BUILDING PROFILE Width (ft) = 120Eave Height (ft) = 26 Length (ft) = 75Roof Slope (Rise/12) = 1.0:12

BUILDING LOADS

A) THIS IS TO CERTIFY THAT THIS STRUCTURE IS DESIGNED UTILIZING THE LOADS INDICATED AND APPLIED AS REQUIRED BY NCBC 18 / IBC 15

THIS CERTIFICATION IS LIMITED TO THE STRUCTURAL DESIGN OF THE FRAMING AND COVERING PARTS MANUFACTURED BY THE BUILDING MANUFACTURER AND AS SPECIFIED IN THE CONTRACT.
ACCESSORY ITEMS SUCH AS DOORS, WINDOWS, LOUVERS, TRANSLUCENT PANELS, VENTILATORS ARE NOT INCLUDED. ALSO EXCLUDED ARE OTHER PARTS OF THE PROJECT NOT PROVIDED BY THE BUILDING MANUFACTURER SUCH AS FOUNDATIONS, MASONRY WALLS, MECHANICAL EQUIPMENT AND THE ERECTION AND INSPECTION OF THE BUILDING. THE BUILDING SHOULD BE ERECTED ON A PROPERLY DESIGNED FOUNDATION IN ACCORDANCE WITH THE BUILDING MANUFACTURER'S DESIGN MANUAL, THE ATTACHED DRAWINGS, AND GOOD ERECTION PRACTICES. THE END USER AND/OR ENGINEER OF RECORD IS TO CONFIRM THAT THESE LOADS COMPLY WITH REQUIREMENTS OF THE

OCCUPANCY/RISK_CATEGORY	<u>Il - Normal</u> <u>Is 1.0000 le 1.00</u>	D) /
WIND LOAD ULTIMATE	120 MPH NOMINAL 92.95 MPH WIND EXPOSURE C	
CLOSURE TYPE	Enclosed INTERNAL WIND COEF0.18 / 0.18	
GROUND SNOW LOAD	10.00 PSF ROOF SNOW LOAD 7 PSF Ce 1.0000 Ct 1.00	E) /
SNOW BANKING LOADS	PER CODE	:
COLLATERAL DEAD LOAD	3 PSF	1
ROOF LIVE LOAD	20.00 PSF (REDUCIBLE Yes)	í
DEAD_LOAD	2.000 PSF (FOR ROOF PANELS AND PURLINS)	
SEISMIC		i
SPECTRAL RESPONSE Ss 0.18	50 \$1_0.0860 Sds_0.1984 Sd1_0.1376	1
SITE CLASS D	DESIGN RISK CATEGORY <u>C</u> Cs <u>0.0662</u>	
RESPONSE MODIFICATION FACTOR, R	3.000* FRAMES 3.000* BRACING	
	TEM (LATERAL DIRECTIONS) = ORDINARY STEEL MOMENT FRAMES	A)
	TEM (REW) = ORDINARY STEEL CONCENTRICALLY BRACED FRAMES	
	TEM (LEW) = ORDINARY STEEL MOMENT FRAMES TEM (LONGITUDINAL DIRECTIONS) = ORDINARY STEEL CONC. BRACED FRAMES	B)
ANALYSIS PROCEDURE	= EQUIVALENT LATERAL FORCE PROCEDURE	C)
	STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR	D)
SERVICE	ABILITY CRITERIA SEISMIC RESISTANCE.	υ)
		(C)

				_	
	MINIA	NUM DESI	GN DEFLECTIONS		
Endwall Column	=	120	Roof Panel (Live)	=	60
Endwall Rafter (Live)	=	180	Roof Panel (Wind)		60
Endwall Rafter (Wind)	=	180	Rigid Frame (Horz)		60
Wall Girt	=	90	Rigid Frame (Vert)	100	180
Roof Purlin (Live)	100	150	Rigid Frame (Seismic)	=	50
Roof Purlin (Wind)	=	150			
Wall Panel	=	60			

GENERAL NOTES

- A) THE STRUCTURE UNDER THIS CONTRACT HAS BEEN DESIGNED AND DETAILED FOR THE LOADS AND CONDITIONS STIPULATED IN THE CONTRACT AND SHOWN ON THESE DRAWINGS. ANY ALTERATIONS TO THE STRUCTURAL SYSTEM OR REMOVAL OF ANY COMPONENT PARTS, OR THE ADDITION OF OTHER CONSTRUCTION MATERIALS OR LOADS MUST BE DONE UNDER THE ADVICE AND DIRECTION OF A REGISTERED ARCHITECT, CIVIL OR STRUCTURAL ENGINEER.
- THE BUILDING MANUFACTURER WILL ASSUME NO RESPONSIBILITY FOR ANY LOADS NOT INDICATED.

 B) THIS METAL BUILDING IS DESIGNED WITH THE BUILDING MANUFACTURER'S STANDARD PRACTICES WHICH ARE BASED ON PERTINENT PROCEDURES AND RECOMMENDATIONS OF THE FOLLOWING ORGANIZATIONS AND CODES.
- 1. AMERICAN INSTITUTE OF STEEL CONSTRUCTION: " AISC SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS-ALLOWABLE STRESS DESIGN
- 2. AMERICAN IRON AND STEEL INSTITUTE: "SPECIFICATION FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL
- 3. AMERICAN WELDING SOCIETY: "STRUCTURAL WELDING CODE" AWS D1.1.
- 4. METAL BUILDING MANUFACTURER'S ASSOCIATION: "LOW RISE BUILDING SYSTEMS MANUAL"
- C) 1) MATERIAL PROPERTIES OF STEEL PLATE USED IN THE FABRICATION OF PRIMARY RIGID FRAMES, AND OTHER MATERIAL PROPERTIES OF STEEL PLATE USED IN THE PARMICIATION OF PRIMARY TRIBLE PROMES, AND OTHER PRIMARY STRUCTURAL EXCLUSIVE OF COLD-FORMED SECTIONS, CONFORM TO A529 WITH A MINIMUM YIELD POINT OF 55,000 psi. FLANGES GREATER THAN 1" IN THICKNESS OR 12" IN WIDTH CONFORM TO A572 WITH A MINIMUM YIELD POINT OF 55,000 psi. WEB MATERIAL CONFORMS TO ASTM—A529 WITH A MINIMUM YIELD POINT OF 55,000 psi.
- 2) MATERIAL PROPERTIES OF PIPE SECTIONS CONFORM TO ASTM-A500, GRADE B WITH A MINIMUM YIELD POINT OF 42,000 psi.
- 3) MATERIAL PROPERTIES OF TUBE SECTIONS CONFORM TO ASTM-A500, GRADE B WITH A MINIMUM YIELD POINT OF 46,000 psi.
- 4) MATERIAL PROPERTIES OF HOT ROLLED CHANNEL AND ANGLE MEMBERS CONFORM TO THE REQUIREMENTS OF ASTM-A529 WITHMINIMUM YIELD POINT OF 50,000 PSI. HOT ROLLED W-SHAPED MEMBERS CONFORM TO THE REQUIREMENTS OF ASTM-A992WITH MINIMUM YIELD POINT OF 50,000 PSI.
- 5) MATERIAL PROPERTIES OF COLD FORMED LIGHT GAGE STEEL MEMBERS CONFORM TO EITHER ASTM A653-06 GR 55 OR A1011-04 HSLAS CRADE 55 WITH YIELD OF 55,000 psi.

 6) MATERIAL PROPERTIES OF ROOF/WALL SHEETING, BASE METAL CONFORM TO ASTM-A792 GRADES 80 CLASS 1, 2 OR 3
- WITH A MINIMUM YIELD STRENGTH OF 80,000 PSI. COATING OF BASE MATERIAL IS 55% ALUMINUM-ZINC ALLOY IN ACCORDANCE WITH AZ55 SPECIFICATIONS.
- 7) CABLE UTILIZED FOR BRACING CONFORMS TO ASTM A475. CABLE BRACING IS TO BE INSTALLED TO A TAUT
- 8) ROD UTILIZED FOR BRACING MEMBERS CONFORM TO ASTM-A36 WITH MINIMUM YIELD POINT OF 36,000 PSI.
 9) IT IS THE RESPONSIBILITY OF ERECTOR TO ENSURE PROPER BOLT TIGHTNESS IN ACCORDANCE WITH APPLICABLE
 "RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING A-325 OR A-490 BOLTS". ALL A-325 BOLTS IN PRIMARY
 FRAMING MUST BE "SNUG-TIGHT", EXCEPT AS FOLLOWS:
 "FULLY-PRETENSION". A-325 BOLTS IF:

 G) BUILDING COCATOE IN A HIGH SEISMIC AREA. FOR IBC-BASED CODE, "HIGH SEISMIC AREA" IS DEFINED AS "SEISMIC DESIGN CATEGORY" OF "D", "E" OR "F".

 b) BUILDING SUPPORTS A CRANE SYSTEM WITH A CAPACITY GREATER THAN 5.00 TONS. 8) ROD LITHITED FOR BRACING MEMBERS CONFORM TO ASTM-A36 WITH MINIMUM YIELD POINT OF 36,000 PSI

- c) BUILDING SUPPORTS MACHINERY THAT CREATES VIBRATION, IMPACT OR STRESS REVERSALS ON THE CONNECTIONS.
- d) ANY CONNECTION DESIGNATED IN THESE DRAWINGS AS "A-325 SC".

- 10) SECONDARY MEMBERS AND FLANGE BRACE CONNECTIONS SHALL ALWAYS BE SNUG TIGHT, UNO.
- 11) ANCHOR BOLTS 3/4" IN DIAMETER THRU 1 1/4" IN DIAMETER CONFORM TO A.S.T.M. F1554 GR. 36.
 ANCHOR BOLTS 1/2" IN DIAMETER CONFORM TO A.S.T.M. A-307.
 D) UNLESS NOTED OTHERWISE ON FRAMING COLOR CHART: ALL STEEL MEMBERS EXCEPT BOLTS, FASTENERS, CABLE
- AND RODS SHALL RECEIVE ONE COAT OF STANDARD RED OXIDE SHOP PRIMER.

 E) SHOP AND FIELD INSPECTIONS AND ASSOCIATED FEES ARE THE RESPONSIBILITY OF THE CONTRACTOR, UNLESS STIPULATED OTHERWISE IN THE CONTRACT.

APPROVAL NOTES

- THE FOLLOWING CONDITIONS APPLY IN THE EVENT THAT THESE DRAWINGS ARE USED AS APPROVAL DRAWINGS:
- A) IT IS IMPERATIVE THAT ANY CHANGES TO THESE DRAWINGS:
- 1) BE MADE IN CONTRASTING INK
- 2) HAVE ALL INSTANCES OF CHANGE CLEARLY INDICATED.
- 3) BE LEGIBLE AND UNAMBIGUOUS.
- B) DATED SIGNATURE IS REQUIRED ON ALL PAGES.
- C) MANUFACTURER RESERVES THE RIGHT TO RESUBMIT DRAWINGS WITH EXTENSIVE OR COMPLEX CHANGES REQUIRED O AVOID MISFABRICATION, THIS MAY IMPACT THE DELIVERY SCHEDULE.
- D) APPROVAL OF THESE DRAWINGS INDICATES CONCLUSIVELY THAT THE MANUFACTURER HAS CORRECTLY INTERPRETED THE CONTRACT REQUIREMENTS, AND FURTHER CONSTITUTES AGREEMENT THAT THE BUILDING AS DRAWN, OR AS DRAWN WITH INDICATED CHANGES REPRESENTS THE TOTAL OF THE MATERIALS TO BE SUPPLIED BY MANUFACTURER.
- E) ANY CHANGES NOTED ON THE DRAWINGS NOT IN CONFORMANCE WITH THE TERMS AND REQUIREMENTS OF THE CONTRACT BETWEEN MANUFACTURER AND ITS CUSTOMER ARE NOT BINDING ON MANUFACTURER UNLESS SUBSEQUENTLY SPECIFICALLY ACKNOWLEDGED AND AGREED TO IN WRITING BY CHANGE ORDER OR SEPARATE DOCUMENTATION, MANUFACTURER RECOGNIZES THAT RUBBER STAMPS ARE ROUTINELY USED FOR INDICATING APPROVAL, DISAPPROVAL, REJECTION, OR MERE REVIEW OF THE DRAWINGS SUBMITTED. HOWEVER, MANUFACTURER DOES NOT ACCEPT CHANGES OR ADDITIONS TO CONTRACTUAL TERMS AND CONDITIONS THAT MAY APPEAR WITH USE OF A STAMP OR SIMILAR INDICATION OF APPROVAL, DISAPPROVAL, ETC. SUCH LANGUAGE APPLIED TO MANUFACTURER'S DRAWINGS BY THE CUSTOMER, ARCHITECT, ENGINEER, OR ANY OTHER PARTY WILL BE CONSIDERED AS UNACCEPTABLE ALTERATIONS TO THESE DRAWING NOTES, AND WILL NOT ALTER THE CONTRACTUAL RIGHTS AND OBLIGATIONS EXISTING BETWEEN MANUFACTURER AND ITS CUSTOMER.

SAFETY COMMITMENT

- THE RUILDING MANUFACTURER HAS A COMMITMENT TO MANUFACTURE QUALITY BUILDING COMPONENTS THAT CAN BE SAFELY ERECTED. HOWEVER, THE SAFETY COMMITMENT AND JOB SITE PRACTICES OF THE ERECTOR ARE BEYOND THE CONTROL OF THE BUILDING MANUFACTURER.
- IT IS STRONGLY RECOMMENDED THAT SAFE WORKING CONDITIONS AND ACCIDENT PREVENTION PRACTICES BE THE TOP PRIORITY OF ANY JOB SITE.
- LOCAL, STATE AND FEDERAL SAFETY AND HEALTH STANDARDS SHOULD ALWAYS BE FOLLOWED TO HELP INSURE WORKER SAFETY.
- MAKE CERTAIN ALL EMPLOYEES KNOW THE SAFEST AND MOST PRODUCTIVE WAY OF ERECTING A BUILDING. EMERGENCY PROCEDURES SHOULD BE KNOWN TO ALL EMPLOYEES.
- DAILY MEETINGS HIGHLIGHTING SAFETY PROCEDURES ARE ALSO RECOMMENDED. THE USE OF HARD HATS, RUBBER SOLE SHOES FOR ROOF WORK, PROPER EQUIPMENT FOR HANDLING MATERIAL, AND SAFETY NETS WHERE APPLICABLE.

ERECTOR / CONTRACTOR RESPONSIBILITIES

- A) IT IS THE RESPONSIBILITY OF THE ERECTOR/CONTRACTOR TO INSURE THAT ALL PROJECT PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT THE BUILDING MANUFACTURER OR ITS DESIGN ENGINEER IS ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT.
- B) THE CONTRACTOR MUST SECURE ALL REQUIRED APPROVALS AND PERMITS FROM THE APPROPRIATE AGENCY AS
- C) APPROVAL OF THE MANUFACTURER'S DRAWINGS AND CALCULATIONS INDICATE THAT THE BUILDING MANUFACTURER CORRECTLY INTERPRETED AND APPLIED THE REQUIREMENTS OF THE CONTRACT DRAWINGS AND SPECIFICATIONS. (SECT. 4.4.1 AISC CODE OF STANDARD PRACTICES, LATEST ED.)
- D) WHERE DISCREPANCIES EXIST BETWEEN THE MANUFACTURER'S STRUCTURAL STEEL PLANS AND THE PLANS FOR
- OTHER TRADES, THE STRUCTURAL STEEL PLANS SHALL GOVERN. (SECT. 3.3 AISC CODE OF STANDARD PRACTICE LATEST ED.)

 E) DESIGN CONSIDERATIONS OF ANY MATERIALS IN THE STRUCTURE WHICH ARE NOT FURNISHED BY THE BUILDING MANUFACTURER ARE THE RESPONSIBILITY OF THE CONTRACTORS AND ENGINEERS OTHER THAN THE BUILDING MANUFACT—URER'S ENGINEERS UNLESS SPECIFICALLY INDICATED.
- URER'S ENGINEERS UNLESS SPECIFICALLY INDICATED.

 F) THE RECTOR/CONTRACTOR IS RESPONSIBLE FOR ALL ERECTION OF STEEL AND ASSOCIATED WORK IN COMPLIANCE WITH
 THE BUILDING MANUFACTURER'S "FOR CONSTRUCTION" DRAWINGS.

 6) PRODUCTS SHIPPED TO ERECTOR/CONTRACTOR OR HIS CUSTOMER SHALL BE INSPECTED BY ERECTOR/CONTRACTOR
 IMMEDIATELY UPON ARRIVAL CLAIMS FOR SHORTAGES OR DEFECTIVE MATERIAL IF NOT PACKAGED MUST BE SENT TO THE
 MANUFACTURER IN WRITING WITHIN FIVE (5) DAYS AFTER RECEIPT OF THE SHIPMENT. HOWEVER, IF A DEFECT IS OF SUCH A NATURE
 THAT REASONABLE VISUAL INSPECTION WOULD FAIL TO DISCLOSE IT, THEN THE CLAIM MUST BE MADE WITHIN FIVE (5) DAYS
 AFTER THE EDECTOR (CONTRACTOR) LEASING OF THE DEFECT THE MANUFACTURER WITHIN FIVE (6) DAYS
 AFTER THE EDECTOR (CONTRACTOR) LEASING OF THE DEFECT THE MANUFACTURER WITHIN FIVE (6) DAYS
 AFTER THE EDECTOR (CONTRACTOR) LEASING OF THE DEFECT THE MANUFACTURER WITHIN THE CALLANDER OF MADE OF THE DEFECT THE SECONDARY DEFECT THIS ESSON AFTER THE ERECTOR/CONTRACTOR LEARNS OF THE DEFECT. THE MANUFACTURER WILL NOT BE LIABLE FOR ANY DEFECT UNLESS CLAIM IS MADE WITHIN ONE (1) YEAR AFTER DATE OF THE ORIGINAL SHIPMENT BY THE MANUFACTURER TO CONTRACTOR OR HIS CUSTOMER. THE MANUFACTURER WILL BE GIVEN A REASONABLE OPPORTUNITY TO INSPECT DEFECTIVE MATERIALS UPON RECEIPT OF CLAIM BY CONTRACTOR.
- OPUN RECEIPT OF COMMERCION.

 IF A DEFECT IS OF SUCH NATURE THAT IT CAN BE REMEDIED BY A FIELD OPERATION AT THE JOB SITE WITHOUT
 THE NECESSITY OF RETURNING THE MATERIAL TO THE MANUFACTURER, THEN UPON WRITTEN AUTHORIZATION OF
 THE MANUFACTURER THE CONTRACTOR MAY REPAIR OR CAUSE THE MATERIAL TO BE REPAIRED AND THE MANUFACTURER
 WILL REMBURSE THE CONTRACTOR FOR THE COST OF THE REPAIR IN ACCORDANCE WITH THE WRITTEN AUTHORIZATION.
- THE CORRECTION OF MINOR MISFITS BY THE USE OF DRIFT PINS TO DRAW THE COMPONENTS IN TO LINE, MODERATE AMOUNTS OF REAMING, CHIPPING AND CUTTING, AND THE REPLACEMENT OF MINOR SHORTAGES OF MATERIAL ARE A NORMAL PART OF ERECTION AND ARE NOT SUBJECT TO CLAIM.
- H) ALL BRACING AS SHOWN AND PROVIDED BY THE MANUFACTURER FOR THIS BUILDING IS REQUIRED AND SHALL BE
- INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE STRUCTURE.

 1) TEMPORARY SUPPORTS, SUCH AS TEMPORARY GUYS, BRACES, FALSE WORK, CRIBBING OR OTHER ELEMENTS REQUIRED. FOR THE ERECTION OPERATION WILL BE DETERMINED AND FURNISHED AND INSTALLED BY THE ERECTOR. THESE TEMPORARY SUPPORTS WILL SECURE THE STEEL FRAMING, OR ANY PARTLY ASSEMBLED STEEL FRAMING, AGAINST LOADS COMPARABLE IN INTENSITY TO THOSE FOR WHICH THE STRUCTURE WAS DESIGNED, RESULTING FROM WIND, SEISMIC FORCES AND ERECTION OPERATIONS, BUT NOT THE LOADS RESULTING FROM THE PERFORMANCE OF WORK BY OR THE ACTS OF OTHERS, NOR SUCH UNPREDICTABLE LOADS AS THOSE DUE TO TORNADO, EXPLOSION OR COLLISION (SECT. 7.10.3 AISC CODE OF STANDARD PRACTICE, LATEST ED.)
- J) METAL BUILDING MANUFACTURER IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL AND WORKMANSHIP OF FOUNDATION. ANCHOR BOLT PLANS PREPARED BY MBM ARE INTENDED TO SHOW ONLY LOCATION, DIAMETER AND PROJECTION OF THE ANCHOR RODS REQUIRED TO ATTACH THE METAL BUILDING SYSTEM TO FOUNDATION. IT IS RESPONSIBILITY OF THE END CUSTOMER TO ENSURE THAT ADEQUATE PROVISIONS ARE MADE FOR SPECIFYING ROD EMBEDMENT, BEARING VALUES, TIE RODS AND OTHER ASSOCIATED ITEMS EMBEDDED IN THE CONCRETE FOUNDATION, AS WELL AS FOUNDATION DESIGN FOR THE LOADS IMPOSED BY MB SYSTEM, OTHER IMPOSED LOAD, AND THE BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE (MBMA 06 SECTIONS 3.2.2 AND A3)
- K) METAL BUILDING MANUFACTURER DOES NOT PROVIDE ANY FIELD SUPERVISION FOR THE ERECTION, NOR DOES MBM PERFORM ANY INSPECTIONS DURING OR AFTER ERECTION.

COMPONENTS & CLADDING (unfactored) 32.081 psf / -34.755 psf psf / -42.695 psf Wall Edge Values = 32.081

IT IS THE RESPONSIBILITY OF THE CUSTOMER TO PROVIDE ALL DOCUMENTATION REQUIRED FOR ANY ACCESSORIES NOT PROVIDED BY MBM TO THEIR LOCAL PERMITTING OFFICE. ALL ACCESSORIES MUST COMPLY AND MEET ALL DESIGN REQUIREMENTS PER LOCAL CODES.

ALL VEHICULAR FRAMED OPENINGS SUPPLIED ON THIS PROJECT HAVE BEEN DESIGNED TO SUPPORT WIND LOADS NORMAL TO A DOOR SYSTEM, BASED ON THE STANDARD BUILDING CODE CRITERIA THE VEHICULAR FRAMED OPENING HAS NOT BEEN DESIGNED FOR ANY ADDITIONAL MOMENT OR CATENARY FORCE FROM THE DOOR SYSTEM. ANY CHANGES TO THE INFORMATION SHOWN HERE WOULD REQUIRE AN ENGINEERING INVESTIGATION AND POSSIBLE BUILDING

Rigid Frame Flange brace	_	<u>ન</u> ૄ		ey Prim			
Angle:	RO		_	The state of			
	Grt	Pur	EvSt	Jmb	88	End Col	wall Ra
U SECTION:	RO	RO	RÓ	RO	R0	RO	RO
C SECTION:	RO	RO	RO	RO	RO	RO	RO
D SECTION:	RO	RO	RQ	RO	R0	RO	RO
Z SECTION:	RO	RO	RO	RO	RO	RO	RO
E SECTION:	RO	RO	RO	RO	RO	RO	RO
R SECTION:	RO	RO	RO	RO	RO	RO	RO
W SECTIONS	RO	RO	RO	RD	RO	RO	RO

EDAMING COLORS

WHEN GALVANIZED PROVIDED: ALL FINISHED PRIMARY BUILT-UP AND HOT ROLL MEMBERS ARE HOT DIPPED GALVANIZED. ALL SECONDARY COLD FORMED MEMBERS ARE PRE-GALVANIZED.



BY AN IAS ACCREDITED FACILITY.

COLORS:

LINER

SOFFIT:

WALLS:

GABLE:

EAVE:

CUTTER

CORNER:

TRAMED OP

DOWNSPOUTS:

DOOR PANEL:

GALVALUME

SADDLE TAN

SADDLE TAN

KOKO BROWN

COLOR

COLOR

COVER PAGE ANCHOR BOLT LAYOUT 1.1 ANCHOR BOLT DETAILS ANCHOR BOLT REACTIONS 1.2 ROOF FRAMING LAYOUT RIGID FRAME CROSS SECTION 2.1-2.4 SIDEWALL FRAMING LAYOUT 3 ENDWALL FRAMING LAYOUT HANGER DOOR FRAMING 4.1 HANGER DOOR DETAILS 4.2 5-5.4 FRAMING DETAILS ROOF PANELS & TRIM 6.1 ROOF PANEL DETAILS SIDEWALL PANELS & TRIM 7.1 SIDEWALL PANEL DETAILS ENDWALL PANELS & TRIM ENDWALL PANEL DETAILS 8.1 SPECIAL DETAILS 10 LINER SHEETING & TRIM

DRAWING INDEX

DESCRIPTION

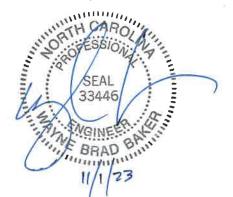
REV.

PAGE

THIS PROJECT IS DESIGNED AS AN ENCLOSED BUILDING, ACCESSORIES (DOORS, WINDOWS, ETC.) BY OTHERS MUST BE DESIGNED AS "COMPONENTS AND CLADDING" IN ACCORDANCE TO SPECIFIC WIND PROVISIONS OF REFERENCED BUILDING CODE.

FOR OCCUPANCY (RISK) CATEGORY I OR II, IBC PROVISIONS INDICATE THAT SINGLE-STORY BUILDINGS SHALL HAVE "NO DRIFT LIMIT" PROVIDED THAT INTERIOR WALLS, PARTITIONS, CEILINGS AND EXTERIOR WALL SYSTEMS HAVE BEEN DESIGNED TO ACCOMMODATE THE SEISMIC STORY DRIFTS. INTERIOR WALLS, PARTITIONS, CEILINGS OR EXTERIOR SYSTEMS NOT PROVIDED BY MBM SHALL BE DESIGNED AND DETAILED BY OTHERS TO ACCOMODATE THE SEISMIC STORY DRIFTS.

> Wayne Brad Baker, P.E. 235 Sanders Rd. Hahira, GA 31632



	BRAD BRAD BRAD BRAD BRAD BRAD BRAD BRAD
	DRAWING STATUS
	FOR APPROVAL: THESE DRAWINGS, BEING FOR APPROVAL, ARE BY DEFINITION NOT FINAL, AND ARE FOR CONCEPTUAL REPRESENTATION ONLY. THEIR PURPOSE IS TO CONFIRM PROPER INTERPRETATION OF THE PROJECT DOCUMENTS. ONLY DRAWINGS ISSUED 'FOR CONSTRUCT CAN BE CONSIDERED AS COMPLETE.
X	FOR PERMIT. THESE DRAWINGS, BEING FOR PERMIT, ARE BY DEFINITION NOT FINAL IN THAT AS A MINIMUM, PIECE MARKINGS A NOT IDENTIFIED., ONLY DRAWINGS SUED FOR CONSTRUCTION CAN BE CONSIDERED AS COMPLETE.
	FOR CONSTRUCTION: THESE DRAWINGS ARE FINAL AND ISSUED FOR FIELD USE FOR BUILDING ERECTION

COURT BUILDINGS LYNDHURST COR SARASOTA, STEEL 4084 JOB NO 8245 DATE

2

ERWIN,

JOBSITE:

28339

2

ERWIN,

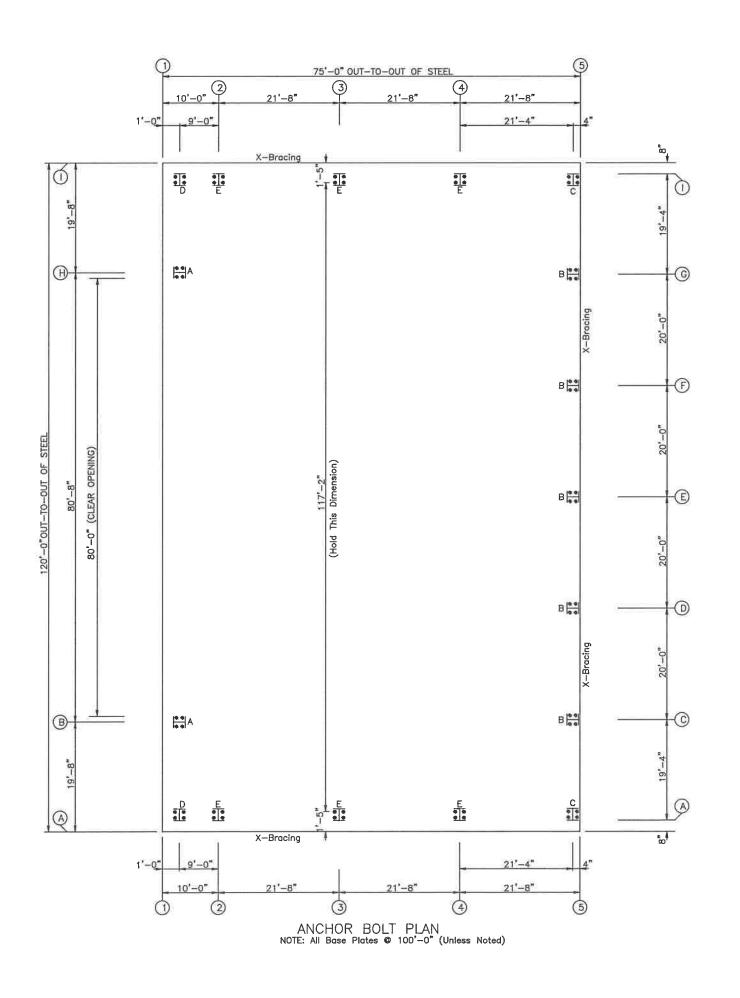
615

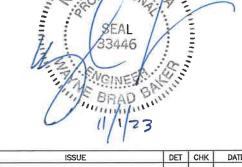
HANGAR AIRPORT

ERWIN

E BY 10/26/23 OJECT ONSTRUCTION NONE DAR MLE COVER PAGE DEFINITION RKINGS ARE NUMBER MPLETE.

PAGE





ISSUE DET CHK DATE

STEELCOR BUILDINGS

CUSTOMER: ERWIN HANGAR

JOB NO: 8245

LOCATION: ENWIN, NC 28339

DET CHK DATE

DATE

10/26/23

DATE

10/26/23

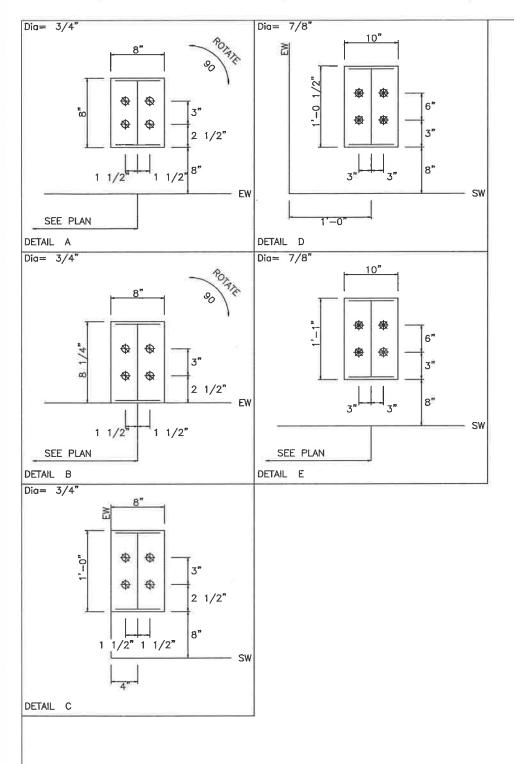
DET CHK DATE

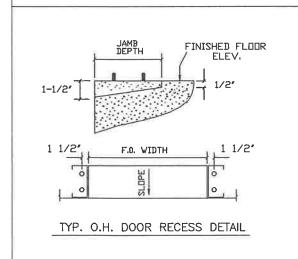
DATE

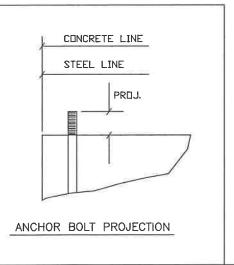
10/26/23

NOTE: ALL FIELD LOCATED FRAMED OPENING LOCATIONS SHALL BE AT THE DISCRETION OF THE ERECTOR/CUSTOMER. IT IS RECOMMENDED THAT THESE ANCHORS BE LOCATED AT TIME OF ERECTION.

FIELD LOCATE: (2) 3070 WALKDOORS

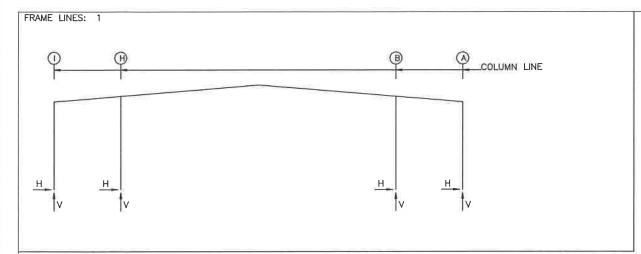


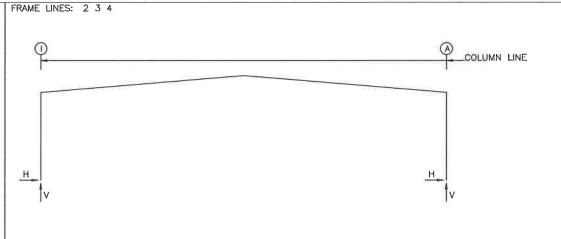


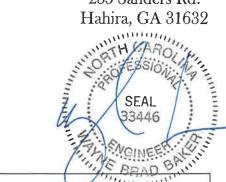




	· ·		
IS	SUE	DET	CHK DATE
STEELCOR BUILD	DINGS		
CUSTOMER: ERWIN HANGAR			
ов NO: 8245		10/	26/23
ERWIN, NC 28	339		
ANCHOR BOLT	DETAILS		
PAGE 1.1	DRAWN BY: DAR	SPW	SCALE: NON







RIGIE	FRAN	⁄1Ε: ε	BASIC COL	UMN REA	ACTIONS ((k)								
Frame Line 1 1 1	Column Line I A H B	Horiz 0.0 0.0 0.0 0.0	-Dead Vert -0.6 -0.6 4.3 4.3	Collo Horiz -0.1 0.1 0.0	teral- Vert -2.2 -2.2 9.3 9.3	Horiz -0.1 0.1 0.0 0.0	-Live Vert -1.4 -1.4 5.7 5.7	Horiz 0.0 0.0 0.0 0.0		Horiz -1.9 -2.5 0.0 0.0	Left1- Vert 1.5 4.8 -14.0 -13.2	-Wind_ Horiz 2.5 1.9 0.0	Right1- Vert 4.8 1.5 -13.2 -14.0	
Frame Line 1 1 1	Column Line I A H B	Wind Horiz -2.5 -1.8 0.0 0.0	1_Left2- Vert 0.8 4.0 -9.4 -8.6	Wind_ Horiz 1.8 2.5 0.0 0.0	Right2- Vert 4.0 0.8 -8.6 -9.4	Wind Horiz 1.3 -1.0 0.0 0.0	d_Long1- Vert 1.8 1.4 -10.7 -7.8	Wind Horiz 1.0 -1.3 0.0 0.0	d_Long2- Vert 1.4 1.8 -7.8 -10.7	-Seism Horiz -0.3 -0.3 0.0 0.0	ic_Left Vert -0.4 0.4 0.5 -0.5	Seismic Horiz 0.3 0.3 0.0 0.0	:_Right Vert 0.4 -0.4 -0.5 0.5	
Frame Line 1 1 1	Column Line I A H B	-MIN_S Horiz -0.1 0.1 0.0 0.0	NOW Vert -1.2 -1.2 4.8 4.8	F1PAT_ Horiz -0.1 0.1 0.0 0.0	Vert -1.4 -2.1 5.8 5.0	F1PAT_ Horiz 0.1 0.1 0.0 0.0	Vert -2.1 -1.4 5.0 5.8	F1PAT_ Horiz 0.0 0.0 0.0 0.0	Vert 0.8 0.8 0.7 0.7	F1PAT_ Horiz -0.1 0.1 0.0 0.0	Vert -2.2 -2.2 5.1 5.1	F1UNB_ Horiz 0.0 0.0 0.0 0.0	SL_L- Vert -1.1 -1.0 4.1 2.3	
Frame Line 1 1 1	Column Line I A H B	F1UNB_ Horiz 0.0 0.0 0.0 0.0	_SL_R- Vert -1.0 -1.1 2.3 4.1											
Frame Line 2 2	Column Line I A	Horiz 3.0 -3.0	Dead Vert 4.9 4.9		teral- Vert 6.9 6.9	 Horiz 9.0 -9.0	Live Vert 11.5 11.5	Horiz 5.3 –5.3	-Snow Vert 6.7 6.7	Wind Horiz -22.5 13.1	Left1- Vert -27.6 -19.7	-Wind_ Horiz -13.1 22.5	Right1— Vert —19.7 —27.6	
Frame Line 2 2	Column Line I A	Wind Horiz -16.7 7.3	d_Left2- Vert -17.3 -9.4	-Wind_ Horiz -7.3 16.7	Right2- Vert -9.4 -17.3	Wind Horiz -11.5 12.2	d_Long1- Vert -22.4 -18.1	Wind Horiz -12.2 11.5	d_Long2- Vert -18.1 -22.4	-Seism Horiz -0.5 -0.5	ic_Left Vert -0.2 0.2	Seismic Horiz 0.5 0.5	:_Right Vert 0.2 -0.2	
Frame Line 2 2	Column Line I A	-MIN_5 Horiz 7.5 -7.5	SNOW Vert 9.6 9.6	F2UNB_ Horiz 5.0 -5.0	SL_L- Vert 7.1 4.3		_SL_R- Vert 4.3 7.1							
Frame Line 3 3	Column Line I A		-Dead Vert 6.0 6.0	Collo Horiz 3.3 -3.3		Horiz 13.2 -13.2	-Live Vert 15.8 15.8	Horiz 7.7 –7.7	-Snow Vert 9.2 9.2	Wind Horiz -27.3 16.6		-Wind_ Horiz 16.6 27.3	Right1— Vert -23.6 -32.1	
Frame Line 3 3	Column Line I A	Wind Horiz -18.8 8.1		-Wind_ Horiz -8.1 18.8	Right2- Vert 9.5 -17.9	Wind Horiz -17.0 18.1	d_Long1- Vert -30.8 -24.9	Wind Horiz -18.1 17.0	d_Long2- Vert -24.9 -30.8	-Seism Horiz -0.6 -0.7	ic_Left Vert -0.3 0.3	Seismic Horiz 0.6 0.7	_Right Vert 0.3 -0.3	
Frame Line 3 3	Column Line I A	-MIN_S Horiz 11.0 -11.0	NOW Vert 13.2 13.2	F3UNB_ Horiz 7.4 –7.4		F3UNB Horiz 7.4 –7.4	_SL_R- Vert 6.0 9.8							
Frame Line 4 4	Column Line I A	Horiz 3.8 3.8	-Dead Vert 6.0 6.0		teral— Vert 4.0 4.0	 Horiz 13.2 -13.2	-Live Vert 15.8 15.8		Vert	———Wind Horiz —31.0 18.7	_Left1- Vert -36.2 -26.1	-Wind_ Horiz -18.7 31.0	Right1— Vert —26.1 —36.2	
Frame Line 4 4	Column Line I A	Wind Horiz -22.5 10.2	i_Left2- Vert -22.1 -11.9	Wind_ Horiz 10.2 22.5	Right2- Vert -11.9 -22.1	Wind Horiz -17.0 18.1	d_Long1- Vert 30.8 24.9	Wind Horiz -18.1 17.0	i_Long2- Vert -24.9 -30.8	-Seism Horiz -0.6 -0.7		Seismic Horiz 0.6 0.7		
Frame Line	Column Line	-MiN_S Horiz	SNOW Vert	F4UNB_ Horiz	SL_L- Vert	F4UNB. Horiz	_SL_R- Vert							

RIGI) FR	AME	: AN	ICHOR I	30LTS &	BASE	PLATES
Frm	Col		_Bolt	Base_	_Plate (i	n)	Grout
Line	Line		Dia	Width	Length	Thick	(in)
1	I	4	0.875	10.00	12.50	0.500	0.0
	A	4	0.875	10.00	12.50	0.500	0.0
	H	4	0.750	8.000	8.000	0.375	0.0
	B	4	0.750	8.000	8.000	0.375	0.0

RIGI	D FR	AME	: AN	ICHOR I	BOLTS &	BASE	PLATES
Frm Line	Col Line		_Bolt Dia	Base_ Width	_Plate (i Length	n) Thick	Grout (in)
2 2	I A	4	0.875 0.875	10.00 10.00	13.00 13.00	0.625 0.625	0.0

RIGI	D FR	AME	: AN	ICHOR I	BOLTS &	BASE	PLATES
Frm Line	Col Line		_Bolt Dia	Base. Width	_Plate (i Length	n) Thick	Grout (in)
3 3	A	4 4	0.875 0.875	10.00 10.00	13.00 13.00	0.750 0.750	0.0

ij	RIGI	D FR	AME	: AN	ICHOR I	BOLTS &	BASE	PLATES
	Frm Line	Col Line	Anc. Qty	_Bolt Dia	Base. Width	_Plate (i Length	n) Thick	Grout (in)
	4 4	A	4	0.875 0.875	10.00 10.00	13.00 13.00	0.625 0.625	0.0

GENERAL NOTES

- FOUNDATION DESIGN AND CONSTRUCTION ARE NOT THE RESPONSIBILITY OF METAL BUILDING MANUFACTURER.
- 2. ALL REACTIONS ARE UNFACTORED.
- 3 ULTIMATE WIND LOADS ARE USED TO DERIVE THE WIND REACTION.
- 4. ANCHOR BOLTS SHALL BE ACCURATELY SET TO A TOLLERANCE OF +/- 1/8" IN BOTH ELEVATION AND LOCATION.
- 5. COLUMN BASE PLATES ARE DESIGNED NOT TO EXCEED A BEARING PRESSURE OF 1050 POUNDS PER SQUARE INCH.

V														1, YE	BRA	DBA
END	WAL		LUMN:	BAS	IC COL	JMN R	EACTION	S (k)					6	VI, YE	ring	in the
Frm Line 1	Col Line H B	Wind Press Horz -3.6 -3.6	Wind Suct Horz 3.9 3.9	–М Ног О.	z Ve .0 4	rt H 1	0.0	ert H 3.6	1UNB_ orz 0.0 0.0	_SLR Vert 2.0 3.6					11/	23
Frm Line 555555555	Col Line A C D E F G I	Dead Vert 0.6 1.2 1.2 1.2 1.2 1.2 0.6	Collar Vert 0.3 0.7 0.6 0.7 0.6 0.7	Ver 1. 4. 4. 4. 4.		Snow /ert 0.6 1.7 1.5 1.6 1.5	Wind Left1 Vert -3.1 -8.8 -8.1 -6.0 -4.2 -4.9	Wind Rigl Verl 2. 4. 6. 8. 8. 3.	nt1 1 9 2 0 1	Wind Left2 Vert -2.0 -6.2 -5.8 -4.0 -1.9 -2.3 -1.0	Wind Right2 Vert -1.0 -2.3 -1.9 -4.0 -5.8 -6.2 -2.0	Wind Press Horz -3.5 -6.7 -7.2 -7.7 -7.2 -6.7 -3.5	Hor 4. 7. 8. 8. 8.	et 7 0 4 0 4	Wind Long1 Vert -3.5 -8.7 -8.0 -5.7 -4.2 -5.1	Wind Long2 Vert -2.1 -5.1 -4.2 -5.7 -8.0 -8.7 -3.5
Frm e 55555555555555555555555555555555555	Col Line A C D E F G I	Seis Left Vert 0.0 0.0 0.0 0.0 0.0	Seis Right Vert 0.0 0.0 0.0 0.0 0.0	Sei: Lon Ver 0. 0. 0. 0. 0.	ng f t 1 .0 .0 .0 .0 .0 .0	-MIN_S Horz 0.0 0.0 0.0 0.0 0.0 0.0	SNOW Vert 0.9 2.4 2.1 2.2 2.1 2.4 0.9	E2UNB_ Horz 0.0 0.0 0.0 0.0 0.0 0.0	SL_L Vert 0.7 1.5 2.6 2.1 0.2 0.6 0.2	- E2UNE Horz 0.0 0.0 0.0 0.0 0.0 0.0	3_SL_R- Vert 0.2 0.6 0.2 2.1 2.6 1.5	E2PAT_ Horz 0.0 0.0 0.0 0.0 0.0 0.0	Vert 1.7 5.1 2.1 -0.3 0.1 0.0	E2PAT, Horz 0.0 0.0 0.0 0.0 0.0 0.0 0.0	Vert -0.2 2.0 4.9 2.1 -0.3 0.1	-
Frm Line 5 5 5 5 5 5 5	Col Line A C D E F G	E2PAT_ Horz 0.0 0.0 0.0 0.0 0.0 0.0 0.0	Vert 0.0 -0.3 2.1 4.9 2.1 -0.3 0.0	E2PAT_ Horz 0.0 0.0 0.0 0.0 0.0 0.0 0.0	Vert 0.0 0.1 -0.3 2.1 4.9 2.0 -0.2	E2PA Horz 0.0 0.0 0.0 0.0 0.0 0.0	T_LL_5- Vert 0.0 0.0 0.1 -0.3 2.1 5.1 1.7	E2PAT Horz 0.0 0.0 0.0 0.0 0.0 0.0	Ver 2. 2. 2. 2. 2. 2.	t Horz 0 0.0 3 0.0 1 0.0 2 0.0 1 0.0 5 0.0	0 -0.2 0 2.5 0 2.1 0 2.2 0 2.1 0 2.3	-				

Building reactions are based on the following building data: Width (ft) = 120.0 Eave Height (ft) = 75.0 Eave Height (ft) = 26.0/ 26.0 Roof Slope (Rise/12) = 1.0/ 1.0 Dead Load (psf) = 2.0 Collateral Load (psf) = 3.0 Roof Live Load(psf) = 12.0 Snow Load (psf) = 12.0 Snow Load (psf) = 7.0 Wind Speed (mph) = 120.0 Wind Code = NCBC 18 (IBC 15) Exposure = C Enclosed/Open/Partial = ENCLOSED = 1.00 Importance Wind = 1.00 Importance Seismic = 1.00 Seismic Zone = C Seismic Coeff (Fa*Ss) = 0.30

END	WALL	COI	LUMN:	ANCI	ANCHOR BOLTS & BASE PLATES							
Frm Line					Base_Plate (in) Grout Width Length Thick (in)							
5	Α	4	0.750	8.000	12.00	0.375	0.0					
5	С	4	0.750	8.000	8.250	0.375	0.0					
5	D	4	0.750	8.000	8.250	0.375	0.0					
5	Ε	4	0.750	8.000	8.250	0.250	0.0					
5	F	4	0.750	8.000	8.250	0.375	0.0					
5	G	4	0.750	8.000	8.250	0.375	0.0					
5	5 I		0.750	8.000	12.00	0.375	0.0					

ANCHOR	BOLT	SUMMAR'

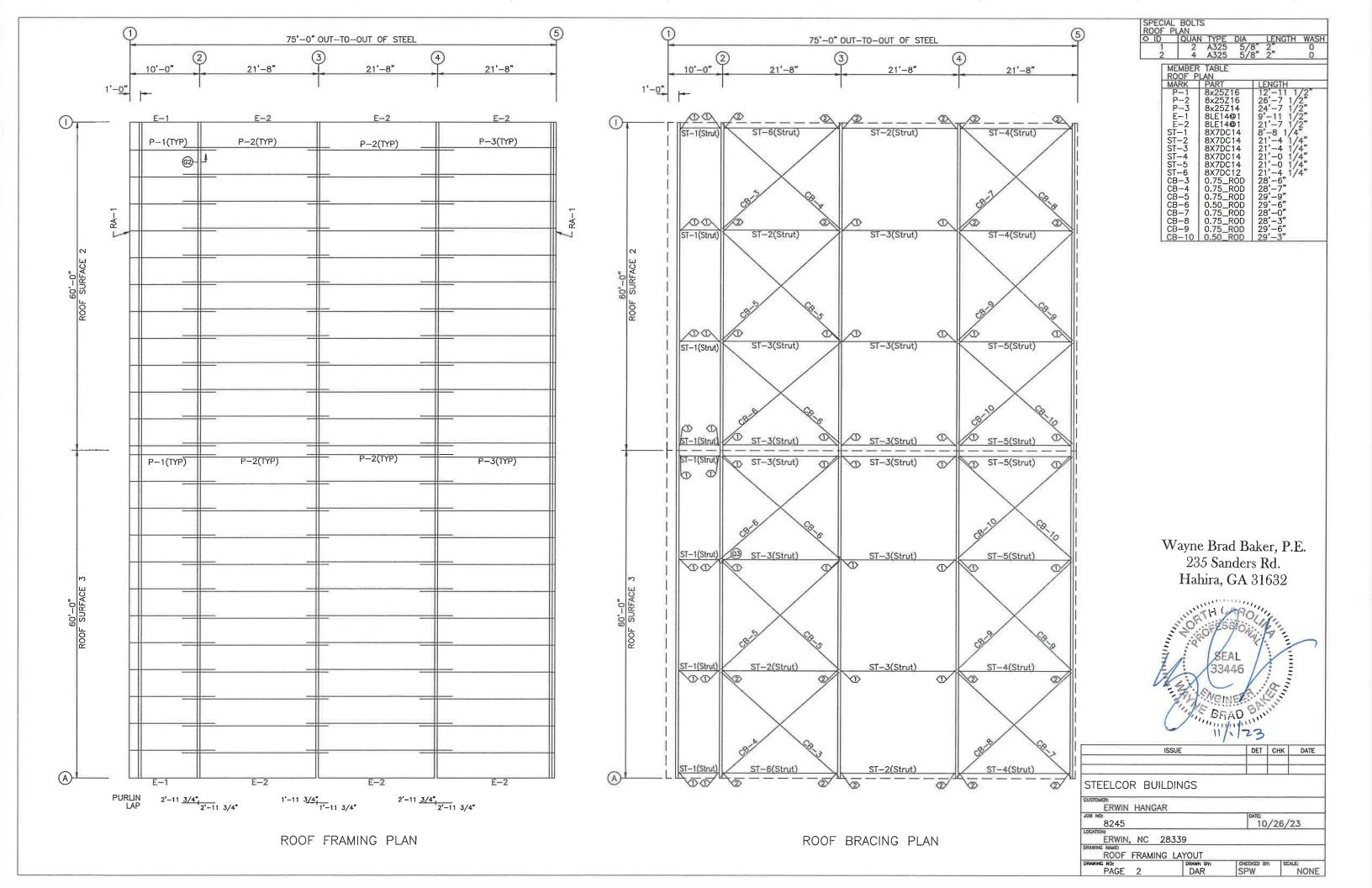
Qty	Locate	Dia (in)	Туре	Projection (in)
\$ 36	Endwall	3/4"	GR36	1.50
\$ 32	Frame	7/8"	GR36	2.50

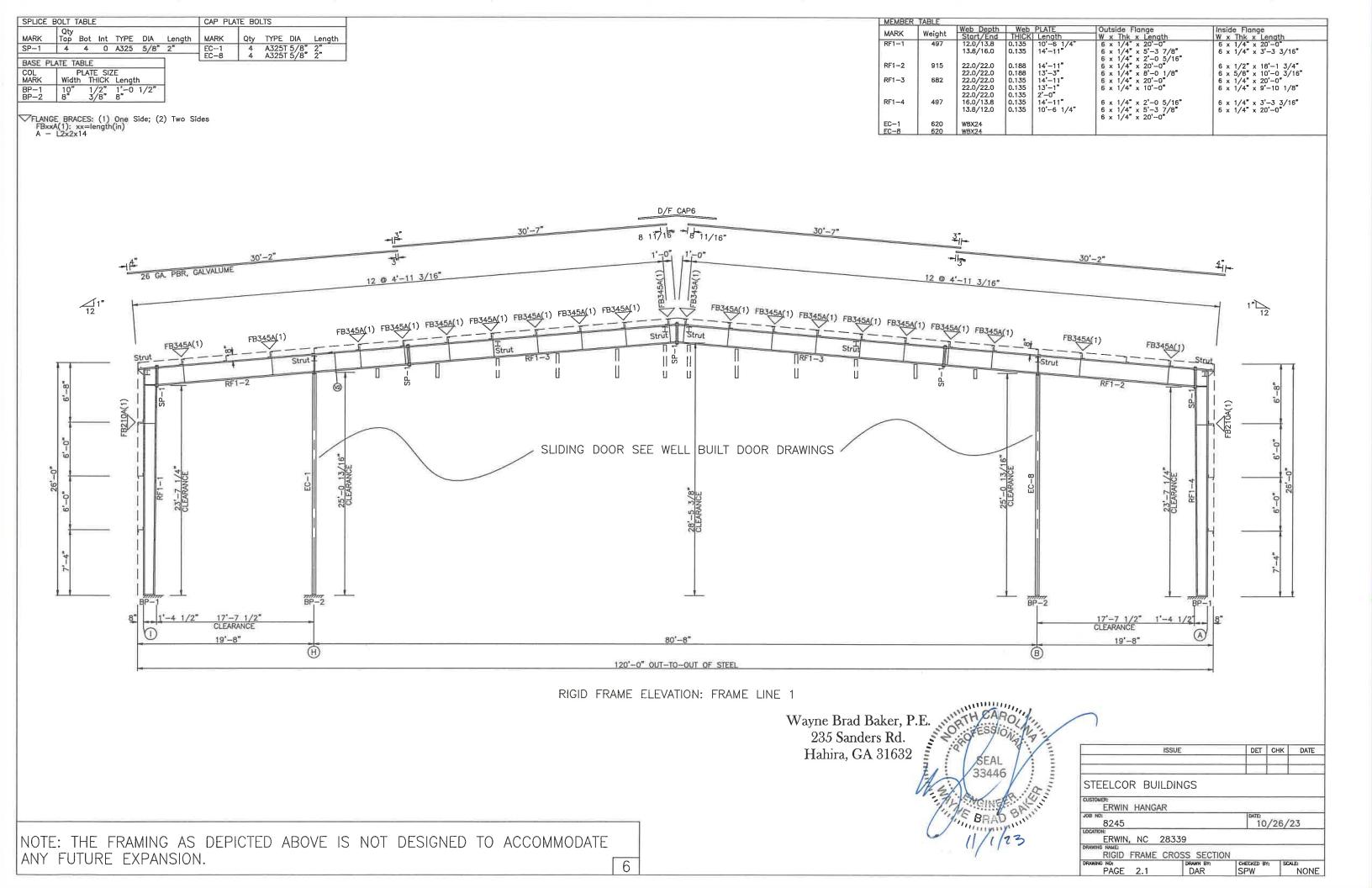
NOTE: THE FRAMING AT BOTH ENDWALLS IS NOT DESIGNED TO ACCOMMODATE FUTURE ADDITIONS. REACTIONS CORRESPONDING TO THESE FRAME LINES REFLECT LOADINGS FOR ACTUAL TRIBUTARY AREA AND ARE NOT INTENDED TO INCLUDE ANY FUTURE MODIFICATIONS UNLESS NOTED OTHERWISE.

BUILDING BRACING REACTIONS

						,	01	
Wa	ılı	Col		React	ions(k —Sei) smic—	_Sheai 7f+)	
Loc	Line	Col Line	Horz			Vert		Note
L EW	1							(h)
F_SW	Á 5	2,3	21.2	23.8	2.4	2.7		()
R_EW	5	C,D	1.7	2.4	0.4	0.6		
B_SW	!	2,3 C,D F,G 3,2	1.7 21.2	2.4 23.8	0.4 2.4	0.6 2.7		
(h)Rigi	id fra	me at	endwall					
	id fra			23.6	2.4	2.7		

ISS	SUE	DET	СНК	DATE
STEELCOR BUILD	INGS			
ERWIN HANGAR				
ов NO: 8245		DATE: 10,	/26/	23
ERWIN, NC 28	339			
ANCHOR BOLT F	REACTIONS			
PAGE 1.2	DRAWN BY: DAR	SPW	: SC	NONE





MEMBER TABLE SPLICE BOLT TABLE Web PLATE
THICK| Length
0.250 6'-7 13/16"
0.250 14'-11"
0.313 4'-0 9/16"
0.250 13'-10 9/16"
0.250 2'-0"
0.188 10'-0"
0.188 10'-0 3/8"
0.188 14'-11"
0.188 5'-1" Outside Flange W x Thk x Lenath 8 x 5/16" x 20'-0" 8 x 5/16" x 5'-3 11/16" 8 x 5/16" x 4'-4 7/16" 8 x 5/16" x 16'-0 7/8" 8 x 1/4" x 9'-6" Weight 1307 MARK Qty Top Bot Int TYPE DIA MARK RF2-1 2 A325 7/8" 0 A325 3/4" 2 A325 5/8" 21.9/44.0 44.0/44.0 44.0/26.5 26.5/24.0 24.0/24.0 24.0/24.0 24.0/31.5 31.5/34.0 RF2-2 1146 BASE PLATE TABLE COL PLATE SIZE
MARK Width THICK Length
BP-1 10" 5/8" 1"-1" 6 x 1/4" x 20'-0" 6 x 1/4" x 10'-0 3/8" RF2-3 FLANGE BRACES: (1) One Side; (2) Two Sides FBxxA(1): xx=length(in) A - L2x2x14 D/F CAP6 8 11/16 26 GA. PBR, GALVALUME 12 @ 4'-11 3/16" 12 @ 4'-11 3/16" 121" FB370A(1) FB370A(1) FB405A(1) Strut Strut

1

RIGID FRAME ELEVATION: FRAME LINE 2

120'-0" OUT-TO-OUT OF STEEL



DET	CHK	DATE
	DEI	

Inside Flange W x Thk x Length 8 x 5/16" x 19'-8 3/4" 8 x 5/16" x 2'-0"

8 x 5/16" x 15'-11 5/8" 8 x 1/4" x 10'-0"

6 x 1/4" x 10'-0 5/16" 6 x 1/4" x 19'-9 3/8"

1" 12

STEELCOR BUILDINGS

21'-11 11/16' CLEARANCE

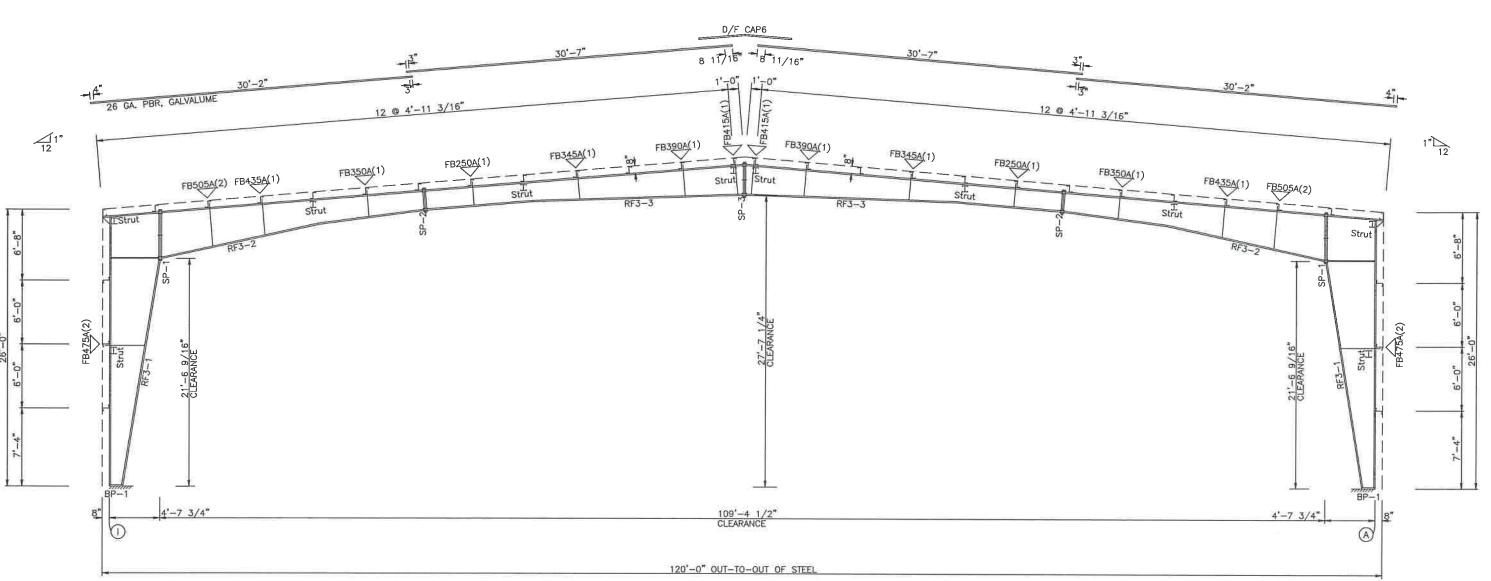
ERWIN HANGAR 8245 10/26/23

ERWIN, NC 28339 RIGID FRAME CROSS SECTION

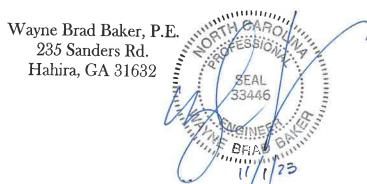
SCALE: NONE PAGE 2.2

MARK	Qty Top	Bot	Int	TYPE	DIA	Length
SP-1 SP-2 SP-3	4 4 4	4 4 4	0	A325 A325 A325	7/8" 3/4" 5/8"	2 1/2" 2" 2"
BASE P	LATE T	ABLE				1
COL		PLAT	F SI2	ZE		1
MARK	Widt			Length	1	
MARK BP−1 ✓FLANC FB×x	10" SE BR/ A(1):	CES:	HICK /4"	Length) Two Si
MARK BP−1 ✓FLANC FB×x	10"	CES:	HICK /4"	Length) Two Si
MARK BP−1 ✓FLANC FB×x	10" SE BR/ A(1):	CES:	HICK /4"	Length) Two Si
MARK BP−1 ✓FLANC FB×x	10" SE BR/ A(1):	CES:	HICK /4"	Length) Two Si

MADIC	W/-*-E4	Web Depth	Web	PLATE	Outside Flange	Inside Flange
MARK	Weight	Start/End	THICK	Length	W x Thk x Length	W x Thk x Length
RF3-1	1715	12.0/33.9	0.313	10'-9 3/16"	8 x 3/8" x 20'-0"	8 x 3/8" x 19'-5 3/16"
		33.9/55.0	0.313	14'-11"	8 x 3/8" x 5'-3 9/16"	B x 3/8" x 2'-0"
					8 x 5/16" x 5'-3 9/16"	1
RF3-2	1170	50.0/29.2	0.250	13'-0 1/16"	8 x 5/16" x 15'-1 7/8"	8 x 5/16" x 15'-1 11/16"
	1	29.2/26.0	0.250	2'-0"	8 x 1/4" x 9'-6"	8 x 5/16" x 10'-0 1/8"
		26.0/20.0	0.188	10"-0"		
RF3-3	851	20.0/20.0	0.188	10'-0 1/4"	6 x 1/4" x 20'-0"	6 x 1/4" x 10'-0 3/16"
	1	20.0/29.1	0.188	14"-11"	6 x 1/4" x 10'-0 1/4"	6 x 1/4" x 19'-9 5/8"
		20 1 / 72 0	A 100	E' 4"	'	



RIGID FRAME ELEVATION: FRAME LINE 3



ISSUE	DET	CHK	DATE

STEELCOR BUILDINGS	
ERWIN HANGAR	
OB NO:	DATE
8245	10/26/23
ERWIN, NC 28339	

RIGID FRAME CROSS SECTION
RIGHD FRAME CROSS SECTION
RAWING NO:
PAGE 2.3 DAR

SPLICE BOLT TABLE MEMBER TABLE Outside Flange
W x Thk x Length
8 x 3/8" x 20"-0"
8 x 3/8" x 5'-3 11/16"
8 x 5/16" x 5'-3 9/16"
8 x 5/16" x 15'-1 7/8"
8 x 1/4" x 9'-6"
 Web Depth
 Web PLATE

 Start/End
 THICK
 Length

 12.0/33.9
 0.313
 10'-9 5/16

 33.9/55.0
 0.313
 14'-11"
 Inside Flange W x Thk x Length 8 x 3/8" x 19'-5 5/16" 8 x 3/8" x 2'-0" Qty Top Bot Int TYPE DIA MARK Weight MARK RF4-1 1678 2 A325 0 A325 2 A325 13'-0 1/16" 2'-0" 10'-0" 10'-0 1/4" 14'-11" 50.0/29.2 29.2/26.0 26.0/20.0 20.0/20.0 20.0/29.1 29.1/32.0 0.250 0.250 0.188 0.188 0.188 8 x 5/16" x 15'-1 11/16" 8 x 5/16" x 10'-0 1/8" RF4-2 BASE PLATE TABLE COL PLATE SIZE
MARK Width THICK Length
BP-1 10" 5/8" 1'-1" 6 x 1/4" x 20'-0" 6 x 1/4" x 10'-0 1/4" 6 x 1/4" x 10'-0 3/16" 6 x 1/4" x 19'-9 5/8" RF4-3 FLANGE BRACES: (1) One Side; (2) Two Sides FBxxA(1): xx=length(in) A - L2x2x14 D/F CAP6 8 11/16" -8 11/16" 26 GA. PBR, GALVALUME (1) FB415A(1) FB330A(1) 12 @ 4'-11 3/16" 12 @ 4'-11 3/16" 121" FB345A(1) FB345A(1) FB505A(2) FB435A(1) Strut FB435A(1) FB505A(2) Strut Strut 21'-6 9/16" CLEARANCE 4'-7 3/4" 120'-0" OUT-TO-OUT OF STEEL RIGID FRAME ELEVATION: FRAME LINE 4 Wayne Brad Baker, P.E. 235 Sanders Rd. DET CHK DATE ISSUE Hahira, GA 31632 STEELCOR BUILDINGS

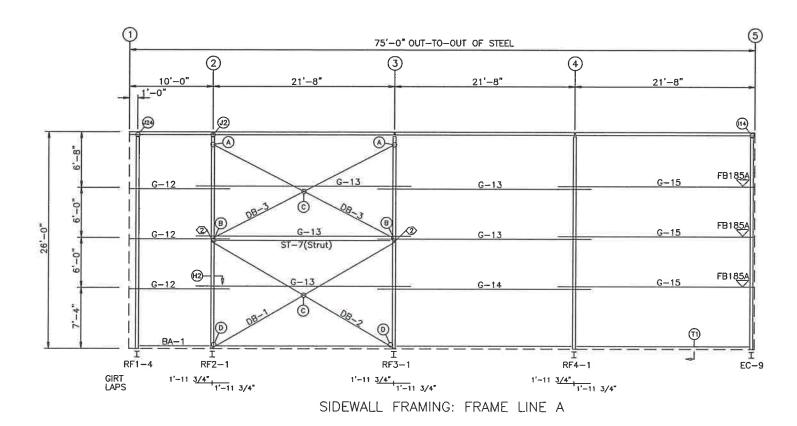
ERWIN HANGAR

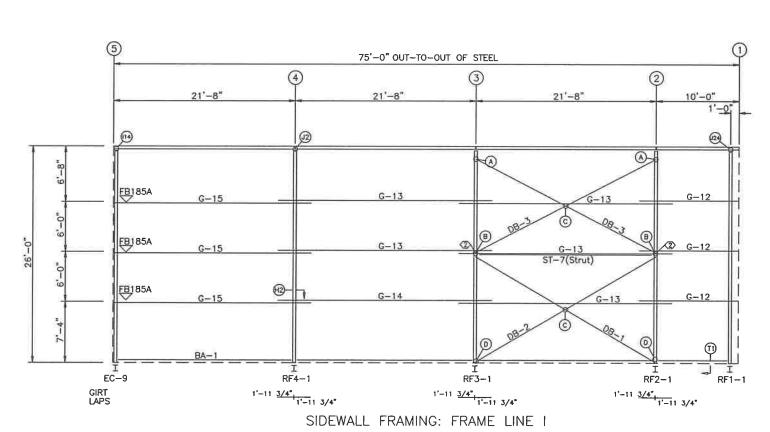
ERWIN, NC 28339

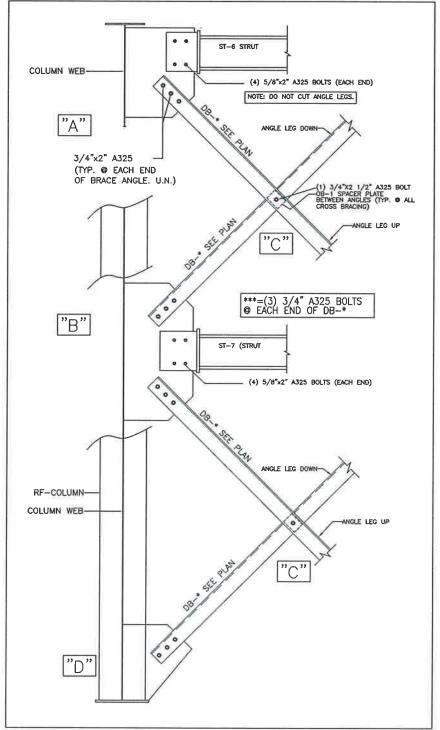
RIGID FRAME CROSS SECTION

8245

10/26/23

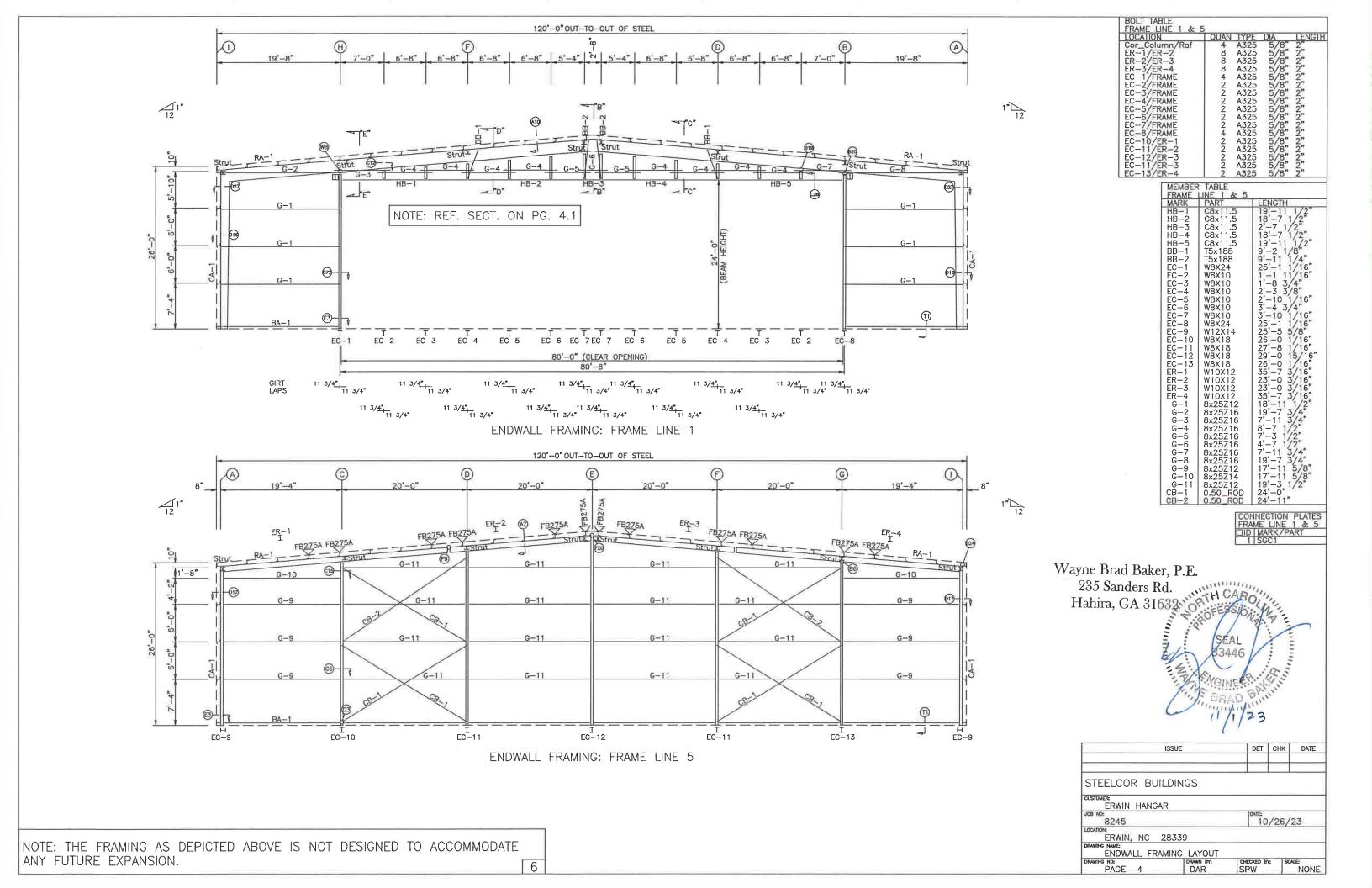


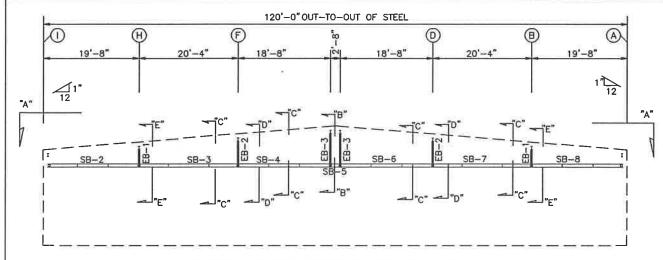




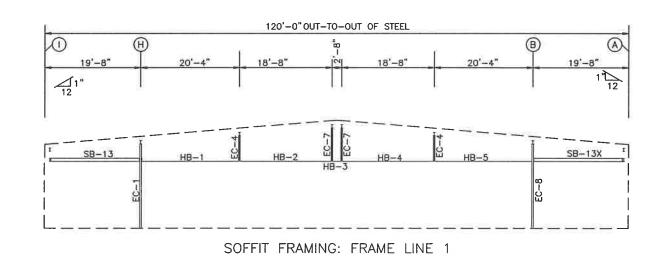


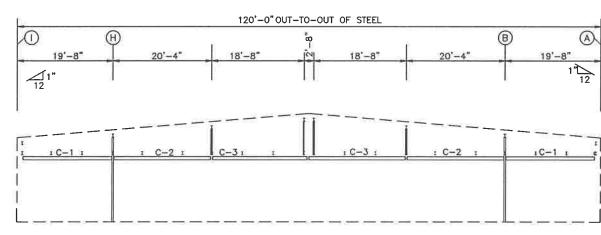
	ISSUE	DET	СНК	DATE
		100	Othic	D/112
			-	
STEELCOR BUIL	DINGS			
STEELCON DOI	LDINGS			
CUSTOMER				
ERWIN HANGA	R			
JOB NO:		DATE:		
			100 1	~ ~
8245		1 10	//0//	2.5
		10	/26/	23
LOCATION:	28339	10	/26/	23
8245 LOCATION: ERWIN, NC 2	28339	10	/26/	23
ERWIN, NC 2	2	10	/20/.	23
ERWIN, NC 2	2	CHECKED BY		





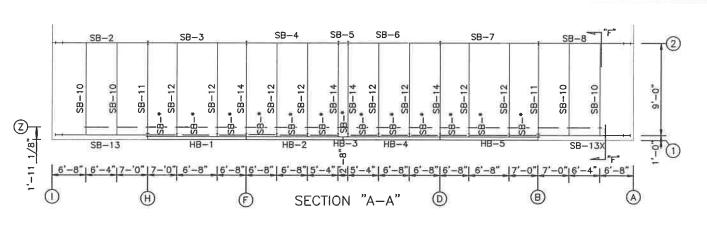
HANGAR DOOR FRAMING: FRAME LINE 2





SOFFIT FRAMING: FRAME LINE Z

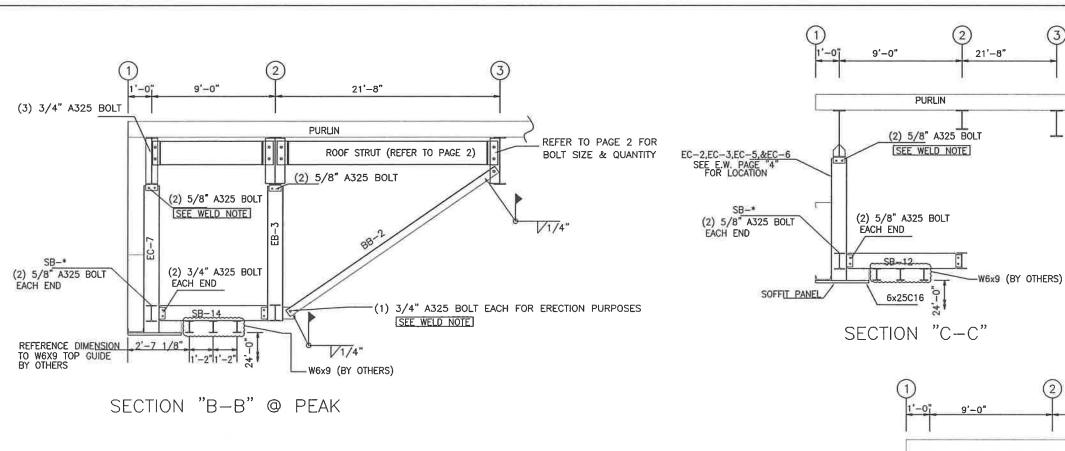
Ì	MEMBER	TABLE LINE 1.1	
ı	MARK	PART	LENGTH
	SB-2	W8X10	18'-7 13/16"
	SB-3	W8X10	20'-2 13/16"
	SB-4	W8X10	18'-10 13/16"
	SB-5	W8X10	1'-10 13/16"
	SB-6	W8X10	18'-10 13/16"
ı	SB-7	W8X10	20'-2 13/16"
	SB-8	W8X10	18'-7 13/16"
	SB-9	W8X10	18'-11"
	SB-10	W8X10	10'-0"
Ñ	SB-11	W8X10	10'-0"
l	SB-12	W8X10	10'-0"
	SB-13 SB-13X	W8X10 W8X10	17'-10 1/8" 17'-10 1/8"
	SB-13A	W8X10 W8X10	10'-0"
	SB-*	W8X10	TBD
ı	EB-1	W8X10	TBD
1	EB-2	W8X10	TBD
1	EB-3	W8X10	l TBD
1	C-1	6x25C16	18'-7 1/2"
1	C-2	6x25C16	20'-3 1/2"
	C-3	6x25C16	19'-11 1/2"

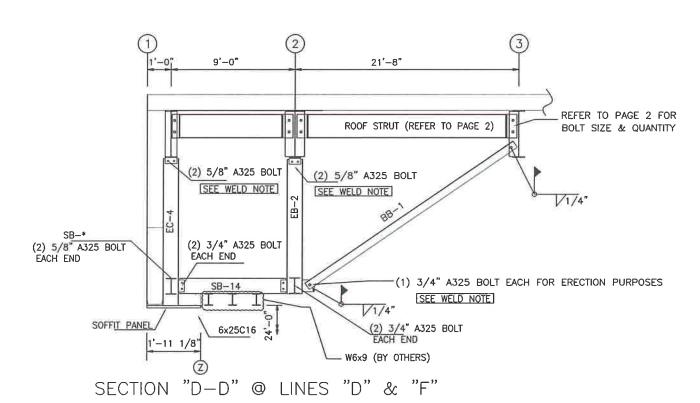


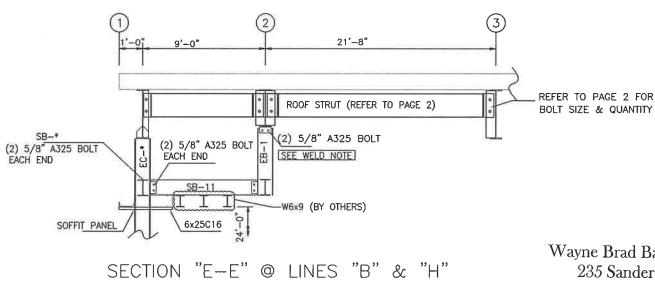
NOTE: REF SECT. DWGS @ PAGE 4.2



IS	SSUE	DET	СНК	DATE	
STEELCOR BUILI	DINGS		:		
CUSTOMER: ERWIN HANGAR					
JOS NO: 8245		DATE: 10/26/23			
ERWIN, NC 28					
HANGAR DOOR	FRAMING TORAIN BY:	CHECKED BY	r: Sca	i F·	
PAGE 4.1	DAR	SPW	500	NON	







Wayne Brad Baker, P.E.

235 Sanders Rd.
Hahira, GA 31632

H CARO

SEAL

33446

BRAD

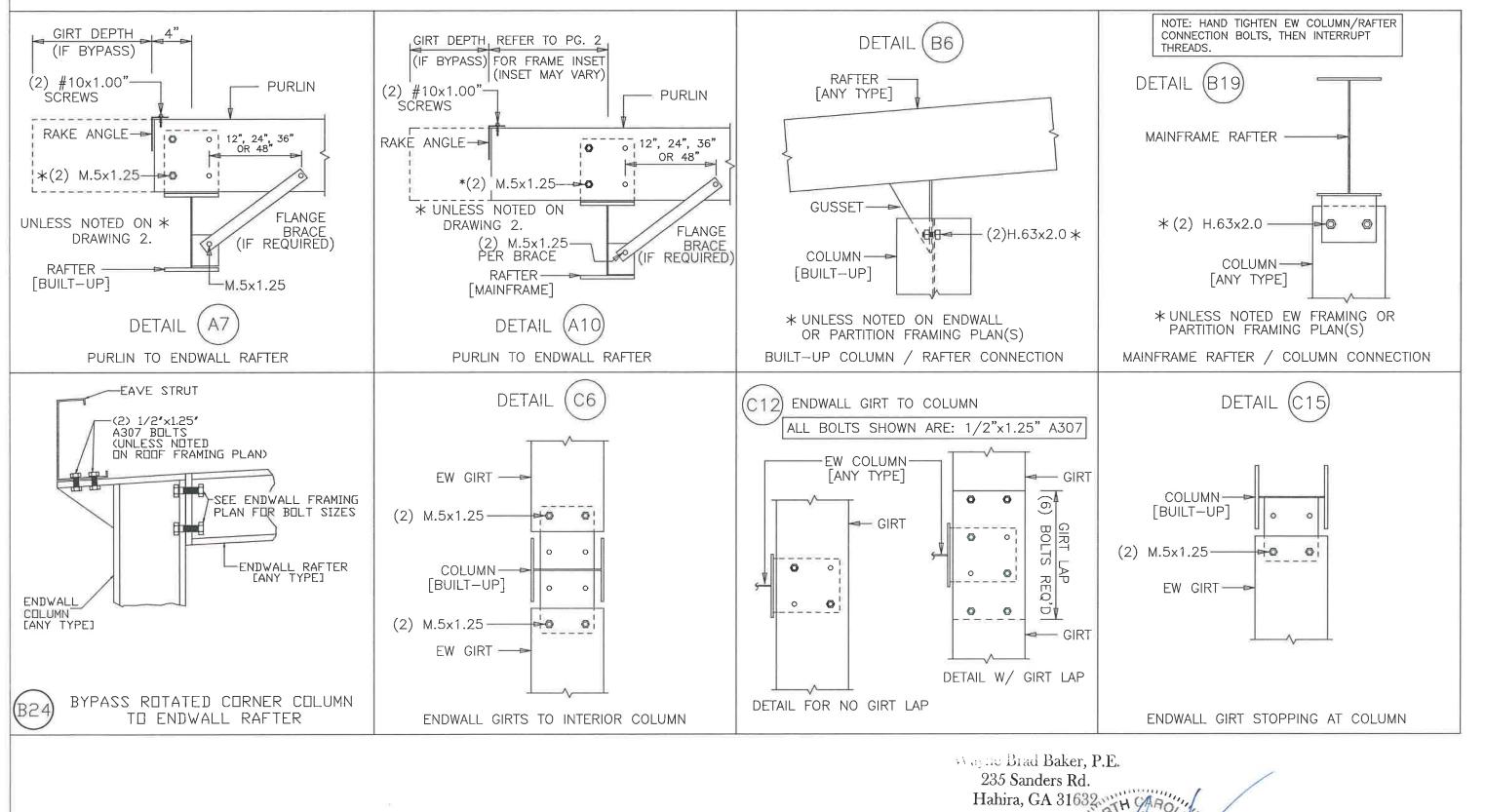
WGINER

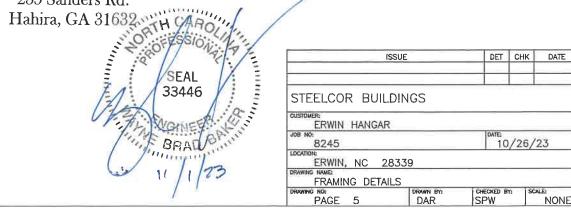
BRAD

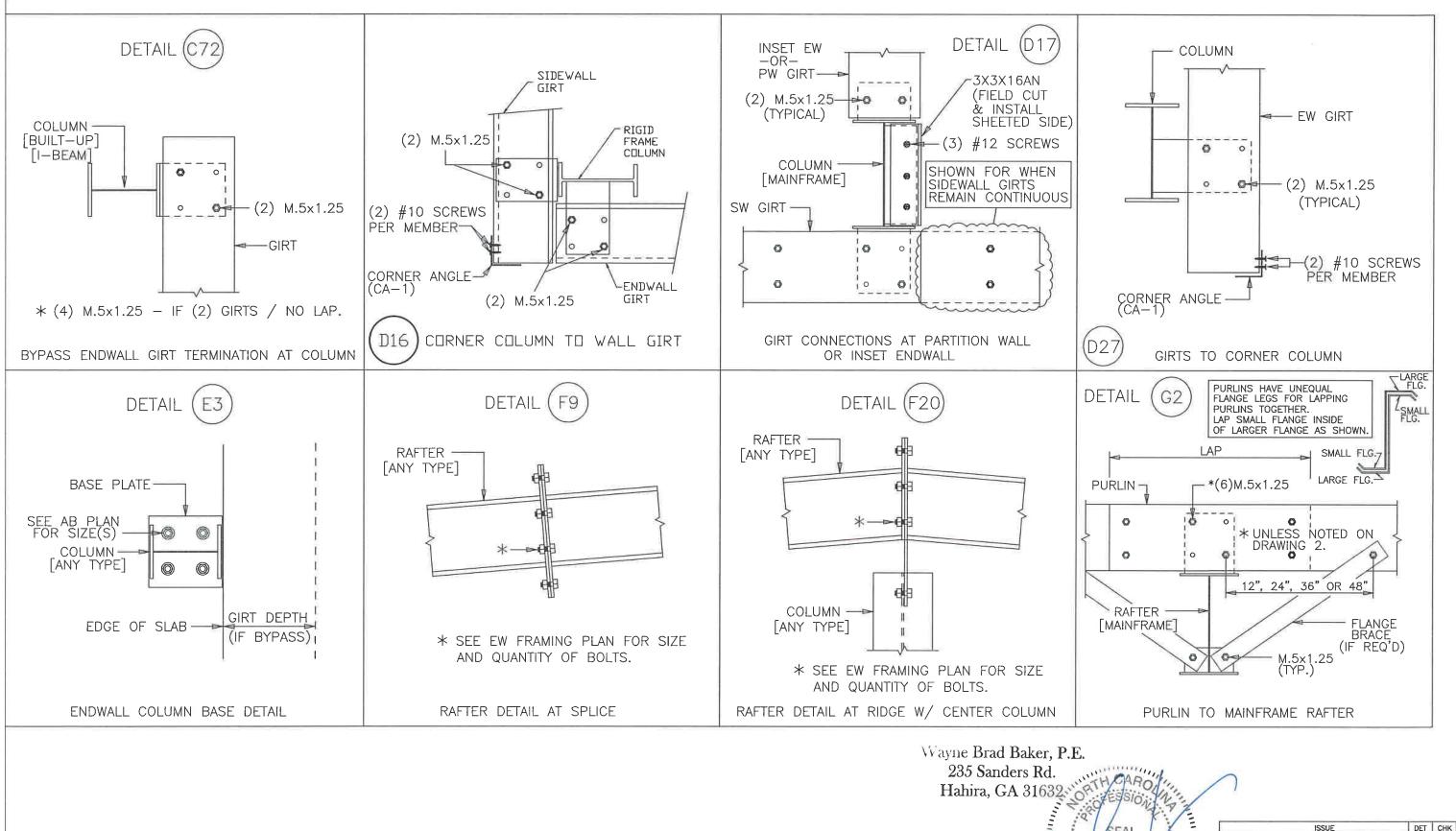
11/1/23

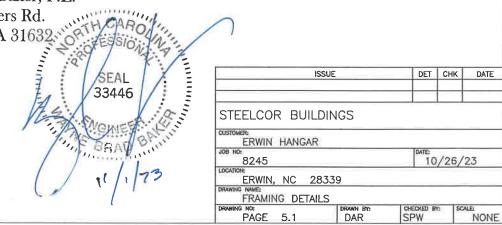
ISSUE		DET	CHK	DATE
STEELCOR BUILD	INGS			
ERWIN HANGAR				
108 NO: 8245	DATE: 10/26/23			23
ERWIN, NC 28	339			
HANGER DOOR I	DETAILS			
PAGE 4.2	DRAWN BY: DAR	SPW BY	: 50	NONE

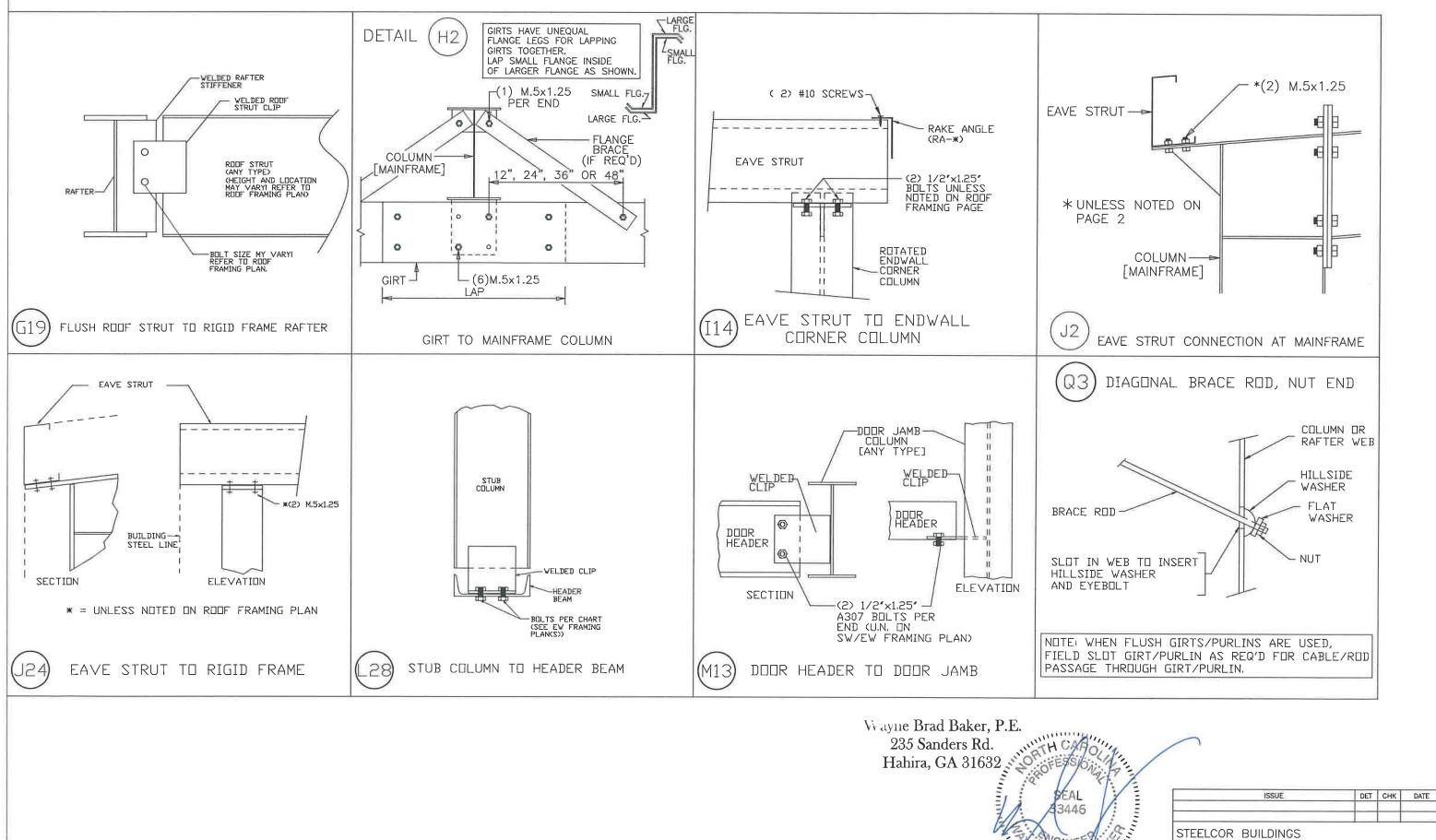
WELD NOTE: FIELD WELD ONCE ALL DEAD LOAD IS APPLIED











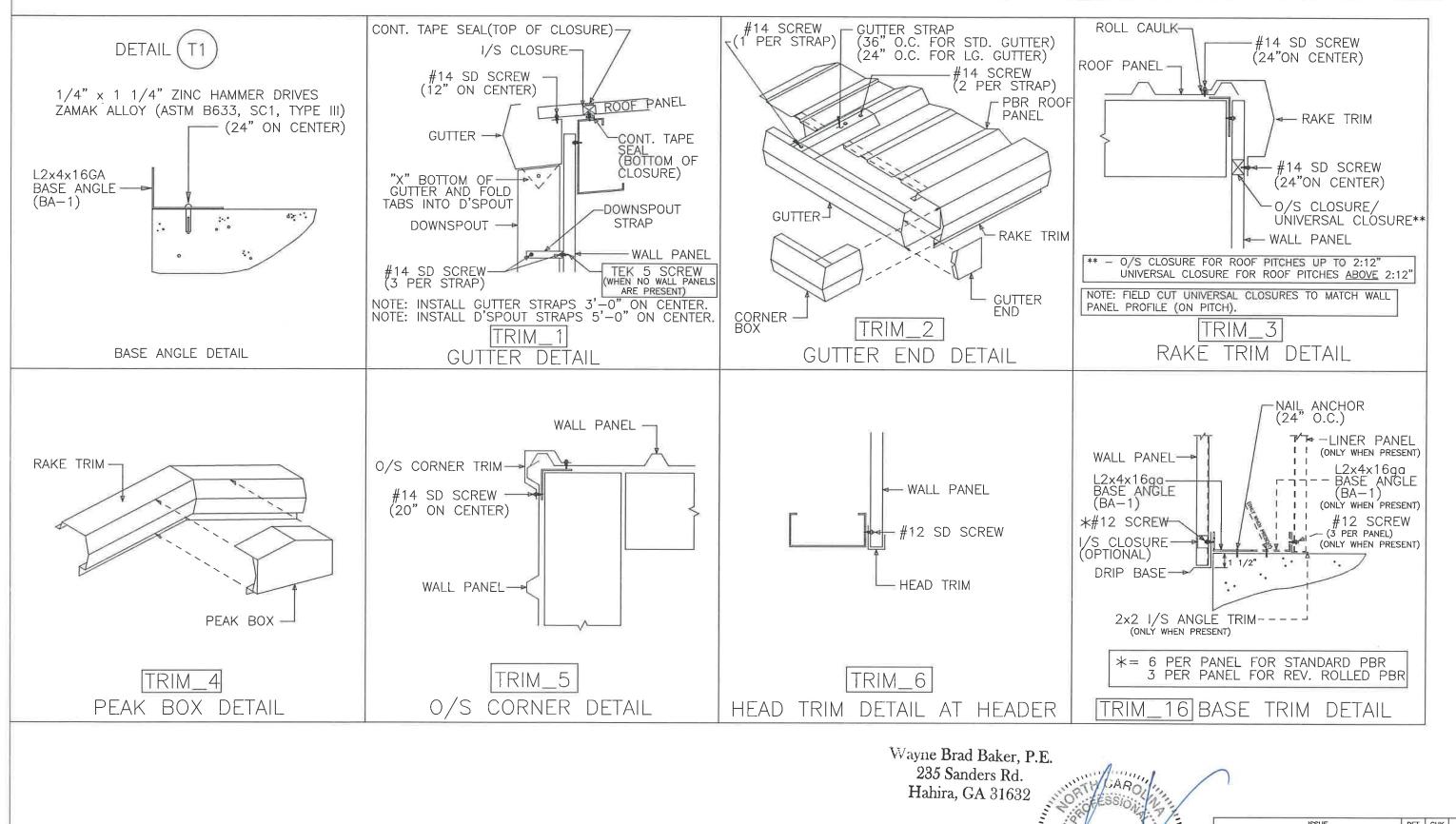
STEELCOR BUILDINGS

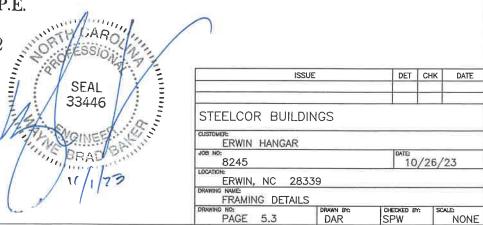
CUSTOMER:
ERWIN HANGAR

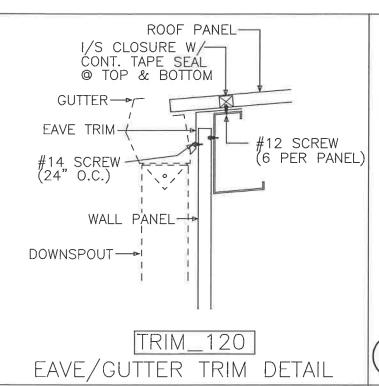
JOB-NO:
82.45
LOCATION:
ERWIN, NC 28339
DEFAMING NAME:
FRAMING DETAILS
DRAWING NO:
PAGE 5.2
DAR

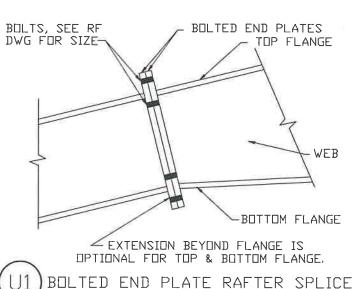
DET CHR DATE
10/26/23

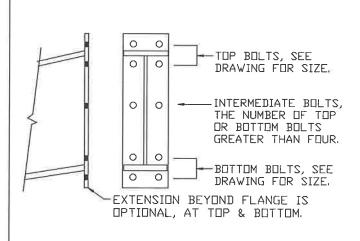
LOCATION:
CHECKED BT:
CHECKED BT:
SCALE:
NONE



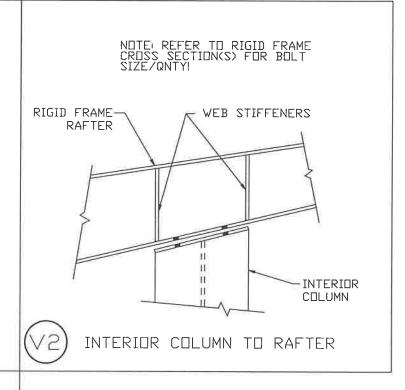


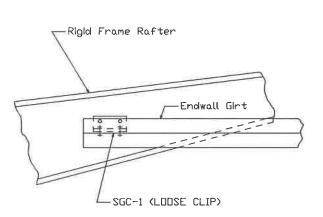






BOLTED END PLATE CONNECTION





ATTACH WITH
CLIP TO RAFTER (2) 1/2'x1.25' A307 BOLT/NUT
CLIP TO GIRT (2) 1/2'x1.25' A307 BOLT/NUT

SECTION OF ENDWALL GIRT TO RAFTER

STRUCTURAL BOLTED CONNNECTIONS

REFER TO COVER PAGE "GENERAL NOTES" PARAGRAPH "C", SECTION "9" FOR INSTRUCTIONS ON TIGHTENING ALL A325 AND A490 CONNECTION BOLTS.

TRIM NOTES:

- [1] SEAL TRIM SPLICES WITH TUBE CAULK.
- [2] SECURE GUTTER SPLICES AND END PLUGS WITH RIVETS.
- [3] SECURE ALL OTHER ROOF TRIM SPLICES WITH TRIM SCREWS UNLESS NOTED OTHERWISE.
- [4] TRIM SCREWS ARE LOCATED 24" ON CENTER UNLESS NOTED OTHERWISE.
- [5] STD. TRIM SPLICES ARE 3" TOTAL UNLESS NOTED OTHERWISE.

MORTISE PREPPED PERSONNEL DOORS

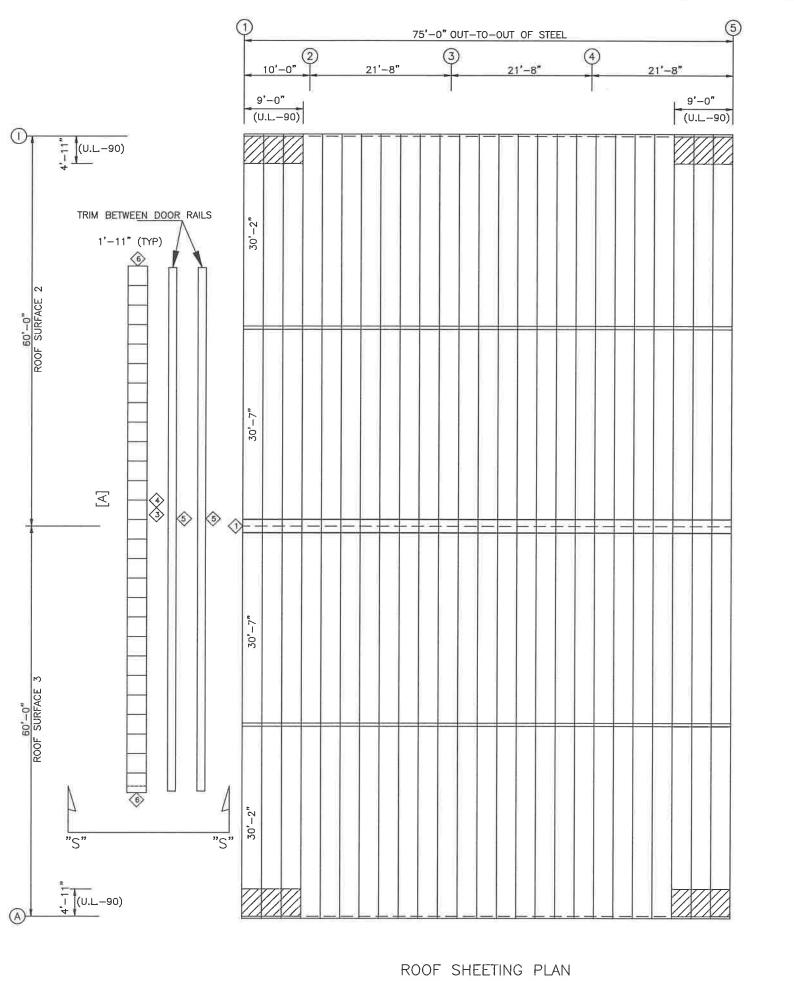
ALL MORTISE PREPPED PERSONNEL DOORS COME AS RIGHTHAND REVERSED SWING.

(i.e. STANDING ON THE OUTSIDE OF THE BUILDING FACING THE DOOR, THE LOCK WILL BE ON THE LEFTHAND SIDE OF THE DOOR AND THE DOOR WILL SWING OUTWARD FROM THE BUILDING.)

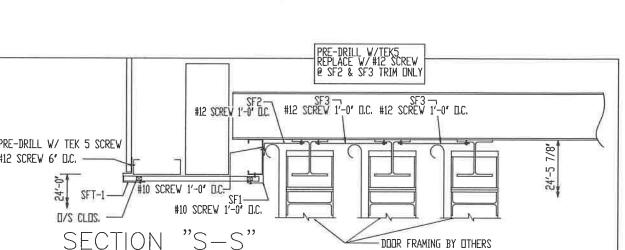
ANY FIELD MODIFICATIONS ARE THE RE— SPONSIBILITY OF THE ERECTOR AND MBM IS NOT LIABLE FOR LABOR CHARGES NOR DAMAGES DUE TO ERROR.

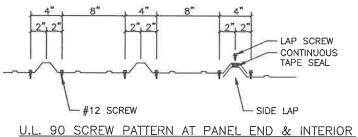


ļ:	ISSUE		СНК	DATE	
STEELCOR BUIL	DINGS				
CUSTOMES: ERWIN HANGAR					
JOB NO: BATE: 10/2			/26/	23	
ERWIN, NC 2	B339				
FRAMING DETAI	LS				
PAGE 5.4	DRAWN BT:	SPW	SC	NONE	

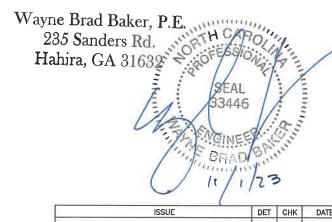


PRE-DRILL W/ TEK 5 SCREW #12 SCREW 6" D.C. D/S CLDS. SECTION "S-S" #12 SCREW





= U.L. 90 SCREW PATTERN @ HATCHED IN AREAS WITHIN DIMENSIONS SHOWN ON ROOF SHEETING PLAN.



TRIM TABLE ROOF PLAN ◇ID PART

1 D/F CAP6 3 SF1 4 SF2 5 SF3 6 R JAMB

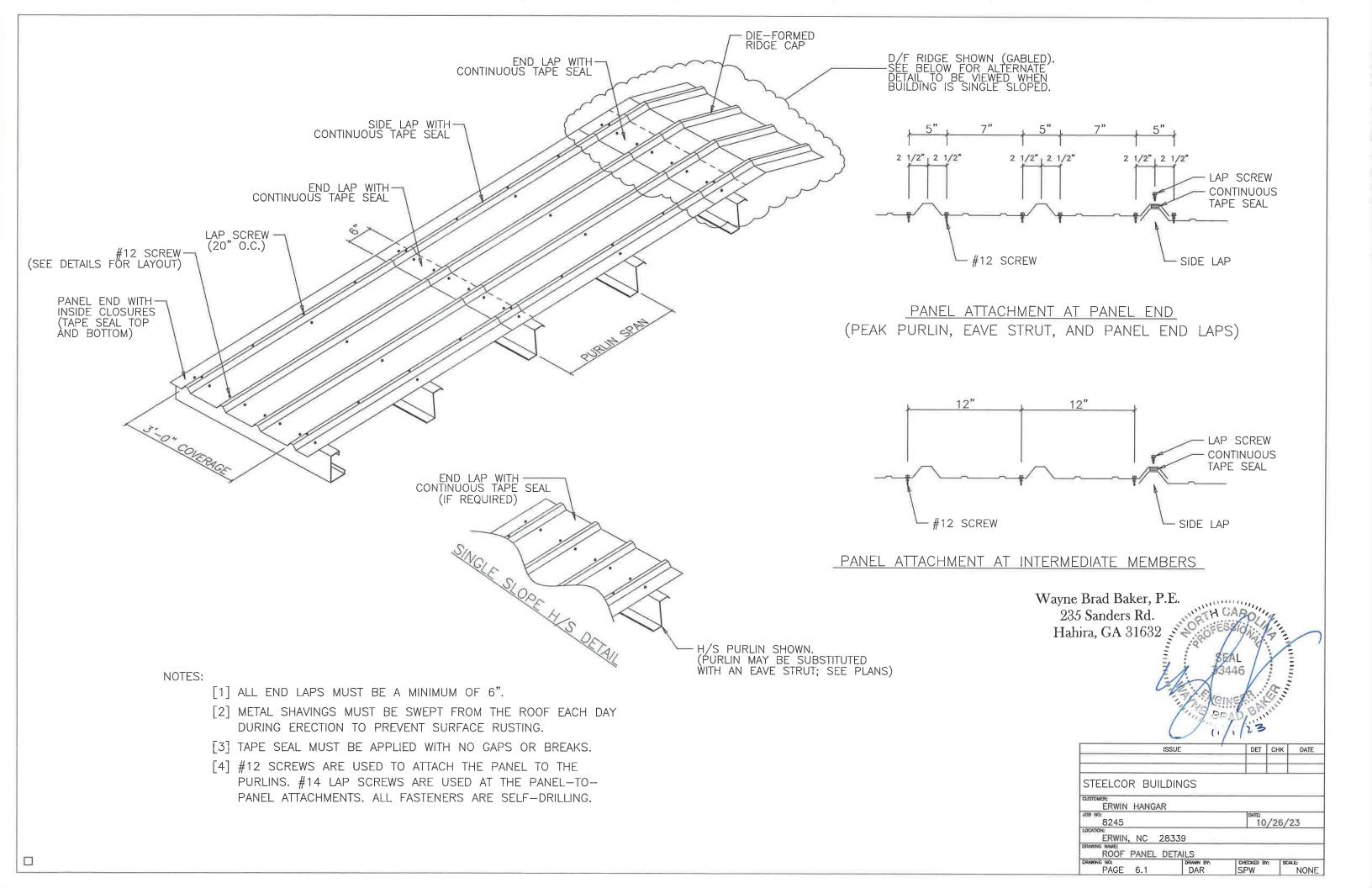
ISSUE DET CHK DATE

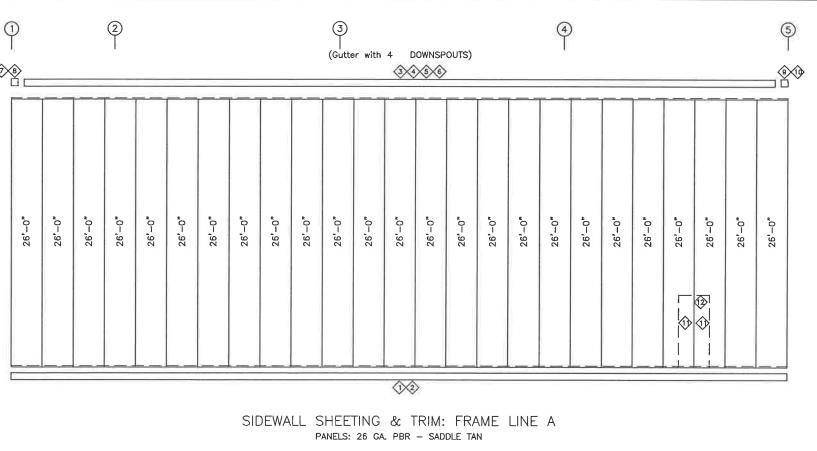
STEELCOR BUILDINGS

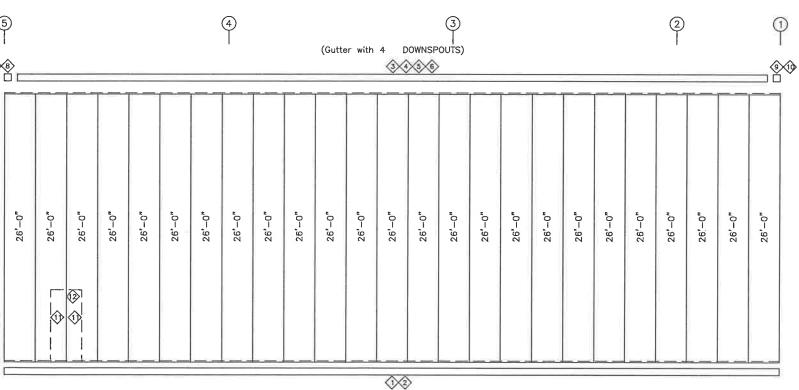
CUSTOMER:
ERWIN HANGAR

JOB NO:
8245
LOCATION:
ERWIN, NC 28339
DEAMING NAME:
ROOF PANELS & TRIM
DRAWING NO:
PAGE 6 DAR SPW NONE

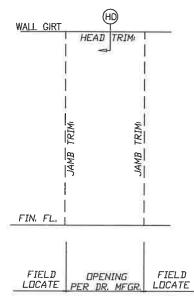
ROOF SHEETING PLAN
PANELS: 26 GA. PBR - GALVALUME
[A] SOFFIT PANELS: 26 GA. PBR - NEED COLOR





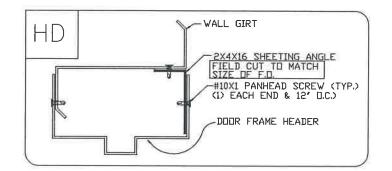


SIDEWALL SHEETING & TRIM: FRAME LINE I
PANELS: 26 GA. PBR - SADDLE TAN



3070 TRIM KIT

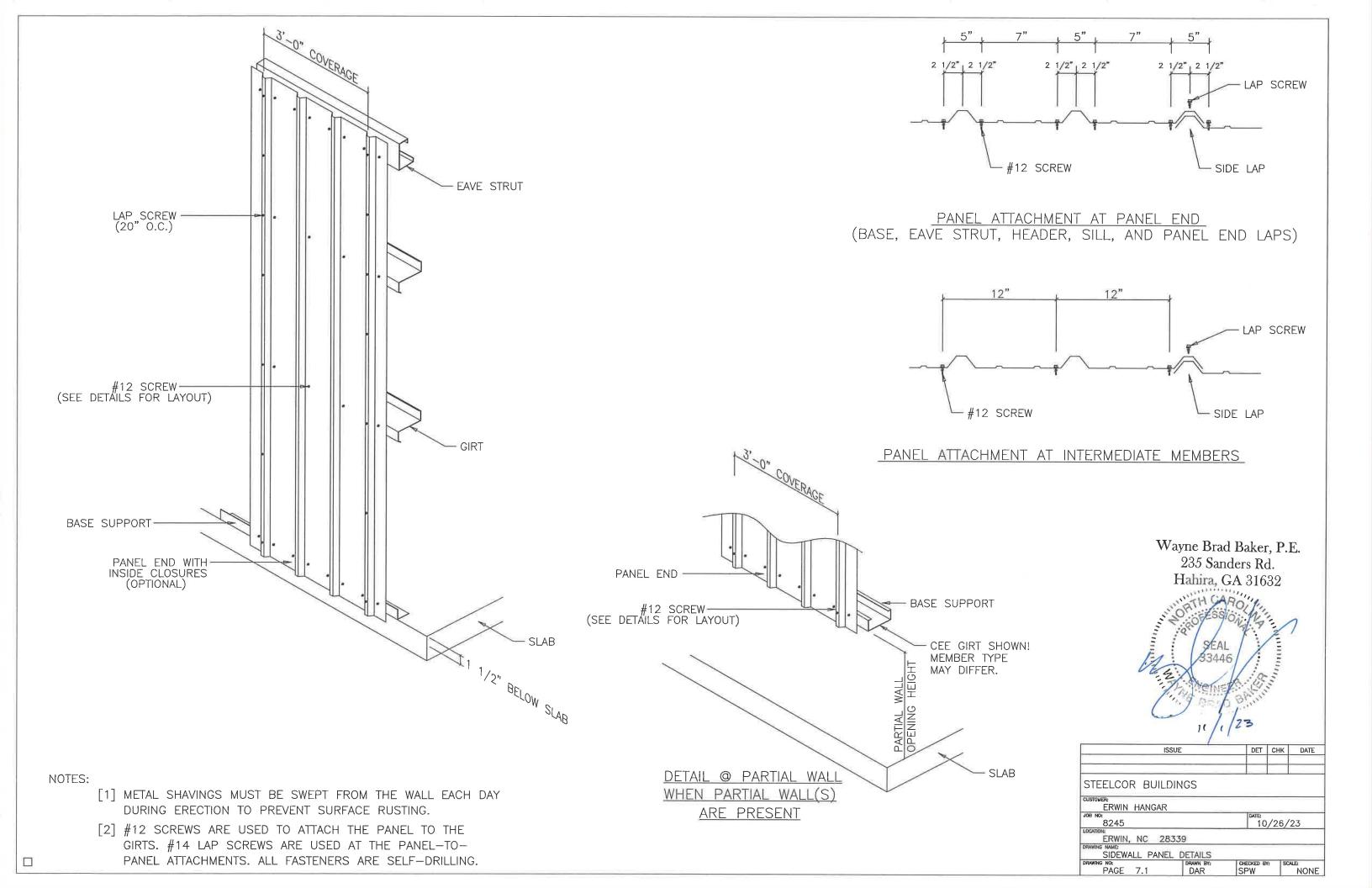
- 1. FIELD CUT AND WORK GIRTS, PANELS, AND TRIM AS REQUIRED.
- 2. REFER TO DETAIL PAGES FOR APPLICABLE TRIM DETAILS.(DETAIL PAGE 5.1)

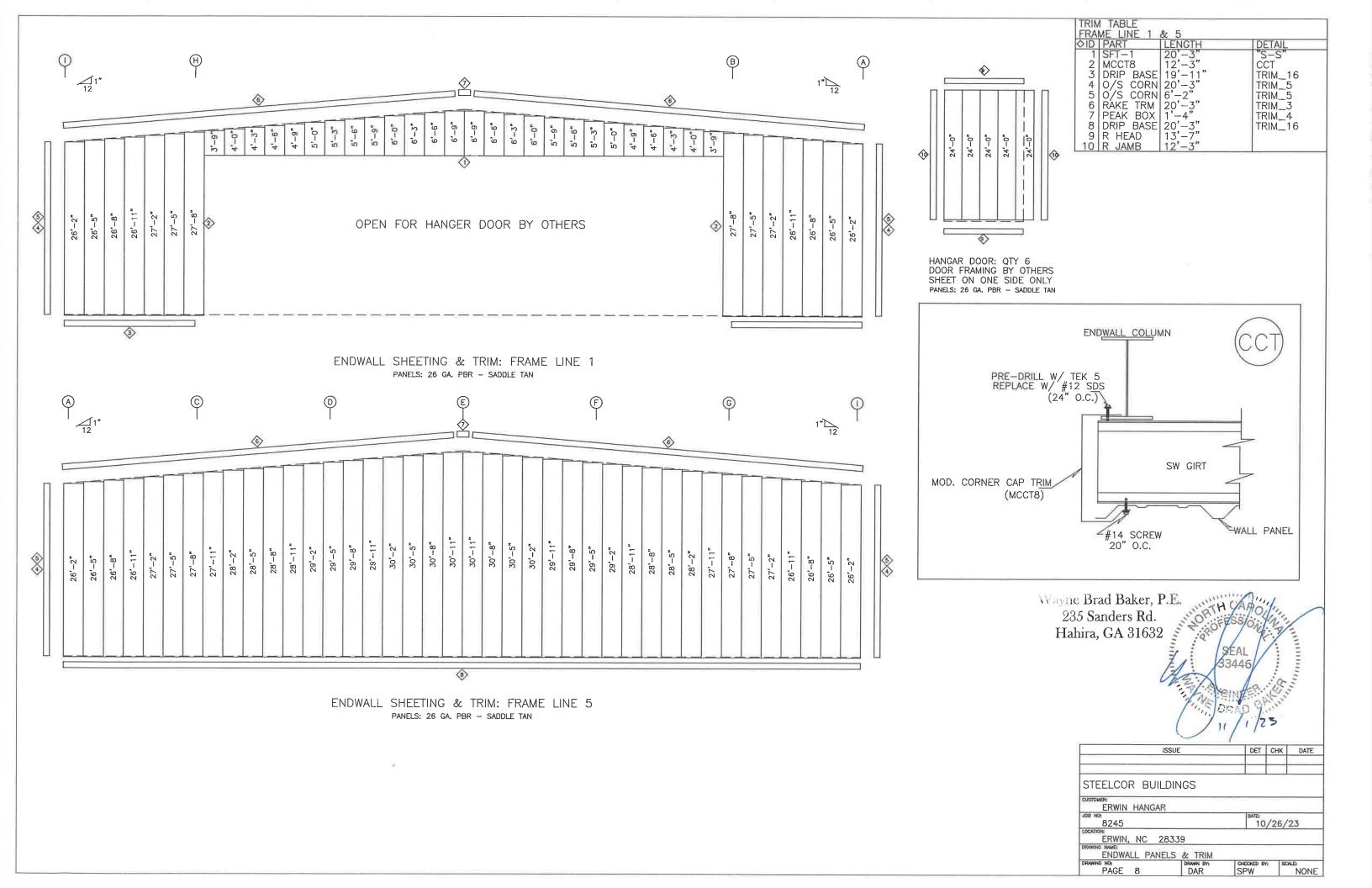


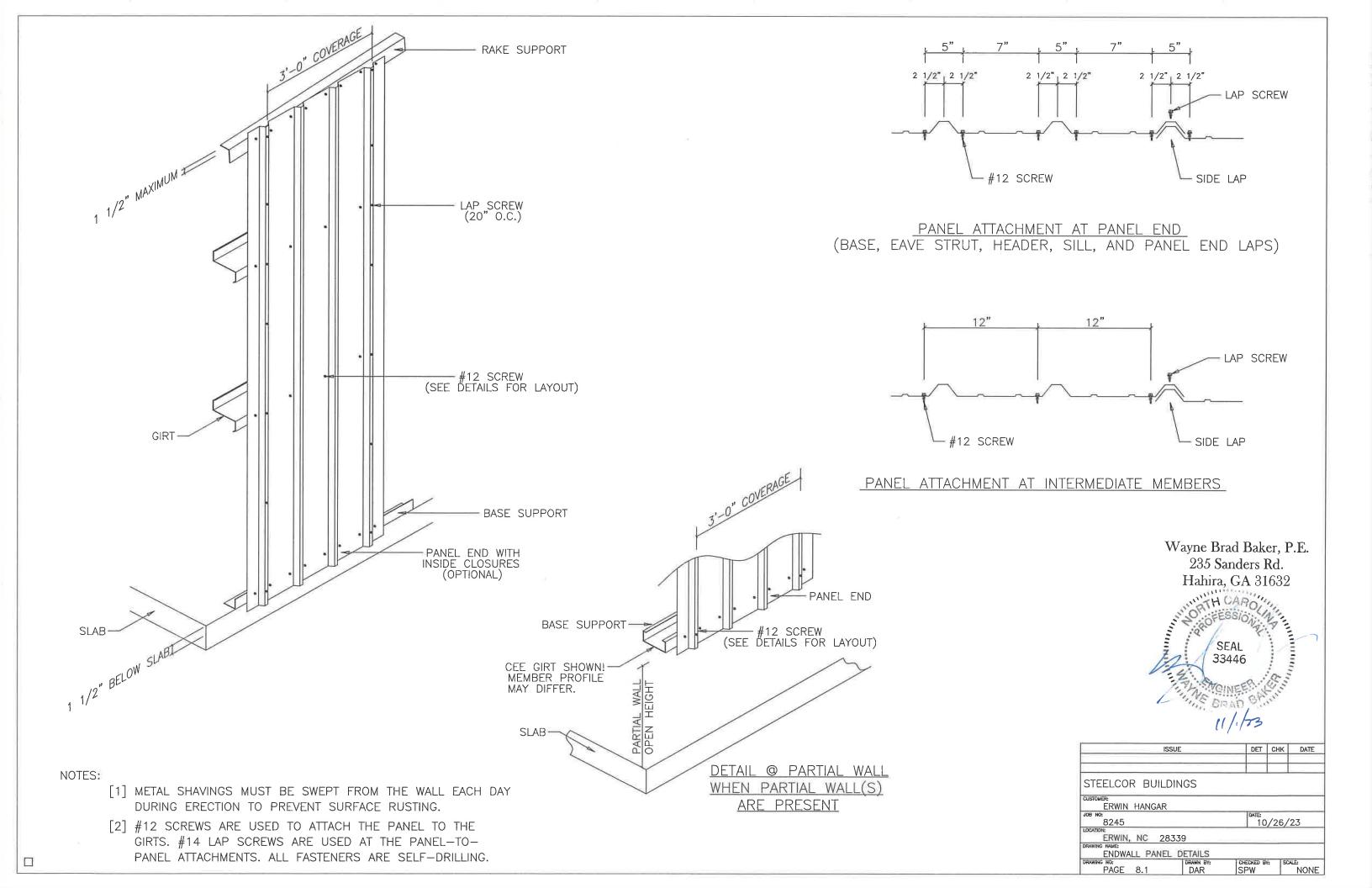
TRIM TABLE	FRAME LINE A &			
○ ID	PART	LENGTH	DETAIL	
1	DRIP BASE	20'-3"	TRIM_16	
2	DRIP BASE	15'-3"	TRIM_16	
3	GUTTER	20'-3"	TRIM_1	
4	GUTTER	15'-0"	TRIM_1	
5	EAVE TRM	20'-3"	TRIM_120	
6	EAVE TRM	15'-0"	TRIM_120	
7	GUTEND	1"	TRIM_2	
8	CORBOX	1'-0"	TRIM_2	
9	GUTEND	1"	TRIM_2	
10	CORBOX	1'-0"	TRIM_2	
11	R	JAMB	7'-3"	TRIM_8
12	R	HEAD	3'-3"	TRIM_61

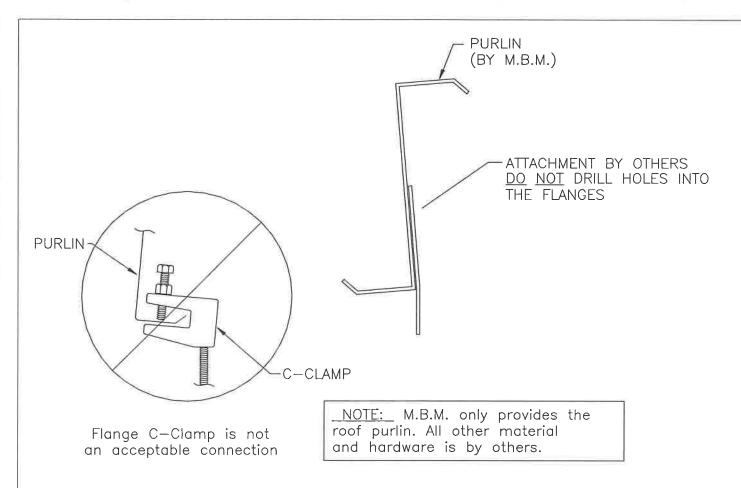


	ISSUE	DET	DET CHK			
STEELCOR BUIL	DINGS					
ERWIN HANGAF	?					
лов но: 8245		10/26/23				
ERWIN, NC 2	8339					
DRAWING NAME: SIDEWALL PAN	ELS & TRIM					
DRAWING NO: PAGE 7	DRAWN BY: DAR	SPW BY	: SCA	NONF		









Recommended Connection Detail

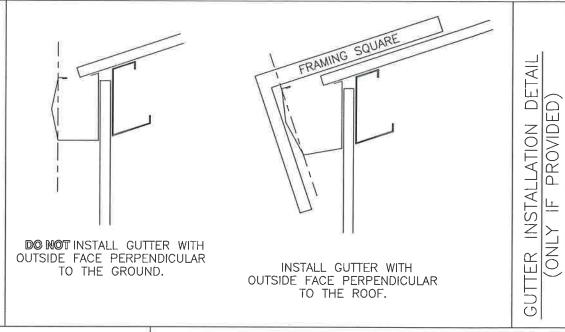
NOTE

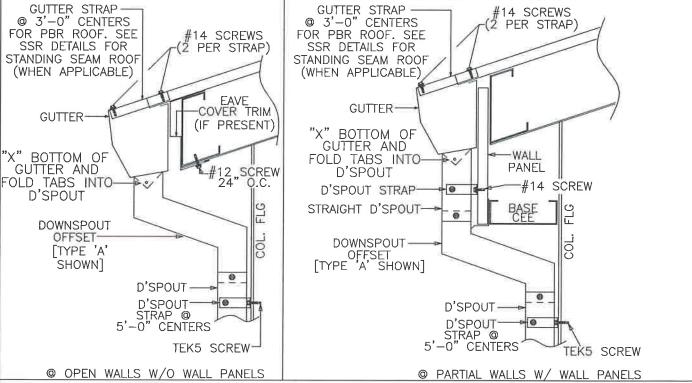
MANY FACTORS BEYOND THE CONTROL OF THE METAL BUILDING SUPPLIER AFFECT THE ABILITY OF A PURLIN TO SAFELY SUPPORT HANGING LOADS COMBINED WITH OTHER REQUIRED ROOF LOADS. DUE TO THE VARIABLES INVOLVED IN HANGING LOADS AND THEIR ATTACHMENTS TO THE PURLINS, THE METAL BUILDING SUPPLIER CANNOT ASSURE THAT THE PURLINS FOR A PARTICULAR BUILDING PROJECT CAN SAFELY SUPPORT THE MAXIMUM ALLOWABLE HANGING LOADS IN COMBINATION WITH OTHER ROOF LOADS.

IT IS THE RESPONSIBILITY OF THE HANGER SYSTEM INSTALLER TO COORDINATE WITH THE ENGINEER OF RECORD FOR THE OVERALL PROJECT TO ENSURE A SAFE HANGING LOAD INSTALLATION. THE METAL BUILDING ENGINEER IS NOT THE ENGINEER OF RECORD FOR THE OVERALL PROJECT. WITHOUT SPECIFIC CERTIFICATION FOR INDIVIDUAL HANGING LOADS, THE NET EFFECTS OF APPLIED HANGER LOADS INSTALLED ON A PARTICULAR PURLIN SHALL NOT EXCEED THE NET EFFECTS OF THE CERTIFIED UNIFORMLY APPLIED DESIGN COLLATERAL LOAD.

HANGING LOADS SHOULD NOT BE APPLIED TO THE PURLIN LIP. WHERE PERMISSIBLE, THE BEST PRACTICE FOR HANGING LOADS IS TO ATTACH TO THE PURLIN WEB USING A BOLT AND NUT, OR SELF-DRILLING SCREWS.

HANGING UNIFORM LOADS SUCH AS SPRINKLER MAINS OR HVAC EQUIPMENT SHOULD BE DISTRIBUTED OVER SEVERAL PURLINS, AND SHOULD NEVER EXCEED THE COLLATERAL LOAD ALLOWANCE FOR THE ROOF SYSTEM. FOR UNIFORM LOADS THAT RUN PARALLEL TO THE PURLINS, IT MAY BE NECESSARY TO USE TRANSVERSE SUPPORT CHANNELS(A.KA. TRAPEZE BEAMS) ATTACHED TO THE WEBS OR FLANGES OF ADJACENT PURLINS TO SPREAD THE LOAD BETWEEN TWO OR MORE PURLINS. IN SUCH CASES, CONTACT THE BUILDING MANUFACTURER OR A LOCAL PROFESSIONAL ENGINEER PRIOR TO ATTEMPTING TO HANG LOADS FROM THE PURLINS





NOTE: REGARDLESS OF DOWNSPOUT OFFSET SCENARIO, TEK5 SCREWS MUST BE USED TO ATTACH DOWNSPOUT STRAPS TO PEMB FRAMING. WHEN WALL PANELS SPAN FROM GROUND TO EAVE (FULL SPAN), #14 SCREWS WILL BE USED TO ATTACH DOWNSPOUT STRAPS TO WALL PANELS.

