GENERAL NOTES:

. MATERIALS

STRUCTURAL STEEL PLATE HOT ROLLED MILLS SHAPES HSS ROUND HSS RECTANGULAR **COLD FORM SHAPES**

ROOF AND WALL SHEETING **BOLTS** CABLE

ASTM DESCRIPTION AS529 / A572 / A1011

A36 / A529 / A572 / A500 A500 A653 / A1011 A653 / A792

A307 / A325 / A490 A475 A529 / A572

2. STRUCTURAL PRIMER NOTE:

SHOP COAT PRIMER IS INTENDED TO PROTECT THE STEEL FRAMING FOR A SHORT PERIOD OF TIME. STORAGE IN EXTREME COLD TEMPERATURES OR WINTER SNOW CONDITIONS, INCLUDING TRANSPORTATION ON SALTED OR CHEMICALLY TREATED ROADS WILL ADVERSELY AFFECT THE DURABILITY AND LONGEVITY OF THE PRIMER. THE COAT OF SHOP PRIMER DOES NOT PROVIDE THE UNIFORMITY OF APPEARANCE, OR THE DURABILITY AND CORROSION RESISTANCE OF A FIELD APPLIED FINISH COAT OF PAINT OVER A SHOP PRIMER. MINOR ABRASIONS TO THE SHOP COAT PRIMER CAUSED BY HANDLING, LOADING, SHIPPING, UNLOADING AND ERECTION ARE UNAVOIDABLE AND ARE NOT THE RESPONSIBILITY OF THE METAL BUILDING MANUFACTURES METAL BUILDING MANUFACTURER IS NOT RESPONSIBLE FOR THE DETERIORATION OF THE PRIMER OR CORROSION THAT MAY RESULT FROM ATMOSPHERIC AND ENVIRONMENTAL CONDITIONS NOR THE COMPATIBILITY OF THE PRIMER TO ANY FIELD APPLIED COATING.

3. BUILDING ERECTION NOTES

THE GENERAL CONTRACTOR AND/OR ERECTOR IS RESPONSIBLE TO SAFELY AND PROPERLY ERECT THE METAL BUILDING SYSTEM IN CONFORMANCE WITH THESE DRAWINGS, OSHA REQUIREMENTS, AND EITHER MBMA OR CSA S16 STANDARDS PERTAINING TO PROPER ERECTION. TEMPORARY SUPPORTS SUCH AS GUYS, BRACES, FALSEWORK, CRIBBING, OR OTHER ELEMENTS FOR ERECTION ARE TO BE DETERMINED, FURNISHED, AND INSTALLED BY THE ERECTOR. THESE SUPPORTS MUST SECURE THE STEEL FRAMING, OR PARTLY ASSEMBLED STEEL FRAMING, AGAINST LOADS COMPARABLE IN INTENSITY TO THOSE FOR WHICH THE STRUCTURE WAS DESIGNED IN ADDITION TO LOADS RESULTING FROM THE ERECTION OPERATION. SECONDARY WALL AND ROOF FRAMING (GIRTS, PURLINS, AND/OR JOISTS) ARE NOT DESIGNED TO FUNCTION AS A WORKING PLATFORM OR TO PRÓVIDE AS ÁN ANCHORAGE POINT FOR A FALL ARREST / SAFETY TIE OFF.

SPECIAL INSPECTIONS AND TESTING THAT MAY BE REQUIRED BY GOVERNMENTAL OR OTHER AUTHORITY DURING CONSTRUCTION AND/OR STEEL FABRICATION (COLLECTIVELY, "INSPECTIONS") ARE NOT THE RESPONSIBILITY OF NBG, AND TO THE EXTENT REQUIRED IT SHALL BE THE RESPONSIBILITY OF THE BUILDEF AND/OR OWNER. IN THE EVENT INSPECTIONS ARE REQUIRED, THE BUILDER AND/OR OWNER SHALL EMPLOY A THIRD PARTY QUALITY ASSURANCE TESTING AGENCY APPROVED BY THE RELEVANT AUTHORITY. IF SUCH REQUIREMENTS ARE NOT SPECIFICALLY INCLUDED IN NBG SALES DOCUMENTS, NO INSPECTIONS BY NBG OR AT ANY NBG FACILITY SHALL BE MADE. ALL NBG FACILITIES ARE ACCREDITED BY IAS AC472.

A325 & A490 BOLT TIGHTENING REQUIREMENTS:

IT IS THE RESPONSIBILITY OF THE ERECTOR TO ENSURE PROPER BOLT TIGHTNESS IN ACCORDANCE WITH APPLICABLE REGULATIONS. FOR PROJECTS IN THE UNITED STATES SEE THE RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING A325 OR A490 BOLTS OR FOR PROJECTS IN CANADA, SEE THE CAN/CSA S16 LIMIT STATES DESIGN OF STEEL STRUCTURES FOR MORE INFORMATION.

THE FOLLOWING CRITERIA MAY BE USED TO DETERMINE THE BOLT TIGHTNESS (I.E., "SNUG-TIGHT" OR "FULLY-PRETENSIONED"), UNLESS REQUIRED OTHERWISE BY LOCAL JURISDICTION OR CONTRACT REQUIREMENTS:

A) ALL A490 BOLTS SHALL BE "FULLY-PRETENSIONED"

B) ALL A325 BOLTS IN PRIMARY FRAMING (RIGID FRAMES AND BRACING) MAY BE "SNUG-TIGHT", EXCEPT AS FOLLOWS: "FULLY-PRETENSION" A325 BOLTS IF:

a) BUILDING SUPPORTS A CRANE SYSTEM WITH A CAPACITY GREATER THAN 5 TONS. b) BUILDING SUPPORTS MACHINERY THAT CREATES VIBRATION, IMPACT OR

STRESS-REVERSALS ON THE CONNECTIONS. THE ENGINEER-OF-RECORD FOR THE PROJECT SHOULD BE CONSULTED TO EVALUATE FOR THIS CONDITION. c) THE PROJECT SITE IS LOCATED IN A HIGH SEISMIC AREA. FOR IBC-BASED CODES, "HIGH SEISMIC AREA" IS DEFINED AS "SEISMIC DESIGN CATEGORY" OF "D", "E", OR "F". SEE THE "BUILDING LOADS" SECTION OF THIS PAGE FOR THE DEFINED SEISMIC DESIGN CATEGORY FOR THIS PROJECT. d) ANY CONNECTION DESIGNATED IN THESE DRAWINGS AS "A325-SC" OR "SLIP-CRITICAL (SC)" CONNECTIONS MUST BE FREE OF PAINT, OIL, OR OTHER MATERIALS THAT REDUCE FRICTION AT CONTACT SURFACES. GALVANIZED OR LIGHTLY RUSTED SURFACES ARE ACCEPTABLE.

C) IN CANADA, ALL A325 AND A490 BOLTS SHALL BE "FULLY PRE-TENSIONED", EXCEPT FOR SECONDARY MEMBERS (PURLINS, GIRTS, OPENING FRAMING, ETC.) AND FLANGE BRACES.

SECONDARY MEMBER (PURLIN, GIRT, OPENING FRAMING, ETC.) AND FLANGE BRACE CONNECTIONS MAY ALWAYS BE "SNUG-TIGHT". UNLÉSS INDICATED OTHÉRWISE IN THESE DRAWINGS.

GENERAL DESIGN NOTES:

1) ALL STRUCTURAL STEEL SECTIONS AND WELDED PLATE MEMBERS ARE DESIGNED IN ACCORDANCE WITH ANSI/AISC 360 "SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS" OR THE CAN/CSA \$16 "LIMIT STATES DESIGN OF STEEL STRUCTURES", AS REQUIRED BY THE SPECIFIED BUILDING CODE. 2) ALL WELDING OF STRUCTURAL STEEL IS BASED ON EITHER AWS D1.1 "STRUCTURAL WELDING CODE - STEEL" OR CAN/CSA W59 "WELDED STEEL CONSTRUCTION (METAL ARC WELDING)", AS

REQUIRED BY THE SPECIFIED BUILDING CODE. 3) ALL COLD FORMED MEMBERS ARE DESIGNED IN ACCORDANCE WITH ANSI/AISI 100 OR THE CAN/CSA S136 "SPECIFICATIONS FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL

MEMBERS", AS REQUIRED BY THE SPECIFIED BUILDING CODE. 4) ALL WELDING OF COLD FORMED STEEL IS BASED ON AWS D1.3 "STRUCTURAL WELDING CODE -SHEET STEEL" OR CAN/CSA W59 "WELDED STEEL CONSTRUCTION (METAL ARC WELDING)",

AS REQUIRED BY THE SPECIFIED BUILDING CODE. 5) THIS MANUFACTURING FACILITY IS IAS AC-472 ACCREDITED AND CAN/CSA A660 AND W47.1

CERTIFIED (IF APPLICABLE) FOR THE DESIGN AND MANUFACTURING OF METAL BUILDING SYSTEMS. 6) IF JOISTS ARE INCLUDED WITH THIS PROJECT, THEY ARE SUPPLIED AS A PART OF THE SYSTEMS ENGINEERED METAL BUILDING AND ARE FABRICATED IN ACCORDANCE WITH THE REQUIREMENTS OF SECTION 1926.758 OF OSHA SAFETY STANDARDS FOR STEEL ERECTION DATED JANUARY 18, 2001

THE DRAWINGS AND THE METAL BUILDING THEY REPRESENT ARE THE PRODUCT OF THE METAL BUILDING MANUFACTURER. THE REGISTERED PROFESSIONAL ENGINEER'S SEAL PERTAINS ONLY TO THE REQUIREMENTS LISTED HEREIN FOR THE MATERIALS DESIGNED AND SUPPLIED BY THE METAL BUILDING MANUFACTURER. THE REGISTERED PROFESSIONAL ENGINEER WHOSE SEAL APPEARS ON THESE DRAWINGS IS EMPLOYED OR ENGAGED BY THE METAL BUILDING MANUFACTURER AND DOES NOT SERVE AS OR REPRESENT THE PROJECT ENGINEER OF RECORD AND SHALL NOT BE CONSTRUED AS SUCH.

7. GLOSSARY OF ABBREVIATIONS:

A.B. = ANCHOR RODS B.U. = BUILT-UPBS = BOTH SIDES DIA = DIAMETER F.S. = FAR SIDE FLG = FLANGE GA. = GAUGE H.S.B. = HIGH STRENGTH BOLTS HT. = HEIGHT

LLV = LONG LEG VERTICAL

M.B. = MACHINE BOLTS MAX = MAXIMUMMBS = METAL BUILDING SUPPLIER MIN = MINIMUM N.S. = NEAR SIDE N/A = NOT APPLICABLE NIC = NOT IN CONTRACT O.A.L. = OVERALL LENGTH O.C. = ON CENTER

PL = PLATE REQ'D = REQUIREDREV. = REVISION SIM = SIMILAR SL = STEEL LINE SLV = SHORT LEG VERTICAL TBD = TO BE DETERMINED TYP = TYPICALU.N.O. = UNLESS NOTED OTHERWISE

?? = PART MARK TO BE DETERMINED AND WILL BE UPDATED ON CONSTRUCTION DRAWINGS

KIRBY BUILDING SYSTEMS 124 KIRBY DRIVE

PORTLAND, TN 37148 PHONE: 615-325-4165

PROJECT BUILDING LOADS

CERTIFICATION EXTENDS ONLY FOR THE LOADS SPECIFIED ON KIRBY'S PURCHASE ORDER TO THE STRUCTURAL COMPONENTS OF THE BUILDING DESIGNED AND SUPPLIED BY KIRBY BUILDING SYSTEMS, IF ERECTED AS INDICATED. KIRBY'S CUSTOMER IS TO CONFIRM THAT THESE LOADS COMPLY WITH THE REQUIREMENTS OF THE LOCAL BUILDING DEPARTMENT. NOTE THAT KIRBY'S ENGINEER IS NOT ACTING AS THE ENGINEER OF RECORD FOR THIS CONSTRUCTION PROJECT. DESIGN LOADS HAVE BEEN APPLIED IN ACCORDANCE WITH THE FOLLOWING

DESIGN CODE: NORTH CAROLINA (NCBC 2018)

ROOF LIVE LOAD: 20.00 psf REDUCIBLE PER CODE

*** RISK CATEGORY: II - STANDARD BUILDINGS

SNOW EXP. FACTOR, Ce: 1.00

*** FOR RISK CATEGORY I OR II

BUILDINGS, IBC ALLOWS FOR

SINGLE STORY BUILDINGS TO

HAVE NO LIMIT FOR SEISMIC

THAT ANY INTERIOR WALLS,

PARTITIONS, CEILINGS, AND

DETAILED (BY OTHERS) TO

DRIFT.

ACCOMMODATE THIS STORY

EXTERIOR WALLS SHOULD BE

STORY DRIFT. PLEASE NOTE

GROUND SNOW LOAD: 10.00 psf

SNOW IMPORTANCE FACTOR, Is: 1.00 ULTIMATE DESIGN WIND SPEED: 121 mph (Vult)

mph (Vasd)

NOMINAL DESIGN WIND SPEED: 94 WIND EXPOSURE: C

DESIGN SUCTION / PRESSURE FOR WALL COMPONENTS AND CLADDING NOT DESIGNED OR PROVIDED BY KBS: + 30

UL-90: NO

SEISMIC INFORMATION: Ss: 0.183 S1: 0.085

DESIGN (Sds / Sd1): 0.195/0.136

SITE CLASS: D

SEISMIC IMP. FACTOR, le: 1.00 SEISMIC DESIGN CATEGORY: C

ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE

BASIC SFRS: NOT DETAILED FOR SEISMIC

COUNTY: HARNETT

STATE: NC

1) COLLATERAL DEAD LOADS, UNLESS OTHERWISE NOTED, ARE ASSUMED TO BE UNIFORMLY DISTRIBUTED WHEN SUSPENDED SPRINKLER SYSTEMS, LIGHTING, HVAC EQUIPMENT, CEILINGS, ETC., ARE SUSPENDED FROM ROOF MEMBERS, CONSULT THE M.B.S. IF THESE CONCENTRATED LOADS EXCEED 500 POUNDS (USING THE WEB MOUNT DETAIL), OR 200 POUNDS (USING THE FLANGE MOUNT DETAIL), OR IF INDIVIDUAL MEMBERS ARE LOADED SIGNIFICANTLY MORE THAN OTHERS.

2) THE DESIGN OF STRUCTURAL MEMBERS SUPPORTING GRAVITY LOADS IS CONTROLLED BY THE MORE CRITICAL EFFECT OF ROOF LIVE LOAD OR ROOF SNOW LOAD, AS DETERMINED BY THE APPLICABLE CODE.

3) ALL WELDING MUST BE PERFORMED BY AWS QUALIFIED WELDERS FOR THE WELDING PROCESSES AND POSITIONS TO BE USED. ALL WELDING AND WELD PREP MUST BE COMPLETED AND VISUALLY INSPECTED TO AWS ACCEPTANCE CRITERIA (TABLE 6.1) IN ACCORDANCE WITH THE APPLICABLE AWS STANDARD. WELD ELECTRODES USED FOR ALL FIELD WELD PROCESSES MUST BE SELECTED FROM TABLE 3.1 IN AWS D1.1 FOR GROUP II MATERIAL GREATER THAN OR EQUAL TO 0.125" THICK OR TABLE1.2 IN AWS D1.3 FOR MATERIAL LESS THAN 0.125" THICK AND ALL FILLER MATERIAL MUST HAVE A Fu OF 70 KSI.

4) ALL EXTERIOR COMPONENTS (WINDOWS, DOORS, ETC) MUST MEET WIND LOADING REQUIREMENTS FOR THE BUILDING CODE LISTED ABOVE OR MUST BE ADEQUATELY PROTECTED DURING A HIGH WIND EVENT. ALL GLAZING AND OTHER APPLICABLE OPENINGS IN WINDBORNE DEBRIS REGIONS MUST BE IMPACT-RESISTANT OR PROTECTED WITH AN IMPACT-RESISTANT COVERING. IMPACT RESISTANT MATERIALS MUST MEET THE LARGE AND/OR SMALL MISSILE TEST OF ASTM E 1996 AND ASTM E 1886.

BUILDING SPECIFIC LOADING INFORMATION

* DEAD LOAD: NORMAL WEIGHT OF METAL BUILDING COMPONENTS, NOT INCLUDING PRIMARY FRAMING. AS SUPPLIED BY THE MANUFACTURER

** Pm IS BASED ON THE MINIMUM ROOF SNOW LOAD CALCULATED PER BUILDING CODE OR THE CONTRACT-SPECIFIED ROOF SNOW LOAD, WHICHEVER IS GREATER. THIS VALUE, Pm, IS ONLY APPLIED IN COMBINATION WITH DEAD AND COLLATERAL LOADS. ROOF SNOW IN OTHER LOADING CONDITIONS IS DETERMINED PER THE SPECIFIED BUILDING CODE.

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		ROOF DEAD	COLLATE	RAL DEAD	SNOW COEFFICIENT		SNOW LOAD		WIND		SEISMIC		
	BLDG.	(psf)*	Pri (psf)	Sec (psf)	Ct	Cs	Ps (psf)	**Pm (psf)	Enclosure	GCpi	R	Cs	V (kips)
	Α	3.0	3.0	3.0	1.0	1.00	7.00	10.00	Enclosed	±0.18	3.00	0.065	3.1

	CONTENTS					
SHEET NUMBER	DESCRIPTION					
· C1	COVER SHEET(S)					
AB1	ANCHOR ROD PLAN					
E1-E4	ERECTION DRAWINGS					

GP - GRAY PRIMER

GP - GRAY PRIMER

GP - GRAY PRIMER

24 Ga. STANDING SEAM 360 (SS3)

GALVALUME PLUS (GM)

26 Ga. KIRBY WALL (KW1)

ROOF LINE TRIM: POLAR WHITE, SP (PW)

DOWNSPOUTS: POLAR WHITE, SP (PW)

BASE TRIM: SANDSTONE, SP (SS)

WALL CORNER TRIM: POLAR WHITE, SP (PW)

NOTE: ANY VARIANCE FROM THE PANEL TYPES OR COLORS

LISTED HERE WILL BE NOTED ON THE ELEVATION DRAWINGS

FRAMED OPENING TRIM: SANDSTONE, SP (SS)

SANDSTONE, SP (SS)

HIGH SYSTEM w/ THERMAL SPACERS

PRIMER

STRUCTURAL FRAMING:

WALL SECONDARY:

ROOF SECONDARY:

ROOF PANELS

WALL PANELS

TRIM COLORS

NTS	
DESCRIPTION	
EET(S)	
OD PLAN	
DRAWINGS	

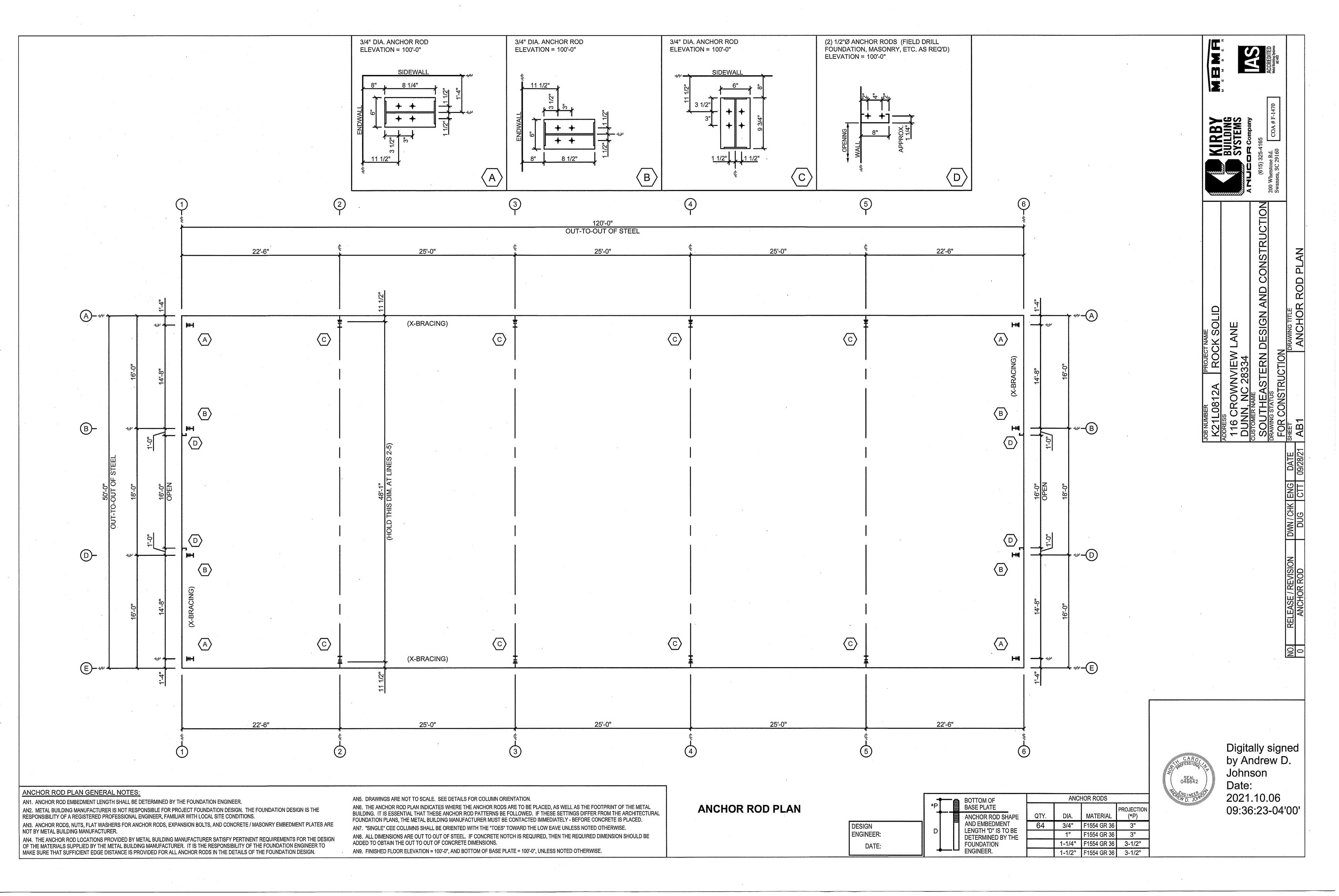
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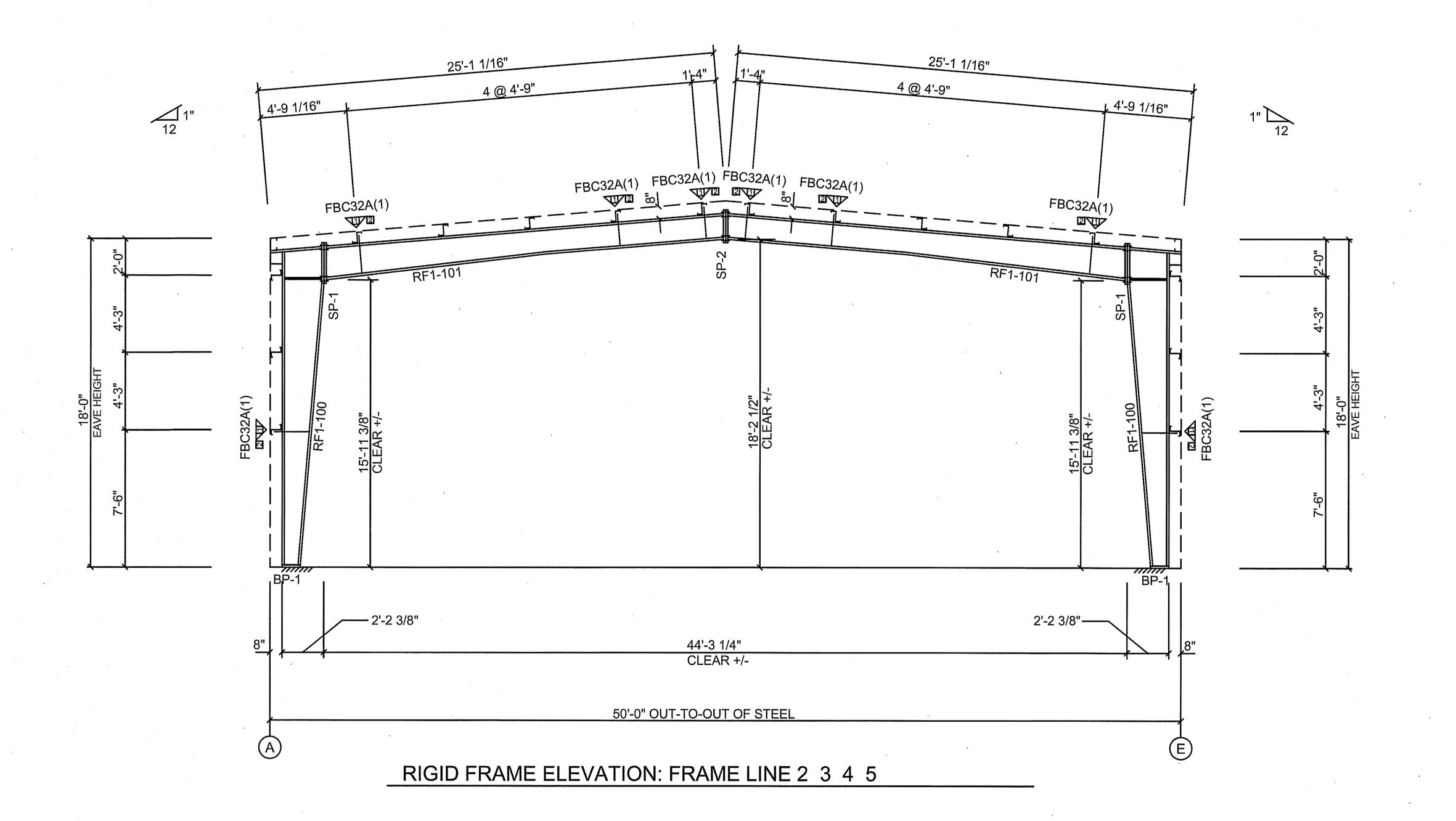
KIRB BUILDING SYSTEMS

CONSTRUCTION



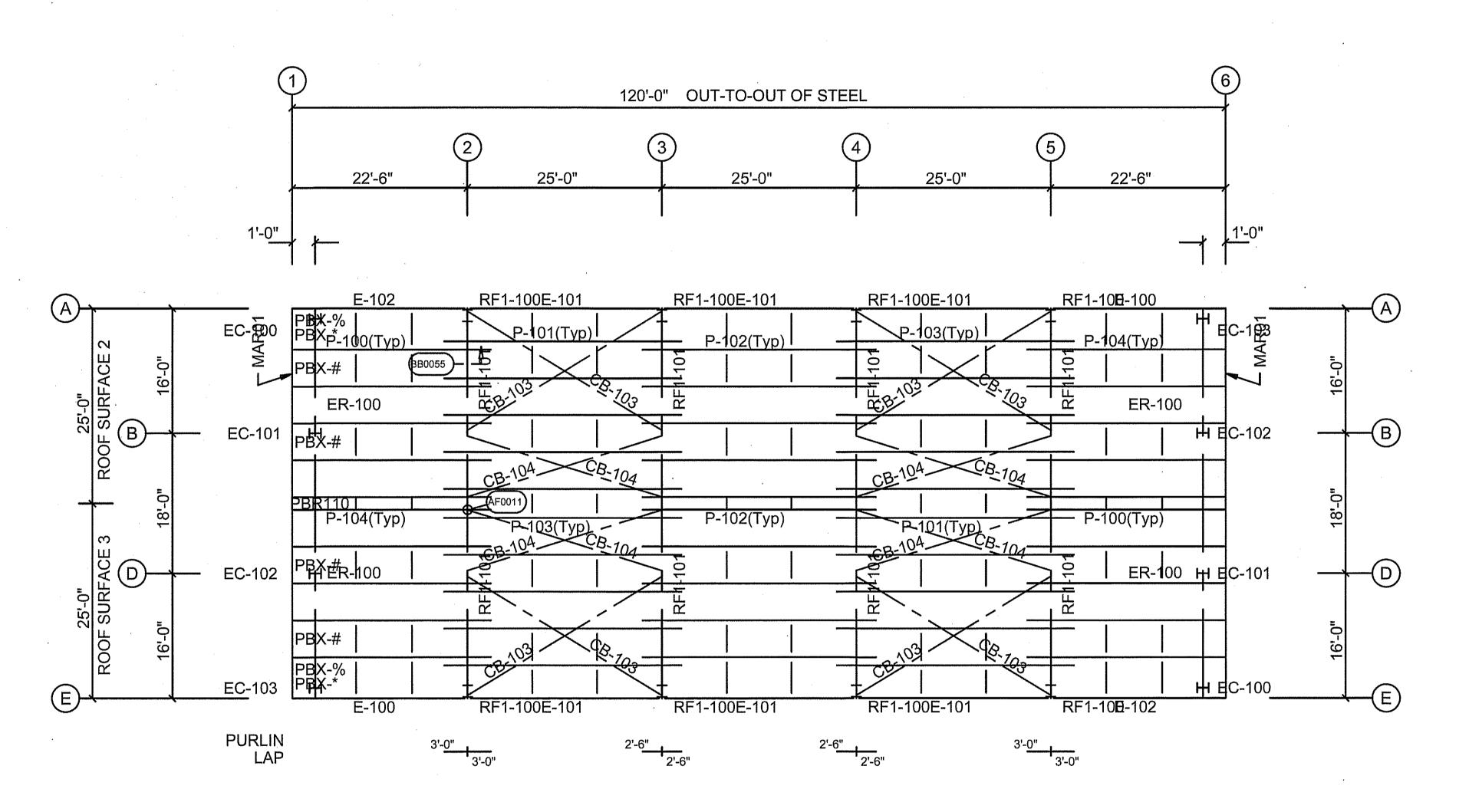
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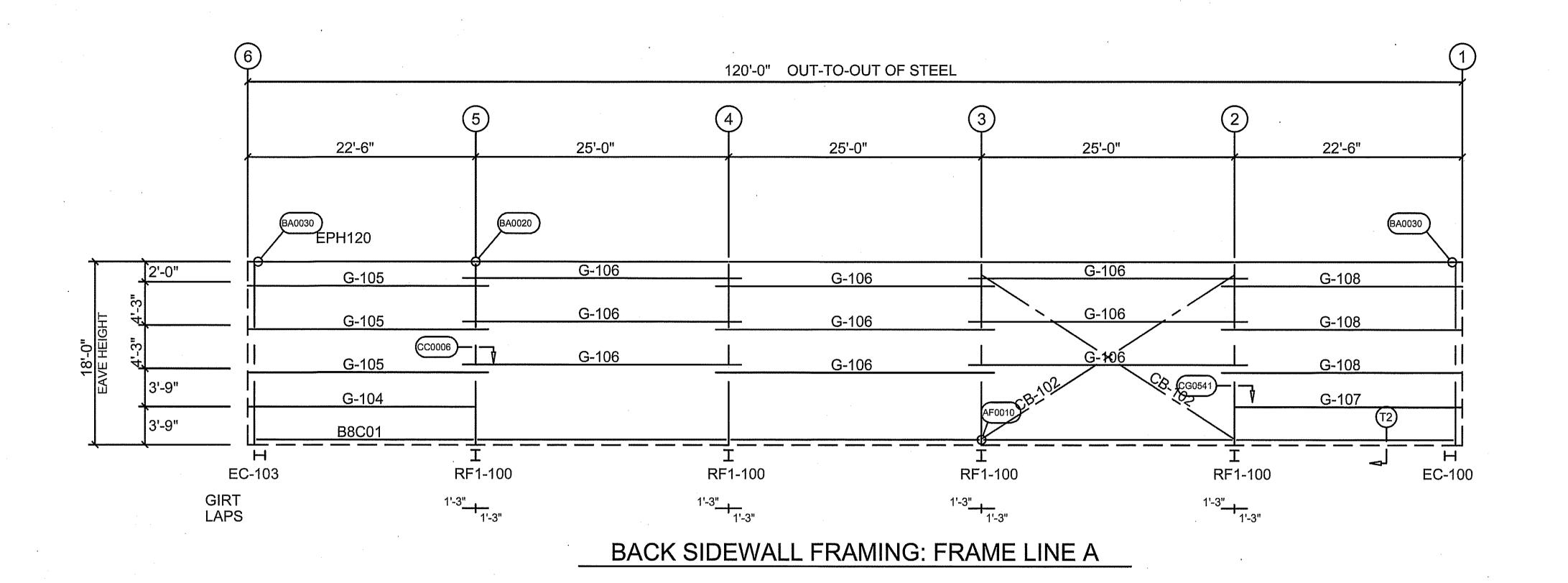


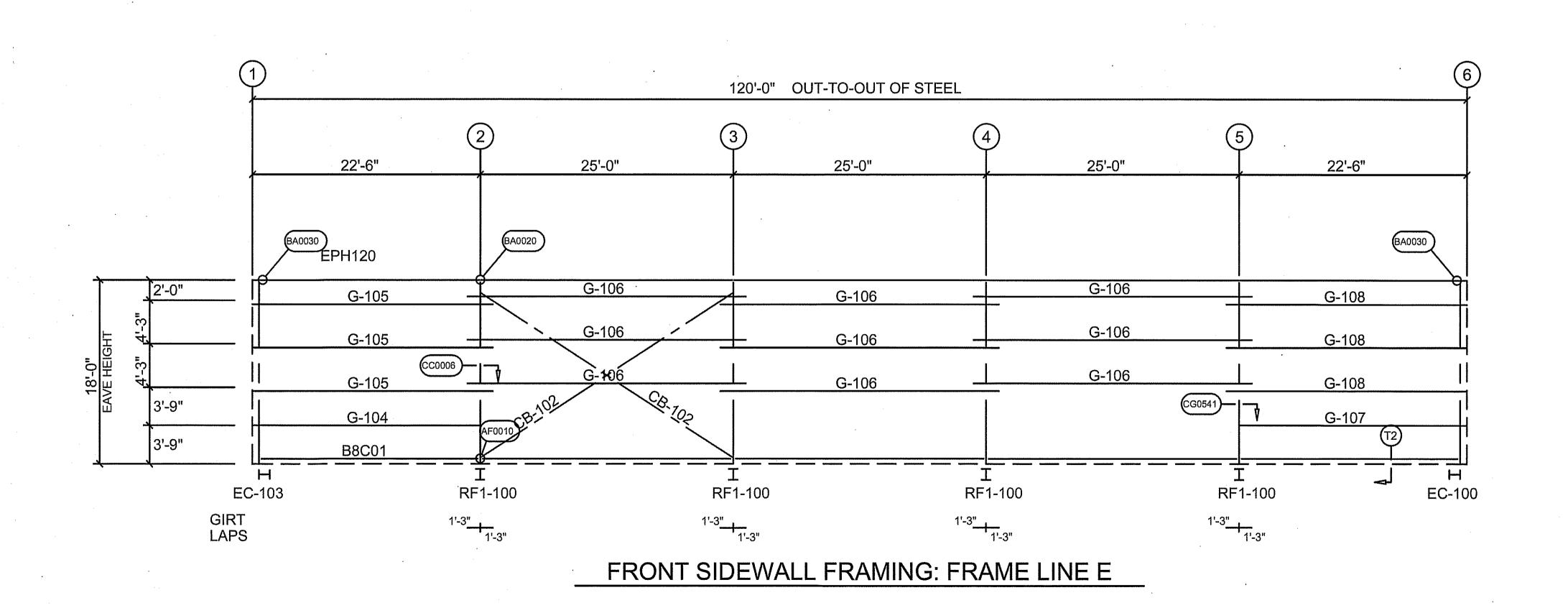
ROOF FRAMING PLAN

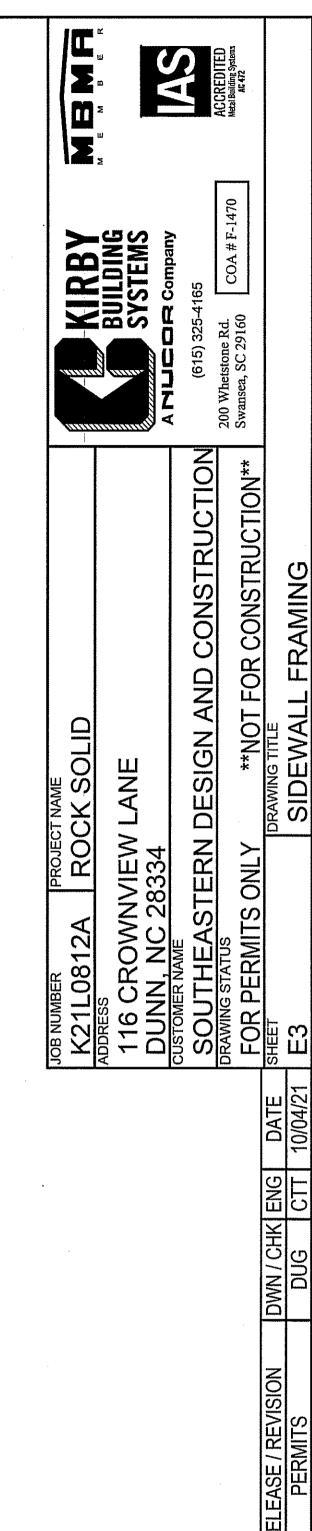


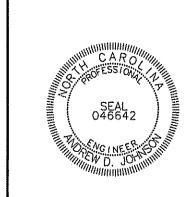
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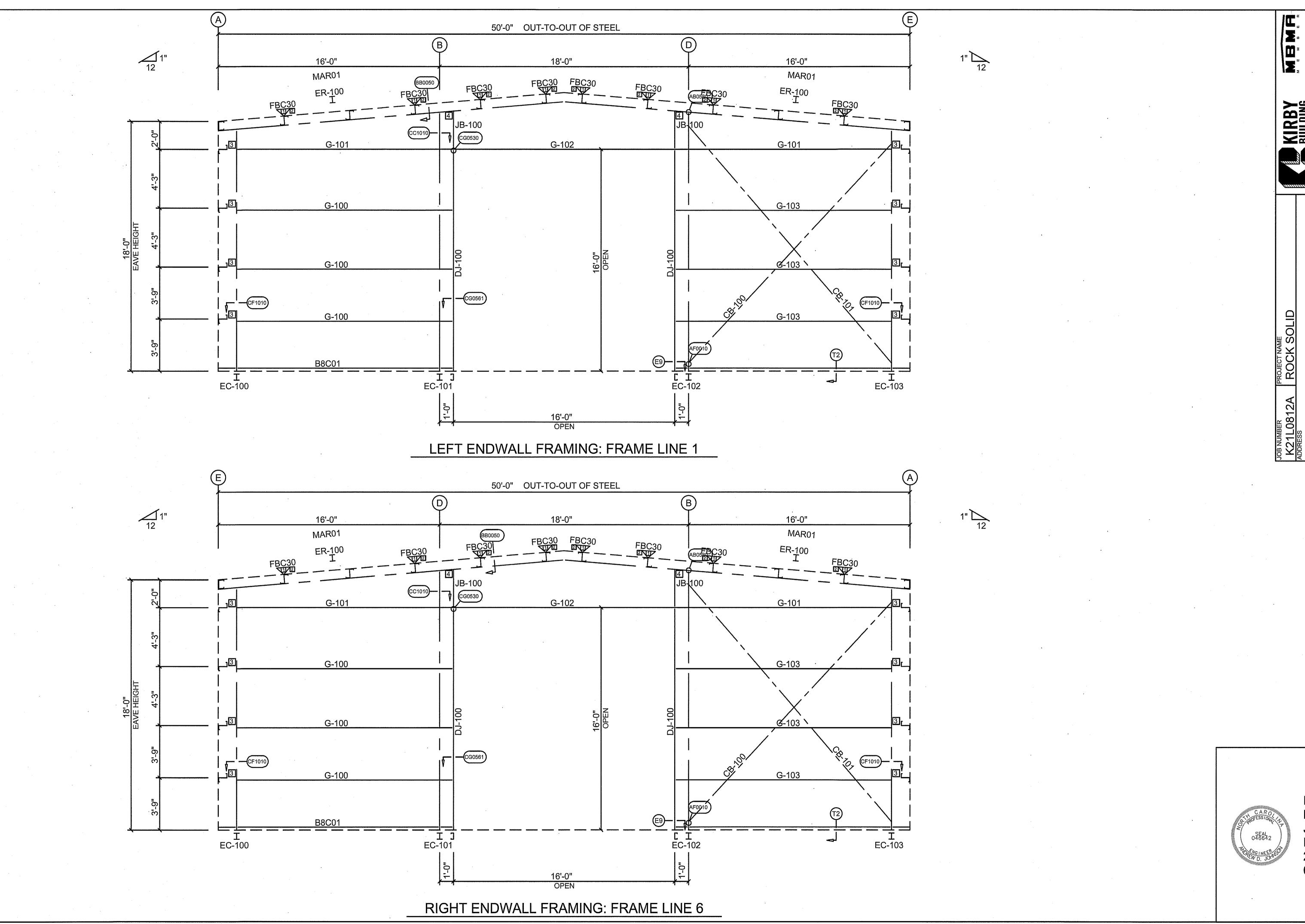


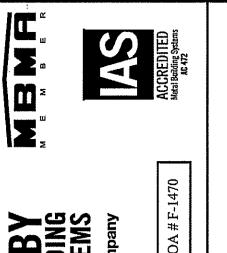






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