR (REF. ELEV. 0'-0"), SEE SITE PLAN.
13. SUBMIT WRITTEN REQUEST TO THE ARCHITECT FOR APPROVAL OF ANY PROPOSED CHANGE TO THE REQUIREMENTS OF THE CONTRACT DOCUMENTS. SPLICING, CUTTING, NOTCHING OR OTHER ALTERATIONS TO THE STRUCTURAL MEMBERS ARE NOT PERMITTED WITHOUT WRITTEN AUTHORIZATION OF THE ENGINEER. ANY UNAUTHORIZED DEVIATION FROM THE CONTRACT DOCUMENTS , AND CORRECTION THEREOF, IS THE RESPONSIBILITY OF THE CONTRACTOR.
14. THE MOST STRINGENT REQUIREMENTS APPLY IN CASE OF CONFLICT BETWEEN SPECIFICATIONS, STANDARDS, CODES, AND DRAWINGS.

DESIGN LOADS

FLOOR/ROOF AREAS	LIVE LOAD	DEAD LOAD
ROOF (TOTAL)	20 PSF	15 PSF
-DECKING		3 PSF
-ROOF COVER		3 PSF
-BAR JOIST		4 PSF
-MECHANICAL	5 PSF	
FLOOR (OFFICE)	250 PSF	

OCCUPANCY CATEGORY : II

WIND DESIGN:

ULTIMATE DESIGN WIND SPD = 120 MPH

IMPORTANCE FACTOR =  $I_w$  = 1.00

EXPOSURE CATAGORY = B

BUILDING CATEGORY = ENCLOSED

WIND BASE SHEAR  $V_x$  = NA KIPS

WIND BASE SHEAR  $V_y$  = NA KIPS

SNOW LOADS:

GROUND SNOW LOAD  $P_g$  = 10 PSF

EXPOSURE FACTOR  $C_e$  = 1.0

THERMAL FACTOR  $C_t$  = 1.0

IMPORTANCE FACTOR =  $I_{pg}$  = 1.0

SNOW LOAD  $P_f$  = 10 PSF

SEISMIC LOADS:

IMPORTANCE FACTOR =  $I_e$  = 1.00

SEISMIC DESIGN CATEGORY = NA

OCCUPANCY = II

SITE CLASS = NA

BASIC STRUCTURAL SYSTEM = BEARING WALL

RESPONSE MOD =  $R$  = 8

$S_s$  = NA %g

$S_1$  = NA %g

ANALYSIS PROCEDURE = NA

DESIGN BASE SHEAR =  $V_x$  = NA KIPS,  $V_y$  = NA KIPS

FOUNDATIONS:

FOUNDATION NOTES:

1. ALL FOOTINGS SHALL BE POURED ON COMPACTED SOIL WITH A MINIMUM BEARING CAPACITY OF 2000 PSF.
2. CONTINUOUS WALL FOOTINGS SHALL BE POURED MONOLITHICALLY WITH COLUMN FOOTINGS.
3. ALL BACKFILL MATERIAL SHALL BE FREE OF DEBRIS. PLACE FILL IN 8" LIFTS WITH COMPACTION BETWEEN LIFTS TO A MINIMUM OF 95% OF MAXIMUM DENSITY AT OPTIMUM MOISTURE CONTENT.
4. INSTALL FOUNDATIONS PER GEOTECHNICAL ENGINEER'S REPORT AND FIELD INSTRUCTIONS.
5. FOUNDATION TYPE AND DESIGN MAY BE FIELD-MODIFIED BASED UPON GEOTECHNICAL ENGINEER'S SITE DETERMINATIONS.
6. NOTIFY ENGINEER OF ALL FIELD DETERMINED CONCLUSIONS.
7. WALLS ACTING AS RETAINING WALLS SHALL NOT BE BACKFILLED WITHOUT BRACING UNTIL ALL SUPPORTING SOIL AND SLABS ARE IN PLACE.

CONCRETE:

CONCRETE NOTES:

1. ALL CONCRETE IS TO BE PROPORTIONED AND PLACED IN ACCORDANCE WITH THE LATEST EDITION OF THE AMERICAN CONCRETE INSTITUTE/ACI 318.
2. CONCRETE SHALL HAVE MINIMUM COMPRESSIVE STRENGTH:
- SLABS: 3000 PSI      FOUNDATIONS: 3000 PSI      FOOTINGS: 3000 PSI
3. ALL CONCRETE STEEL REINFORCEMENT TO BE GRADE 60.
4. ALL INTERIOR SLABS TO BE 5" THICK.
5. CONTROL JOINTS ARE TO BE PLACED AS SPECIFIED ON PLANS BOTH DIRECTIONS.

MASONRY:

MASONRY NOTES

1. CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C 90, GRADE N-1 UNLESS OTHERWISE NOTED, COMPRESSIVE STRENGTH ON NET CROSS SECTIONAL AREA: 2000 PSI.
2. MASONRY SHALL BE LAID IN ASTM C 270, TYPE "S" MORTAR, UNLESS OTHERWISE NOTED AND SHALL HAVE FULL MORTAR COVERAGE OF THE FACE SHELLS IN BOTH HORIZONTAL AND VERTICAL JOINTS.
3. GROUT FOR REINFORCED MASONRY SHALL HAVE A COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS AND SHALL CONFORM TO ASTM C476.
4. GROUT FOR REINFORCED MASONRY SHALL HAVE A SLUMP OF 8 TO 11 INCHES. COURSE GROUT SHALL BE USED IN SPACES 2-1/2" X 3" AND GREATER. FINE GROUT SHALL BE USED IN SPACES 2" X 3" AND SMALLER.
5. MAXIMUM HEIGHT TO WHICH MASONRY SHALL BE LAID BEFORE FILLING IS 4 FEET FOR COARSE GROUT AND 2 FEET FOR FINE GROUT.
6. REINFORCING GRADE AND DETAILS SHALL BE THE SAME AS FOR CONCRETE. TIE IN POSITION AND PLACE CONCRETE AROUND REINFORCING DURING CONSTRUCTION OF MASONRY. DO NOT PUSH REINFORCING DOWN INTO PREVIOUSLY PLACED GROUT FILL. SET BOLTS SIMILARLY.
7. HORIZONTAL REINFORCING BARS MAY BE SPLICED WITH A MINIMUM LAP OF 48 TIMES THE BAR DIAMETER UNO
8. REINFORCE ALL WALLS WITH 9 GAGE CONTINUOUS LADDER TYPE REINFORCING AT 16" VERTICAL SPACING UNO ON PLANS.
9. SPLICED REINFORCING BARS SHALL OCCUPY THE SAME CELL. MINIMUM LAP SPLICE SHALL BE 48 TIMES THE BAR DIAMETER. SPLICED BARS NEED NOT BE TIED TOGETHER.
10. VERTICAL REINFORCEMENT IN WALLS SHALL BE SUPPORTED AND SECURED AGAINST DISPLACEMENT AT 6 FOOT INTERVALS FOR #3 AND #4 BARS AND 8 FOOT INTERVALS FOR #5 AND #6 BARS.
11. WHERE INTERIOR CONCRETE MASONRY PARTITIONS INTERSECT WITH OTHER INTERIOR PARTITIONS OR EXTERIOR WALLS, A MASONRY BOND, OR THE EQUIVALENT IN APPROVED METAL TIES, SHALL BE PROVIDED UNO ON THE DRAWINGS.
12. MORTAR JOINTS SHALL BE 3/8" THICK WITH FULL MORTAR COVERAGE ON VERTICAL AND HORIZONTAL FACE SHELLS. VERTICAL JOINTS SHALL BE SHOVED TIGHT.
13. BACKFILLING AGAINST MASONRY WALLS SHALL NOT BE PERMITTED UNTIL SUFFICIENT LATERAL SUPPORT IS PROVIDED BY THE CONTRACTOR.
14. PROVIDE BOND BEAMS AT MASONRY ELEVATIONS AS SHOWN ON THE ARCHITECTURAL AND STRUCTURAL DRAWINGS. BOND BEAMS SHALL HAVE TWO #4 BARS, CONTINUOUS, UNLESS OTHERWISE NOTED.
15. ALL VERTICAL REINFORCING BARS SHALL EXTEND 6" MINIMUM INTO BOND BEAM AT TOP OF WALL.

METAL DECKING:

METAL DECKING NOTES

1. STEEL ROOF DECK SHALL BE DESIGNED, MANUFACTURED AND INSTALLED IN ACCORDANCE WITH THE LATEST EDITION OF THE STEEL DECK INSTITUTE'S "DESIGN SPECIFICATION FOR COMPOSITE FLOOR DECK, NON-COMPOSITE FLOOR DECK, AND ROOF DECK.
2. ROOF DECKING/FLOOR DECKING AS SPECIFIED BY PLANS.

STRUCTURAL STEEL:

STEEL NOTES

1. ALL STEEL IS TO BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST EDITION OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION'S "MANUAL OF STEEL CONSTRUCTION" AND "CODE OF STANDARD PRACTICEFOR STEEL BUILDINGS AND BRIDGES."
2. PRODUCTS:
- STRUCTURAL STEEL: ASTM A992/ASTM 572  $F_y$ = 50 ksi
- TUBULAR STEEL: ASTM A500 B  $F_y$  = 46 KSI
- CONNECTION BOLTS: ASTM 325 (THREADS INCLUDED)
- ANCHOR BOLTS: ASTM A307
- WELD ELECTRODES: E70XX OR AWS EQUIVALENT
- SHOP PRIMER: ONE COAT OF RUST INHIBITIVE PRIMER
3. USE A-325 BOLTS FOR ALL STEEL CONNECTIONS UNLESS OTHERWISE NOTED. ALL CONNECTIONS ARE BEARING TYPE AND BOLT THREADS ARE INCLUDED FROM SHEAR PLANE UNLESS OTHERWISE SPECIFIED.
4. USE A-307 ANCHOR BOLTS UNLESS OTHERWISE NOTED. ALL ANCHOR BOLTS SHALL HAVE STANDARD HOOK OF NOT LESS THAN 3" UNLESS SPECIFIED OTHERWISE.
5. BOLT HOLE NOTATION AND SPECIFICATION:
- $d$  = BOLT DIAMETER
- STD = STANDARD HOLE =  $d + 1/16"$
- OVS = OVERSIZED HOLE =  $d + 5/16"$
- SSL = SHORT SLOTTED HOLE =  $(d + 1/16") \times (d + 3/8")$
- LSL = LONG SLOTTED HOLE =  $(d + 1/16") \times (d + 2.5Xd)$
- NSL = LONG OR SHORT SLOTTED HOLE NORMAL TO LOAD DIRECTION
6. CONNECTION TYPE NOTATION AND SPECIFICATION:
- SC - SLIP CRITICAL
- N -BEARING TYPE W/ THREADS INCLUDED IN SHEAR PLANE
- X - BEARING TYPE W/ THREADS EXCLUDED FROM SHEAR PLANE
- ALL WELDING SHALL CONFORM TO THE LATEST EDITION OF THE AMERICAN WELDING SOCIETY'S "STRUCTURAL WELDING CODE FOR STEEL." ALL WELDERS SHALL BE CERTIFIED PER AWS.

FABRICATION AND ERECTION:

FABRICATION AND ERECTION NOTES

1. SHOP FABRICATION AND ERECTION IN ACCORDANCE WITH AISC CODE OF STANDARD PRACTICE.
2. QUALITY CONTROL SHALL BE PER ASIC "QUALITY CRITERIA AND INSPECTION STANDARDS".
3. ALL HOLES TO BE DRILLED HOLES TO BE DRILLED OR PUNCHED. DO NOT PRODUCE HOLES OR ENLARGE HOLES BY MEANS OF BURNING OR FLAME TORCHING.
4. SET STRUCTURAL STEEL ACCURATELY TO LINES AND ELEVATIONS AS SPECIFIED AND ADJUST PRIOR TO FINAL FASTENING WHETHER BOLTING OR WELDING.
5. SPLICES SHALL BE ALLOWED ONLY WHERE DENOTED UNLESS APPROVED.
6. TEMPORARY SHORING AND BRACING SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO SAFELY RESIST ALL CONSTRUCTION AND SHORT TERM LOADS PRIOR TO COMPLETE INSTALLATION OF BUILDING COMPONENTS.
7. GROUT BELOW BASE PLATES USING NON-SHRINK GROUT.

STEEL BAR JOIST AND GIRDERS:

STEEL BAR JOIST AND GIRDERS NOTES

1. ALL JOIST AND JOIST GIRDERS AE TO BE DESIGNED, FABRICATED, AND ERECTED IN ACCORDANCE WITH THE LATEST EDITION OF THE "STEEL JOIST INSTITUTE'S SPECIFICATION".
2. BRIDGING SHALL BE INSTALLED AS SHOWN ON THE FRAMING AND PROPERLY ANCHORED TO THE TOP AND BOTTOM CHORDS OF ALL JOISTS AND ANCHORED TO END WALLS.
3. BOTTOM CHORD EXTENSIONS OF ALL JOISTS AND GITRDERS SHALL NOT BE ATTACHED TO COLUMN OR WALL STABILIZER PLATES UNTILL ALL DEAD LOADS HAVE BEEN APPLIED.

WOODARD SEASE & ASSOCIATES, PC

STRUCTURAL ENGINEERS

4915 WATERS EDGE DRIVE, SUITE 225 RALEIGH, NC 27606

OFFICE: (919) 307-3995

OFFICE: (919) 307-3995

OFFICE: (919) 307-3995

WOODARD SEASE & ASSOCIATES, PC

STRUCTURAL ENGINEER

29423

SEAL DATE: 06/24/2025

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3607 HODGES CHAPEL ROAD

DUNN, NORTH CAROLINA

CONTRACTOR: SUNDAWG RENOVATIONS INC.

PURPOSE: REPAIR/REINFORCEMENT PLAN PLAN: ROOF/MASONRY REPAIR

PROJECT	25-20-106	DATE	06/24/25
ENGINEER	WPS	REV #	DATE
		1	06/24/25
OWN BY	WPS	2	06/24/25
CHD BY	BEW	3	06/24/25
		4	06/24/25

SCALE 1/4" = 1'-0"

SPECIFICATIONS

SHEET:

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