HOUSTON

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#### **ERECTION NOTES**

- 1. All bracing shown and provided by the Metal Building Provider (MBP) for this building is required and shall be installed by the erector as a permanent part of the structure ("Code of Standard Practice for Steel Buildings" in the ANSI/AISC 303—16; Section 7.10).
- Temporary supports, such as guys, braces, falsework, cribbing or other elements required for the erection operation shall be determined and furnished by the erector ("Code of Standard Practice for Steel Buildings and Bridges" in the ANSI/AISC 303—16; Section 7.10.3).
- 3. Normal erection operations include the correction of minor misfits by moderate amounts of reaming, grinding, welding or cutting, and the drawing of elements into line through use of drift pins. Errors which require major changes in the member configuration are to be reported immediately to the Metal Building Provider by the customer to enable whoever is responsible either to correct the error or to approve the most efficient and economic method of correction to be used by others ("Code of Standard Practice for Steel Buildings and Bridges" in the ANSI/AISC 303—16; Section 7.14).
- 4. Erection tolerances are set forth in the "Code of Standard Practice for Steel Buildings and Bridges" in the ANSI/AISC 303—16; Section 7.13 note that individual members are considered plump, level and aligned if the deviation does not exceed 1:500. Variations in finished overall dimensions of structure steel framing are deemed within the limits of good practice when they do not exceed the cumulative effect of rolling, fabricating, and erection tolerances.
- 4.1. When crane support systems are part of the metal building system erection tolerances Section 6.8, Erection Tolerances, 2018 MBMA Metal Building Systems manual shall apply. To achieve the required tolerances grouting of the columns and shimming of the runway beams may be required. The customer shall provide grout if required. The contractor erecting the runway beams is responsible for shimming, plumbing, and leveling of the runway system. When aligning the runway beams the alignment shall be with respect to the beam webs so that the center of the aligned rail is over the runway web.
- 5. As a general rule field welding is not used to assemble a metal building system. In cases where the drawings indicate field welding and in cases where approved corrections are to be made by field welding the following requirements shall be met;
- 5.1. welders must be qualified by an independent testing agency, with suitable documentation to AWS D1.1 Structural Welding Code Steel or AWS D1.3 Structural Welding Code Sheet as applicable, for the processes, positions, and materials involved.
- 5.2. All welds must be made in conformance to a documented and approved Welding Procedure Specification (WPS). All joints which are not prequalified must be supported by a certified Procedure Qualification Record (PQR) by an independent testing agency.
- 6. All documentation and records shall be the responsibility of the customer.
- 7. Any claims or shortages by buyer must be made to the Metal Building Provider within seven (7) working days after delivery, or such claims will be considered to have been waived by the customer and disallowed. All claims should be directed to the Metal Building Provider's Customer Service Department.
- 8. Claims for correction of alleged misfits will be disallowed unless the Metal Building Provider shall have received prior notice thereof and allowed reasonable inspection of such misfits. Ordinary inaccuracies of shop work shall not be construed as misfits. No part of the building may be returned or charges assessed for alleged misfits without prior approval from the Metal Building Provider.
- 9. Neither the Metal Building Provider nor the customer will cut, drill or otherwise after their work, or the work of other trades to accommodate other trades unless such work is clearly specified in the contract documents. Whenever such work is specified the customer is responsible for furnishing complete information as to materials, size, location, and number of alterations prior to preparation of shop drawings ("Code of Standard Practice for Steel Buildings and Bridges" in the ANSI/AISC 303—16, Section 7.15).
- 10. The Metal Building Provider Field Modifications Policy:
- 10.1. The Metal Building Provider will only be responsible for the field—modified parts designed and approved by the Metal Building Provider's Customer Service Department.
- 10.2. Any field modifications designed by third parties may not be approved by the Metal Building Provider and may limit the Metal Building Provider's warranty and liability.
- 10.3. The Metal Building Provider makes no warranty and hereby disclaims any responsibility with respect to the design, engineering, or construction of any field—modified parts performed by third parties.
- 11. WARNING SOME PANELS AND TRIM PARTS ARE FURNISHED WITH A PROTECTIVE PEEL—OFF FILM. PARTS PROVIDED WITH THIS FILM CANNOT BE EXPOSED TO SUNLIGHT WITHOUT FIRST REMOVING THE FILM. THIS FILM MUST BE REMOVED PRIOR TO INSTALLATION. FILM MUST ALSO BE REMOVED FROM ALL NON EXPOSED PARTS WITHIN SIX MONTHS FROM FILM APPLICATION OR IRREPARABLE DAMAGE WILL OCCUR TO THE SURFACE CLAIMS WILL NOT BE ACCEPTED FOR THIS ISSUE.

## <u>RESPONSIBILITIES</u>

- 1. The Metal Building Provider Customer, hereafter referred to as the "customer," obtains and pays for all building permits, licenses, public assessments, paving or utility pro rata, utility connections, occupancy fees and other fees required by any governmental authority or utility in connection with the work provided for in the Contract Documents. The customer provides at his expense all plans and specifications required to obtain a building permit. it is the customer's responsibility to ensure that all plans and specifications comply with the applicable requirements of any governing building authorities.
- 2. The customer is responsible for identifying all applicable building codes, zoning codes, or other regulations applicable to the Construction Project, including the Metal Building system
- 3. It is the responsibility of the customer to interpret all aspects of the End User's specifications and incorporate the appropriate specifications, design criteria, and design loads into the Order Documents submitted to the Metal Building Provider.
- 4. It is the responsibility of the Metal Building Provider to furnish the metal building system to meet the specifications including the design criteria and design loads incorporated by the Contractor into the Order Documents. The Metal Building Provider is not responsible for making an independent determination of any local codes or any other requirements not part of the Order Document.
- 5. The Metal Building Provider's standard specifications apply unless stipulated otherwise in the Contract Documents. The Metal Building Provider design, fabrication, quality criteria, standards, practice, methods and tolerances shall govern the work any other interpretations to the contrary not with standing, it is understood by both parties that the customer is responsible for clarifications of inclusions or exclusions from the Architectural plans.
- 6. In case of discrepancies between the Metal Building Provider's structural steel plans and plans for other trades, the Metal Building Provider's shall govern ("Code of Standard Practice for Steel Buildings and Bridges" in the AISC 303-16; Section 3.3).
- 7. The customer is responsible for overall project coordination. All interface, compatibility and design considerations concerning any materials not furnished by the Metal Building Provider and the Metal Building Provider's steel system are to be considered and coordinated by the customer. Specific design criteria concerning this
- interface between materials must be furnished by the customer before release for fabrication or the Metal Building Provider's assumptions will govern.

  8. Foundations, anchor rods, and anchor rod embedment are designed, furnished, and set by the customer in accordance with an approved drawing. Dimensional
- accuracy shall satisfy the requirements of Section 7.5 1 of "Code of Standard Practice for Steel Buildings and Bridges" in the AISC 303-16.
- 9. All other embedded items or connection materials between the structural steel and the work of other trades are located and set by the customer in accordance with approved location on erection drawings. Accuracy of these items must satisfy the erection tolerance requirements.
- 10. The Metal Building Provider does not investigate the influence of the metal building system on existing buildings or structures. The End Customer assures that such buildings and structures are adequate to resist snow drifts, wind loads, or other conditions as a result of the presence of the metal building system.

## GENERAL SPECIFICATIONS

- 1. Wall and liner panels are an integral part of the structural system. Unauthorized removal of panels or cutting panels for framed openings not shown is prohibited
- therefor is not a cause for rejection.
- 3. The Metal Building Provider's red—oxide and gray—oxide primer are designed for short term field protection from exposure to ordinary atmospheric conditions.
- 4. All bolts are 1/2" x 1-1/4" A307 unless noted. Refer to the erection drawings for specific framing connections and the cross-section(s) for main frame connections.
   5. Unless noted otherwise on the frame cross section(s), all bolted joints with ASTM F3125 Grade A325 bolts are specified as snug-tightened joints in accordance with the specification for Structural Joints Using High-Strength Bolts, June 11, 2020. Installation Inspection requirements for Snug-Tight Bolts

Oil—canning, a perceived waviness inherent to light gauge metal, may exist. This condition does not affect the structural integrity or the finish of the panel, and

- (Specification for Structural joints, Section 9.1) is suggested.

  6. Unless noted otherwise, all bolted connections are designed as bearing type connections with bolt threads not excluded from the shear plane.
- 7. Any type of suspended or load inducing system(s) is prohibited if zero collateral and zero sprinkler loads are designated on the contract. This would include lights, duct work, piping, and insulation types other than 3" standard duty fiberglass blanket insulation, etc.

BUILDING DESIGN CODES	
Building Code:	NCBC 18
Steel Specification:	MSC 360-10

Steel Specification:

Cold-Formed Specification:

AISC 360-10

AISI S100-12

GENERAL LOADS

Roof Dead Load:

 Roof Dead Load:
 2.50

 Roof Collateral Load:
 6.00 psf

 Sprinkler Load:
 0 psf

 Roof Live Load:
 20.00 psf

Tributary Live Load Reduction: Yes

Rainfall Intensity (5—minute duration 5—year recurrence): 7.06 in/hr

MIND I OVD

Wind Speed (3—sec gust) Vult:

Va≤ol:

V serv:

Wind Exposure Category:

Wind Condition:

Internal Pressure Coefficient (GCpi):

120 mph

92.00 mph

76.00 mph

Enclosed

0.18, -0.18

CNOW LOAD

 SNOW LOAD
 7.00 psf

 Roof Snow Load (Pf):
 7.00 psf

 Ground Snow Load (Pg):
 10 psf

 Snow Exposure Factor (Ce):
 1.00

 Snow Load Importance Factor (Is):
 1.00

 Thermal Factor (Ct):
 1.00

DEFLECTION CRITERIA

H/100 Roof Panels: Main Frames Lateral: L/60 L/180 Main Frames Vertical: L/180 Purlins: Bearing Frame Rafter: L/180 L/240 Wall Panels: L/240 Endwall Columns: L/240 Girts:

SEISMIC LOAD

Risk Category:

Seismic Importance Factor (le):

Spectral Response Acceleration (Ss):

Spectral Response Acceleration (S1):

Site Class:

Spectral Response Coefficients (Sds):

Spectral Response Coefficients (Sds):

Spectral Response Coefficients (Sd1):

Seismic Design Category:

C

Basic Seismic Force Resisting Systems\*:

Basic Seismic Force Resisting Systems\*: 'Structural Steel Systems Not Specifically Detailed for Seismic Resistance'

LongitudinalLateralTotal Design Base Shear:13.4 Kips14.2 KipsSesimic Response Coefficient(s) (Cs):0.0660.066Response Modification Factor(s) (R):33Deflection Amplification Factor(s):3.03.0

Analysis Procedure: Equivalent Lateral Force

\* Ordinary Steel Concentrically Braced Frame(s)
and/or Ordinary Steel Moment Frame(s)

ROOF PANEL

Profile: Super Seam—Plus Gauge: 24 Color: Galvalume Plus
UL580 Class 90: Yes
Clip Type: High Float

WALL PANEL

Profile: Super Span X Gauge: 26 Color: Ash Gray

APPROVAL SPECIFICATIONS

- 1. Approval of the Metal Building Provider drawings and/or calculations indicate that the Metal Building Provider has correctly interpreted the contact requirements. This approval constitutes the customer acceptance of the Metal Building Provider design, concepts, assumptions, and loadings.
- 2. Failure to respond to clouded areas and areas to verify may result in additional costs and/or schedule delays for which the Metal Building Provider will not be responsible.
- 3. Any changes made after the Metal Building Provider's customer has signed and returned the Metal Building Provider drawings and/or calculations and the project is released for fabrication shall be billed to the Metal Building Provider customer including material, engineering, and other costs. An additional fee may be charged if the project must be moved in the fabrication and/or the shipping schedule.
- 4. It is the responsibility of the customer to field verify all existing conditions prior to fabrication.
- 5. It is imperative that any changes to these drawings:
- 5.1. Be made in contrasting ink.
- 5.2. Be legible and unambiguous.
- 5.3. Have all instances of changes clearly indicated.
- 6. A dated signature, in the designated areas, is required on all pages. The signature must be from the person authorized on the contract or a person authorized, in writing, by the Metal Building Provider customer.
- 7. The Metal Building Provider reserves the right to resubmit drawings with extensive or complex changes required to avoid misfabrication. This may impact the delivery schedule.
- 8. Any changes noted on the drawings not in conformance with the terms and requirements of the contract between the Metal Building Provider and its customer are not binding on the Metal Building Provider unless subsequently acknowledged and agreed to in writing by change order or separate documentation.
- 9. Waiving the approval process by designating the order "For Production" supercedes notes 1,2,5,6, and 8 in this section, and constitutes the customer acceptance of the Metal Building Provider's design, concepts, assumptions, and loadings.

PRIMARY FRAMING
Built-Up & Hot-Rolled: Gray Oxide Primer

SECONDARY FRAMING

Pre-Galvanized
Pre-Galvanized
: Pre-Galvanized

Hot-Dip Galvanizing conforms to the ASTM A123 specification. Pre-Galvanized members conform to the ASTM A653, Grade 50, Coating G-90 specification.

	<u> </u>	MINO	JULILIOLL
WG NO.	ISSUE	DATE	DESCRIPTION
C1	P2	05.28.24	COVER SHEET
F1	1	05.28.24	ANCHOR BOLT PLAN
F2	1	05.28.24	ANCHOR BOLT DETAILS
F3	0	05.09.24	ANCHOR BOLT REACTIONS
P1	P1	05.09.24	RIGID FRAME ELEVATION
P2	P1	05.09.24	RIGID FRAME ELEVATION
E1	P1	05.09.24	ROOF FRAMING PLAN
E2	P1	05.09.24	ROOF SHEETING PLAN
E3	P1	05.09.24	ENDWALL FRAME & SHEETING ELEVATION
E4	P1	05.09.24	ENDWALL FRAME & SHEETING ELEVATION
E5	P1	05.09.24	SIDEWALL FRAME & SHEETING ELEVATION
E6	P1	05.09.24	SIDEWALL FRAME & SHEETING ELEVATION
E7-E10	P1	05.09.24	SECTION DETAILS PAGE
E11	P1	05.09.24	ROOF CURB DETAILS
D1	P1	05.09.24	STANDARD DETAILS PAGE
D2	P1	05.09.24	STANDARD DETAILS PAGE

DRAWING SCHEDULE

General

-X-Bracing is to be installed to a taut condition with all slack removed. Do not tighten beyond this state.

-Framed openings, walk doors, and open areas shall be located in the bay and elevation as shown in the erection drawings. The cutting or removal of girts shown on the erection drawings due to the addition and/or relocation of framed openings, walk doors, or open areas not shown may void the design certifications supplied by the metal building manufacturer.

-This metal building system is designed as enclosed. All exterior components not by the metal building manufacturer (i.e. doors, windows, vents, etc.) must be designed to withstand the specified wind loading for the design of components and cladding in accordance with the specified building code. Doors are to be closed when a maximum of 50% of design wind velocity is reached.

-The design collateral load has been uniformly applied to the design of the building. Hanging loads are to be attached to the purlin web. This may not be appropriate for heavily concentrated loads. Any attached load in excess of 150 pounds shall be accounted for by special design performed by a licensed engineer using concentrated loads and may require separate support members within the roof system.

-Drift snow is applied in accordance with the building code. The framing provided by WWSB is designed with Drift Snow at the following locations:

-Drift snow is applied in accordance with the building code. The framing provided by WWSB is designed with Drift Snow at the following locations:

Trapezoidal Surcharge: 10.9 psf to 0 psf with width of 5.7 ft wide at Line 1 between Lines A & C.8 Trapezoidal Surcharge: 31.3 psf to 0 psf with width of 8.2 ft wide at Line 1 between Lines C.8 & E.2 Trapezoidal Surcharge: 10.9 psf to 0 psf with width of 5.7 ft wide at Line 1 between Lines E.2 & H

Roof top units are to be supported by beams. The framing provided by the manufacturer has been designed with roof top units at the following locations. Purlins have been designed to be cut for

installation and support of RTU. The dimensions shown are to the center of the unit.

1,600 lbs RTU located 50' from BSW and 23'-6", 123'-6" from LEW.
 1,800 lbs RTU located 50' from BSW and 48'-6", 86'-6" from LEW.

ther Construction

-The support member provided by the metal building manufacturer has been designed to support the wall system not by metal building manufacturer and to deflect less than L/240 under wind or seismic loading. Maximum weight of wall material considered is 30 psf

TRIM COLOR	<u>:</u>		
GUTTER:	SMP STEEL GRAY	GAUGE:	26 GA
RAKE:	SMP STEEL GRAY	GAUGE:	26 GA
CORNER:	SMP ASH GRAY	GAUGE:	26 GA
ACCESSORY:	SMP ASH GRAY	GAUGE:	26 GA
DOWNSPOUT:	SMP ASH GRAY	GAUGE:	26 GA
BASE:	SMP ASH GRAY	GAUGE:	26 GA

OTHER LOADS:

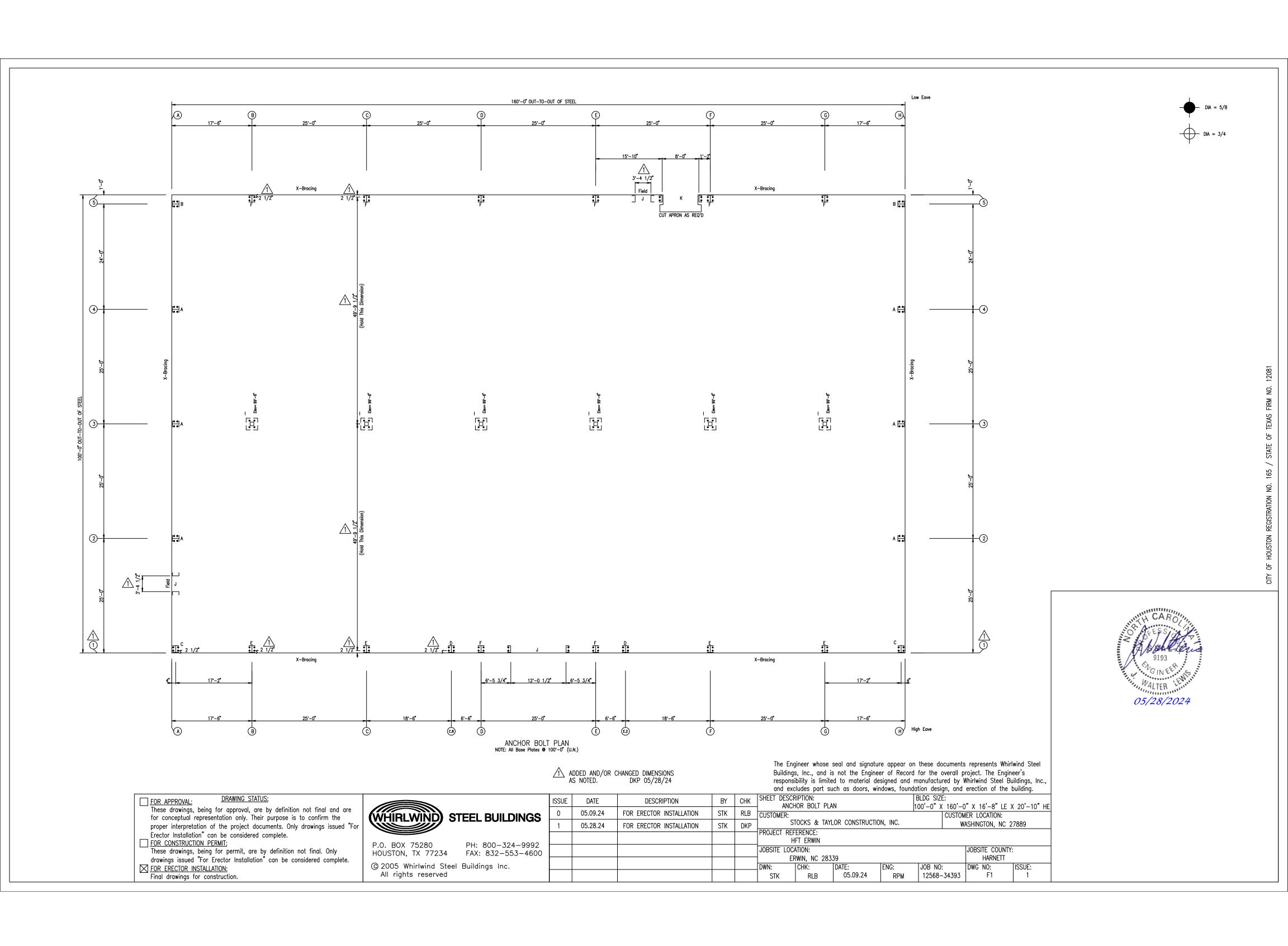
(2) 1,600 lbs RTU

(2) 1,800 lbs RTU

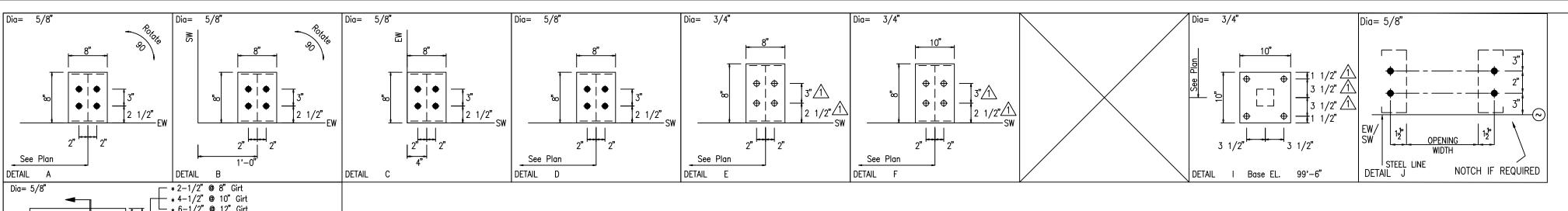
The Engineer whose seal and signature appear on these documents represents Whirlwind Steel Buildings, Inc., and is not the Engineer of Record for the overall project. The Engineer's responsibility is limited to material designed and manufactured by Whirlwind Steel Buildings, Inc., and excludes part such as doors, windows, foundation design, and erection of the building.

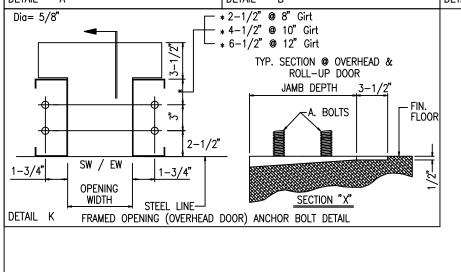
							ana exc	ciuaes part	such as abors, w	indows, tound	aation aesign, ana	erection of the	building.
FOR APPROVAL:  DRAWING STATUS:		ISSUE	DATE	DESCRIPTION	BY	СНК	SHEET DESC	RIPTION: COVER	CHEET		BLDG SIZE: 100'-0" X 160'-	`` ∨ 16' 9" I ⊏	· V 20' 10" UE
These drawings, being for approval, are by definition not final and are for conceptual representation only. Their purpose is to confirm the	(WHIRLWIND) STEEL BUILDINGS	P1	05.09.24	FOR CONSTRUCTION PERMIT	STK	RLB	CUSTOMER:					MER LOCATION:	X 20 - 10 HE
proper interpretation of the project documents. Only drawings issued "For		P2	05.28.24	FOR CONSTRUCTION PERMIT	STK	DKP			AYLOR CONSTRUCT	ION, INC.	w	ASHINGTON, NC	27889
Erector Installation" can be considered complete.							PROJECT REF	-ERENCE: IFT FRWIN					
FOR CONSTRUCTION PERMIT:  These drawings, being for permit, are by definition not final. Only	P.O. BOX 75280 PH: 800-324-9992 HOUSTON, TX 77234 FAX: 832-553-4600						JOBSITE LOC	ATION:				JOBSITE COUNT	iΥ:
drawings issued "For Erector Installation" can be considered complete.							EI	RWIN, NC 28	8339	TENO	LIOD NO	HARNETT	LICOUE
FOR ERECTOR INSTALLATION: Final drawinas for construction.	© 2005 Whirlwind Steel Buildings Inc. All rights reserved						J <sup>DWN:</sup> STK	CHK:	DATE: 05.09.24	ENG: RPM	JOB NO: 12568-34393	DWG NO:	ISSUE: P2













OMITTED DETAIL 'H' & ADDED DIMENSIONS AT DETAIL 'I' FOR CLARITY. ALSO CHANGED DIM'S AT DETAILS 'E' & 'F'. DKP 05/28/24 The Engineer whose seal and signature appear on these documents represents Whirlwind Steel Buildings, Inc., and is not the Engineer of Record for the overall project. The Engineer's responsibility is limited to material designed and manufactured by Whirlwind Steel Buildings, Inc., and excludes part such as doors, windows, foundation design, and erection of the building.

HEET DESCRIPTION:

BLDG SIZE:

FOR APPROVAL:  DRAWING STATUS:
These drawings, being for approval, are by definition not final and are
for conceptual representation only. Their purpose is to confirm the
proper interpretation of the project documents. Only drawings issued "For
Erector Installation" can be considered complete.
FOR CONSTRUCTION PERMIT:
These drawings, being for permit, are by definition not final. Only

FOR CONSTRUCTION PERMIT:

These drawings, being for permit, are by definition not final. Only drawings issued "For Erector Installation" can be considered complete.

FOR ERECTOR INSTALLATION:

Final drawings for construction.

WH	IIRL	.WIND	STEEL	. BUILDINGS
РΛ	BOX	75280	DH.	800-324-9992

P.O. BOX 75280	PH: 800-324-9992
HOUSTON, TX 77234	FAX: 832-553-4600
2005 Whirlwind Steel All rights reserved	Buildings Inc.

	ISSUE	DATE	DESCRIPTION	BY	CHK	SHEET DESC		TAILC		BLDG SIZE:	." ∨ 40' 0" ⊢ \	, 00° 40° UE
1	0	05.09.24	FOR ERECTOR INSTALLATION	STK	RLB	CUSTOMER:	HOR BOLT DE	TIAILS		100'-0" X 160'-0   CUSTOM	) X 16 -8 LE X IER LOCATION:	( 20 – 10 HE
'	1	05.28.24	FOR ERECTOR INSTALLATION	STK	DKP			YLOR CONSTRUCTIO	N, INC.	W/	ASHINGTON, NC 2	7889
						PROJECT RE	FERENCE: HFT ERWIN					
						JOBSITE LOC					JOBSITE COUNTY:	
							RWIN, NC 28				HARNETT	
						DWN:	CHK:	DATE:	ENG:	JOB NO:	DWG NO:	ISSUE:
						STK	RLB	05.09.24	RPM	12568-34393	F2	1



MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Column\_Reactions(k )
Load Hmax V Load Hmin V Bolt(in) Base\_Plate(in)
Id H Vmax Id H Vmin Qty Dia Width Length

FOR APPROVAL:

0.3 22 -1.8 0.3 4 0.625 8.000 8.000 0.375 0.0 0.5

0.3 22 -1.8 0.3 4 0.625 8.000 8.000 0.375 0.0 0.5

FOR CONSTRUCTION PERMIT:

FOR ERECTOR INSTALLATION: Final drawings for construction.

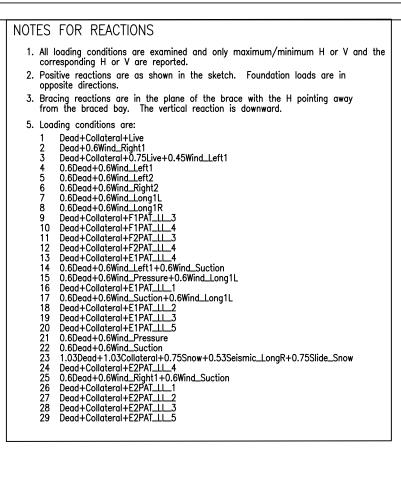
— Column\_Reactions(k )

SOLDIER COLUMN:

Frm Col Line Line

C.8 E

E.2 E 21 23



RIGIE	FRAN	ЛЕ:	BASI	C COLUM	n reacti	ONS (k )							
Frame Line B* B* B*		Horz 0.2 -0.2 0.0	-Dead Vert 2.3 2.3 5.8	Horz 0.4	Vert 3.0	Horz 0.7 -0.7 0.0	-Live Vert 6.0 6.0 18.0	Horz 0.4 -0.4 0.0	-Snow Vert 3.5 3.5 10.5	Snow Horz 0.0 0.0 0.0	v_Drift- Vert 0.0 0.8 0.0	Wind Horz -5.6 -2.6 0.0	_Left1− Vert −12.9 −11.5 −34.6
Frame Line B* B* B*	Column Line 5 E 3	Horz 6.4 4.7	Right1- Vert -5.7 -8.3 -23.4	Horz	Vert −7.8 −6.4	-Wind_ Horz 5.4 5.7 0.0	Vert -0.6	Horz 1.9	LLong1- Vert -14.6 -15.9 -34.8	Horz 1.8	_Long2- Vert -10.5 -11.5 -21.6	-Seism Horz -1.4 -0.7 0.0	ic_Left Vert -0.5 0.2 0.3
Frame Line B* B* B*	Column Line 5 E 3	Seismic Horz 1.4 0.7 0.0	Vert	-Seism Horz 0.0 0.0 0.0	Vert −1.8 −2.7	Horz	NOW Vert 5.0 5.0 15.0	Horz 0.1 -0.1	SL_1- Vert 1.9 -0.3 2.7	Horz 0.1	SL_2- Vert -0.2 2.0 2.6	F1PAT_L Horz 0.3 -0.3 0.0	L_3- Vert 6.7 -0.9 9.2
Frame Line B* B* B*		F1PAT_L Horz 0.4 -0.4 0.0	L_4- Vert -0.7 6.9 8.8										
Frame Line D* D* D*	Ē	Horz	-Dead Vert 2.5 2.5 5.7		Vert 3.0 3.1	Horz 0.9 -0.9 0.0	-Live Vert 6.0 6.1 17.9	Horz 0.5 -0.5 0.0	-Snow Vert 3.5 3.6 10.4	Snow Horz 0.0 0.0 0.0	v_Drift- Vert 0.0 2.4 0.1	Wind Horz -7.4 -3.0 0.0	Left1- Vert -13.5 -11.1 -34.4
Frame Line D* D* D*	Column Line 5 E 3	Horz 8.5	Right1- Vert -5.0 -9.2 -23.1	Horz -8.5	Left2- Vert -8.4 -5.9 -20.3	Horz 7.6	Right2- Vert 0.1 -4.1 -9.0	Horz 1.6	LLong1- Vert -12.1 -12.4 -34.6	Wind Horz 1.6 -1.9 0.0	_Long2- Vert -8.0 -7.9 -21.5	-Seism Horz -1.4 -0.7 0.0	ic_Left Vert -0.5 0.2 0.3
Frame Line D* D*	Column Line 5 E 3	Horz 1.4 0.7	Vert	-MIN_S Horz 0.7 -0.7 0.0	NOW Vert 5.0 5.1 14.9	F2PAT_S Horz 0.1 -0.1 0.0	SL_1- Vert 2.0 -0.2 2.7	Horz 0.1	SL_2- Vert -0.2 2.0 2.6	F2PAT_L Horz 0.4 -0.4 0.0	L_3- Vert 6.7 -0.8 9.1	F2PAT_L Horz 0.5 -0.5 0.0	L_4- Vert -0.6 6.9 8.7
	Frame lin Frame lin		B C F D E	G									

### GENERAL NOTES

- 1. All anchor bolts (by others) to have nuts and flat washers.
- All anchor bolts are designed to full S.A.E. diameters with cut threads. No substitutions are allowed.
- 3. The Metal Building Provider is not responsible for the design, materials and workmanship of the foundation. Anchor bolt plans prepared by the Metal Building Provider are intended to show only location, diameter, and projection of anchor bolts required to attach the Metal Building System to the foundation. The Metal Building Provider is responsible for providing to the Builder the loads imposed by the Metal Building System on the foundation. It is the responsibility of the End Customer to ensure that adequate provisions are made for specifying bolt embedment, bearing angles, the rades and/or other associated items embedded in the concrete foundation. tie rods, and/or other associated items embedded in the concrete foundation, as well as foundation design for the loads imposed by the Metal Building System, other imposed loads, and the bearing capacity of the soil and other conditions of the building site. This is typically the responsibility of the Design Professional or Engineer of Record, which is another reason that their involvement in the Construction Project from the outset is highly recommended. (2012 MBMA Metal Building Systems Manual, Section 3.2.2)

Š.

FIRM

TEXAS

9F

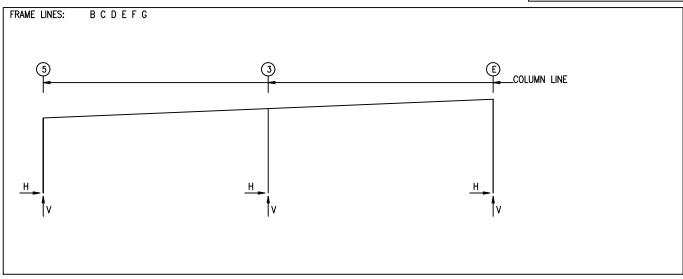
STATE

165

9

REGISTRATION

ΘF

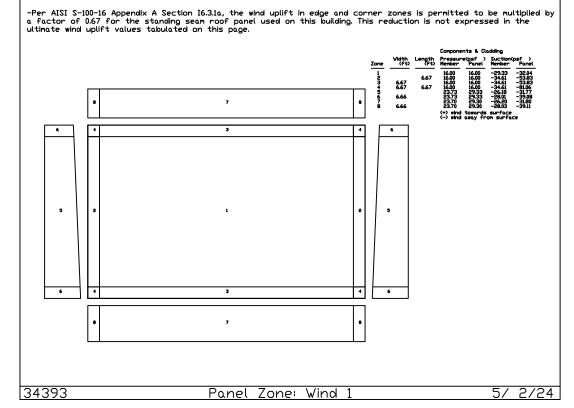


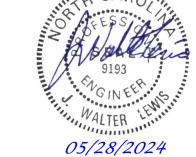
ANCH	OR BOLT	SUMMA Dia	ARY (GF	RADE 3 Proi
Qty	Locate	(in)	Туре	(in)
<b>♦</b> 8	Jamb	5/8"	F1554	2.50
40	Endwall	5/8"	F1554	2.50
<del>+</del> 48	Frame	3/4"	F1554	3.00
<b>♦</b> 8	Soldier	5/8"	F1554	2.50
<del>-</del> ◆ 24	Int. Col.	3/4"	F1554	2.50

——w	nll —	- Col	—— <del>*</del>	Reacti	ons(k ) - — Sei	emic –	Panel_Shea - (lb/ft)
Loc "	Line	Line	Horz "	Vert	Horz	Vert	Wind Sei
∟EW	A E	4,3	3.7	2.6	1.0	0.7	
F_SW	E	4,3 B,C F.G	4.9 4.9	3.7 3.7	3.6 3.6	2.7 2.7	
R_EW	H 5	F,G 3,4 G,F	3.7	2.6	1.0	0.7	
B_SW	5	G,F	4.5	2.6	3.1	1.8	
		C,B	4.5	2.6	3.1	1.8	

RIGID	FRAME:		MAXIMUM	REACTION	S, ANCH	IOR BOLTS	S, & BASE	PLATE	S				
Frm Line	Col Line	Load Id	Hmax H	umn_React V Vmax	tions(k ) Load Id	Hmin H	V Vmin	Bol Qty	t(in) Dia	Base Width	e_Plate(in) Length	Thick	Elev. (in)
B*	5	2	4.1 0.9	-1.1 11.9	5 7	-3.9 1.3	-3.3 -7.4	4	0.750	10.00	9.500	0.375	0.0
B*	Е	6 10	3.3 -1.0	-0.5 12.2	3 7	-2.3 -0.9	4.7 -8.2	4	0.750	8.000	9.500	0.375	0.0
B*	3	7 1	0.0 0.0	-17.4 34.4	8	0.0	-17.4	4	0.750	10.00	10.00	0.375	-6.0

RIGID	FRAME:		MAXIMUM	REACTION	S, ANCH	OR BOLTS	S, & BASE	PLATE	S				
Frm Line	Col Line	Load Id	Hmax H	umn_React V Vmax	tions(k ) Load Id	Hmin H	V Vmin	Bol Qty	t(in) Dia	Base Width	e_Plate(in) Length	Thick	Elev. (in)
D*	5	2 11	5. <b>4</b> 1.1	-0.5 12.2	5 4	-4.9 -4.3	-3.5 -6.7	4	0.750	10.00	9.500	0.375	0.0
D*	E	6 12	3.9 -1.2	-1.0 12.4	3 8	-2.7 -0.7	5.1 -6.0	4	0.750	10.00	9.500	0.375	0.0
D*	3	8 1	0.0 0.0	-17.3 33.7	8	0.0	-17.3	4	0.750	10.00	10.00	0.375	-6.0
D*	Frame lir	nes:	DE										



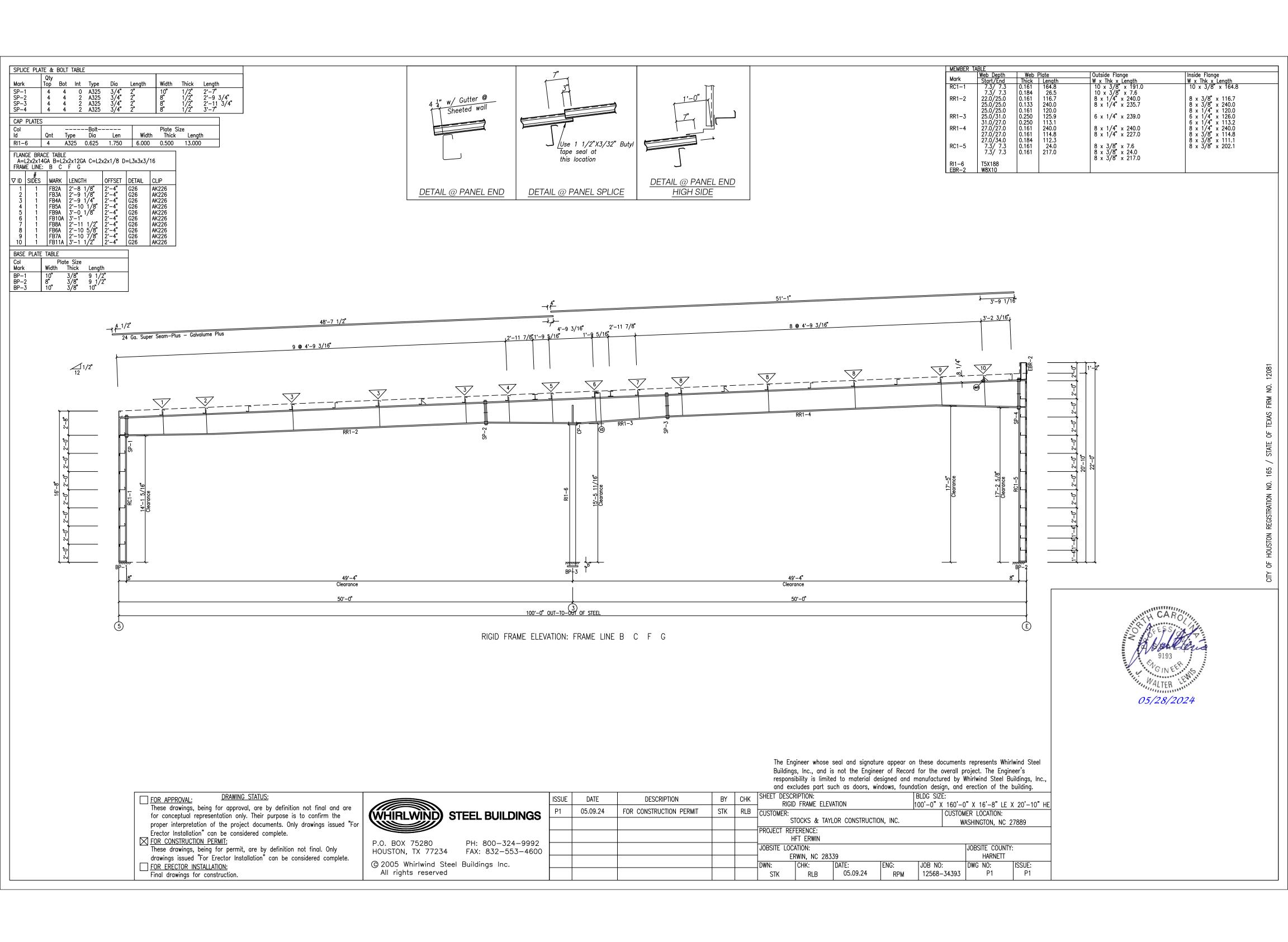


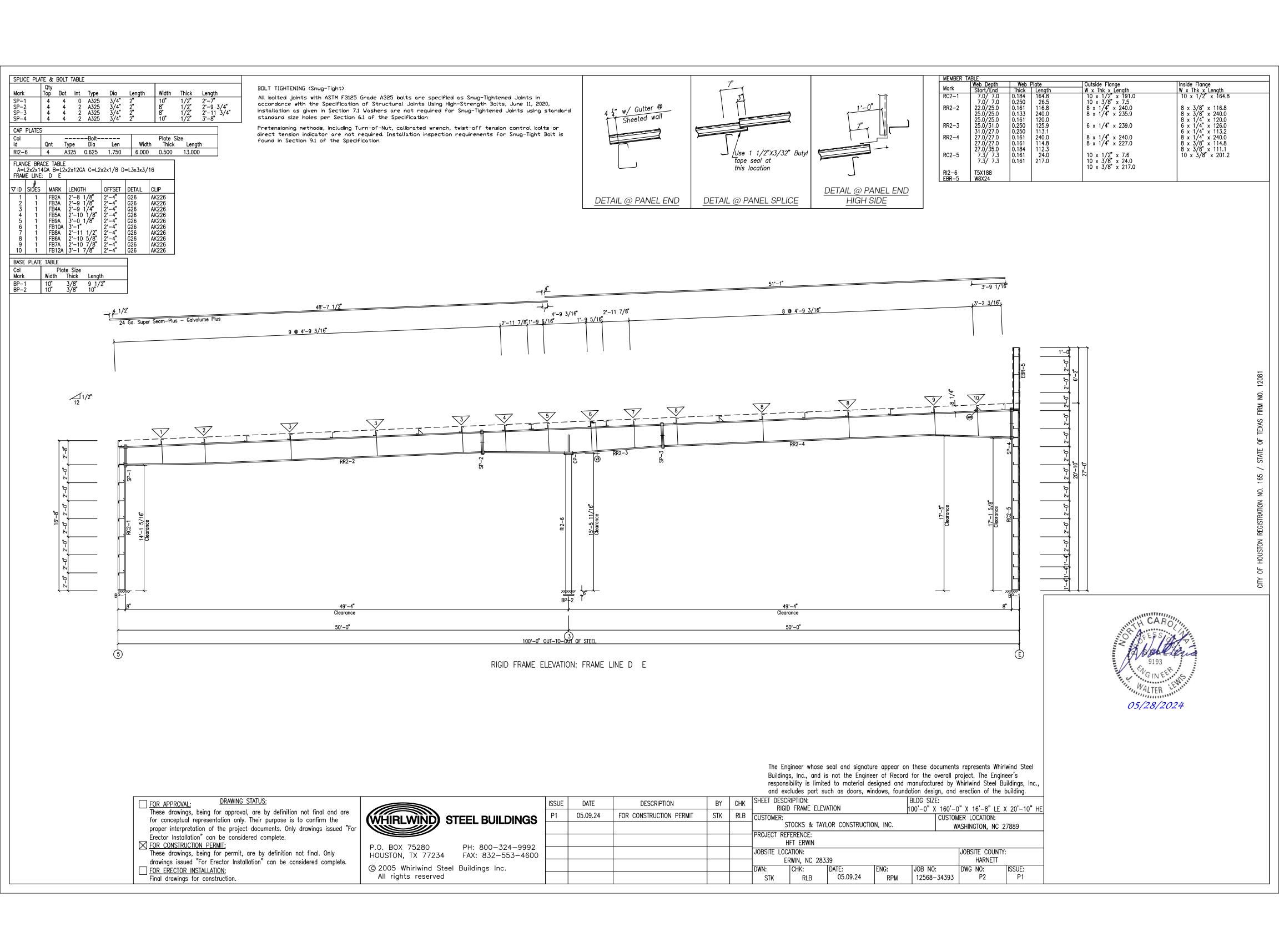
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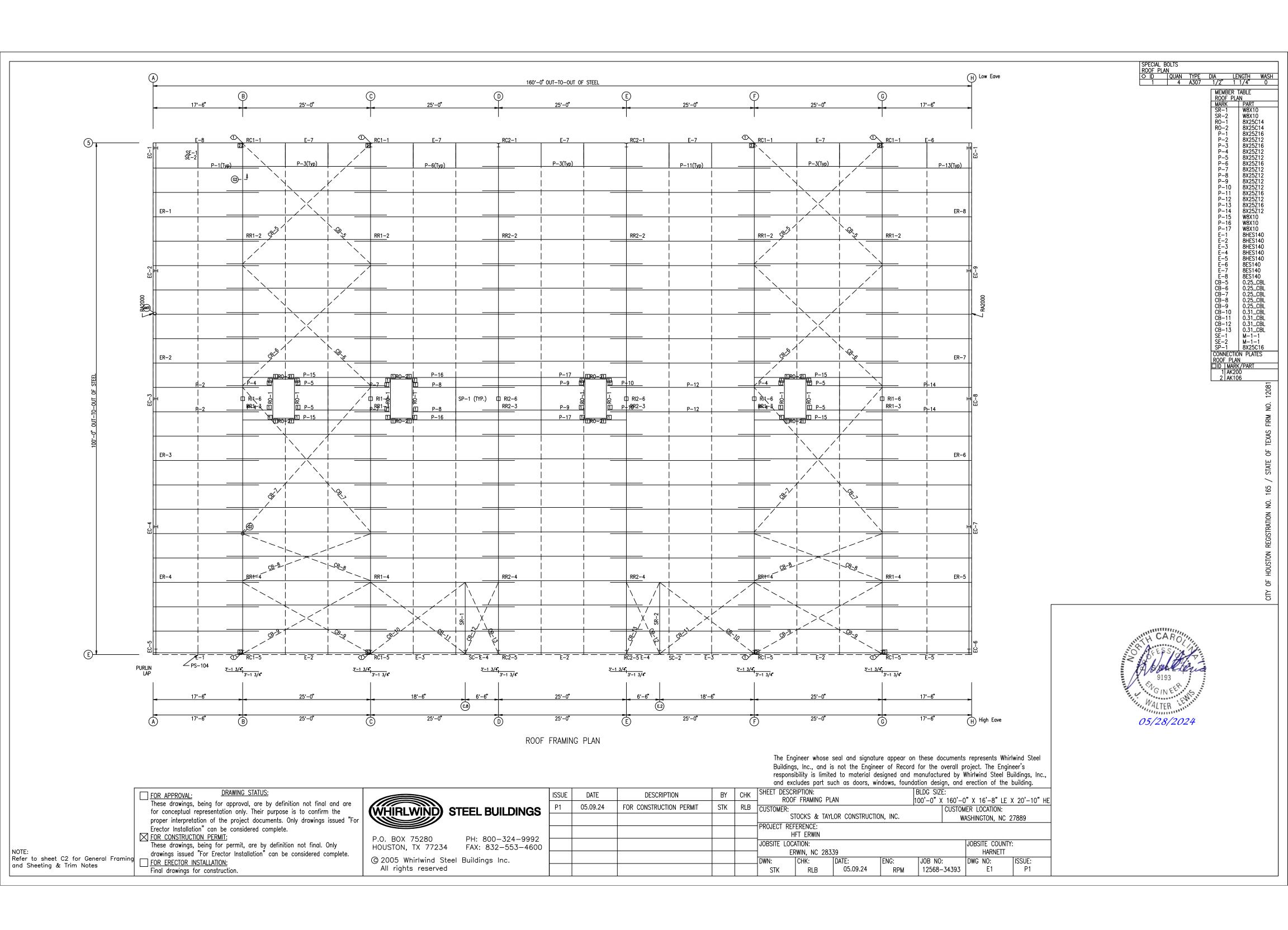
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FOR CONSTRUCTION PERMIT: These drawings, being for permit, are by definition not final. Only	P.O. BOX 75280 HOUSTON, TX 77234	PH: 800-324-9992 FAX: 832-553-4600
drawings issued "For Erector Installation" can be considered complete.		

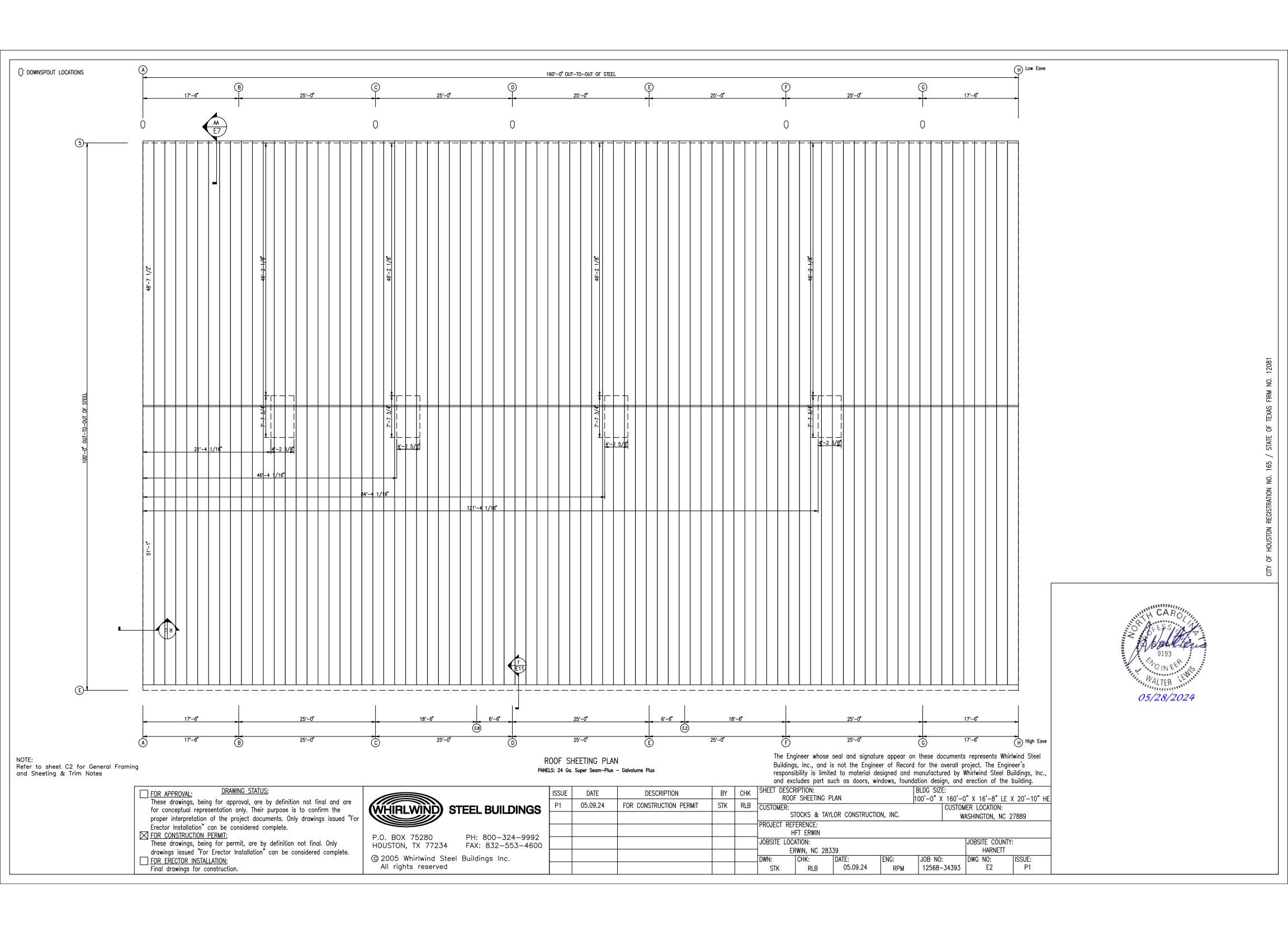
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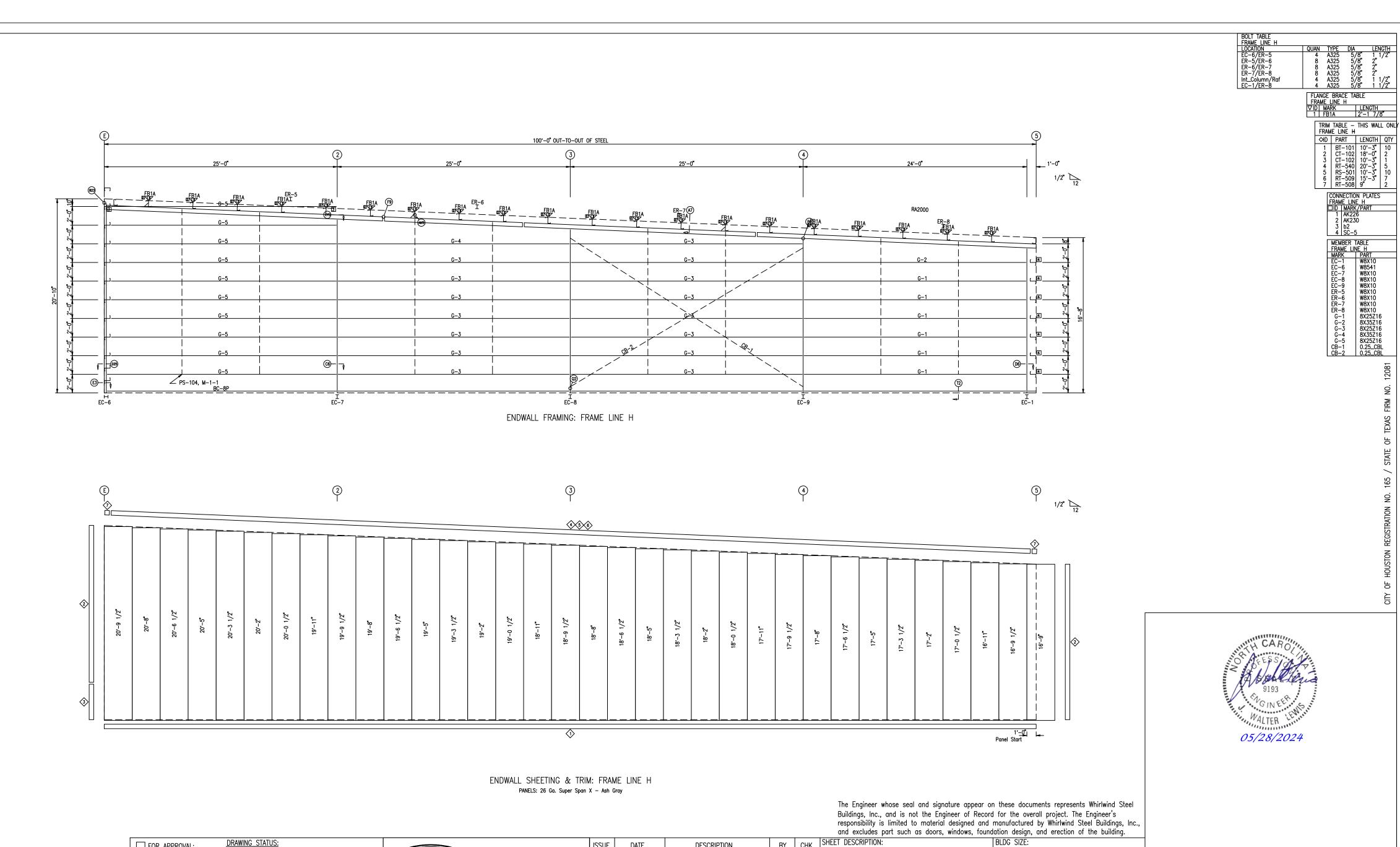
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92 600						PROJECT REFERENCE:  HFT ERWIN				mornitoring no 27000			
						JOBSITE LOCATION: ERWIN, NC 28339					JOBSITE COUNTY: HARNETT		
						DWN: STK	CHK: RLB	DATE: 05.09.24	ENG: RPM	JOB NO: 12568-34393	DWG NO: F3	ISSUE: 0	











NOTE: Refer to sheet C2 for General Framing and Sheeting & Trim Notes FOR APPROVAL:

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FOR CONSTRUCTION PERMIT:

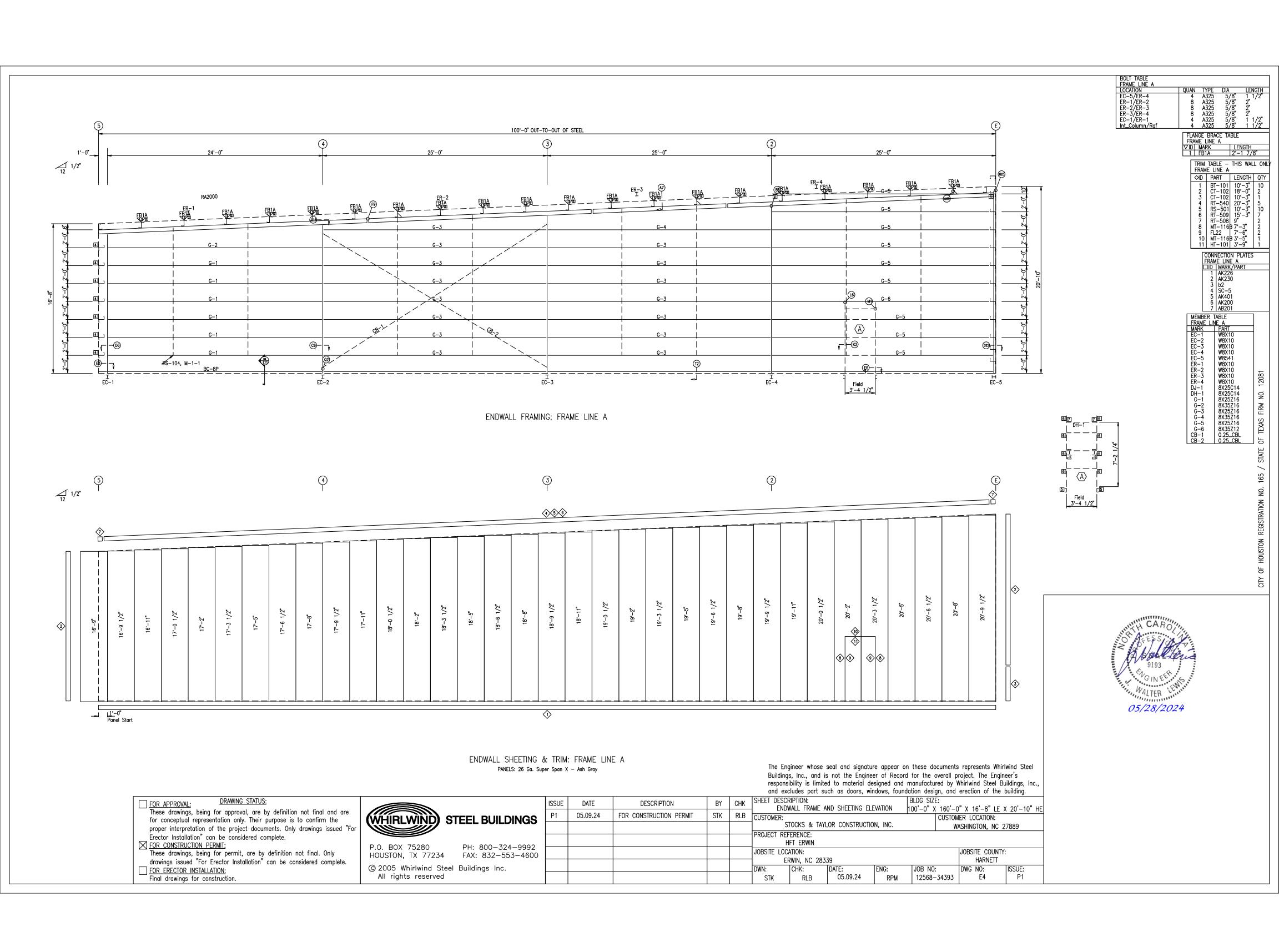
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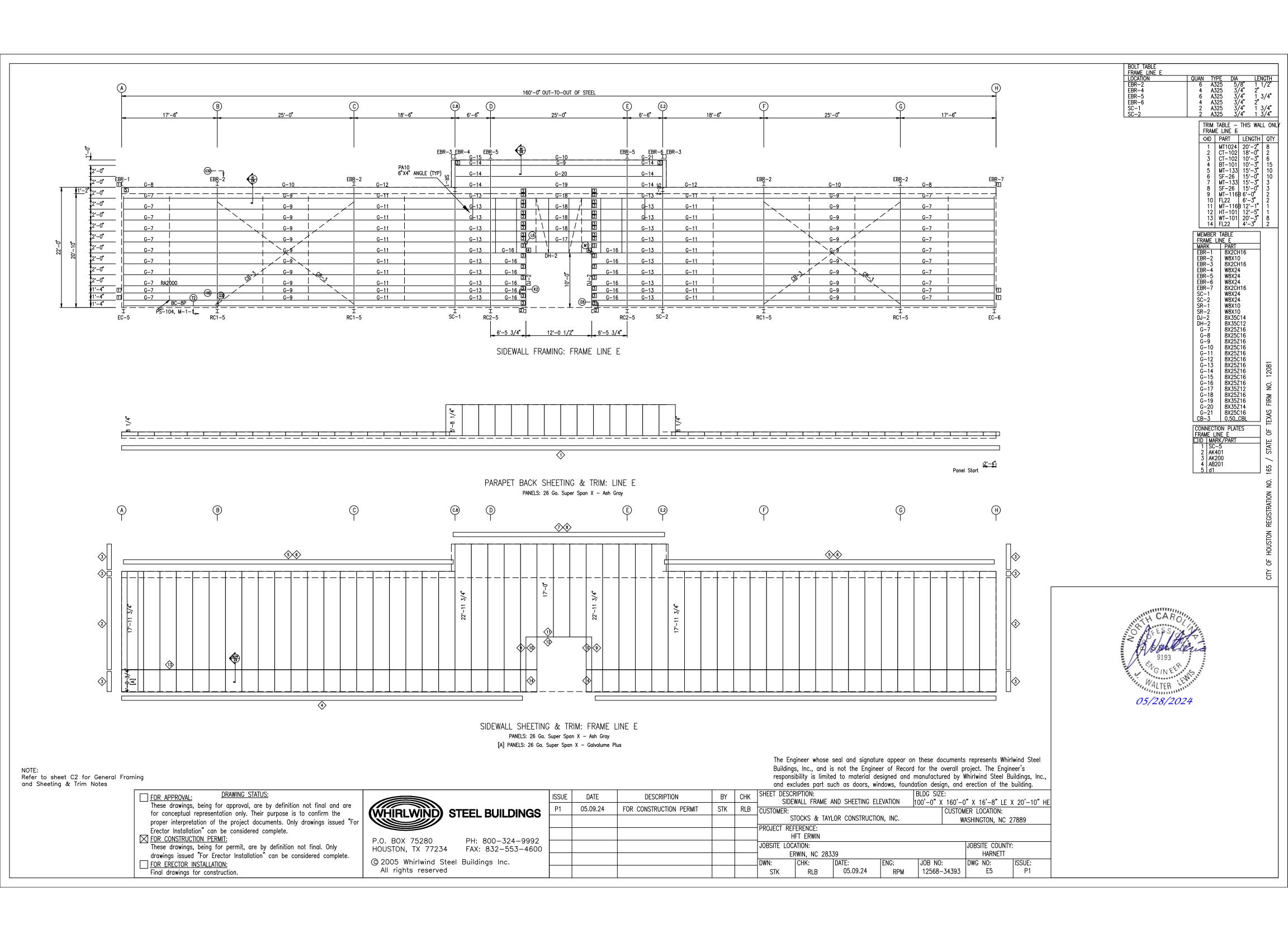
FOR ERECTOR INSTALLATION:
Final drawings for construction.

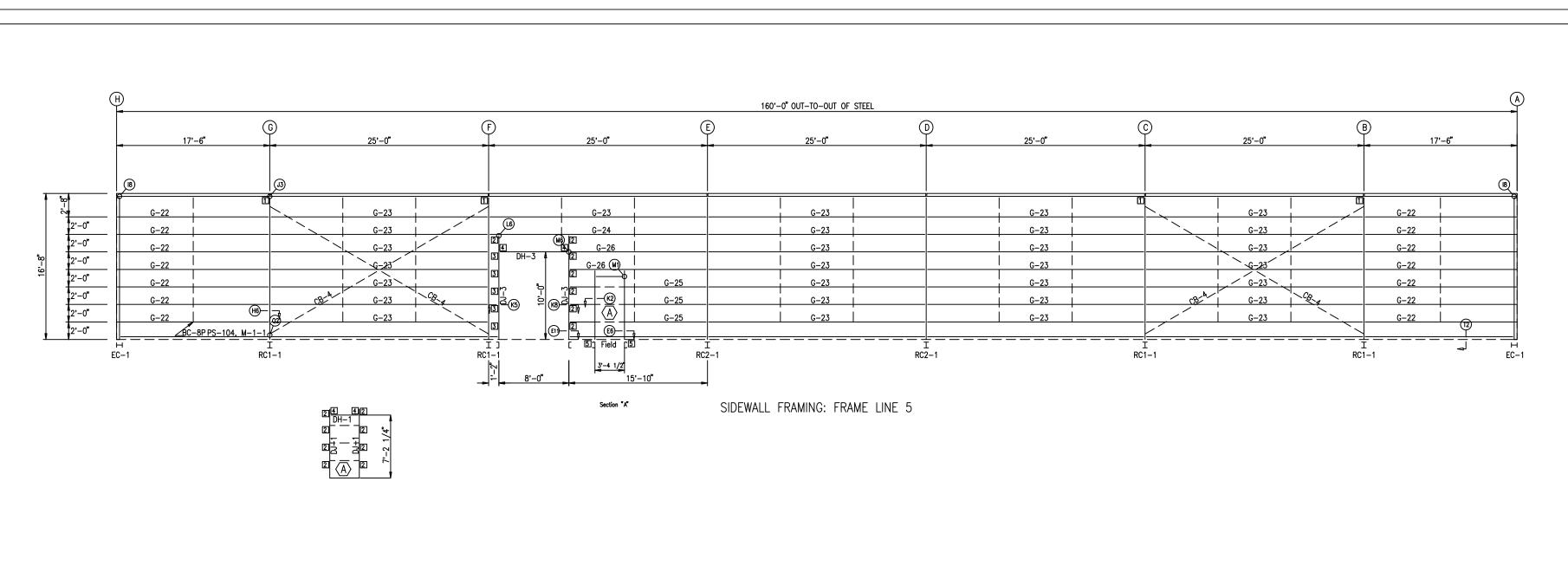
WHIRLWIND STEEL BUILDINGS

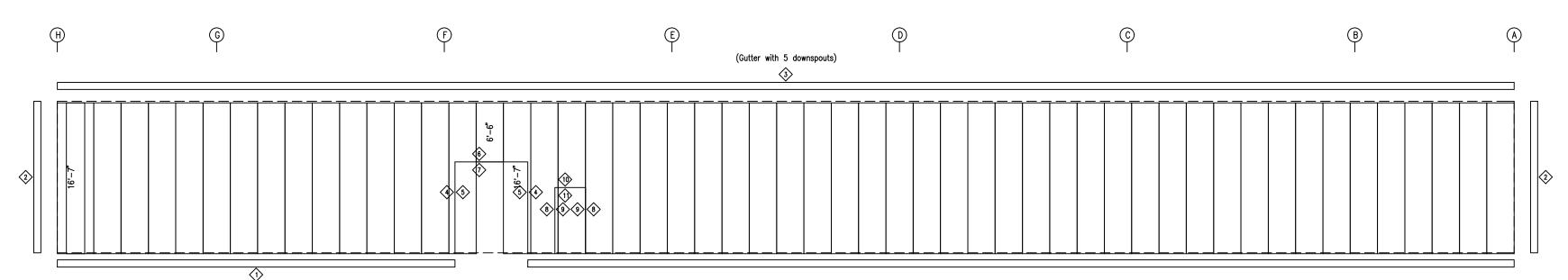
P.O. BOX 75280 PH: 800-324-9992 FAX: 832-553-460 PAX: 832-555-460 PAX: 832

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92 500						JOBSITE LOCA	ATION:			JOBSITE COUNTY:			
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SIDEWALL SHEETING & TRIM: FRAME LINE 5

PANELS: 26 Ga. Super Span X - Ash Gray

Refer to sheet C2 for General Framing and Sheeting & Trim Notes

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Erector Installation can be considered complete.

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| FOR ERECTOR INSTALLATION:
| Final drawings for construction.

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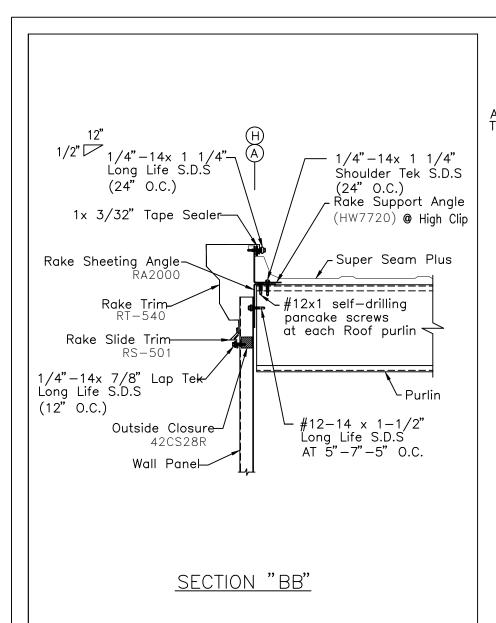
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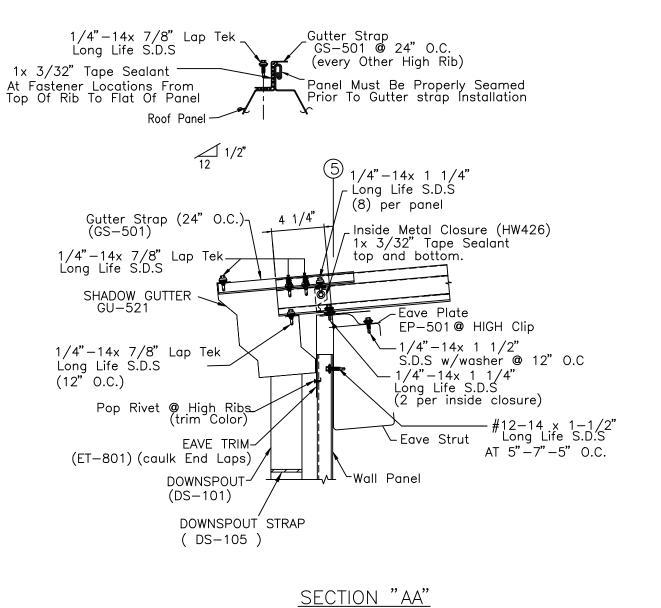
MEMBER TABLE
FRAME LINE 5

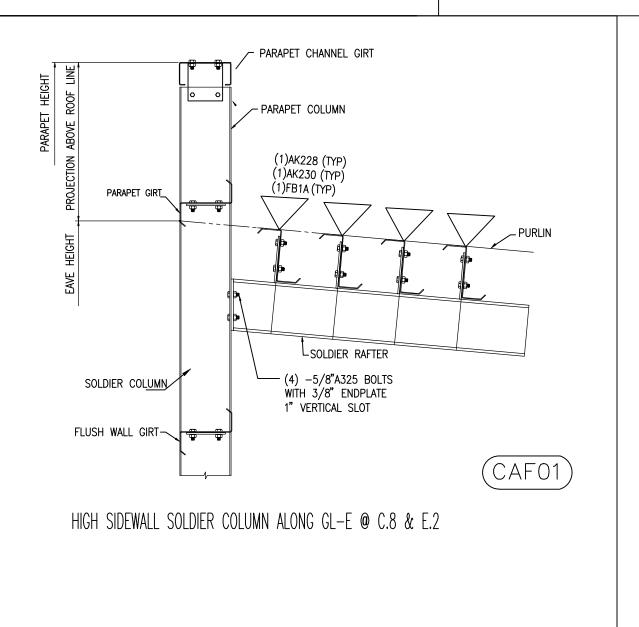
MARK PART
DJ-1 8X25C14
DJ-3 C8X11.5
DH-1 8X25C14
DH-3 C8X11.5
G-22 8X25Z16
G-23 8X25Z16
G-24 8X35Z12
G-25 8X25Z16
G-26 8X25Z16
G-26 8X25Z16
G-26 8X25Z16
G-27 RSZ5Z16
G-28 RSZ5Z16
G-28 RSZ5Z16
G-29 RS

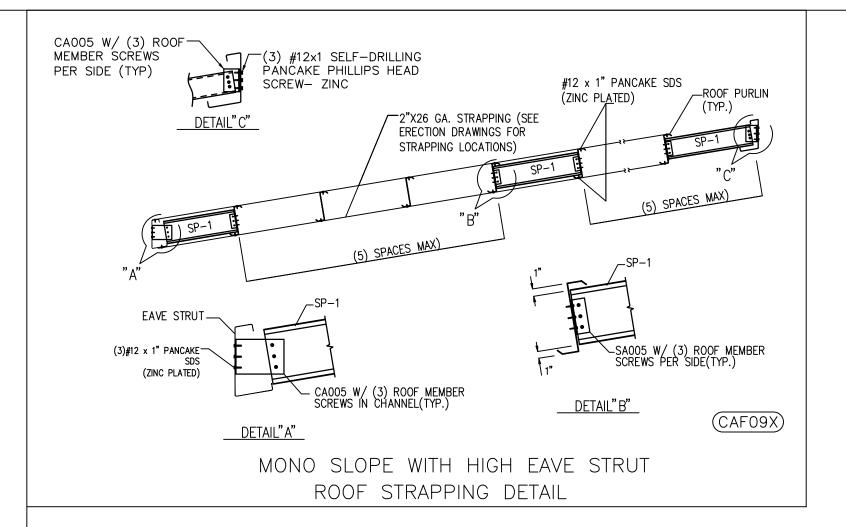
CONNECTION PLATES
FRAME LINE 5
DID | MARK/PART
1 | AK106
2 | AK200
3 | b4
4 | AB201
5 | AK401

OF HOUSTON REGISTRATION NO. 165 / STATE OF TEXAS FIRM NO. 12081











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FOR ERECTOR INSTALLATION:

Final drawings for construction.

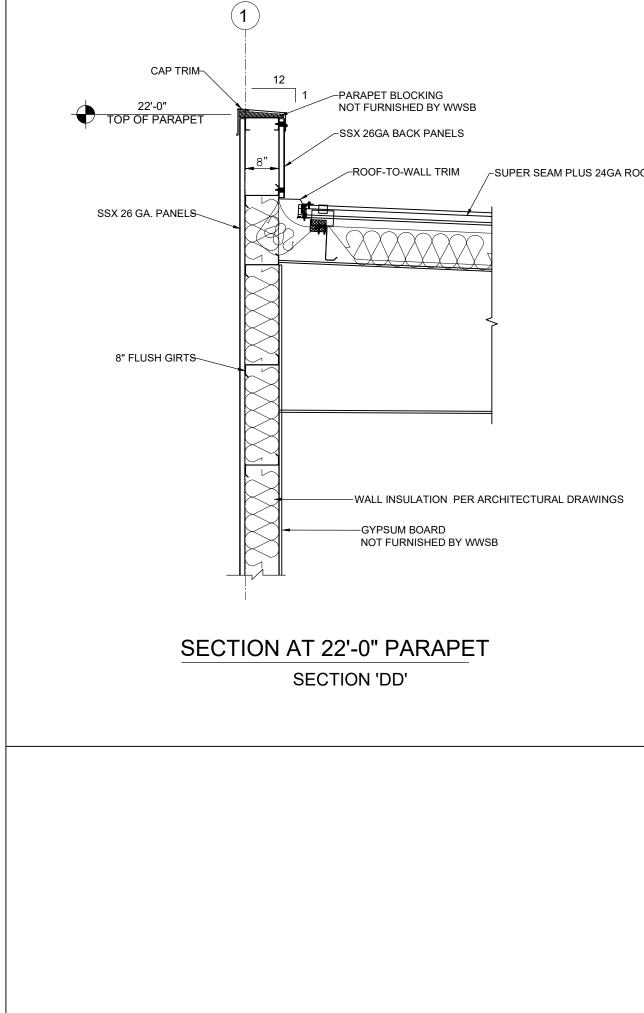
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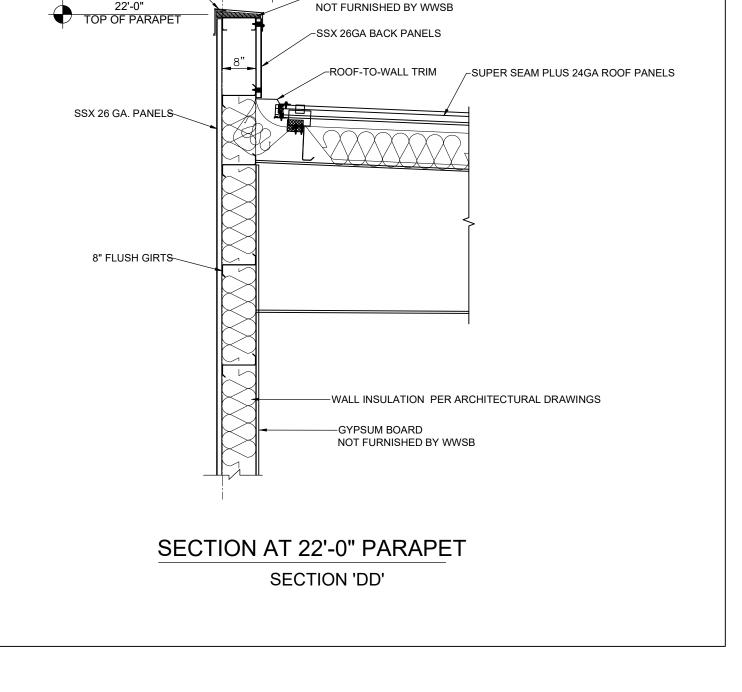
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<b>b</b>	STEEL BUILDINGS

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	ISSUE	DATE	DESCRIPTION	BY	CHK	SHEET DESCF SECT	RIPTION: TON DETAILS F	PAGE		BLDG SIZE: 100'-0" X 160'-0	" X 16'–8" IF X	x 20'-10" HF		
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**SECTION AT 27'-0" PARAPET** 

SECTION 'CC'

TWO PIECE CAP TRIM

1 ½ EIFS

-PARAPET BLOCKING

NOT FURNISHED BY WWSB

-ROOF-TO-WALL TRIM

-GYPSUM BOARD

NOT FURNISHED BY WWSB

\_SUPER SEAM PLUS 24GA ROOF PANELS

-WALL INSULATION PER ARCHITECTURAL DRAWINGS

-SUPER SPAN 26GA BACK PANELS

27'-0"

TOP OF PARAPET

L7x3x16GA (LLH) SHEETING-/

ANGLE PROVIDED BY WW FOR PARAPET CAP SUPPORT.

WITH Properties DENSGLASS SHEATHING NOT FURNISHED BY WWSB

GENERAL CONTRACTOR TO PROVIDE (2) LAYERS 1/2" CDX STRUCTURAL GRADE FRT PLYWOOD SHEATHING - OUTSIDE FACE OF PLYWOOD SHALL

BE FLUSH WITH OUTSIDE FACE OF FLTWOOD SHALL
BE FLUSH WITH OUTSIDE FACE OF WALL GIRTS.
ATTACH PLYWOOD TO SHEETING ANGLES AND
VERTICAL SAG ANGLES WITH #12 TEK SCREWS AT
6" O.C. AROUND PERIMETER.

GENERAL CONTRACTOR TO PROVIDE CONTINUOUS

L6x4x16GA (LLV) SHEETING ANGLE PROVIDED BY WW INSET 1" FROM THE OUTSIDE FACE OF WALL GIRTS

GENERAL CONTRACTOR TO PROVIDE CONTINUOUS 2X8 PRESSURE TREATED WOOD BLOCKING FOR AWNING ANCHORING - COORDINATE LOCATIONS WITH AWNING VENDOR.

AWNING FRAMING, SHEETING & TRIM

HEADER HEIGHT

NOT FURNISHED BY WWSB

2X8 PRESSURE TREATED WOOD BLOCKING FOR SIGNAGE ANCHORING - COORDINATE LOCATIONS

18'-0" GIRT ELEVATION

SIGN NOT FURNISHED BY WWSB

FOR PLYWOOD ATTACHMENT.

NOT FURNISHED BY WWSB

WITH SIGN VENDOR.

LAG BOLT-

SPECIAL HEADER TRIM

STOREFRONT NOT FURNISHED BY WWSB

ALUMINUM-

RR SSX 24 GA. PANEL<del>S</del>

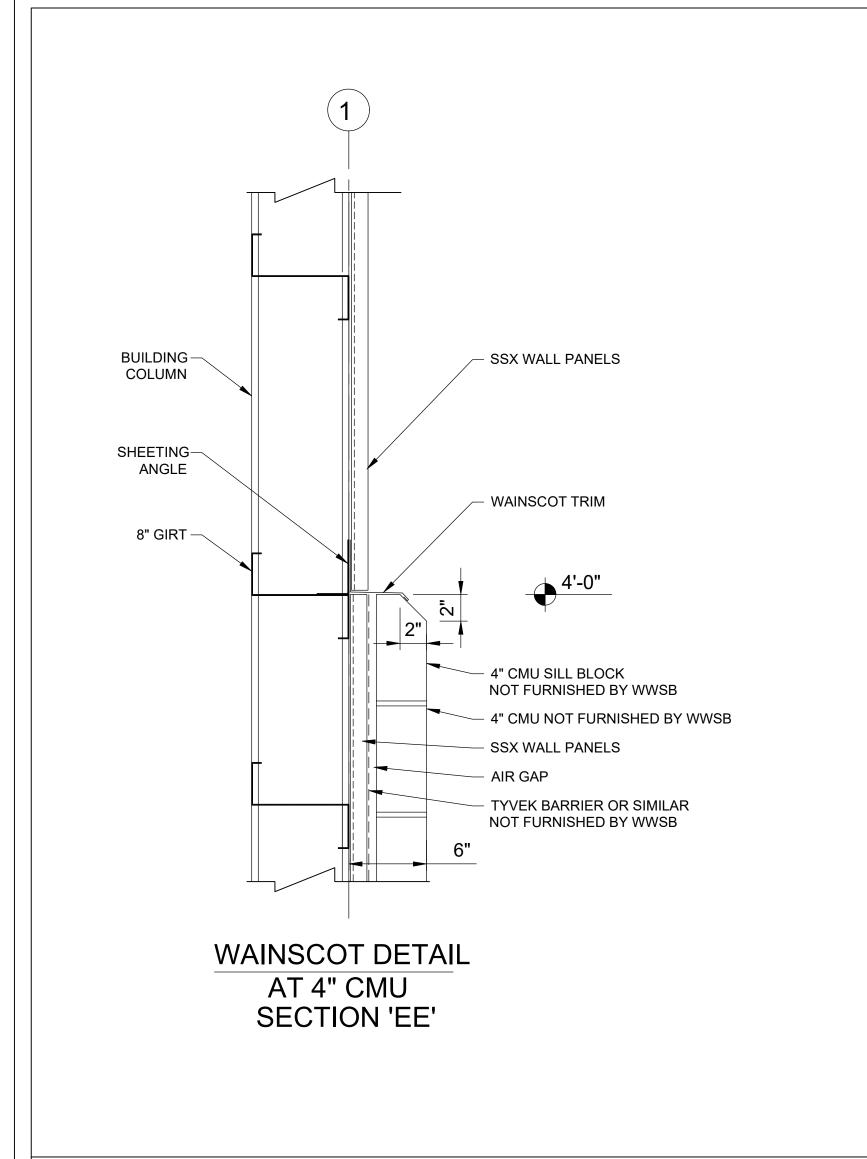
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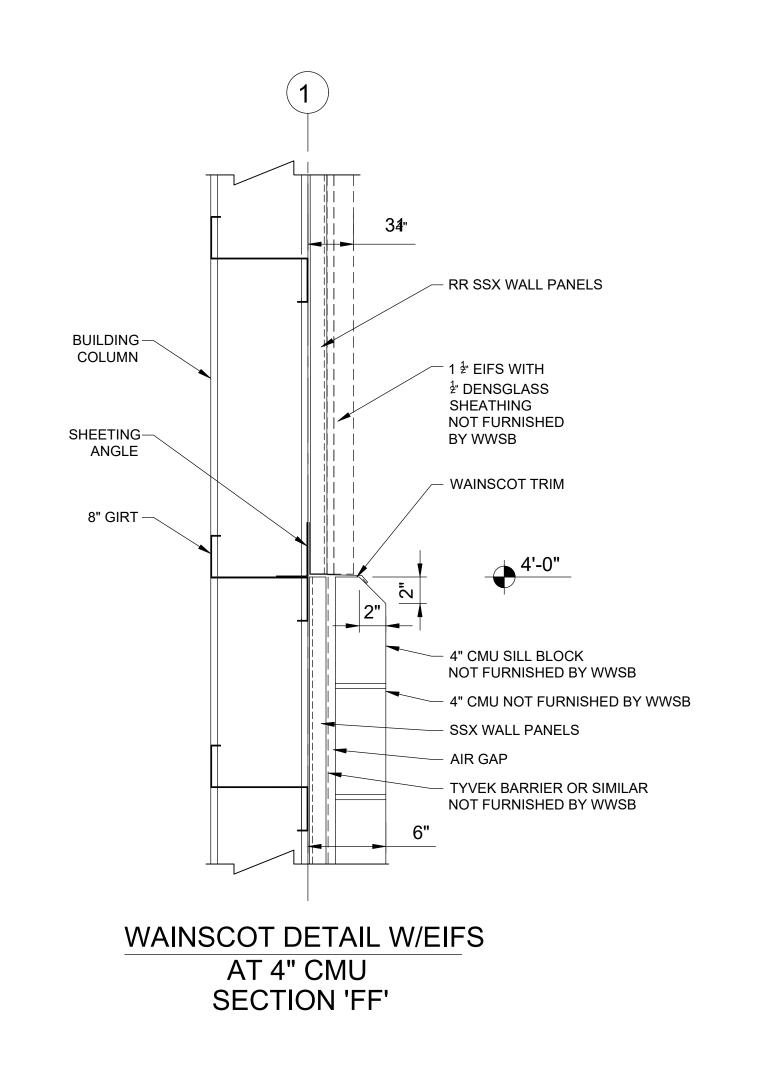
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PΛ	ROY	75280	DH.	800-324-9992

. BOX 75280 JSTON, TX 77234	PH: 800-324-9992 FAX: 832-553-4600

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HOUSTON, TX 77234	FAX: 832-553
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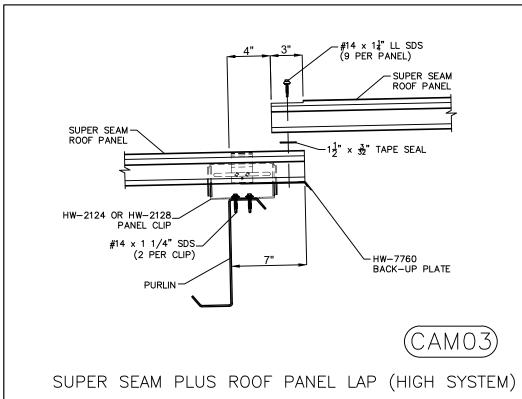
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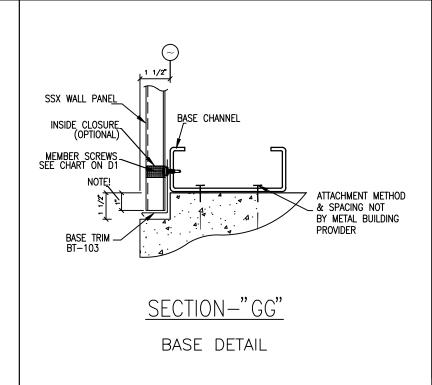
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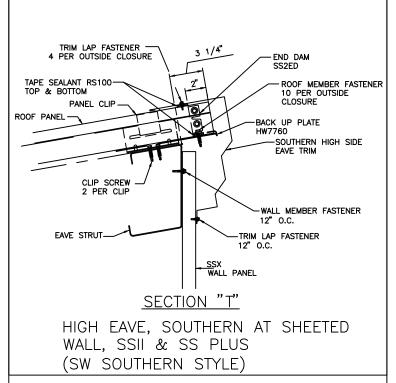


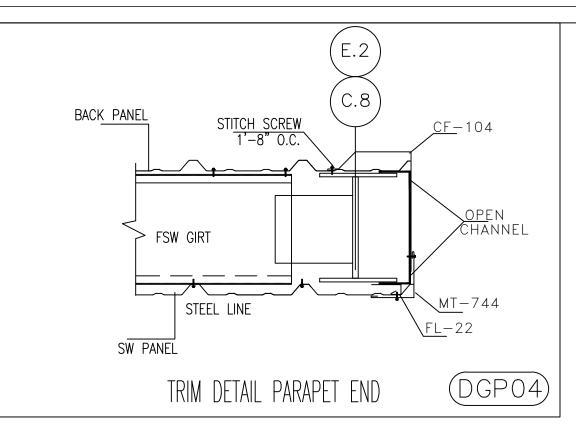
WHILEWIND STEEL BOILDINGS						
D.O. DOY 75000	DII 000 704 0000					
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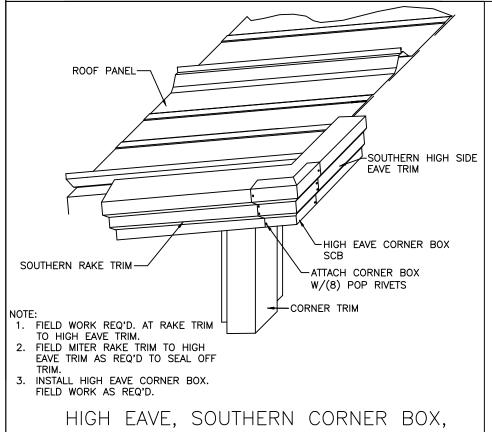
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SUPER SEAM II & SUPER SEAM PLUS (SW SOUTHERN STYLE)



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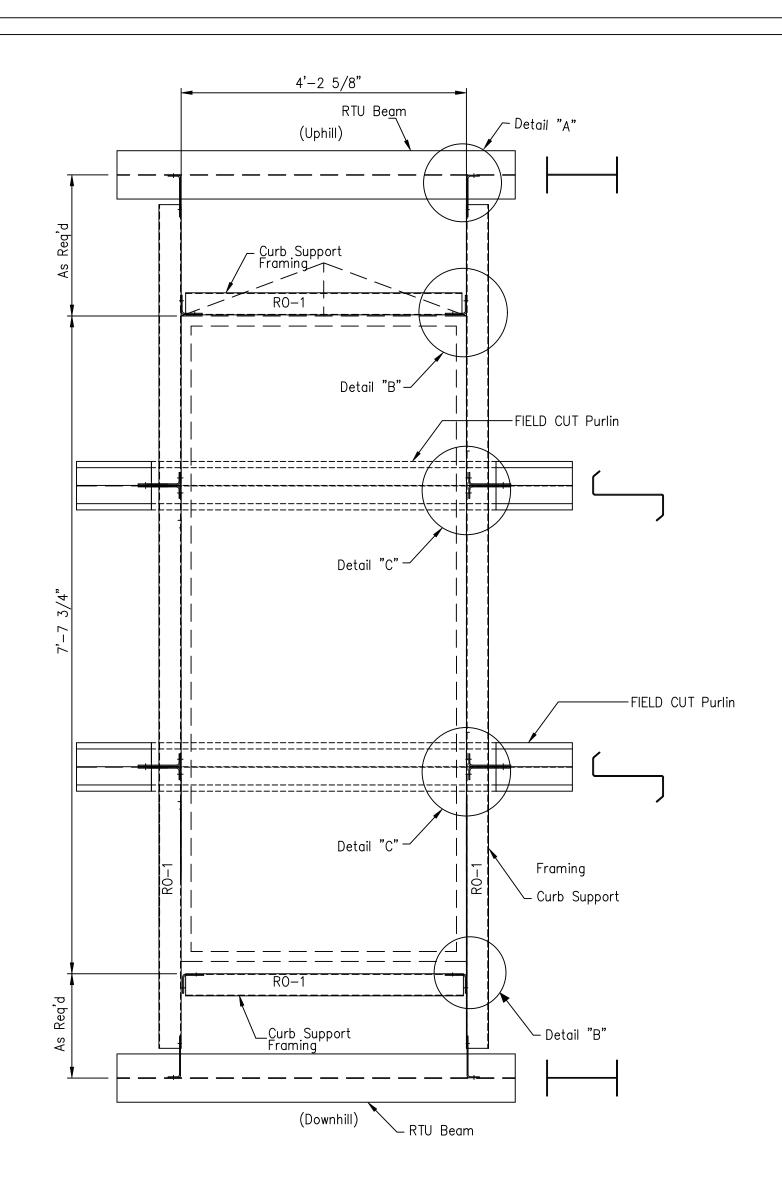
Final drawings for construction.

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HOUSTON, TX 77234	FAX: 832-553-4600

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77234	FAX: 832-553-4600

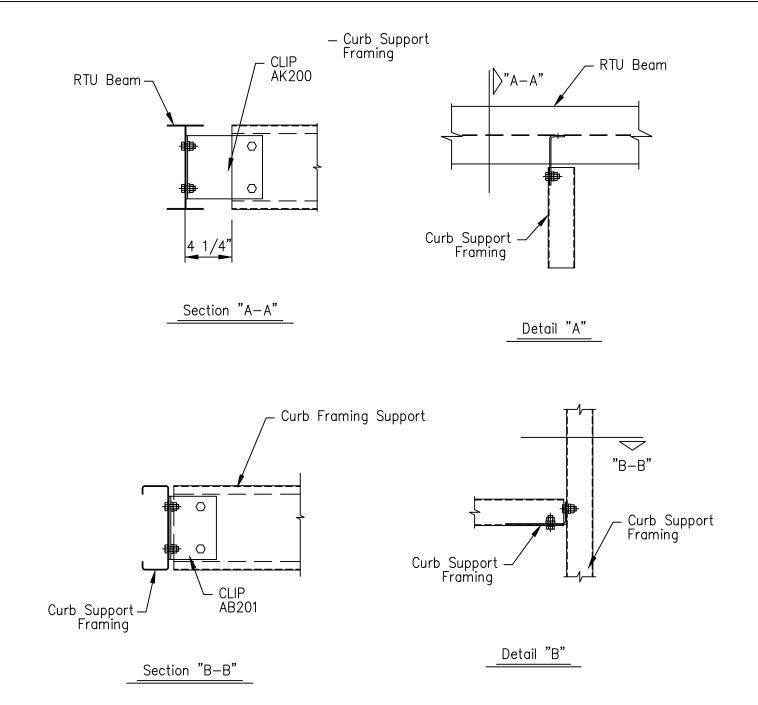
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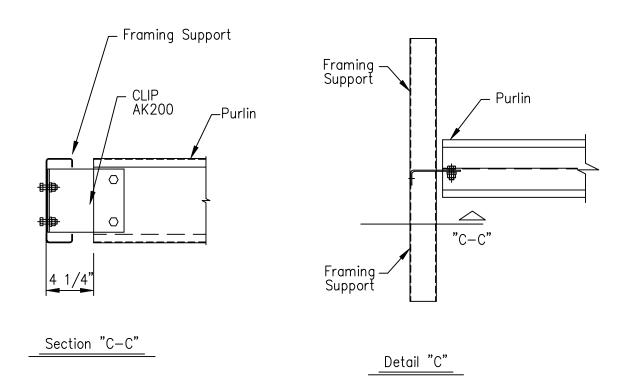
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						STK	RLB	05.09.24	RPM	12568-34393	E10	P1	



# Plan View Curb Support Framing For RTU 1-4

ROOF OPENING FRAMING IS LOCATED AT SAME ELEVATION AS TOP OF THE PURLINS AND RO.(SUPPORT CHANNEL) CURB SUPPORT FRAMING INSTALLATION INSTRUCTIONS WILL BE SUPPLIED BY THE ROOF CURB COMPANY. RTU SUPPORT CHANNEL (RO'S) FILED CUT AND  $\emptyset$  9/16" FIELD DRILL AS PER CURB OPENINGS PURLINS ARE TO BE CUT







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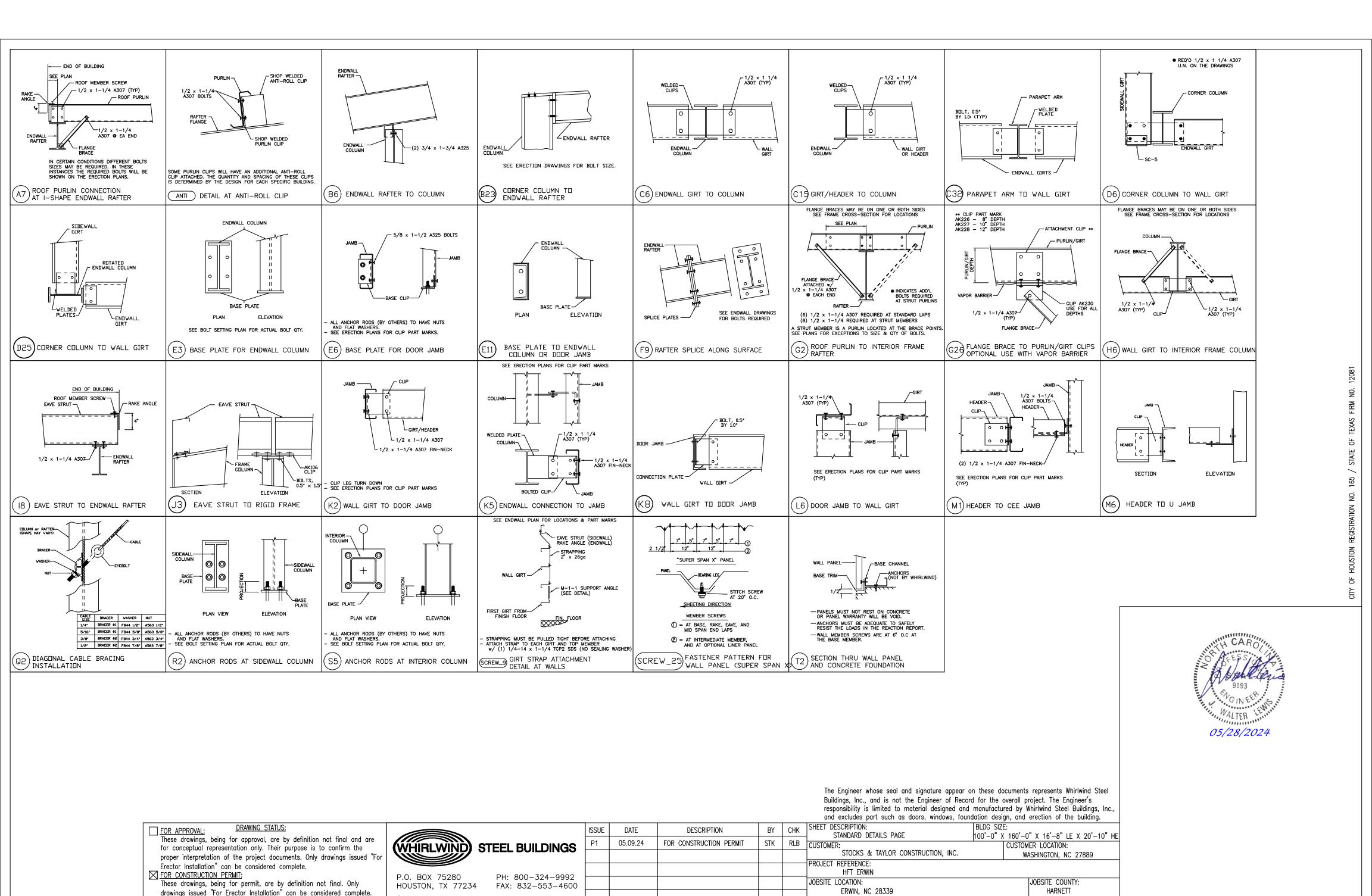
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FOR ERECTOR INSTALLATION:	0
Final drawings for construction.	

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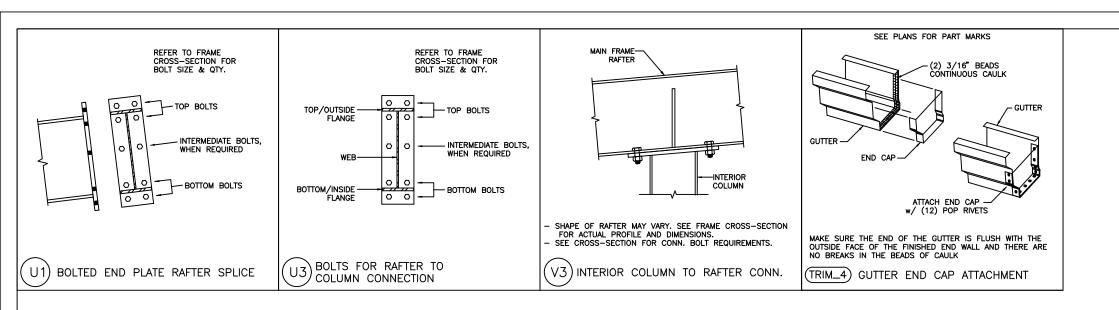
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FOR ERECTOR INSTALLATION:

Final drawings for construction.





The Engineer whose seal and signature appear on these documents represents Whirlwind Steel Buildings, Inc., and is not the Engineer of Record for the overall project. The Engineer's responsibility is limited to material designed and manufactured by Whirlwind Steel Buildings, Inc., and excludes part such as doors, windows, foundation design, and erection of the building.

FOR APPROVAL: <u>DRAWING STATUS:</u>
These drawings, being for approval, are by definition not final and are
for conceptual representation only. Their purpose is to confirm the
proper interpretation of the project documents. Only drawings issued "For
Erector Installation" can be considered complete.
FOR CONSTRUCTION PERMIT:
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These drawings, being for permit, are by definition not final. Only drawings issued "For Erector Installation" can be considered complete.

FOR ERECTOR INSTALLATION:
Final drawings for construction.

WHIRLWIND STEEL BUILDINGS
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P1 05.09.24 FOR CONSTRUCTION PERMIT STK RLB CUSTOMER: STOCKS & TAYLOR CONSTRUCTION, INC. WASHINGTON, NC 27889  PROJECT REFERENCE: HFT ERWIN  JOBSITE LOCATION: ERWIN, NC 28339  HARNETT	ISSUE	DATE	DESCRIPTION	BY	СНК		ESCRIPTION: STANDARD DETAIL	S PAGE		BLDG SIZE: 100'-0" X 160'-	-0" X 16'-8" H	X 20'-10" HE
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