

T.B.E. (SHOP)

T.B.E.









WAKEBOARD DEALERSHIP

SANFORD, NC

Project Name

Sheet Title

EXTERIOR ELEVATIONS

PROJECT #: 24-067 | DATE: 02/26/2024 | No. | Revision | Date | Date | DATE: |

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APPROVED BY:

STRUCTURAL NOTES

GENERAL NOTES:

STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH JOB SPECIFICATIONS AND ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING AND SITE DRAWINGS. CONSULT THESE DRAWINGS FOR SLEEVES, DEPRESSIONS AND OTHER DETAILS NOT SHOWN ON STRUCTURAL INDIAMANGS.

THE STRUCTURE IS DESIGNED TO BE SELF SUPPORTING AND STABLE AFTER THE BUILDING IS COMPLETE. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE REFECTION PROCEDURES AND SEQUENCE TO ISSUERS SAFTLY OF THE BUILDING AND ITS COMPONENTS DURING REFECTION. THES INCLUDES THE ADDITION OF INCESSARY SYNCHIS, SHEETING, TEMPORARY BROADING (AND ACCOMPANYING FOOTINGS), GUYS OR TIEDOMA'S.

ADDITIONAL OBSERVATIONS AS A RESULT OF REJECTION OF WORK COMPLETED AND/OR ADDITIONAL OBSERVATIONS DUE TO THE DEPICIENCIES IN WORK OBSERVED WILL BE AT THE EXPENSE OF THE CONTRACTOR.

DESIGN CODES:

ACI 318-19 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE AND COMMENTARY.

2018 NATIONAL DESIGN SPECIFICATIONS (NDS) FOR WOOD CONSTRUCTION

AISC SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS, ALLOWABLE STRESS DESIGN.

DESIGN LOADS:

THE STRUCTURAL SYSTEM FOR THIS BUILDING HAS BEEN DESIGNED WITH THE FOLLOWING SUPERIMPOSED LOADINGS:

ROOF: GROUND SNOWLOAD, DESIGN ROOF SNOWLOAD, SNOW EXPOSURE FACTOR, SNOW LOAD INFORTNACE FACTOR, THERMAL FACTOR, ROOF LIVE LOAD.	Pg= Pf= Ce= is= Ct= 20 psf	10 psf 10 psf 0.9 1.0 1.2
DESIGN LIVE LOADS: FLOOR	100 ps/	
WARD: BASIC WIND SPEED (3 SEC GUST) EXPOSURE CATEGORY RISK CATEGORY WIND BASE SHEARS,	115 mph C II : Vx = Vv =	17.9k 46.3k
COMPONENT & CLADDING: ALL RUILDING COMPONENTS AND CLADDING ENGINEERED BY THE		NENTM

ALL BUILDING COMPONENTS AND CLADOING FINGINEERED BY THE COMPONENT MANUFACTURER ARE TO BE DESIGNED BY THE MANUFACTURER'S ENGINEER FOR WIND LOADS DETERMANED PER THE NORTH CARGOLING STATE BUILDING CODE FOR THE BASIC DESIGN WIND VELOCITY, IMPORTANCE FACTOR AND EXPOSURE LISTED ABOVE.

SEISMIC: IMPORTANCE FACTOR,	1e =	1,0
USE GROUP MAPPED SPECTRAL RESPONSE ACCELERATIONS,	Ss = S1 =	0.139g 0.067g
SPECTRAL RESPONSE COEFF.,	Sds = Sd1 =	0.149g 0.108g
SEISMIC RESISTING SYSTEM: ORDINARY WOOD SHEATHED SHEAR WALLS		

FOUNDATIONS ARE DESIGNED FOR AN ALLOWABLE SOIL BEARING PRESSURE OF 2,000 pdf. ON EXISTING SOILS, BEFORE CONSTRUCTION COMMENCES, SOIL BEARING OPPACTT SHALL BE EVERIFIED BY A BUBSURFACE INVESTIGATION, A CENTRED TESTING LAGORATORY, WHALL BE SHALL INCLUDE INVALVES AND RECORDING THE STRUCTURE LAGORATORY, WHOSE BEAR SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR RECVENITY PROPERTY BEAUTION CONSTRUCTION BEGINS.

PLUMBING SLEEVES:

SHALL BE A POLYMER INJECTION SYSTEM SUCH AS RAMSET "EPOOK", MOLLY "PARAMOUNT HVC". SIKA "SIKADUR INJECTION SEL", "HUTH-HIGH STREINSTH EPOOK", OR APPROVED EQUIA, INSTALLED IN ACCORDANCE WITH THE MANUFACTURERS INSTRUCTIONS. INSTALLERS SHALL BE TRAINED BY THE MANUFACTURERS REPRESENTATIVE.

ANCHOR BOLTS:

SHALL BE A38 THREADED ROD. PROVIDE HOT DIP GALVANIZE FINISH ON ALL ANCHOR BOLTS PERMANENTLY EXPOSED TO EXTERIOR.

CONCRETE TESTING:

CONCRETE TESTING SHALL BE PAID FOR BY THE OWNER, TESTING LABORATORY SHALL PERFORM THE FOLLOWING TESTS ON CAST-IN-PLACE CONCRETE:

A) ASTM CH3 - "STANDARD TEST METHOD FOR SLUMP OF PORTLAND CEMENT CONCRETE."

"STANDARD TEST METHOD FOR COMPRESSIVE STRENGTH OF CONDITIONS OF CONCRETE SPECIALS." A SEPARATE TEST SHALL BE CONDUCTED FOR EACH CLASS, FOR EVERY 50 CUBIC YARDS (OR FRACTION THEREOP), PLACED PER DAY. REQUIRED CYLINDERS, QUALITIEST, AND TEST THEREOP, PLACED PER DAY. REQUIRED CYLINDERS, QUALITIEST, AND TEST

PROVIDE ONE ADDITIONAL RESERVE CYLINDER TO BE YESTED UNDER THE DIRECTION OF THE ENGINEER, IF REQUIRED, IF 28 DAY STRENGTH IS ACHIEVED, THE ADDITIONAL CYLINDER(S) MAY BE DISCARDED.

NO PENETITATIONS SHALL BE MADE IN ANY STRUCTURAL MEMBERS OTHER THAN THOSE LOCATED ON THESE DRAWINGS WITHOUT PREVIOUS APPROVAL OF THE ENGINEER.

SHALL BE MIX DESIGNED BY A RECOGNIZED TESTING LABORATORY TO ACHIEVE A STRENGTH AT 28 DAYS AS LISTED BELOW WITH A PLASTIC AND WORKARI F MIX:

FOUNDATION WALLS AND FOOTINI INTERIOR SLABS-ON-GRADE ALL OTHER CONCRETE

ALOUPER CONCRETE

SUMMIT PROPOSED MIX DESIGN WITH RECENT FIELD CYLINDER OR LAD
TESTS FOR REVIEW/PRIOR TO USE, MIX SHALL BE UNDOLETY DESTRICT.

THE REVIEW PRIOR TO USE, MIX SHALL BE UNDOLETY DESTRICT.

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4. THE CONCRETE STRENGTHS SHOWN IN THE SECTION ABOVE AND IN THE SPECIFICATIONS ARE MINIMUM COMPRESSIVE STRENGTHS. THE ENGINEER SHALL DETERMINE IF THE CONCRETE IS ACCEPTABLE, OR TO BE REMOVED, OR TO RECEIVE SPECIAL CURING IF THE COMPRESSIVE STRENGTHS ARE LESS THAN SPECIFIES.

5. ALL CONCRETE EXPOSED TO WEATHER OR EARTH SHALL BE AIR ENTRAINED TO 5% TO 7%.

6. WATER REDUCING AGENTS MAY BE USED INTHE CONCRETE MIX. PLASTICIZERS AND SUPER-PLASTICIZERS MAY BE USED ONLY WHEN WRITTEN PERMISSION OF THE ENGINEER IS GIVEN.

7. NO SALTS OF ANY KIND MAY BE USED IN CONCRETE BEFORE OBTAINING THE ENGINEER'S WRITTEN PERMISSION FOR THEIR USE.

1, ALL CONCRETE SHALL BE PLACED IN ACCORDANCE WITH ACI 301 AND ACI 117 EXCEPT AS MODIFIED BELOW:

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CASE THAN TOZERANCES AS MEASURED BY PLACING A FREESTANDING (INJURED BY THE STANDARD (INJURED BY THE STANDARD (INJURED BY THE STANDARD BY THE S

2. ALL REINFORCING STEEL TO BE ASTM AB15, GRADE 60 (M AND LARGER), EXCEPT WHERE NOTED OTHERWISE, REINFORCING SHALL NOT BE WELDED.

WELDED WIRE FABRIC TO CONFORM TO ASTM A185 AND SHALL BE FREE FROM CIL, SCALE AND RUST. PLACE WIFFIN ACCORDANCE WITH THE TYPICAL PLACING DETAILS OF ACT STANDARDS AND THE SPECIFICATIONS. MINIMAM LAPS SHALL BE ONE SPACE PLUS 2".

ALL REINFORCING STEEL BARS TO BE DETAILED AND PLACED IN ACCORDANCE WITH THE LATEST ACI MANUALS.

PROVIDE POUADATION DOWELS AS SHOWN, MINIMUM SIZE DOWELS TO BE # 4, UNLESS OTHERWISE NOTED, ALL VERTICAL REINFORCING STEEL IN COLUMNS AND PIERS, OR VERTICAL REINFORCING IN WALLS, SHALL BE DOWELED INTO THE FOOTINGS WITH SAME SIZE AND QUANTITY DOWEL AS THE VERTICAL.

8. WHERE SHOWN ON THE DRAWINGS, PROVIDE WELD PLATES, WELDMENTS, OR CONCRETE INSERTS FOR FASTENING AND SECURING OTHER COMPONENTS. CONCRETE INSERTS SHOLL BE FURNISHED BY THE CONTRACTOR RECURRING HEM AND INSTALLED BY THE CONTRACTOR CONTRACTOR RECURRING THE MADE. CLIP ANGLES SHALL BE FURNISHED BY THE CONTRACTOR RECURRING THEM.

CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH	3*
EXPOSED TO EARTH OR WEATHER #5 THROUGH #18 BARS #5 BARS OR SMALLER	2* 1 1/2*
NOT EXPOSED TO EARTH OR WEATHER OR IN CONTACT WITH THE GROUND, SLABS AND WALLS	245
#14 AND #18	1 1/2"
BEAMS AND COLUMNS	1 1/2"
10. PROVIDE TWO (2) #5'S, ONE AT EACH FACE, UNLES	IS NOTED OTHERWISE,

-- FORMWORK AND SHORING:

NO STRUCTURAL CONCRETE SHALL BE STRIPPED UNTIL IT HAS REACHED AT LEAST TWO-THIRDS OF THE 28 DAY DESIGN STRENGTH. DESIGN, ERECTION AND REMOVAL OF ALL FORMMORK, SHORES AND RESHORES SHALL MEET THE REQUIREMENTS SET FORTH IN ACI STANDARDS 301 AND 347.

1. STRUCTURAL 2× WOOD COMPONENTS HAVE BEEN DESIGNED AS SOUTHERN YELLOW PINE (SYP) OR HEM-FIR (HIP) NO. 2 OR BETTER AND SHALL HAVE THE FOLLOWING MINRIUM ALLOWABLE FIBERS STRESSES AND PROPERTIES:

MICOULAS OF ELASTICITY (E) 1,300,000 PSI BENDING (Fb) 850 PSI SHEAR (Fv) 75 PSI

WOOD IN CONTACT WITH CONCRETE OR MASONRY SHALL BE PROTECTED OR PRESSURE TREATED IN ACCORDANCE WITH AITC-109.

MEMBER SIZES SHOWN ARE NOMINAL UNLESS NOTED OTHERWISE.

A DOLT IN IMPOOR ARE MACHINE BOLTS, INJECTION OF THE MACHINE BOLTS, INJECTION OF THE MACHINE BOLTS, INJECTION OF THAT SPECIFIED, BOLTS ARE ASTIM 201 STEEL BOLT HOLES IN MOOD SHALL BE 1672 OVERSEZE. WHERE STEEL IS CONNECTED TO WOOD, HOLES IN STEEL SHALL BE 1115 OVERSEZE. PROVIDES STANDARD CUIT WASHERS UNDER HEAD AND HOLT WHERE BEARINGS AND AND AND THE MACHINE STEEL SIDE PLATES ARE USED FOR CONNECTION, THE PLATE SHALL BE USED AS A TRIMINATION.

ALL WOOD ELEMENTS SHALL BE ATTACHED PER THE FASTENING SCHEDULE OF THE 2812 NOSBC (TABLE 2304.9.1) UNLESS OTHERWISE NOTED.

6. SEE ARCHITECTURAL DRAWINGS FOR WEATHER PROTECTION OF ALL EXPOSED WOOD MEMBERS.

WOOD SHEATHING:

1. PLYWOOD ROOF, FLOOR AND WALL SHEATHING ARE DESIGNED AS DIAPHRAGMS AND SHALL COMPLY WITH APPLICABLE PROVISIONS OF CHAPTER 23 OF THE 2012 KCSBC.

SHEATHING SHALL BE FASTENED IN ACCORDANCE WITH PLANS SHOWN
SPECIAL NAULING REQUIREMENTS AND WITH THE APPROPRIATE SCHEDULE IN CHAPTER
23, UNLESS NOTED OTHERWISE.

3. IN GENERAL, SHEETS SHALL BE 4-0'Y8-0' AND SHALL BE LAID WITH FACE PLES ACROSS FRAMING MEMBERS AND WITH END JOINTS STAGGERED 4-0', NO PANEL SHALL BE USED WHICH IS LESS THAM 2 FI IM WORTH ON FLORES AND ROOFS. SHEATHING SHALL BE CONTINUOUS ACROSS 2 SPANS, MINIMUM.

PRE-ENGINEERED WOOD ROOF TRUSSES:

1. EMSINEERED WOOD TRUSS SYSTEMS SHALL BY DESIGNED BY SUPPLIER TO THE CONTIGURATION AND LOAD CARRYING CAPACITY SHOWN ON THE DRAWINGS AND SPECIFICATIONS. TRUSSES SHALL BE DESIGNED TO SUSTAIN BEEL PRESIDED TO SUSTAIN BEEL PRESIDED TO SUSTAIN BEEL PRESIDED TO SUSTAIN BEEL PRESIDED TO THE TRUSSES AND UNIFICIANT LOADS AS INDICATED ON THIS SHEET AND AS FOLLOWS:

DEAD LOAD = 10 psf LIVE LOAD = 20 psf SNOW LOAD = 6.3 psf WIND LOAD ≤ SEE DESIGN LOADS

B) BOTTOM CHORD: DEAD LOAD = 10 pcf LIVE LOAD = 10 pcf

WIND LOAD: WHEN CALCULATING NET UPLIFT REACTIONS, USE MAXIMUM RESISTING DEAD LOAD EQUAL TO 6 PSF ON THE TOP CHORD AND 9 PSF ON THE BOTTOM CHORD. 3, ROOF TRUSSES SHALL BE DESIGNED FOR A MAXIMUM VERTICAL DEFLECTION OF LOSG LIVE LOAD AND LOSG TOTAL LOAD.

4. ALTERNATE TRUSS LAYOUTS ARE ACCEPTABLE ONLY AS A CHANGE ORDER WHICH WILL INCLUDE ENGINEERING CHARGES TO THE CONTRACTOR FOR REDESIGN FOR REVIEW PRIOR TO FABRICATION.

5. SUBMIT SHOP DRAWINGS FOR SENIEW AND APPROVAL PRIOR TO FABRICATION. SHOP DRAWINGS SHALL SHOW AND SECRET HALL CORNECTOR TYPES UTILIZED WITHIN THE PROPERTY OF THE SHOULD HAVE AN ADDITIONAL OTHER CORNECTIONS AND EXAMPLED BY STREET THASSES OR COMPONENTS SUPPLIED AS PART OF THE ENGINEERED THUSS SYSTEM. A RECECTION PROPINES SHALL BE INCLUDED, IDENTIFYING ALL TRUSS SYSTEM. A RECECTION PROPINGS SHALL BE INCLUDED, IDENTIFYING ALL TRUSS SYSTEM COMPONENTS, AS WELL AS ALL PERMANENT BRACKING RECORDED FOR THIS DESIGN. SHOP DRAWINGS SHALL BE INCLUDED. SHALL BEAR THE SIGNATURE AND SEAL OF A PROFESSIONAL ENGINEERED REGISTERED IN THE STATE OF THE PROLECT LOCATION.

WOOD FRAMING CONNECTORS;

CONNECTOR MODEL NUMBERS SHOWN ARE "Strong-Tile" CONNECTORS AS MANUFACTURERED BY "SMPSCOM Strong-Tile Co.", 1450 DOOLITILE DR., PO BOX 1558, SAN LEARDRO, CA. 34571. SUBSTITUTIONS ARE ACCEPTABLE ONLY WITH THE APPROVAL OF THE STRUCTURAL ENGINEER.

ALL CONNECTORS SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM-ABS3, CONNECTORS IN CONTACT WITH PRESSURE TREATED MATERIALS SHALL HAVE 6-485 COATING, CONNECTORS NOT IN CONTACT WITH TREATED MATERIALS SHALL HAVE STANDARD G-60 COATING.

STRUCTURAL STEEL:

1. STEEL SHALL CONFORM TO ASTM ASS2 (F)=50 kB) FOR ALL W-SHAPES, AND ASTM ASS (F)=36 kB) FOR ALL OTHER MISCELLANEOUS SHAPES AND PLATES. STRUCTURAL TUBNIS SHALL CONFORM TO ASTM ASO, GRADE B, TYPE TE OR TS (F)=42 kB).

3. ALL STRUCTURAL STEEL EXPOSED TO EXTERIOR SHALL BE HOT-DIPPED GALVANIZED.

5. WELDS FOR ALL EXPOSED STRUCTURAL STEEL SHALL BE GROUND SMOOTH UNLESS NOTED OTHERWISE.

8. ALL BOLTED CONNECTIONS SHALL CONSIST OF SIZE DIAMETER (MIN.) ASTM ARBS HIGH STRENGTH EDUT, S. UNLESS NOTED OTHERWISE. BEAM CONNECTIONS SHALL BE DESIGNED BY THE FARBURGOR TO SUPPORT AN END REACTION OF WHAT HER BY IN ACCORDANCE WITH PART 1: "BEAM AND GRODE OF SIZE OF SHALL BUT SHALL AD SIZE OF SIZE O

CONTRACTOR TO FURNISH AND INSTALL 500 Ibs. OF ADDITIONAL MISCELANEOUS STEEL TO BE USED AT ENGINEER'S DISCRETION.

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ABOVE FINSHED FLOOR
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B

EDGE OF STEEL
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EACH WAY
EXISTING
EXPANSION
EXTERIOR
FINISH
FLOOR
FLOOR DRAIN
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FACE OF MASON
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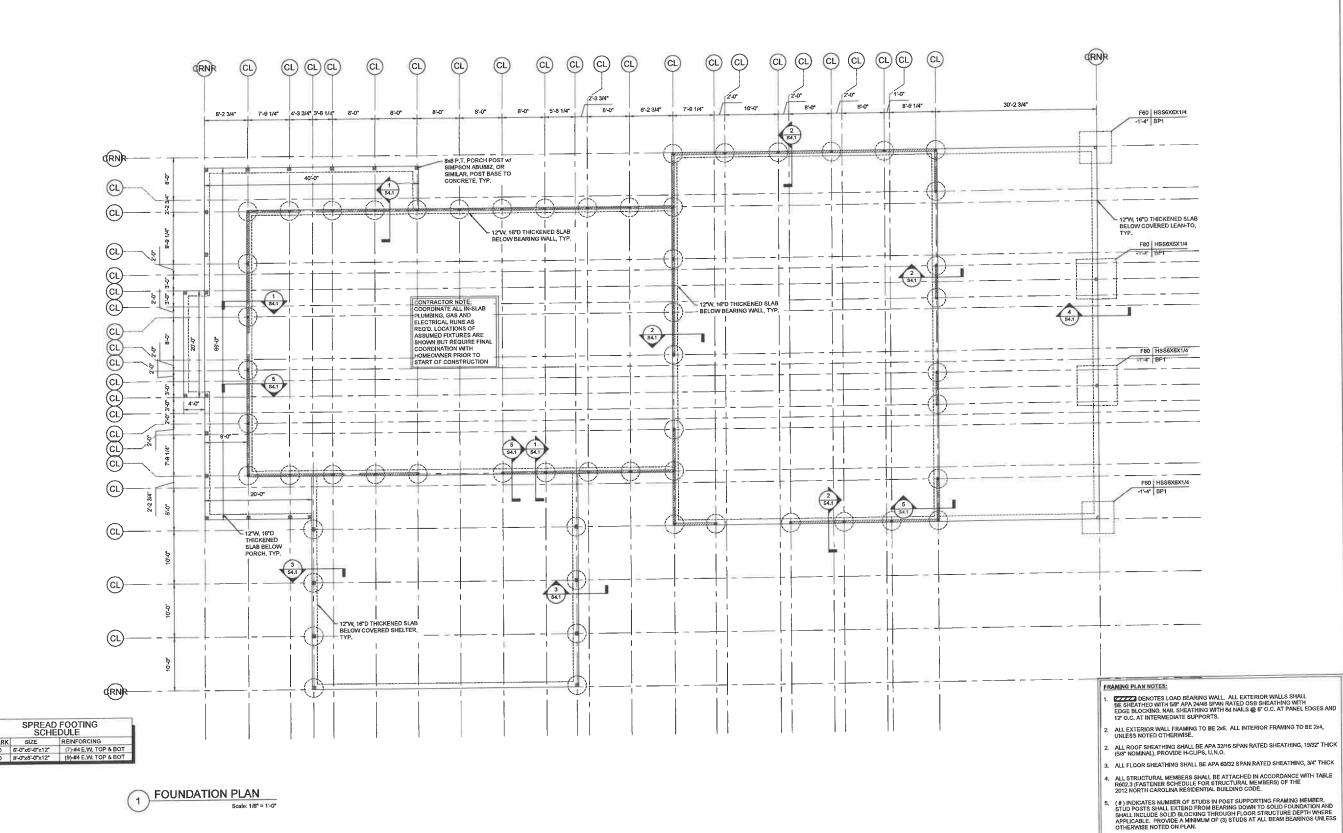
Ш し 13200 STRICKLAND ROAD 919.957.5100 - 1, 919.957.510

jfejfar@fdr-eng.com SEE SEL Heath M. Hendrick Date: 2024-03.06 Date: 2024-03.06 P.1418 P-1418

NOTES GENERAL

DESIGNED BY: DRAWN BY: APPROVED BY нмн PROJECT #: 24-067 02/26/2024 DATE: Revision Date

S1.1



FOR APPROVAL ONLY

ALL EXTERIOR, AND INTERIOR LOAD-BEARING HEADERS TO BE CONSTRUCTED W/MIN, (2)-2-10 AND SUPPORTED BY (1) JACK STUDS AND (1) KING STUD UNLESS NOTED OTHERWISE.

PROVIDE SIMPSON H10A CLIPS AT THE ENDS OF ALL ROOF FRAMING MEMBERS U.N.O.

13200 STRICKLAND ROAD SUITE 114, BOX 332 RALECH, NC 27813 0. 919.987.5100 - f. 919.987.5101 www.for-ong.com





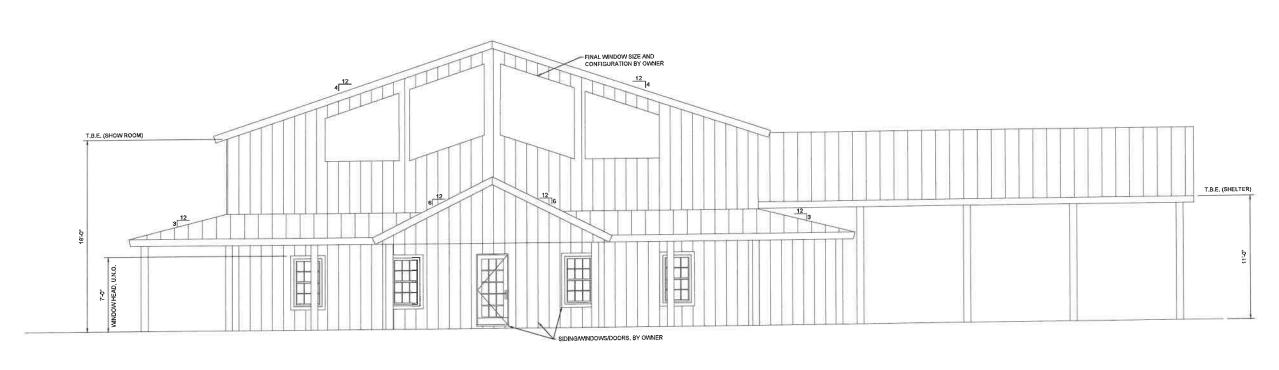
WAKEBOARD DEALERSHIP

Project Name

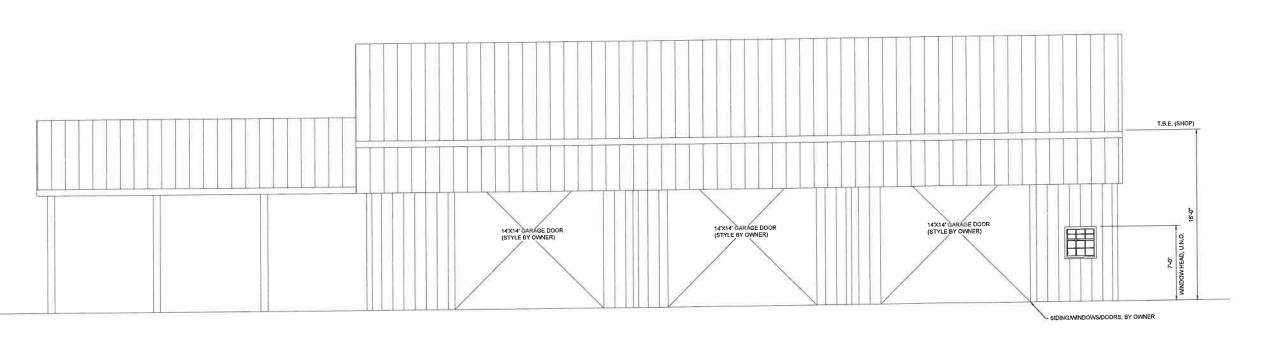
FOUNDATION PLAN

DESIGNED BY: A.JI
DRAWN BY: A.JI
APPROVED BY: HMH
PROJECT #: 24-067
DATE: 0225072024
No. Revision Date

S3.1



1 FRONT ELEVATION
Scale: 1/4" = 1'-0"



13200 STRICKLAND ROAD SUITE 114, BOX 332 RALEIGH, NC 27813 p. 919.957,5100 -1, 919.967,5101 www.fire-eng.com





WAKEBOARD DEALERSHIP

SANFORD, NC

Project Name

EXTERIOR ELEVATIONS

DESIGNED BY: AJI

DRAWN BY: AJI

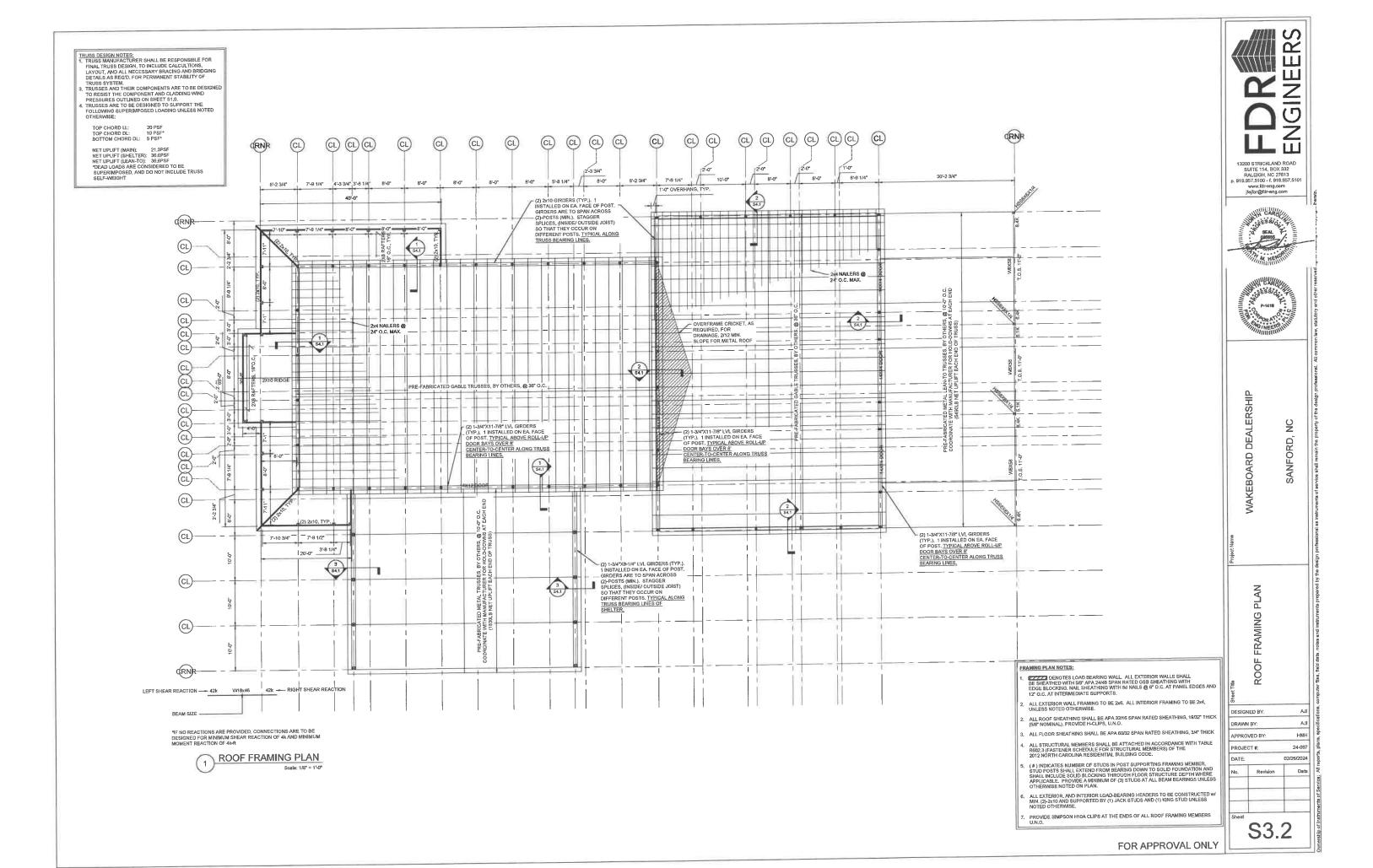
APPROVED BY: HMH

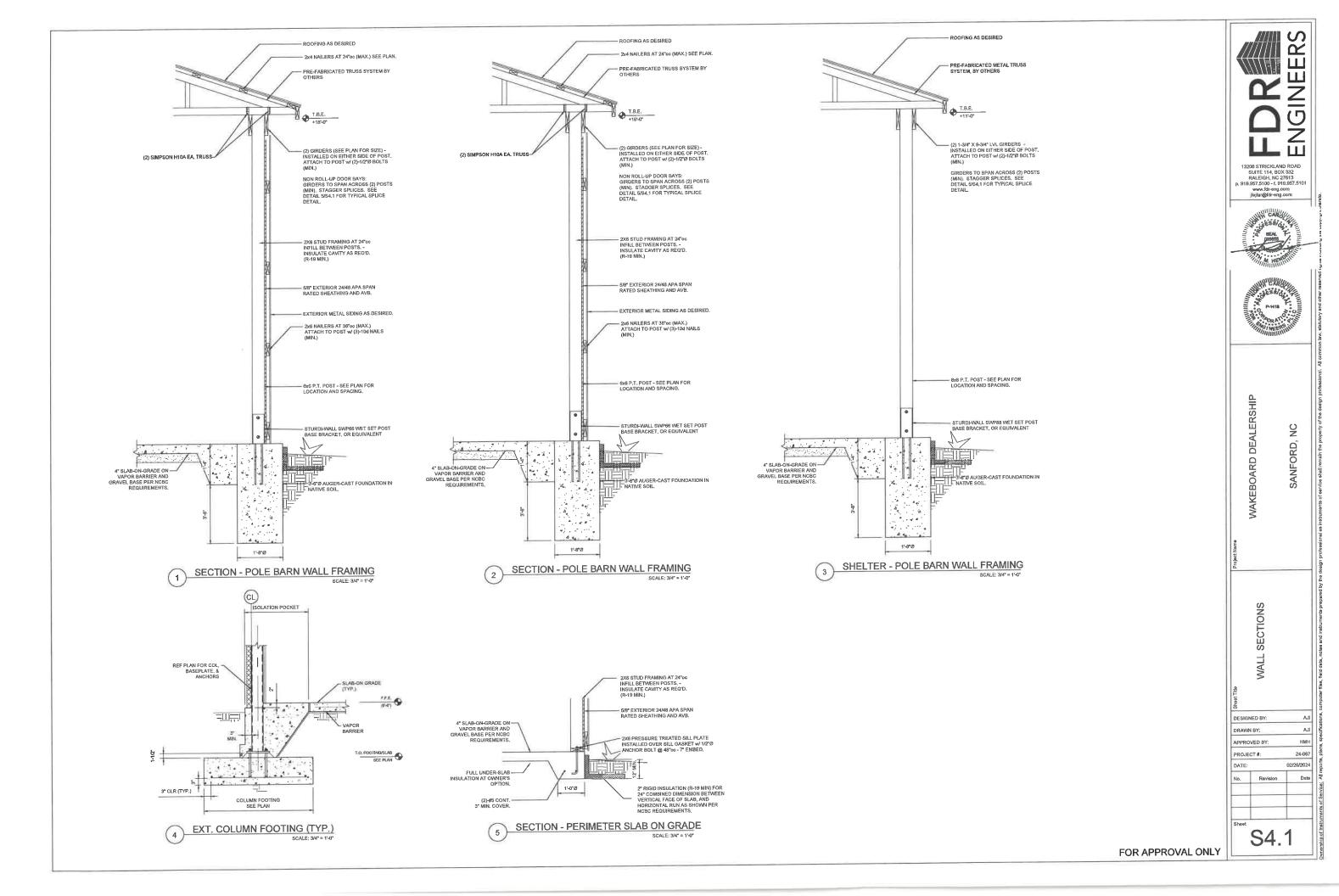
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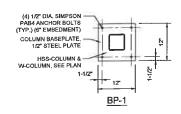
DATE: 02/26/2024

No. Revision Date

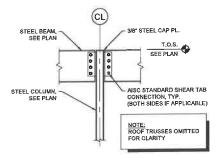
S2.1



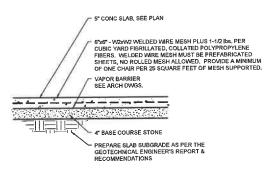




STEEL BASE PLATE DETAILS SCALE: 3/4" = 1'-0"



2 TYPICAL BEAM SHEAR CONNECTION Scale: 3/4" = 1'-0"



NOTES:

1. SEE ARCHITECTURAL DRAWINGS FOR SLOPES, DROPS, AND DRAIN LOCATIONS IN FLOOR SLABS.

5" SLAB ON GRADE DETAIL SCALE: 3/4" = 1'-0"

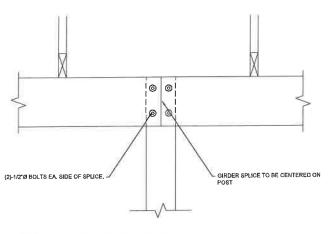
NOTES:

1. CONTRACTORS OPTION - USE REMOVABLE CONTROL JOINT MATERIAL SUCH AS "ZIP STRIP," "STRESSLOCK", OR APPROVED EQUAL.

2. SLAB ON GRADE CONTROL JOINTS SHALL BE TOOLED OR SAWCUT. THE JOINT PATTERN SHALL BE APPROXIMATELY SOUARE AND LIMITED TO AN AREA NOT TO EXCEED 225 S.F. JOINTS SHALL BE CUT WITHIN 12 HOURS OF POLISING SLAB. SEE PLAN FOR PROPOSED JOINT LAYOUT. FINAL JOINT LAYOUT TO BE DETERMINED BY THE GENERAL CONTRACTOR.

3/16" SAWCUT CONTROL JOINT. HAND CUT ENDS TO 3/4" MAX. FROM VERTICAL SURFACES. CLEAN JOINT AND FILL W ELASTOMERIC JOINT SEALANT, CUT JOINT TO A DEPTH OF 1/4 OF THE SLAB THICKNESS _____ SLAB REINFORCING, SEE TYP SLAB ON GRADE DETAIL

5" SLAB ON GRADE CONTROL JOINT SCALE: 3/4" = 1'-0"



GIRDER SPLICE DETAIL SCALE: 1-1/2" = 1'-0"







WAKEBOARD DEALERSHIP

SANFORD,

DETAILS TYPICAL

PROJECT #: 02/26/2024 Revision Date

S5.1

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