BUILDING PROFILE

Width (ft) = 120Eave Height (ft) = 26 Length (ft) = 75Roof Slope (Rise/12) = 1.0:12

BUILDING LOADS

A) THIS IS TO CERTIFY THAT THIS STRUCTURE IS DESIGNED UTILIZING THE LOADS INDICATED AND APPLIED AS REQUIRED BY NCBC 18 / IBC 15

B) THIS CERTIFICATION IS LIMITED TO THE STRUCTURAL DESIGN OF THE FRAMING AND COVERING PARTS MANUFACTURED BY THE BUILDING MANUFACTURER AND AS SPECIFIED IN THE CONTRACT. ACCESSORY ITEMS SUCH AS DOORS, WINDOWS, LOUVERS, TRANSLUCENT PANELS, VENTILATORS ARE NOT INCLUDED. ALSO EXCLUDED ARE OTHER PARTS OF THE PROJECT NOT PROVIDED BY THE BUILDING MANUFACTURER SUCH AS FOUNDATIONS, MASONRY WALLS, MECHANICAL EQUIPMENT AND THE ERECTION AND INSPECTION OF THE BUILDING. THE BUILDING SHOULD BE ERECTED ON A PROPERLY DESIGNED FOUNDATION IN ACCORDANCE WITH THE BUILDING MANUFACTURER'S DESIGN MANUAL, THE ATTACHED DRAWINGS, AND GOOD ERECTION PRACTICES. THE END USER AND/OR ENGINEER OF RECORD IS TO CONFIRM THAT THESE LOADS COMPLY WITH REQUIREMENTS OF THE LOCAL BUILDING DEPT.

OCCUPANCY/RISK CATEGORY	<u>II — Normal Is 1.0000 le 1.00</u>
WIND LOAD ULTIMATE	120 MPH NOMINAL 92.95 MPH WIND EXPOSURE C
CLOSURE TYPE	Enclosed INTERNAL WIND COEF, -0.18 / 0.18
GROUND SNOW LOAD	10.00 PSF ROOF SNOW LOAD 7 PSF Ce 1.0000 Ct 1.00
SNOW BANKING LOADS	PER CODE
COLLATERAL DEAD LOAD	3 PSF
ROOF LIVE LOAD	20.00 PSF (REDUCIBLE Yes)
DEAD LOAD	2.000 PSF (FOR ROOF PANELS AND PURLINS)
SEISMIC	
SPECTRAL RESPONSE Ss 0.	1860 S1_0.0860 Sds_0.1984 Sd1_0.1376
SITE CLASS D	DESIGN RISK CATEGORY C Cs 0.0662
RESPONSE MODIFICATION FACTOR,	R <u>3.000*</u> FRAMES <u>3.000*</u> BRACING
BASIC SEISMIC FORCE RESISTING	SYSTEM (LATERAL DIRECTIONS) = ORDINARY STEEL MOMENT FRAMES
	SYSTEM (REW) = ORDINARY STEEL CONCENTRICALLY BRACED FRAMES
BASIC SEISMIC FORCE RESISTING	SYSTEM (LEW) = ORDINARY STEEL MOMENT FRAMES
BASIC SEISMIC FORCE RESISTING	SYSTEM (LONGTHORNAL DIRECTIONS) = ORDINARY STEEL CONC. BRACED FRAMES

= EQUIVALENT LATERAL FORCE PROCEDURE

SERVICEABILITY CRITERIA

ANALYSIS PROCEDURE

STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR D)

	MINI	NUM DESI	ON DEFLECTIONS			
Endwall Column	=	120	Roof Panel (Liv	re)	=	60
Endwall Rafter (Live)	=	180	Roof Panel (Wir	nd)	=	60
Endwall Rafter (Wind)	=	180	Rigid Frame (H	lorz)	=	60
Wall Girt	=	90	Rigid Frame (V	ert)	=	180
Roof Purlin (Live)	-	150	Rigid Frame (S	eismic)	=	50
Roof Purlin (Wind)	=	150				
Wall Panel	=	60				

GENERAL NOTES

- A) THE STRUCTURE UNDER THIS CONTRACT HAS BEEN DESIGNED AND DETAILED FOR THE LOADS AND CONDITIONS STIPULATED IN THE CONTRACT AND SHOWN ON THESE DRAWINGS. ANY ALTERATIONS TO THE STRUCTURAL SYSTEM OR REMOVAL OF ANY COMPONENT PARTS, OR THE ADDITION OF OTHER CONSTRUCTION MATERIALS OR LOADS MUST BE DONE UNDER THE ADVICE AND DIRECTION OF A REGISTERED ARCHITECT, CMIL OR STRUCTURAL ENGINEER. THE BUILDING MANUFACTURER WILL ASSUME NO RESPONSIBILITY FOR ANY LOADS NOT INDICATED.
- B) THIS METAL BUILDING IS DESIGNED WITH THE BUILDING MANUFACTURER'S STANDARD PRACTICES WHICH ARE BASED N PERTINENT PROCEDURES AND RECOMMENDATIONS OF THE FOLLOWING ORGANIZATIONS AND CODES.
- AMERICAN INSTITUTE OF STEEL CONSTRUCTION: " AISC SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS—ALLOWABLE STRESS DESIGN"
- 2. AMERICAN IRON AND STEEL INSTITUTE: "SPECIFICATION FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS"
- 3. AMERICAN WELDING SOCIETY: "STRUCTURAL WELDING CODE" AWS D1.1.
- 4. METAL BUILDING MANUFACTURER'S ASSOCIATION: "LOW RISE BUILDING SYSTEMS MANUAL"
- C) 1) MATERIAL PROPERTIES OF STEEL PLATE USED IN THE FABRICATION OF PRIMARY RIGID FRAMES, AND OTHER PRIMARY STRUCTURAL EXCLUSIVE OF COLD—FORMED SECTIONS, CONFORM TO ASTM—A529 OR A572. FLANGES WITH THICKNESS OF ONE INCH OR LESS AND WIDTH OF 12" OR LESS CONFORM TO A529 WITH A MINIMUM YIELD POINT OF 55,000 psi. FLANGES GREATER THAN 1" IN THICKNESS OR 12" IN WIDTH CONFORM TO A572 WITH A MINIMUM YIELD
- POINT OF 50,000 psi. WEB MATERIAL CONFORMS TO ASTM-A529 WITH A MINIMUM YIELD POINT OF 55,000 psi.

 2) MATERIAL PROPERTIES OF PIPE SECTIONS CONFORM TO ASTM-A500, GRADE B WITH A MINIMUM YIELD POINT OF 42,000 ps
- 3) MATERIAL PROPERTIES OF TUBE SECTIONS CONFORM TO ASTM-A500, GRADE B WITH A MINIMUM YIELD POINT OF 46,000 psi
- 4) MATERIAL PROPERTIES OF HOT ROLLED CHANNEL AND ANGLE MEMBERS CONFORM TO THE REQUIREMENTS OF ASTM-A529 WITHMINIMUM YIELD POINT OF 50,000 PSI. HOT ROLLED W-SHAPED MEMBERS CONFORM TO THE REQUIREMENTS OF ASTM-A992WITH MINIMUM YIELD POINT OF 50,000 PSI.
- 5) MATERIAL PROPERTIES OF COLD FORMED LIGHT GAGE STEEL MEMBERS CONFORM TO EITHER ASTM A653-06 GR 55 OR A1011-04 HSLAS GRADE 55 WITH YIELD OF 55,000 psi.
- 6) MATERIAL PROPERTIES OF ROOF/WALL SHEETING, BASE METAL CONFORM TO ASTM-A792 GRADES 80 CLASS 1, 2 OR 3 WITH A MINIMUM YIELD STRENGTH OF 80,000 PSI. COATING OF BASE MATERIAL IS 55% ALUMINUM-ZINC ALLOY IN ACCORDANCE WITH AZ55 SPECIFICATIONS.
- 7) CABLE UTILIZED FOR BRACING CONFORMS TO ASTM A475. CABLE BRACING IS TO BE INSTALLED TO A TAUT CONDITION.
- 8) ROD UTILIZED FOR BRACING MEMBERS CONFORM TO ASTM-A36 WITH MINIMUM YIELD POINT OF 36,000 PSI. 9) IT IS THE RESPONSIBILITY OF ERECTOR TO ENSURE PROPER BOLT TIGHTNESS IN ACCORDANCE WITH APPLICABLE "RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING A-325 OR A-490 BOLTS". ALL A-325 BOLTS IN PRIMARY FRAMING MUST BE "SNUG-TIGHT". EXCEPT AS FOLLOWS: "FULLY-PRETENSION" A-325 BOLTS IF:
- a) BUILDING LOCATED IN A HIGH SEISMIC AREA. FOR IBC-BASED CODE, "HIGH SEISMIC AREA" IS DEFINED AS "SEISMIC DESIGN CATEGORY" OF "D", "E" OR "F".
- b) BUILDING SUPPORTS A CRANE SYSTEM WITH A CAPACITY GREATER THAN 5.00 TONS.
- c) BUILDING SUPPORTS MACHINERY THAT CREATES VIBRATION, IMPACT OR STRESS REVERSALS ON THE CONNECTIONS
- d) ANY CONNECTION DESIGNATED IN THESE DRAWINGS AS "A-325 SC".

- 10) SECONDARY MEMBERS AND FLANGE BRACE CONNECTIONS SHALL ALWAYS BE SNUG TIGHT, UND.
- 11) ANCHOR BOLTS 3/4" IN DIAMETER THRU 1 1/4" IN DIAMETER CONFORM TO A.S.T.M. F1554 GR. 36.
- ANCHOR BOLTS 1/2" IN DIAMETER CONFORM TO A.S.T.M. A-307.

 D) UNLESS NOTED OTHERWISE ON FRAMING COLOR CHART: ALL STEEL MEMBERS EXCEPT BOLTS, FASTENERS, CABLE AND RODS SHALL RECEIVE ONE COAT OF STANDARD RED OXIDE SHOP PRIMER.
- E) SHOP AND FIELD INSPECTIONS AND ASSOCIATED FEES ARE THE RESPONSIBILITY OF THE CONTRACTOR, UNLESS STIPULATED OTHERWISE IN THE CONTRACT

APPROVAL NOTES

- THE FOLLOWING CONDITIONS APPLY IN THE EVENT THAT THESE DRAWINGS ARE USED AS APPROVAL DRAWINGS:
- A) IT IS IMPERATIVE THAT ANY CHANGES TO THESE DRAWINGS:
- 1) BE MADE IN CONTRASTING INK.
- 2) HAVE ALL INSTANCES OF CHANGE CLEARLY INDICATED.
- 3) BE LEGIBLE AND UNAMBIGUOUS.
- B) DATED SIGNATURE IS REQUIRED ON ALL PAGES.
- C) MANUFACTURER RESERVES THE RIGHT TO RESUBMIT DRAWINGS WITH EXTENSIVE OR COMPLEX CHANGES REQUIRED TO AVOID MISFABRICATION. THIS MAY IMPACT THE DELIVERY SCHEDULE.
- D) APPROVAL OF THESE DRAWINGS INDICATES CONCLUSIVELY THAT THE MANUFACTURER HAS CORRECTLY NTERPRETED THE CONTRACT REQUIREMENTS, AND FURTHER CONSTITUTES AGREEMENT THAT THE BUILDING AS DRAWN, OR AS DRAWN WITH INDICATED CHANGES REPRESENTS THE TOTAL OF THE MATERIALS TO BE SUPPLIED BY MANUFACTURER.
- E) ANY CHANGES NOTED ON THE DRAWINGS NOT IN CONFORMANCE WITH THE TERMS AND REQUIREMENTS OF THE CONTRACT BETWEEN MANUFACTURER AND ITS CUSTOMER ARE NOT BINDING ON MANUFACTURER UNLESS SUBSEQUENTLY SPECIFICALLY ACKNOWLEDGED AND AGREED TO IN WRITING BY CHANGE ORDER OR SEPARATE DOCUMENTATION, MANUFACTURER RECOGNIZES THAT RUBBER STAMPS ARE ROUTINELY USED FOR INDICATING APPROVAL, DISAPPROVAL, REJECTION, OR MERE REVIEW OF THE DRAWINGS SUBMITTED. HOWEVER, MANUFACTURER DOES NOT ACCEPT CHANGES OR ADDITIONS TO CONTRACTUAL TERMS AND CONDITIONS THAT MAY APPEAR WITH USE OF A STAMP OR SIMILAR INDICATION OF APPROVAL, DISAPPROVAL, ETC. SUCH LANGUAGE APPLIED TO MANUFACTURER'S DRAWINGS BY THE CUSTOMER, ARCHITECT, ENGINEER, OR ANY OTHER PARTY WILL BE CONSIDERED AS UNACCEPTABLE ALTERATIONS TO THESE DRAWING NOTES, AND WILL NOT ALTER THE CONTRACTUAL RIGHTS AND OBLICATIONS EXISTING BETWEEN MANUFACTURER AND ITS CUSTOMER

SAFETY COMMITMENT

- A) THE BUILDING MANUFACTURER HAS A COMMITMENT TO MANUFACTURE QUALITY BUILDING COMPONENTS THAT CAN BE SAFELY ERECTED. HOWEVER, THE SAFETY COMMITMENT AND JOB SITE PRACTICES OF THE ERECTOR ARE BEYOND THE CONTROL OF THE BUILDING MANUFACTURER.
- IT IS STRONGLY RECOMMENDED THAT SAFE WORKING CONDITIONS AND ACCIDENT PREVENTION PRACTICES BE THE TOP
- C) LOCAL STATE AND FEDERAL SAFETY AND HEALTH STANDARDS SHOULD ALWAYS BE FOLLOWED TO HELP INSURE
- MAKE CERTAIN ALL EMPLOYEES KNOW THE SAFEST AND MOST PRODUCTIVE WAY OF ERECTING A BUILDING EMERGENCY PROCEDURES SHOULD BE KNOWN TO ALL EMPLOYEES.
- DAILY MEETINGS HIGHLIGHTING SAFETY PROCEDURES ARE ALSO RECOMMENDED. THE USE OF HARD HATS, RUBBER SOLE SHOES FOR ROOF WORK, PROPER EQUIPMENT FOR HANDLING MATERIAL, AND SAFETY NETS WHERE APPLICABLE, ARE RECOMMENDED.

ERECTOR / CONTRACTOR RESPONSIBILITIES

- A) IT IS THE RESPONSIBILITY OF THE ERECTOR/CONTRACTOR TO INSURE THAT ALL PROJECT PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT THE BUILDING MANUFACTURER OR ITS DESIGN ENGINEER IS ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT.
- B) THE CONTRACTOR MUST SECURE ALL REQUIRED APPROVALS AND PERMITS FROM THE APPROPRIATE AGENCY AS
- C) APPROVAL OF THE MANUFACTURER'S DRAWINGS AND CALCULATIONS INDICATE THAT THE BUILDING MANUFACTURER CORRECTLY INTERPRETED AND APPLIED THE REQUIREMENTS OF THE CONTRACT DRAWINGS AND SPECIFICATIONS. (SECT. 4.4.1 AISC CODE OF STANDARD PRACTICES, LATEST ED.)
- D) WHERE DISCREPANCIES EXIST BETWEEN THE MANUFACTURER'S STRUCTURAL STEEL PLANS AND THE PLANS FOR OTHER TRADES, THE STRUCTURAL STEEL PLANS SHALL GOVERN. (SECT. 3.3 AISC CODE OF STANDARD PRACTICE LATEST ED.)
 E) DESIGN CONSIDERATIONS OF ANY MATERIALS IN THE STRUCTURE WHICH ARE NOT FURNISHED BY THE BUILDING
- MANUFACTURER ARE THE RESPONSIBILITY OF THE CONTRACTORS AND ENGINEERS OTHER THAN THE BUILDING MANUFACT-URER'S ENGINEERS UNLESS SPECIFICALLY INDICATED.

 F) THE ERECTOR/CONTRACTOR IS RESPONSIBLE FOR ALL ERECTION OF STEEL AND ASSOCIATED WORK IN COMPLIANCE WITH
- THE BUILDING MANUFACTURER'S "FOR CONSTRUCTION" DRAWINGS.

 G) PRODUCTS SHIPPED TO ERECTOR/CONTRACTOR OR HIS CUSTOMER SHALL BE INSPECTED BY ERECTOR/CONTRACTOR
- IMMEDIATELY UPON ARRIVAL CLAIMS FOR SHORTAGES OR DEFECTIVE MATERIAL IF NOT PACKAGED MUST BE SENT TO THE MANUFACTURER IN WRITING WITHIN FIVE (5) DAYS AFTER RECEIPT OF THE SHIPMENT. HOWEVER, IF A DEFECT IS OF SUCH A NATURE THAT REASONABLE VISUAL INSPECTION WOULD FAIL TO DISCLOSE IT, THEN THE CLAIM MUST BE MADE WITHIN FIVE (5) DAYS AFTER THE ERECTOR/CONTRACTOR LEARNS OF THE DEFECT. THE MANUFACTURER WILL NOT BE LIABLE FOR ANY DEFECT UNLESS CLAIM IS MADE WITHIN ONE (1) YEAR AFTER DATE OF THE ORIGINAL SHIPMENT BY THE MANUFACTURER TO CONTRACTOR OR HIS CUSTOMER. THE MANUFACTURER WILL BE GIVEN A REASONABLE OPPORTUNITY TO INSPECT DEFECTIVE MATERIALS UPON RECEIPT OF CLAIM BY CONTRACTOR.
- IF A DEFECT IS OF SUCH NATURE THAT IT CAN BE REMEDIED BY A FIELD OPERATION AT THE JOB SITE WITHOUT THE NECESSITY OF RETURNING THE MATERIAL TO THE MANUFACTURER, THEN UPON WRITTEN AUTHORIZATION OF THE MANUFACTURER THE CONTRACTOR MAY REPAIR OR CAUSE THE MATERIAL TO BE REPAIRED AND THE MANUFACTURER WILL REIMBURSE THE CONTRACTOR FOR THE COST OF THE REPAIR IN ACCORDANCE WITH THE WRITTEN AUTHORIZATION.
- THE CORRECTION OF MINOR MISFITS BY THE USE OF DRIFT PINS TO DRAW THE COMPONENTS IN TO LINE, MODERATE AMOUNTS OF REAMING, CHIPPING AND CUTTING, AND THE REPLACEMENT OF MINOR SHORTAGES OF MATERIAL ARE A NORMAL PART OF ERECTION AND ARE NOT SUBJECT TO CLAIM.
- H) ALL BRACING AS SHOWN AND PROVIDED BY THE MANUFACTURER FOR THIS BUILDING IS REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE STRUCTURE.
- TEMPORARY SUPPORTS, SUCH AS TEMPORARY GUYS, BRACES, FALSE WORK, CRIBBING OR OTHER ELEMENTS REQUIRED FOR THE ERECTION OPERATION WILL BE DETERMINED AND FURNISHED AND INSTALLED BY THE ERECTOR. THESE TEMPORARY SUPPORTS WILL SECURE THE STEEL FRAMING, OR ANY PARTLY ASSEMBLED STEEL FRAMING, AGAINST LOADS COMPARABLE IN INTENSITY TO THOSE FOR WHICH THE STRUCTURE WAS DESIGNED. RESULTING FROM WIND SEISMIC FORCES AND ERECTION OPERATIONS, BUT NOT THE LOADS RESULTING FROM THE PERFORMANCE OF WORK BY OR THE ACTS OF OTHERS, NOR SUCH UNPREDICTABLE LOADS AS THOSE DUE TO TORNADO, EXPLOSION OR COLLISION 7.10.3 AISC CODE OF STANDARD PRACTICE, LATEST ED.)
- J) METAL BUILDING MANUFACTURER IS NOT RESPONSIBLE FOR THE DESIGN MATERIAL AND WORKMANSHIP OF FOUNDATION, ANCHOR BOLT PLANS PREPARED BY MBM ARE INTENDED TO SHOW ONLY LOCATION, DIAMETER AND PROJECTION OF THE ANCHOR RODS REQUIRED TO ATTACH THE METAL BUILDING SYSTEM TO FOUNDATION, IT IS RESPONSIBILITY OF THE END CUSTOMER TO ENSURE THAT ADEQUATE PROVISIONS ARE MADE FOR SPECIFYING ROD EMBEDMENT, BEARING VALUES, TIE RODS AND OTHER ASSOCIATED ITEMS EMBEDDED IN THE CONCRETE FOUNDATION, AS WELL AS FOUNDATION DESIGN FOR THE LOADS IMPOSED BY MB SYSTEM, OTHER IMPOSED LOAD, AND THE BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE (MBMA 06 SECTIONS 3.2.2 AND A3)
- K) METAL BUILDING MANUFACTURER DOES NOT PROVIDE ANY FIELD SUPERVISION FOR THE ERECTION. NOR DOES MRM PERFORM ANY INSPECTIONS DURING OR AFTER FRECTION.

COMPONENTS & CLADDING (unfactored) Wall Field Values = 32.081 psf / -34.755 psf Wall Edge Values = 32.081 psf / -42.695 psf

IS THE RESPONSIBILITY OF THE CUSTOMER TO PROVIDE ALL DOCUMENTATION REQUIRED FOR ANY ACCESSORIES NOT PROVIDED BY MBM TO THEIR LOCAL PERMITTING OFFICE.
ALL ACCESSORIES MUST COMPLY AND MEET ALL DESIGN REQUIREMENTS PER LOCAL CODES.

ALL VEHICULAR FRAMED OPENINGS SUPPLIED ON THIS PROJECT HAVE BEEN DESIGNED TO SUPPORT WIND LOADS NORMAL TO A SYSTEM, BASED ON THE STANDARD BUILDING CODE CRITERIA THE VEHICULAR FRAMED OPENING HAS NOT BEEN DESIGNED FOR ANY ADDITIONAL MOMENT OR CATENARY FORCE FROM THE DOOR SYSTEM. ANY CHANGES TO THE INFORMATION SHOWN HERE WOULD REQUIRE AN ENGINEERING INVESTIGATION AND POSSIBLE BUILDING

Flange broce: R0 R0 EvSt Jmb 88 U SECTION: RO RO RO RO RO RO RO C SECTION: RO RO RO RO RO RO RO RO W SECTION: RO RO RO RO RO RO

FRAMING COLORS

WHEN GALVANIZED PROVIDED: ALL FINISHED PRIMARY BUILT-UP AND HOT ROLL MEMBERS ARE HOT DIPPED GALVANIZED. ALL SECONDARY COLD FORMED MEMBERS ARE PRE-GALVANIZED.



BY AN IAS ACCREDITED FACILITY.

COVER PAGE ANCHOR BOLT LAYOUT 1 1 ANCHOR BOLT DETAILS 1.2 ANCHOR BOLT REACTIONS ROOF FRAMING LAYOUT RIGID FRAME CROSS SECTION 2.1 - 2.4SIDEWALL FRAMING LAYOUT ENDWALL FRAMING LAYOUT 4.1 HANGER DOOR FRAMING 4.2 HANGER DOOR DETAILS 5-5.4 FRAMING DETAILS ROOF PANELS & TRIM 6.1 ROOF PANEL DETAILS SIDEWALL PANELS & TRIM 7.1 SIDEWALL PANEL DETAILS ENDWALL PANELS & TRIM 8.1 ENDWALL PANEL DETAILS SPECIAL DETAILS 10 LINER SHEETING & TRIM THIS PROJECT IS DESIGNED AS AN ENCLOSED BUILDING. ACCESSORIES

DRAWING INDEX

DESCRIPTION

28339

2

ERWIN,

JOBSITE:

NONE

28339

2

ERWIN,

8

5

6

COURT

BUILDINGS

HANGAR AIRPORT

ERWIN

REV.

PAGE

(DOORS, WINDOWS, ETC.) BY OTHERS MUST BE DESIGNED AS "COMPONENTS AND CLADDING" IN ACCORDANCE TO SPECIFIC WIND PROVISIONS OF REFERENCED BUILDING CODE. FOR OCCUPANCY (RISK) CATEGORY I OR II, IBC PROVISIONS INDICATE THAT SINGLE-STORY BUILDINGS SHALL HAVE "NO DRIFT LIMIT" PROVIDED THAT INTERIOR WALLS, PARTITIONS, CEILINGS AND EXTERIOR WALL SYSTEMS HAVE

BEEN DESIGNED TO ACCOMMODATE THE SEISMIC STORY DRIFTS. INTERIOR WALLS, PARTITIONS, CFILINGS OR EXTERIOR SYSTEMS NOT PROVIDED BY MRM SHALL BE DESIGNED AND DETAILED BY OTHERS TO ACCOMODATE THE SEISMIC STORY DRIFTS.

> FINAL 1/4/23

LYNDHURST STEELCOR SARAS 4084 JOB NO DRAWING STATUS 8245 FOR APPROVAL:
THESE DRAWINGS, BEING FOR APPROVAL, ARE BY
DEFINITION NOT FINAL, AND ARE FOR CONCEPTUAL
REPRESENTATION ONLY. THEIR PURPOSE IS TO
CONFIRM PROPER INTERPRETATION OF THE PROJECT
DOCUMENTS. ONLY DRAWINGS ISSUED "FOR CONSTRUCTION"
CAN BE CONSIDERED AS COMPLETE. 10/26/23 SCALE DAR FOR PERMIT:
THESE DRAWINGS, BEING FOR PERMIT, ARE BY DEFINITION NOT FINAL IN THAT, AS A MINIMUM, PIECE MARKINGS ARE NOT IDENTIFIED, ONLY DRAWINGS ISSUED FOR CONSTRUCTION "CAN BE CONSIDERED AS COMPLETE. COVER PAGE PAGE FOR CONSTRUCTION: THESE DRAWINGS ARE FINAL AND ISSUED FOR FIELD USE FOR BUILDING ERECTION

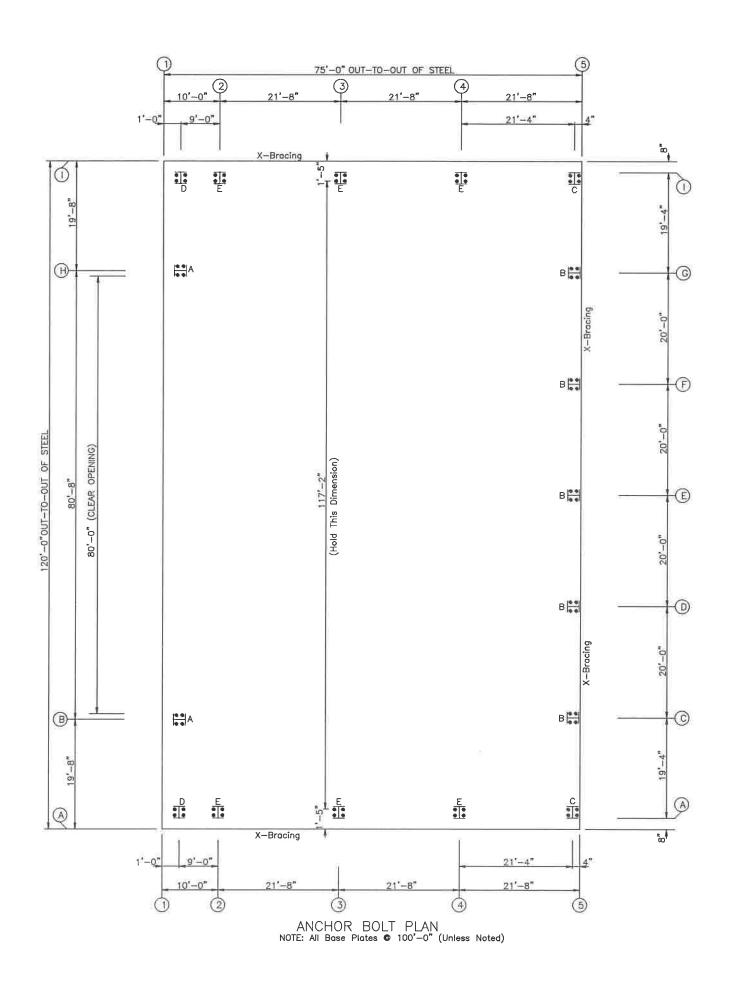
ROOF: GALVALUM LINER: POLAR WHITE DOOR PANEL: SADDLE TAN SOFFIT: KOKO BROWN WALLS: SADDLE TAN GABLE: KOKO BROWN EAVE: KOKO BROWN CORNER KOKO BROWN FRAMED OPENINGS KOKO BROWN

COLORS:

GUTTER: KOKO BROWN KOKO BROWN

KOKO BROWN

♣ Dia= 7/8"

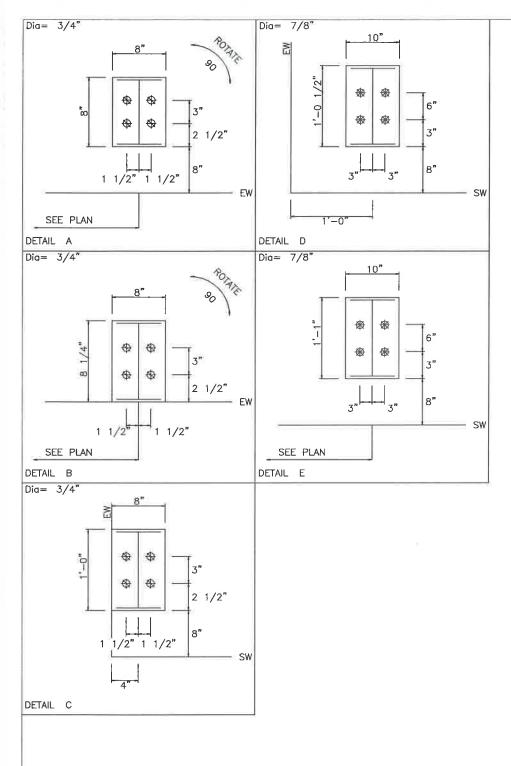


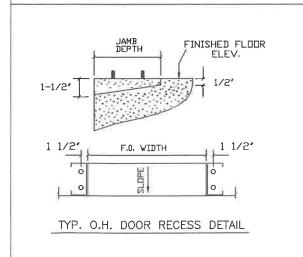
NOTE: ALL FIELD LOCATED FRAMED OPENING LOCATIONS SHALL BE AT THE DISCRETION OF THE ERECTOR/CUSTOMER. IT IS RECOMMENDED THAT THESE ANCHORS BE LOCATED AT TIME OF ERECTION.

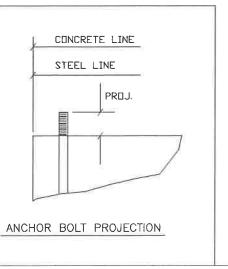
FIELD LOCATE:

(2) 3070 WALKDOORS

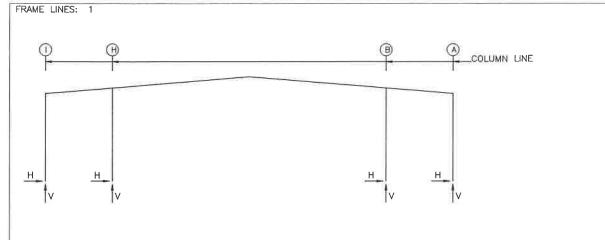
ISSUE		DET	СНК	DATE
STEELCOR BUILDIN	IGS			
ERWIN HANGAR				
6 NO:		DATE:		
8245		10	/26/	23
ERWIN, NC 2833	9			
ANCHOR BOLT LAY	OUT			
AWING NO:	DRAWN BY:	CHECKED BY	r: SC	ALE:
PAGE 1	DAR	IRAW		NONE

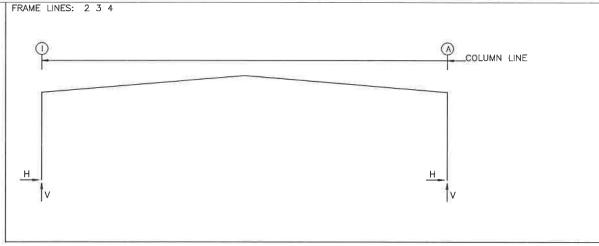






ISS	UE	DET	СНК	DATE
STEELCOR BUILD	INGS			
CUSTOMER: ERWIN HANGAR				
308 NO: 8245		DATE:	/26/	23
ERWIN, NC 283	339			
DRAWING NAME: ANCHOR BOLT D	ETAILS			
DRAWING NO:	DRAWN BY:	CHECKED EN	: SC	N.E.





Line 1	Column Line !	Horiz 0.0	Vert	Horiz	Vert	Horiz -0.1	-Live Vert -1.4	Horiz	Vert	Horiz	Vert	Horiz 2.5	Vert 4.8
1 1 1	A H B	0.0 0.0 0.0	-0.6 4.3 4.3	0.1 0.0 0.0	-2.2 9.3 9.3	0.1 0.0 0.0	-1.4 5.7 5.7	0.0 0.0 0.0	-0.8 -0.8 3.3 3.3	-2.5 0.0 0.0	4.8 -14.0 -13.2	1.9 0.0 0.0	1.5 -13.2 -14.0
Frame Line 1 1 1 1	Column Line I A H B	Wind Horiz -2.5 -1.8 0.0	_Left2- Vert 0.8 4.0 -9.4 -8.6	-Wind_ Horiz 1.8 2.5 0.0 0.0	Right2- Vert 4.0 0.8 -8.6 -9.4	Wind Horiz 1.3 1.0 0.0 0.0	I_Long1 Vert 1.8 1.4 -10.7 -7.8	Horiz 1.0 –1.3	I_Long2- Vert 1.4 1.8 -7.8 -10.7			Seismid Horiz 0.3 0.3 0.0 0.0	c_Right Vert 0.4 -0.4 -0.5
Frame Line 1 1 1 1	Column Line I A H B	-MIN_S Horiz -0.1 0.1 0.0 0.0	NOW Vert -1.2 -1.2 4.8 4.8	Horiz -0.1	LI_1- Vert -1.4 -2.1 5.8 5.0	F1PAT_ Horiz -0.1 0.1 0.0 0.0	Vert -2.1 -1.4 5.0 5.8	F1PAT_ Horiz 0.0 0.0 0.0 0.0	Vert 0.8 0.8 0.7 0.7	F1PAT_ Horiz -0.1 0.1 0.0 0.0	Vert	F1UNB Horiz 0.0 0.0 0.0 0.0	Vert -1.1 -1.0 4.1
Frame Line 1 1 1 1	Column Line I A H B	Horiz 0.0	SL_R- Vert -1.0 -1.1 2.3 4.1										
Frame Line 2 2	Column Line I A		-Dead Vert	-—-Collo Horiz 5.1 -5.1	nteral— Vert 6.9 6.9	Horiz 9.0 –9.0	-Live Vert 11.5 11.5	Horiz 5.3 -5.3	Snow—— Vert 6.7 6.7	Wind Horiz -22.5 13.1	_Left1- Vert -27.6 -19.7	-Wind_ Horiz -13.1 22.5	
Frame Line 2 2	Column Line I A		_Left2- Vert -17.3 -9.4	-Wind_ Horiz -7.3 16.7	Right2- Vert -9.4 -17.3	Wind Horiz -11.5 12.2	_Long1- Vert -22.4 -18.1	Horiz	_Long2- Vert 18.1 22.4	-Seism Horiz -0.5 -0.5	ic_Left Vert -0.2 0,2	Seismic Horiz 0.5 0.5	:_Right Vert 0.2 -0.2
Frame Line 2 2	Column Line I A	-MIN_S Horiz 7.5 -7.5	NOW Vert 9.6 9.6	F2UNB_ Horiz 5.0 -5.0	SL_L- Vert 7.1 4.3	F2UNB_ Horiz 5.0 –5.0	SL_R- Vert 4.3 7.1						
Frame Line 3 3	Column Line I A	Horiz 3.8 -3.8	Dead Vert 6.0 6.0	-—-Colle Horiz 3.3 -3.3	oteral- Vert 4.0 4.0	Horiz 13.2 –13.2	-Live Vert 15.8 15.8	Horiz 7.7 –7.7	-Snow Vert 9.2 9.2	Wind Horiz 27.3 16.6	_Left1- Vert -32.1 -23.6	-Wind_ Horiz -16.6 27.3	
Frame Line 3 3	Column Line I A	Wind Horiz -18.8 8.1	_Left2- Vert -17.9 -9.5	-Wind_ Horiz -8.1 18.8	Right2- Vert -9.5 -17.9	Wind Horiz -17.0 18.1	_Long1- Vert 30.8 24.9	Wind Horiz -18.1 17.0	_Long2- Vert -24.9 -30.8	-Seism Horiz -0.6 -0.7	ic_Left Vert -0.3 0.3	Seismid Horiz 0.6 0.7	:_Right Vert 0.3 -0.3
Frame Line 3 3	Column Line I A	-MIN_S Horiz 11.0 -11.0	Vert	F3UNB_ Horiz 7.4 -7.4	SL_L- Vert 9.8 6.0	F3UNB_ Horiz 7.4 –7.4	SL_R- Vert 6.0 9.8						
Frame Line 4 4	Column Line I A	Horiz 3.8 -3.8	Dead Vert 6.0 6.0	-—-Colle Horiz 3.3 -3.3	oteral- Vert 4.0 4.0	Horiz 13.2 –13.2	-Live Vert 15.8 15.8	Horiz 7.7 –7.7	-Snow Vert 9.2 9.2	Wind Horiz -31.0 18.7	_Left1- Vert -36.2 -26.1	Horiz -18.7	Right1 – Vert –26.1 –36.2
Frame Line 4 4	Column Line I A			-10.2	Right2- Vert -11.9 -22.1	Wind Horiz -17.0 18.1	Long1- Vert -30.8 -24.9	Horiz 18.1	_Long2- Vert -24.9 -30.8	-Seism Horiz -0.6 -0.7	ic_Left Vert -0.3 0.3	Seismic Horiz 0.6 0.7	
Frame Line 4 4	Column Line I A	-MIN_S Horiz 11.0 -11.0	NOW Vert 13.2 13.2	F4UNB_ Horiz 7.4 -7.4	SL_L- Vert 9.8 6.0	F4UNB_ Horiz 7.4 –7.4							

RIGI	D FF	RAME	.: AN	ICHOR E	BOLTS &	BASE	PLATES
Frm Line	Col Line	Anc. Qty	_Bolt Dia	Base Width	_Plate (i Length	n) Thick	Grout (in)
1	I A	4	0.875 0.875	10.00	12.50 12.50	0.500 0.500	0.0
1	H B	4	0.750 0.750	8.000	8.000	0.375	0.0

RIGI	D FR	AME	: AN	ICHOR I	BOLTS &	BASE	PLATES
Frm Line	Col Line		_Bolt Dia	Base_ Width	_Plate (i Length	n) Thick	Grout (in)
2 2	I A	4	0.875 0.875		13.00 13.00	0.625 0.625	0.0

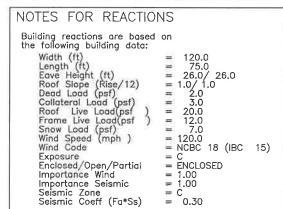
RIGII	D FR	AME	: AN	ICHOR E	30LTS &	BASE	PLATES
Frm Line	Col Line		_Bolt Dia	Base_ Width	_Plate (i Length	in) Thick	Grout (in)
3 3	Å	4 4	0.875 0.875		13.00 13.00	0.750 0.750	0.0

RIGI	D FR	RAME	: AN	ICHOR E	BOLTS &	BASE	PLATES
Frm Line	Col Line	Anc. Qty	_Bolt Dia	Base_ Width	_Plate (i Length	n) Thick	Grout (in)
4 4	I A				13.00 13.00	0.625 0.625	0.0

GENERAL NOTES

- FOUNDATION DESIGN AND CONSTRUCTION ARE NOT THE RESPONSIBILITY OF METAL BUILDING MANUFACTURER.
- 2. ALL REACTIONS ARE UNFACTORED.
- 3 ULTIMATE WIND LOADS ARE USED TO DERIVE THE WIND REACTION.
- ANCHOR BOLTS SHALL BE ACCURATELY SET TO A TOLLERANCE OF +/- 1/8" IN BOTH ELEVATION AND LOCATION.
- 5. COLUMN BASE PLATES ARE DESIGNED NOT TO EXCEED A BEARING PRESSURE OF 1050 POUNDS PER SQUARE INCH.

END	WALI		LUMN:	BAS	SIC COL	UMN R	EACTIONS	S (k)								
Frm Line 1	Col Line H B	Wind Press Horz -3.6 -3.6	Wind Suct Horz 3.9 3.9	Ho).0 Ve	ert H	0.0	ert H		SL_R- Vert 2.0 3.6						
Frm Line 55 55 55 55	Col Line A C D E F G	Dead Vert 0.6 1.2 1.2 1.2 1.2 1.2 0.6	Collar Vert 0.3 0.7 0.6 0.7 0.6 0.7	Ve 1 4 4 4 4		Snow Vert 0.6 1.7 1.5 1.6 1.5 1.7 0.6	Wind Left1 Vert -3.1 -8.8 -8.1 -6.0 -4.2 -4.9 -2.1	Wind Righ Vert -2. -4. -6. -8. -3.	1111 192 201 18	Wind Left2 Vert -2.0 -6.2 -5.8 -4.0 -1.9 -2.3 -1.0	Wind Right2 Vert -1.0 -2.3 -1.9 -4.0 -5.8 -6.2 -2.0	Wind Press Horz -3.5 -6.7 -7.2 -7.7 -7.2 -6.7 -3.5	Wind Suct Horz 4.0 7.4 8.0 8.4 8.0 7.4	L V	Vind Long1 -3.5 -8.7 -8.0 -5.7 -4.2 -5.1	Wind Long2 Vert -2.1 -5.1 -4.2 -5.7 -8.0 -8.7 -3.5
Frm Line 5 5 5 5 5 5	Col Line A C D E F G	Seis Left Vert 0.0 0.0 0.0 0.0 0.0	Seis Right Vert 0.0 0.0 0.0 0.0 0.0	Ve: 0 0 0 0	ng	-MIN_S Horz 0.0 0.0 0.0 0.0 0.0 0.0	SNOW Vert 0.9 2.4 2.1 2.2 2.1 2.4 0.9		SL_L- Vert 0.7 1.5 2.6 2.1 0.2 0.6 0.2	- E2UNB Horz 0.0 0.0 0.0 0.0 0.0 0.0		0.0 0.0 0.0		E2PAT_ Horz 0.0 0.0 0.0 0.0 0.0 0.0	Vert -0.2 2.0 4.9 2.1 -0.3 0.1 0.0	
Frm Line 5 5 5 5 5 5 5 5 5	Col Line A C D E F G	E2PAT_ Horz 0.0 0.0 0.0 0.0 0.0 0.0 0.0	Vert 0.0 -0.3 2.1 4.9 2.1 -0.3 0.0	E2PAT_ Horz 0.0 0.0 0.0 0.0 0.0 0.0 0.0	LL_4- Vert 0.0 0.1 -0.3 2.1 4.9 2.0 -0.2	E2PA Horz 0.0 0.0 0.0 0.0 0.0 0.0	T_LL_5- Vert 0.0 0.0 0.1 -0.3 2.1 5.1 1.7	E2PAT Horz 0.0 0.0 0.0 0.0 0.0 0.0	Vert 2.0 2.2 2.2 2.2 2.5 -0.3	Horz 0.0 3 0.0 1 0.0 2 0.0 1 0.0 5 0.0	-0.2 2.5 2.1 2.2 2.1 2.3					



END	WALL	COI	_UMN:	ANC	HOR BOL	TS & B	ASE PLATE	S
Frm Line	Col Line	Anc. Qty	_Bolt Dia	Base_ Width	_Plate (i Length	n) Thick	Grout (in)	
5	Α	4	0.750	8.000	12.00	0.375	0.0	
5	С	4	0.750	8.000	8.250	0.375	0.0	
5	D	4	0.750	8.000	8.250	0.375	0.0	
5	Ε	4	0.750	8.000	8.250	0.250	0.0	
5	F	4	0.750	8.000	8.250	0.375	0.0	
5	G	4	0.750	8.000	8.250	0.375	0.0	
5	1	4	0.750	8.000	12.00	0.375	0.0	

ANCHOR	BOLT	SUMMARY	ľ

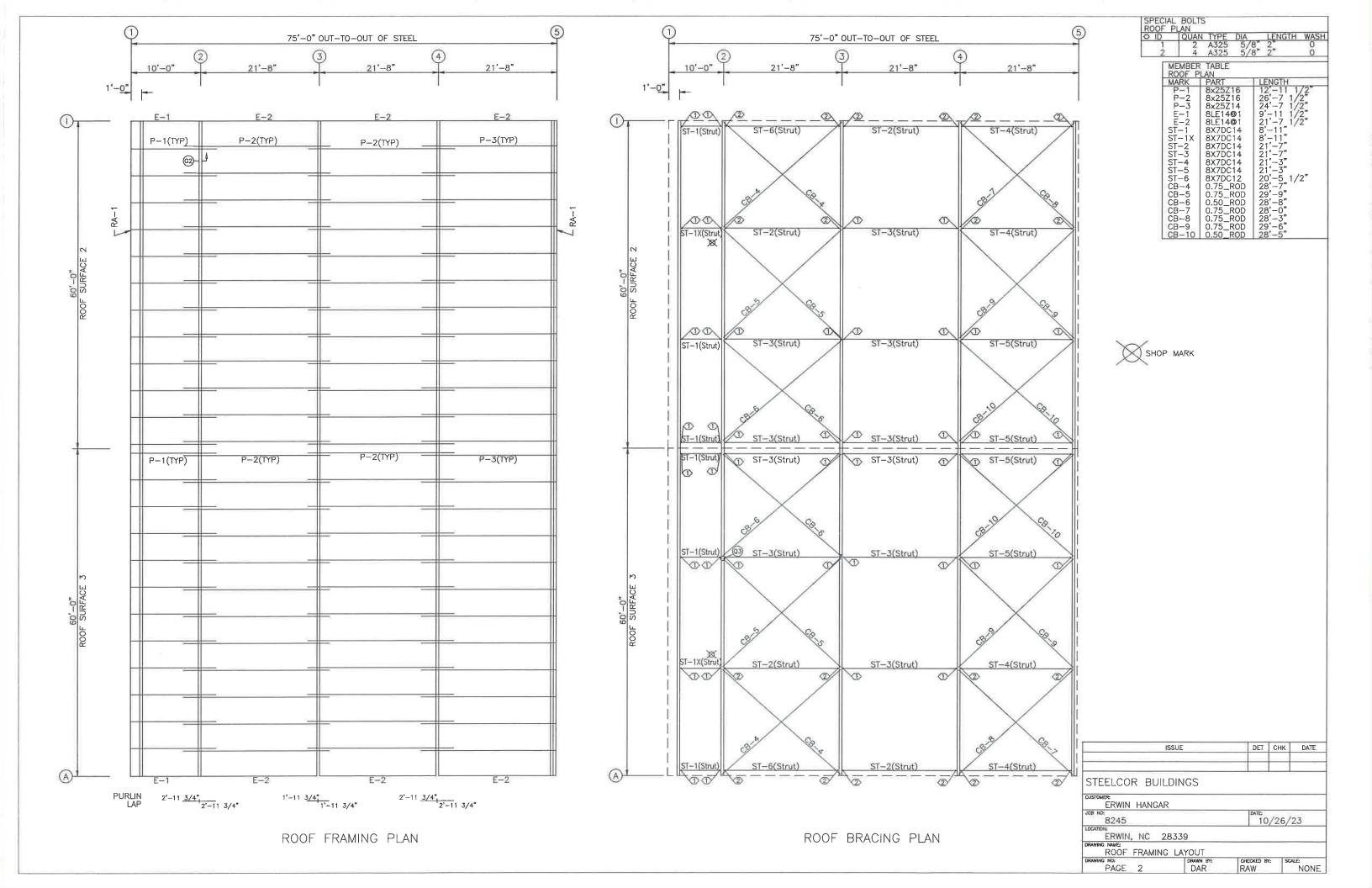
			Dia		Projectio
	Qty	Locate	(in)	Туре	(iń)
B	36 32	Endwall Frame	3/4" 7/8"	GR36 GR36	1.50 2.50

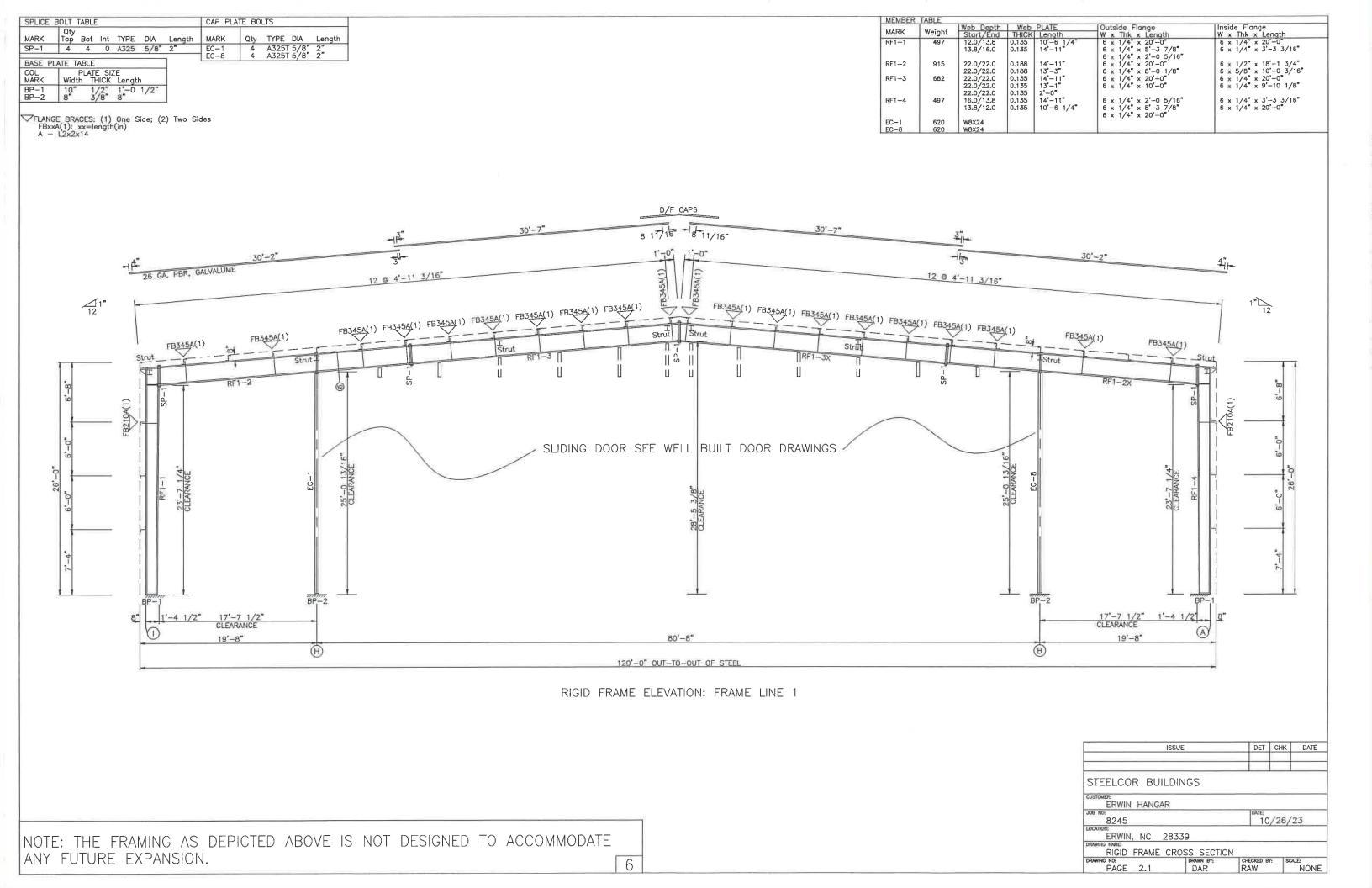
NOTE: THE FRAMING AT BOTH ENDWALLS IS NOT DESIGNED TO ACCOMMODATE FUTURE ADDITIONS. REACTIONS CORRESPONDING TO THESE FRAME LINES REFLECT LOADINGS FOR ACTUAL TRIBUTARY AREA AND ARE NOT INTENDED TO INCLUDE ANY FUTURE MODIFICATIONS UNLESS NOTED OTHERWISE.

BUILDING BRACING REACTIONS

—Wall — Col Loc Line Line	± Reactions(k 	smic- (lb/ft)
L_EW 1 F_SW A 2,3 R_EW 5 C,D F,G B_SW 3,2	21.2 23.8 2.4 1.7 2.4 0.4 1.7 2.4 0.4 21.2 23.8 2.4	2.7 0.6 0.6 2.7
(h)Rigid frame at e	endwall	

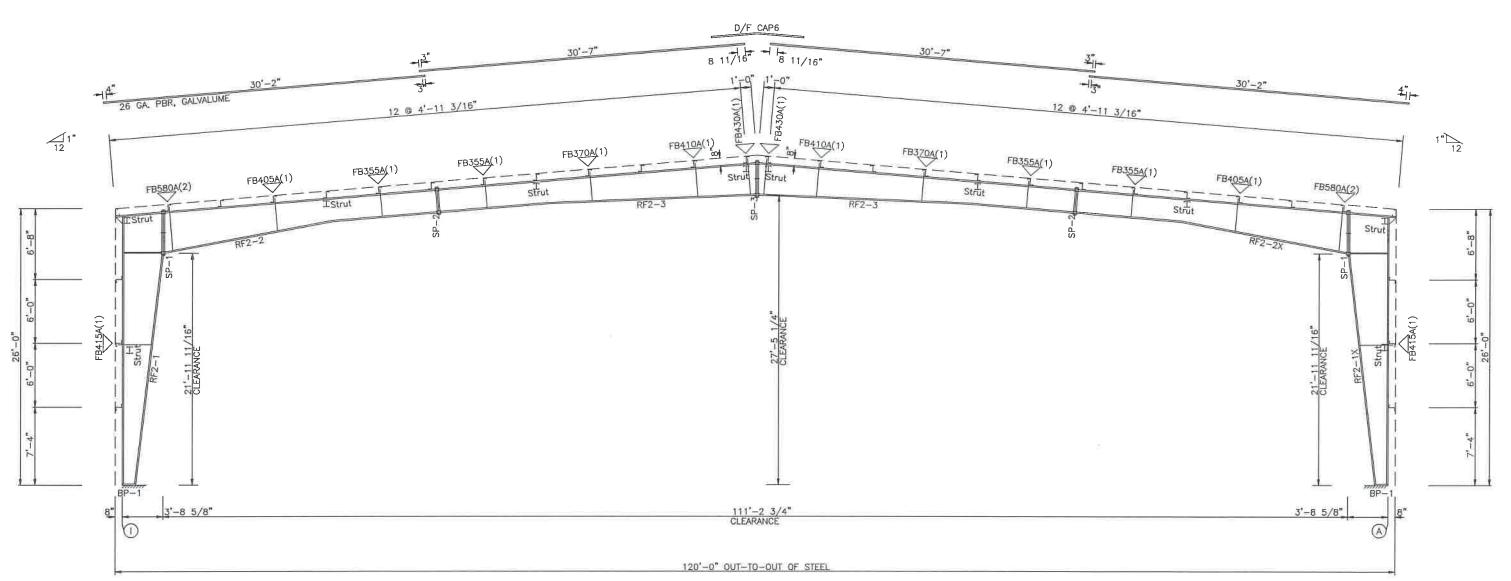
ISS	ISSUE						
STEELCOR BUILD	INGS						
CUSTOMER: ERWIN HANGAR							
лов NO: 8245		10/26/23					
ERWIN, NC 283	339						
ANCHOR BOLT F	REACTIONS						
DRAWING NO: PAGE 1.2	DRAWN BY: DAR	RAW BY	: SC	NONE			





MARK	Qty Top	Bot	Int	TYPE	DIA	Length
SP-1 SP-2 SP-3	4 4 4	4 4 4	2 0 2	A325		3" 2" 2"
BASE PL	ATE TA	ABLE				1
COL MARK		PLATE h Th		ZE Length		
3P-1	10"	5,	/8"	1'-1"		1
FBxx/ A —	A(1): L2x2x	кх=lе 14	ngth	(in)		

144514	1	Web Depth	Web PL	ATE	Outside Flange	Inside Flange
MARK	Weight	Start/End	THICK! L	ength	W x Thk x Length	W x Thk x Length
RF2-1	1307	12.0/21.9	0.250 6	-7 13/16"	8 x 5/16" x 20'-0"	8 x 5/16" x 19'-8 3/4"
	1	21.9/44.0	0.250 14	4'-11"	8 x 5/16" x 5'-3 11/16"	8 x 5/16" x 2'-0"
	1	44.0/44.0	0.313 4	-0 9/16"	8 x 5/16" x 4'-4 7/16"	
RF2-2	1146	44.0/26.5	0.250 13	3'-10 ['] 9/16"	8 x 5/16" x 16'-0 7/8"	8 x 5/16" x 15'-11 5/8"
	1	26.5/24.0	0.250 2	-0"	8 x 1/4" x 9'-6"	8 x 1/4" x 10"-0"
	1	24.0/24.0	0.188 10	o'-o"	A	
RF2-3	919	24.0/24.0	0.188 10	0'-0 3/8"	6 x 1/4" x 20'-0"	6 x 1/4" x 10'-0 5/16"
	1	24.0/31.5	0.188 14	4'-11"	6 x 1/4" x 10'-0 3/8"	6 x 1/4" x 19'-9 3/8"
	1	31 5/34 0	0.188 5	-1"	, , , , , , , , , , , , , , , , , , , ,	

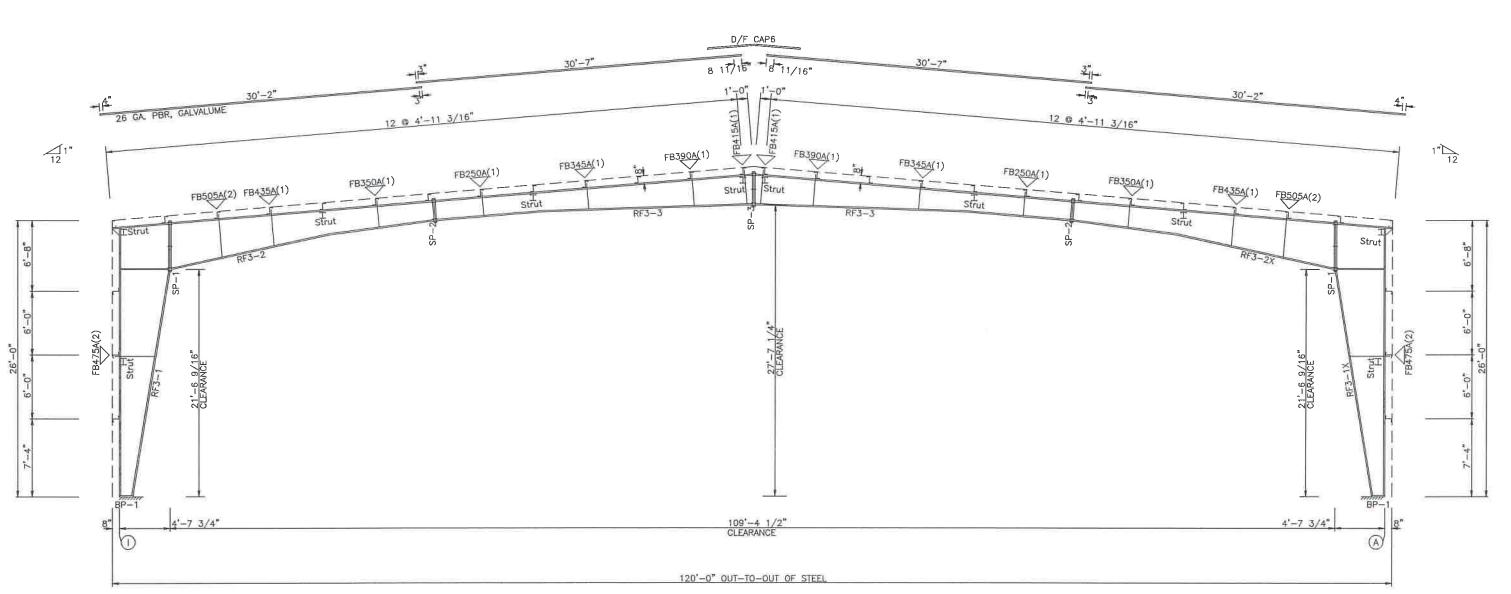


RIGID FRAME ELEVATION: FRAME LINE 2

ISSUE	DET	СНК	DATE	
		_		
STEELCOR BUILDIN	GS			
CUSTOMER: ERWIN HANGAR				
лов но; 8245		DATE:	/26	/23
ERWIN, NC 28339)			
DRAWING NAME: RIGID FRAME CROS	S SECTION			
PAGE 2.2	DAR	RAW	r: S	NONE

MARK	Qty Top	Bot	Int	TYPE	DIA	Length
SP-1 SP-2 SP-3	4 4 4	4 4 4	0	A325 A325 A325	3/4"	2 1/2" 2" 2"
BASE PL	ATE T	ABLE				Ī
COL MARK	Widt	PLATI		ZE Length	1	
BP-1	10"	3,	/4"	1'-1"	7	1
/FLANICI	E BRA	CES:	(1)	One S (in)	ide; (2) Two S

MEMBER	TABLE	more or we	10=0.00=	50.50 til		1944 - 204 - 704
MADIC		Web Depth	Web	PLATE	Outside Flange	Inside Flange
MARK	Weight	Start/End	THICK	Length	W x Thk x Length	W x Thk x Length
RF3-1	1715	12.0/33.9	0.313	10'-9 3/16"	8 x 3/8" x 20"-0"	8 x 3/8" x 19'-5 3/16"
		33.9/55.0	0.313	14'-11"	8 x 3/8" x 5'-3 9/16"	8 x 3/8" x 2'-0"
					8 x 5/16" x 5'-3 9/16"	
RF3-2	1170	50.0/29.2	0.250	13'-0 1/16"	8 x 5/16" x 15'-1 7/8"	8 x 5/16" x 15'-1 11/16"
	1	29.2/26.0	0.250	2'-0"	8 x 1/4" x 9'-6"	8 x 5/16" x 10'-0 1/8"
		26.0/20.0	0.188	10"-0"		
RF3-3	851	20.0/20.0	0.188	10'-0 1/4"	6 x 1/4" x 20"-0"	6 x 1/4" x 10"-0 3/16"
	1	20.0/29.1	0.188	14"-11"	6 x 1/4" x 10"-0 1/4"	6 x 1/4" x 19"-9 5/8"
	1	29 1/32 0	0.188	5'-1"		



RIGID FRAME ELEVATION: FRAME LINE 3

ISSUE	DET	CHK	DATE	
		-11		
STEELCOR BUILDIN	GS			
CUSTOMER: ERWIN HANGAR				
иов но: 8245		DATE:	/26/	/23
ERWIN, NC 28339	9			
RIGID FRAME CROS	S SECTION			
PAGE 2.3	DRAWN BY: DAR	RAW RAW	5	NONE

MARK	Qty Top	Bot	Int	TYPE	DIA	Lenath
SP-1	4	4			7/8"	2 1/2"
SP-2 SP-3	4	4		A325 A325	3/4" 5/8"	2" 2"
BASE PL	ATE T	ABLE				1
COL MARK		PLATI		ZE Lenath		1
BP-1	10"	5	/8"	1'-1"		1

MACH	144 1 1 1	Web Depth	Web	PLATE	Outside Flange	Inside Flange
MARK		Start/End	THICK	Length	W x Thk x Length	W x Thk x Length
RF4-1	1678	12.0/33.9	0.313	10'-9 5/16"	8 x 3/8" x 20'-0"	8 x 3/8" x 19"-5 5/16"
		33.9/55.0	0.313	14'-11"	8 x 3/8" x 5'-3 11/16"	8 x 3/8" x 2'-0"
			1 1		8 x 5/16" x 5'-3 9/16"	
RF4-2	1163	50.0/29.2	0.250	13'-0 1/16"	8 x 5/16" x 15'-1 7/8"	8 x 5/16" x 15'-1 11/16"
	1	29.2/26.0	0.250	2'-0"	8 x 1/4" x 9'-6"	8 x 5/16" x 10'-0 1/8"
		26.0/20.0	0.188	10'-0"		
RF4-3	851	20.0/20.0	0.188	10"-0 1/4"	6 x 1/4" x 20'-0"	6 x 1/4" x 10'-0 3/16"
	1	20.0/29.1	0.188	14'-11"	6 x 1/4" x 10'-0 1/4"	6 x 1/4" x 19'-9 5/8"
		29.1/32.0	0.188	5'-1"		

DET CHK DATE

10/26/23

ISSUE

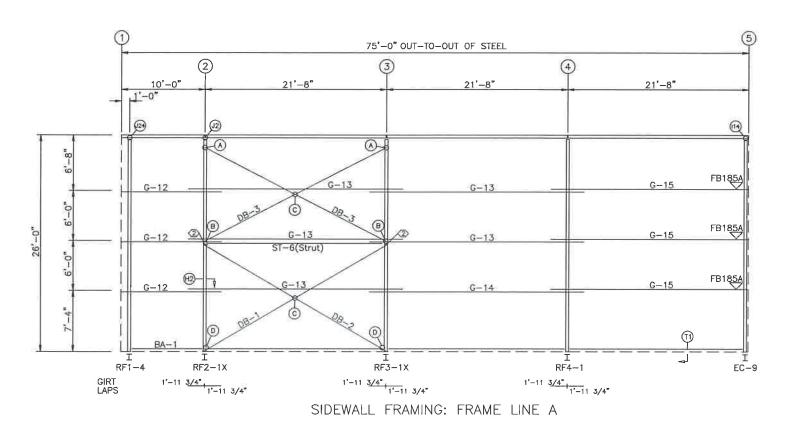
STEELCOR BUILDINGS

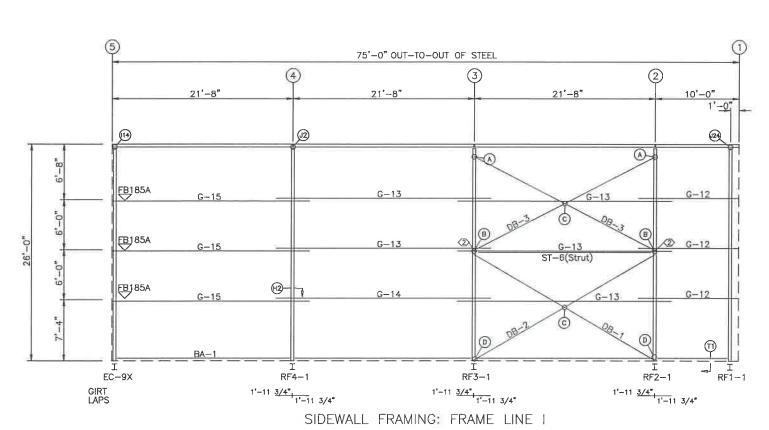
JOB NO:
B245
LDCATIONE
ERWIN, NC 28339
DRAWNED HAME:
RIGID FRAME CROSS SECTION
DRAWNIC NO:
PAGE 2.4
DRAWN BY:
DAR

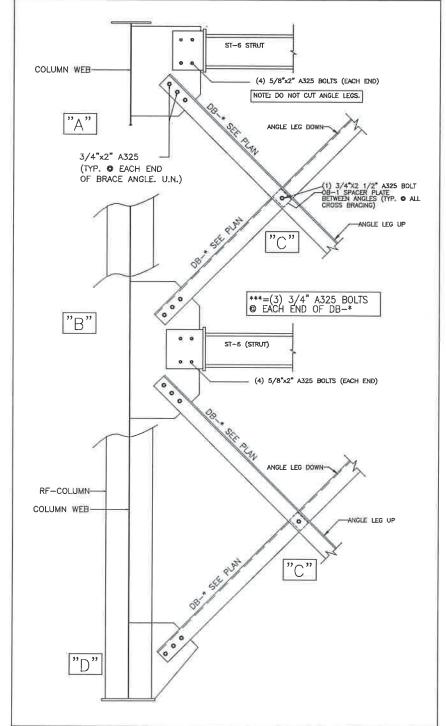
ERWIN HANGAR

→1 ^{4"} 26 GA. PBR.	30'-2" GALVALUME	-1 ³ / ₃ /1- 12 3 4'-11 3/16"	30'-7"	D/F CAP6 8 11/16" 8 11/16" 1 10" 10"	30'-7"	3" - - 3" 12 @ 4'-11 3/16"	30'-2"	411-
7'-4" 6'-0" 6'-0" FB475A(2) FR74-1	FB505A(2) FB435A(1) Strut RF4-2 -7 3/4"	FB350A(1) FB250A(1) Strut	FB345A(1) FRF4-3	B390A(1) Strut RF4-3 O RF4-3 O RF4-3 O RF4-3 O O O O O O O O O O O O O	Strut	FB350A(1) S C C S	FB435A(1) FB505A(2) Trut RF4-2 4'-7 3/4"	FB475A(2) FB475A(2) FB475A(2) FB475A(2) FB475A(2)

RIGID FRAME ELEVATION: FRAME LINE 4

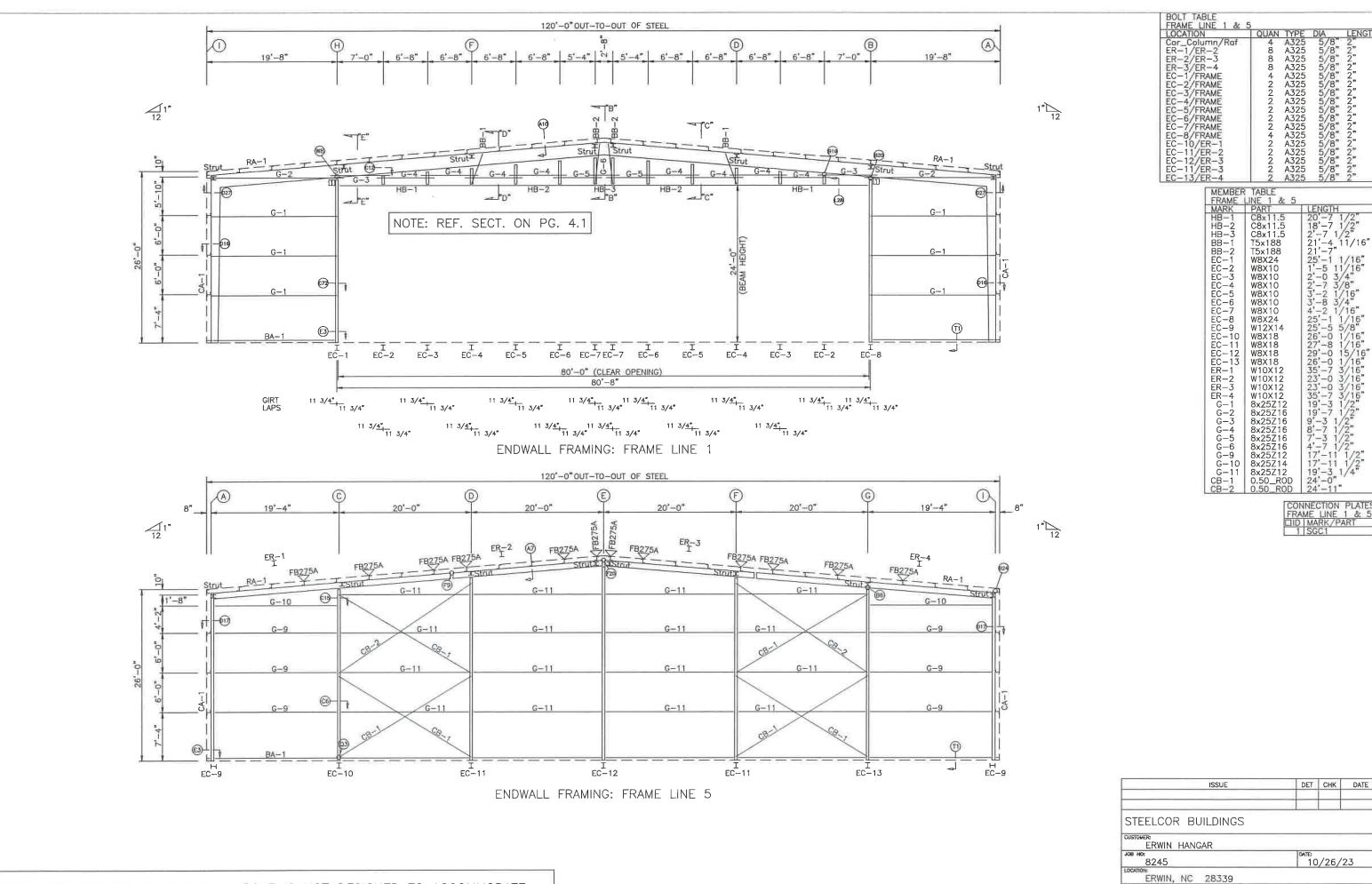




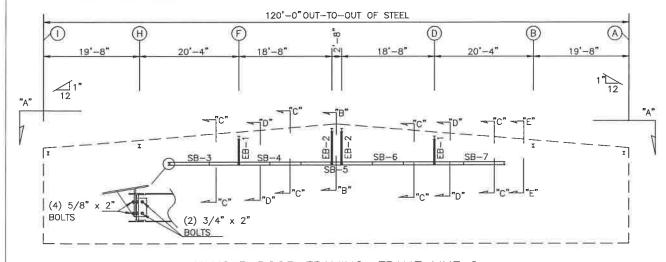


T	SPECIAL	BOLT	S		
	O ID	QUAN	TYPE DIA	LENGTH	WASH
L	2	4	A325 5/	8" 2"	0
	ME	MBER	TABLE		
	FR	AME L	INE A & I		
	MA	RK	PART	LENGTH	- 20
	ST	-6	8X7DC12	20,-5 1/	2"
	, , ,	-12	8x25Z16	11,-11,1	/2
		-13 -14	8x25Z16 8x25Z14	25,-/ 1/	2,,
	"	-14 -15	8x25Z14	23 -7 1/	2"
	l l DB		L3X3X188	24'-0 1/	2"
	1 1 2 2	: I	L3X3X188	24'-0 5/	16"
	DB	$-\overline{3}$	L3X3X188	23'-6 1/	2"

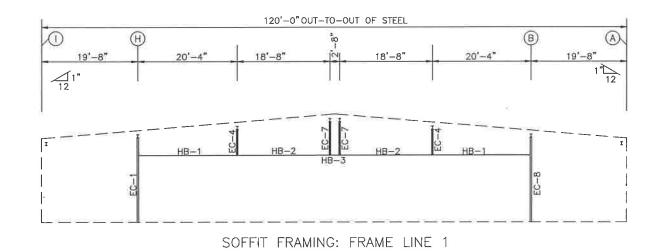
!	SSUE	DET	СНК	DATE
STEELCOR BUIL	DINGS			
ERWIN HANGAR				
108 NO: 8245		DATE: 10	/26/	23
ERWIN, NC 28	3339			
SIDEWALL FRAN	ING LAYOUT			
PAGE 3	DRAWN BY:	CHECKED BY	: SC	ALE: NONE

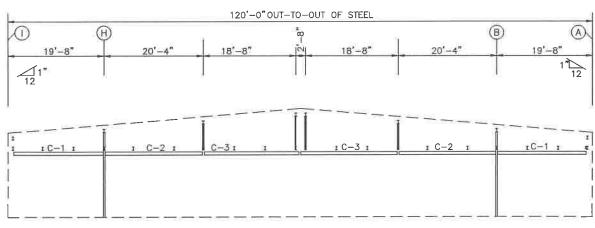


NOTE: THE FRAMING AS DEPICTED ABOVE IS NOT DESIGNED TO ACCOMMODATE ANY FUTURE EXPANSION.



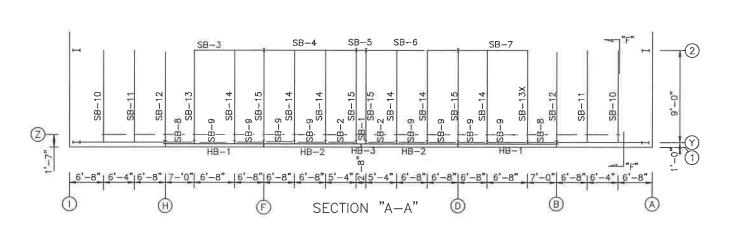
HANGAR DOOR FRAMING: FRAME LINE 2





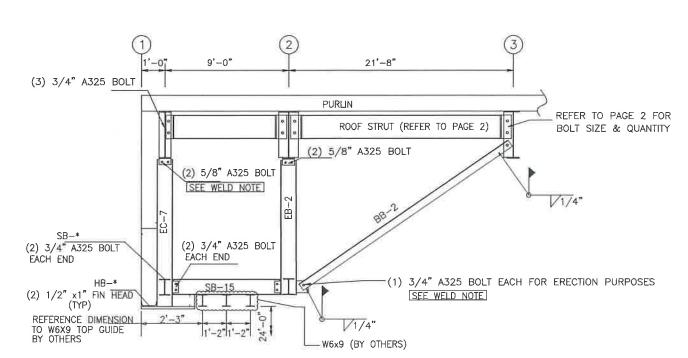
SOFFIT FRAMING: FRAME LINE Z

MEMBER	TABLE	
FRAME	LINE 1.1	
MARK	PART	LENGTH
SB-1	W8X10	2'-6 13/16"
SB-2	W8X10	5'-2 13/16"
		17' 0 17/16"
SB-3	W8X10	13'-2 13/16"
SB-4	W8X10	18'-6 13/16"
SB-5	W8X10	2'-6 13/16"
SB-6	W8X10	18'-6 13/16"
SB7	W8X10	13'-2 13/16"
SB-8	W8X10	6'-10 13/16"
SB-9	W8X10	6'-6 13/16"
SB-10	W8X10	8'-11"
SB-11	W8X10	8'-11"
SB-12	W8X10	8'-3 1/16"
SB-13	W8X10	
SB-13X		8'-11 1/8"
SB-14	W8X10	8'-7 3/8"
SB-15	W8X10	8'-3 1/8"
EB-1	W8X10	1'-11 7/16"
EB-2	W8X10	2'-8 11/16"
C-1	6x25C16	13'-3 1/2"
C-2	6x25C16	20'-3 1/2"
$10-\overline{3}$	6x25C16	19'-11 1/2"
	1	

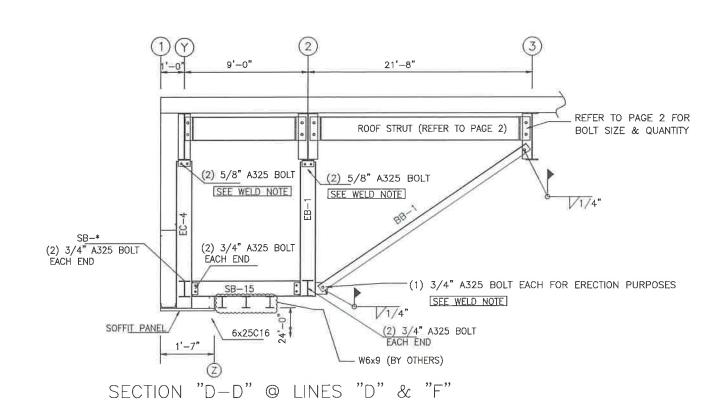


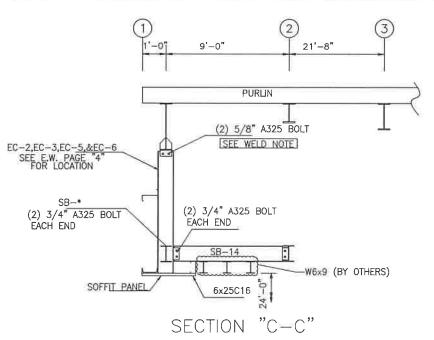
NOTE: REF SECT. DWGS @ PAGE 4.2

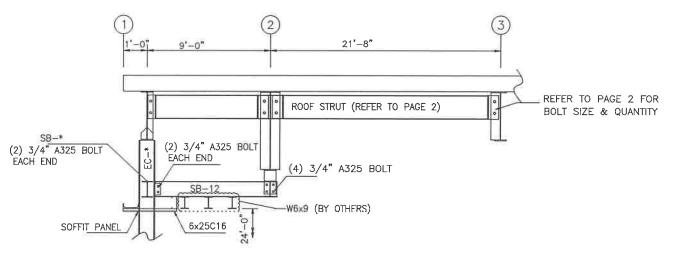
ISS	ISSUE		СНК	DATE
STEELCOR BUILD	INGS			
CUSTOMBRE ERWIN HANGAR				
лов но: 8245		10)	/26/	23
ERWIN, NC 28	339			
HANGAR DOOR	FRAMING			
DRAWING NO: PAGE 4.1	DRAWN BY: DAR	RAW	SCA	NONE



SECTION "B-B" @ PEAK



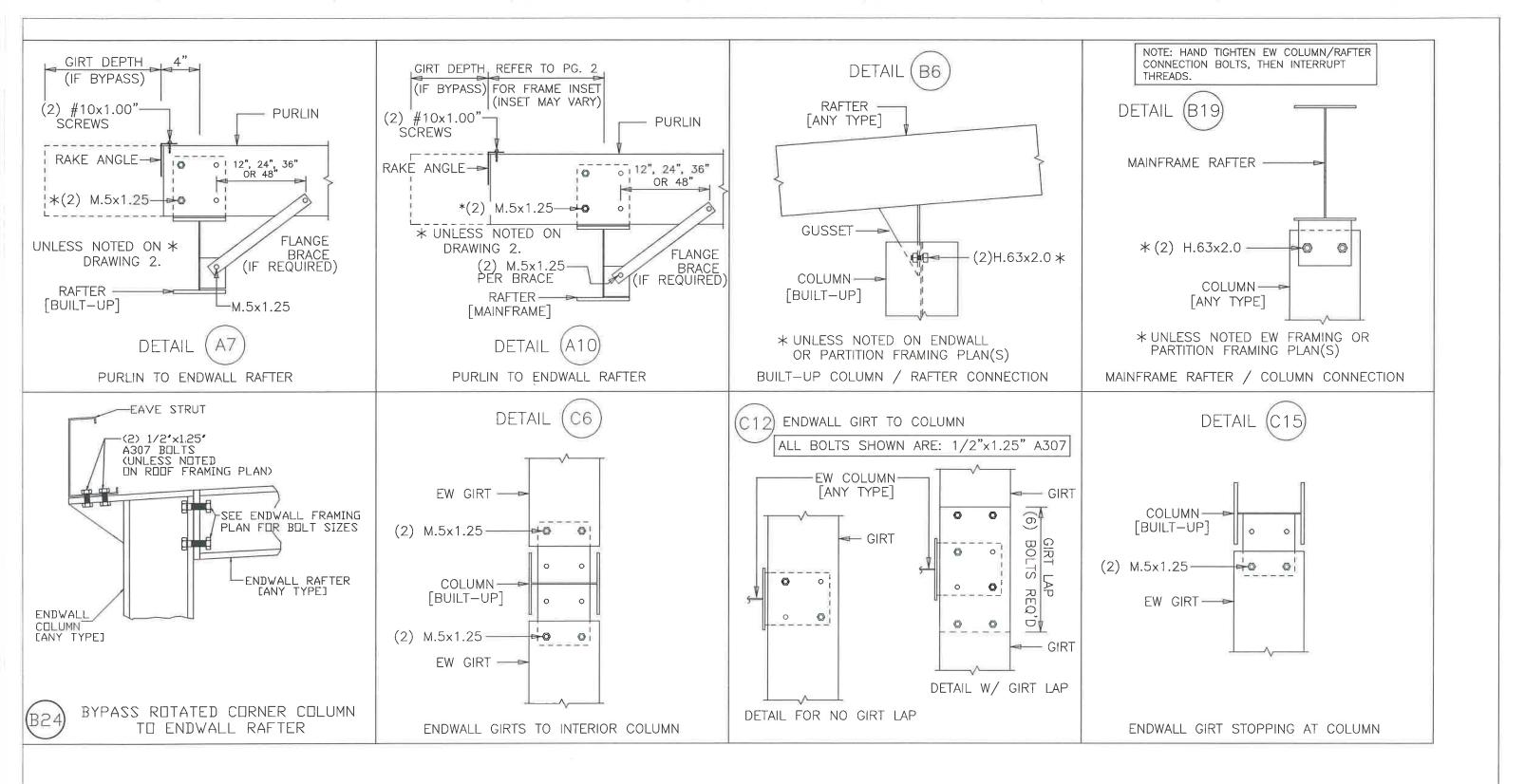




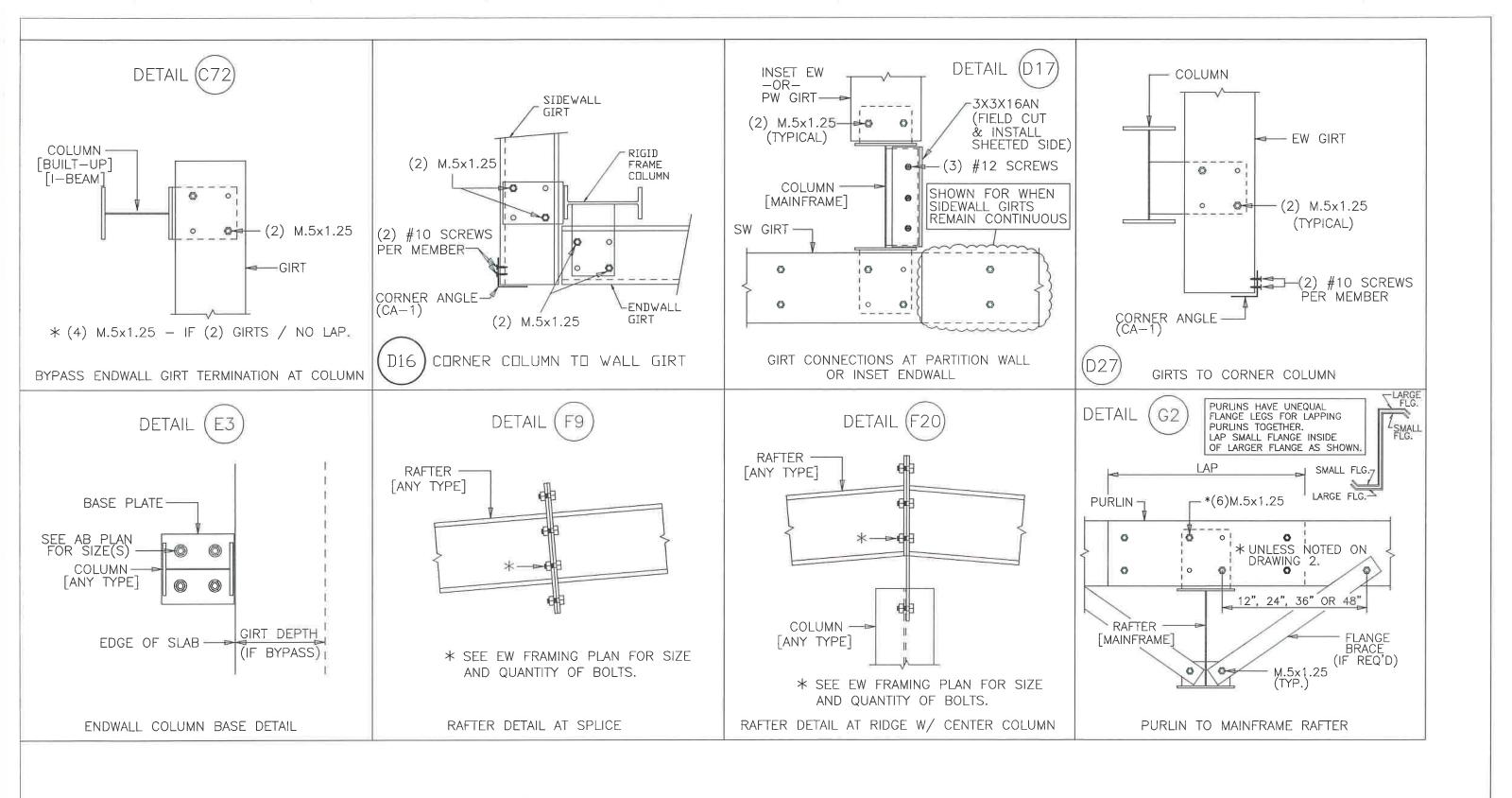
SECTION "E-E" @ LINES "B" & "H"

WELD NOTE: FIELD WELD ONCE ALL DEAD LOAD IS APPLIED

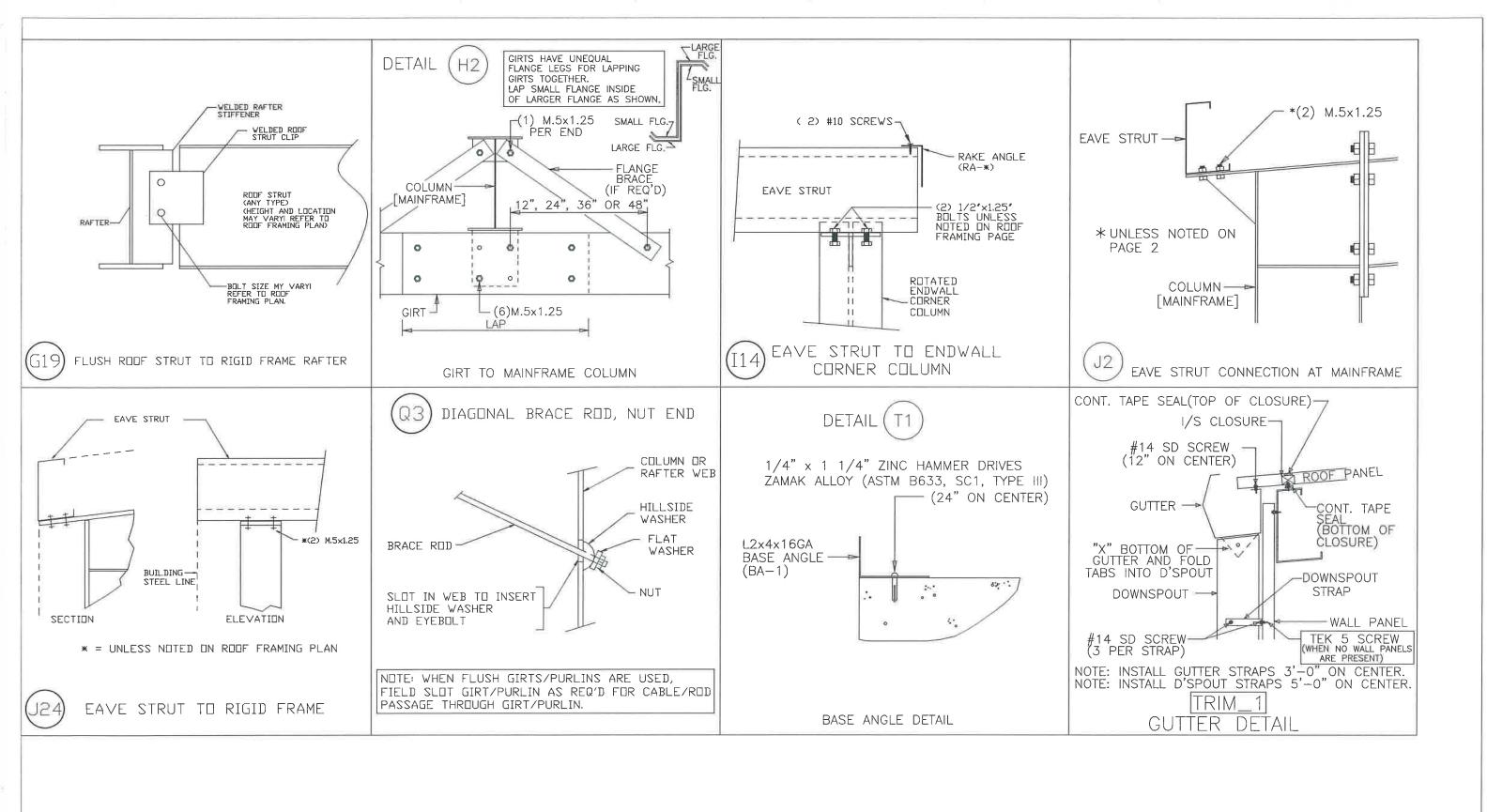
ISSUE		0	ET C	нк	DATE
STEELCOR BUILDIN	GS				
ERWIN HANGAR					
8245		DA	TE 10/2	26/2	23
ERWIN, NC 2833	9				
PAWING MAME! HANGER DOOR DE	TAILS				
PAGE 4.2	DAR	RAW		SCA	E NONE



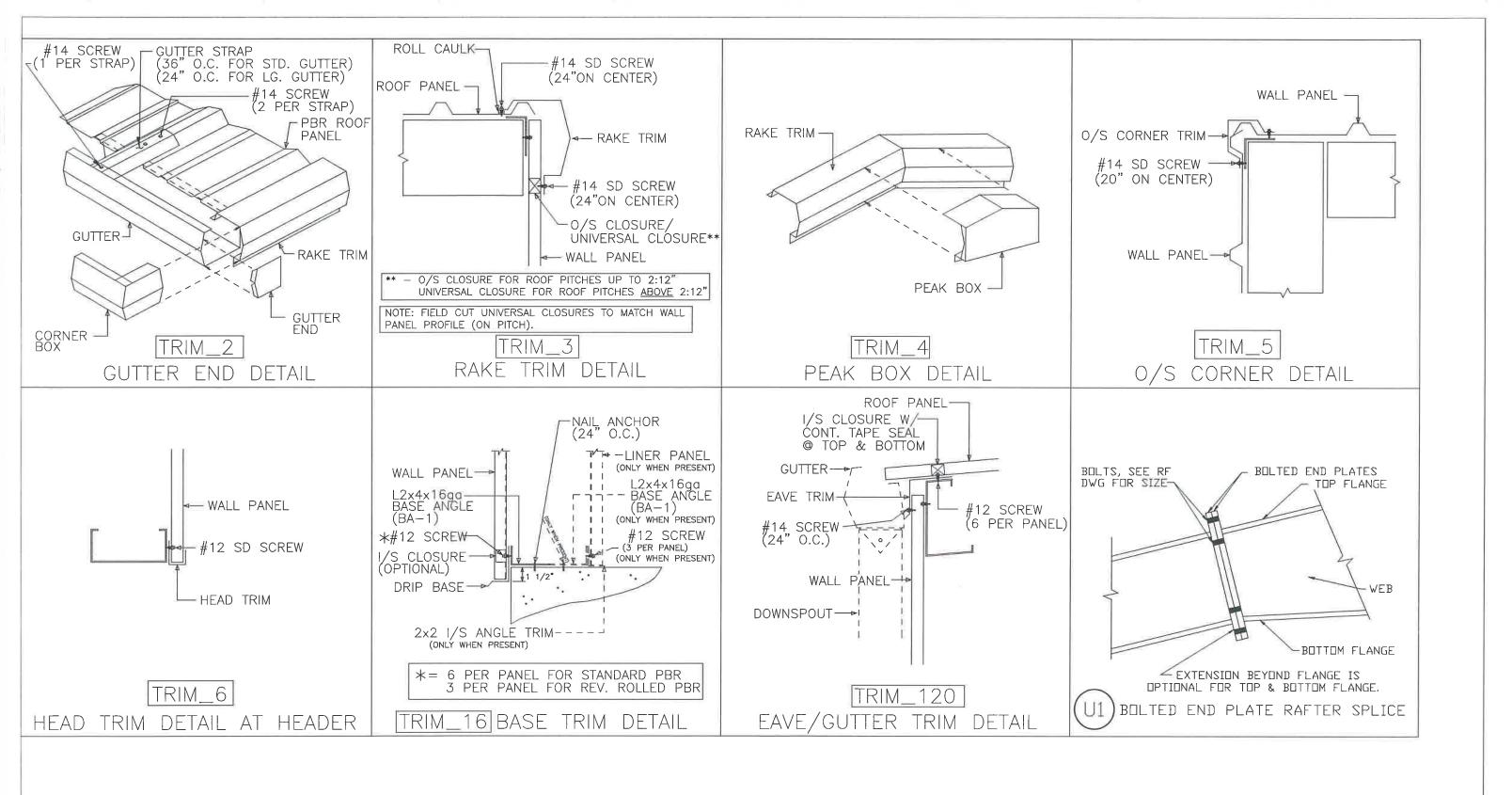
ISSUE		DET	СНК	DATE
STEELCOR BUILDIN	IGS			
ERWIN HANGAR				
ов NO: 8245		10	/26/:	23
ERWIN, NC 2833	9			
FRAMING DETAILS				
PAGE 5	DRAWN BY: DAR	RAW	r: SCA	NONE



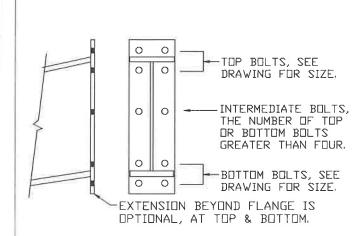
SUE	DET	СНК	DATE
	_		
DINGS			
	77.		
	DATE:	/26/	23
339			
S			
DAR	RAW RAW	r: SC	NONE
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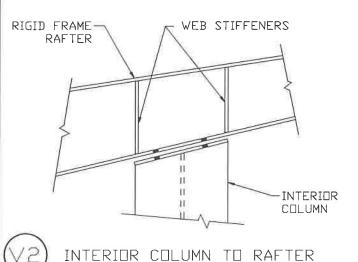
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STEELCOR BUILD	INGS			
ERWIN HANGAR				
008 NO: 8245		DATE: 10,	/26/	23
ERWIN, NC 28	339			
FRAMING DETAIL	S			
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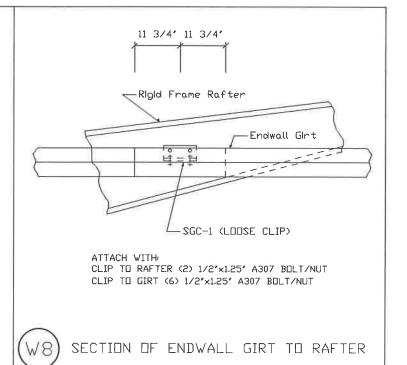


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STEELCOR BUILDIN	NGS			
ERWIN HANGAR				
NOB NO: 0345			/26/	′23
ERWIN, NC 2833	19			
FRAMING DETAILS				
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NOTE: REFER TO RIGID FRAME CROSS SECTION(S) FOR BOLT SIZE/QNTY!





STRUCTURAL BOLTED CONNECTIONS

BOLTED END PLATE CONNECTION

REFER TO COVER PAGE "GENERAL NOTES" PARAGRAPH "C", SECTION "9" FOR INSTRUCTIONS ON TIGHTENING ALL A325 AND A490 CONNECTION BOLTS.

TRIM NOTES:

- [1] SEAL TRIM SPLICES WITH TUBE CAULK.
- [2] SECURE GUTTER SPLICES AND END PLUGS WITH RIVETS.
- [3] SECURE ALL OTHER ROOF TRIM SPLICES WITH TRIM SCREWS UNLESS NOTED OTHERWISE.
- [4] TRIM SCREWS ARE LOCATED 24" ON CENTER UNLESS NOTED OTHERWISE.
- [5] STD. TRIM SPLICES ARE 3" TOTAL UNLESS NOTED OTHERWISE.

MORTISE PREPPED PERSONNEL DOORS

ALL MORTISE PREPPED PERSONNEL DOORS COME AS RIGHTHAND REVERSED SWING.

(i.e. STANDING ON THE OUTSIDE OF THE BUILDING FACING THE DOOR, THE LOCK WILL BE ON THE LEFTHAND SIDE OF THE DOOR AND THE DOOR WILL SWING OUTWARD FROM THE BUILDING.)

ANY FIELD MODIFICATIONS ARE THE RE-SPONSIBILITY OF THE ERECTOR AND MBM IS NOT LIABLE FOR LABOR CHARGES NOR DAMAGES DUE TO ERROR.

STEELCOR BUILDINGS

CUSTOMER:
ERWIN HANGAR

JOB NO:
8245

DET CHK DATE

CHK DATE

CHK DATE

CHK DATE

DATE

10/26/23

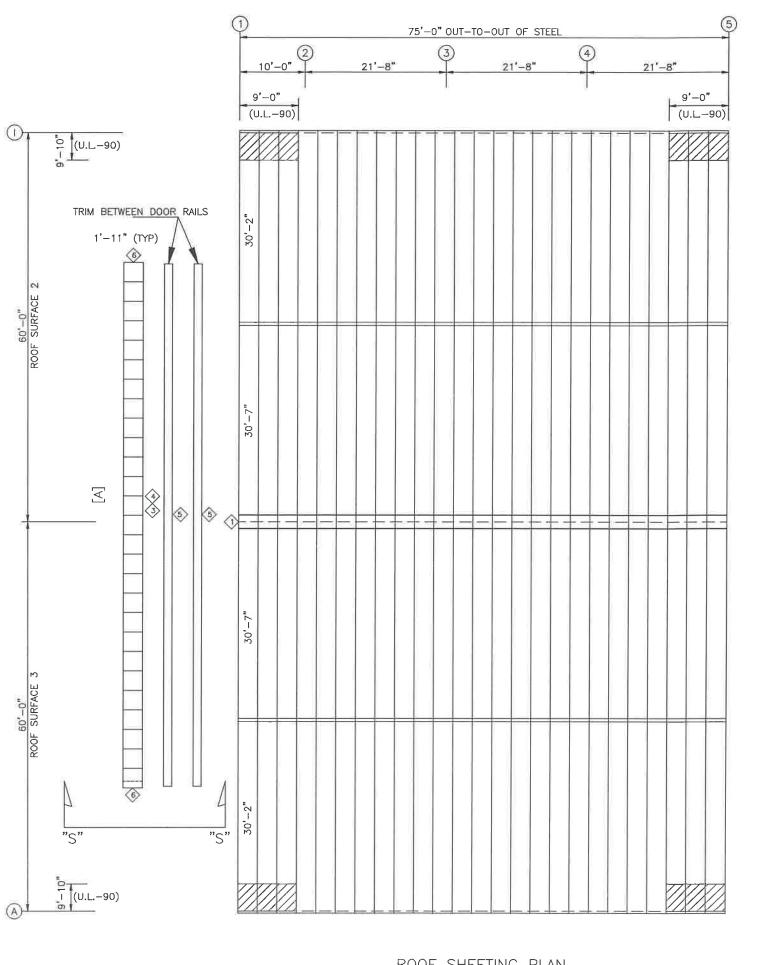
ERWIN, NC 28339
FRAMING DETAILS

PAGE 5.4

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ROOF SHEETING PLAN

PANELS: 26 GA. PBR - GALVALUME

[A] SOFFIT PANELS: 26 GA. PBR - KOKO BROWN

TRIM TABLE
ROOF PLAN

◇ID PART LENGTH

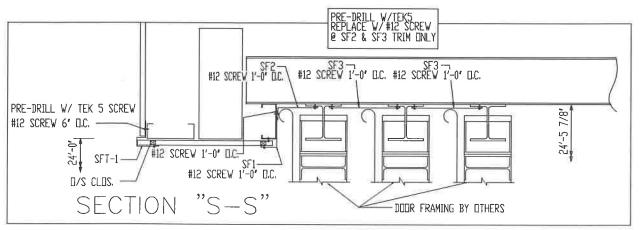
1 D/F CAP6 3'-0"

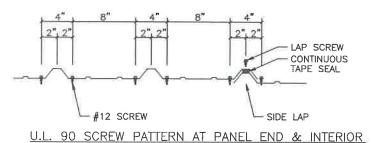
3 SF1 20'-3"

4 SF2 20'-3"

5 SF3 20'-3"

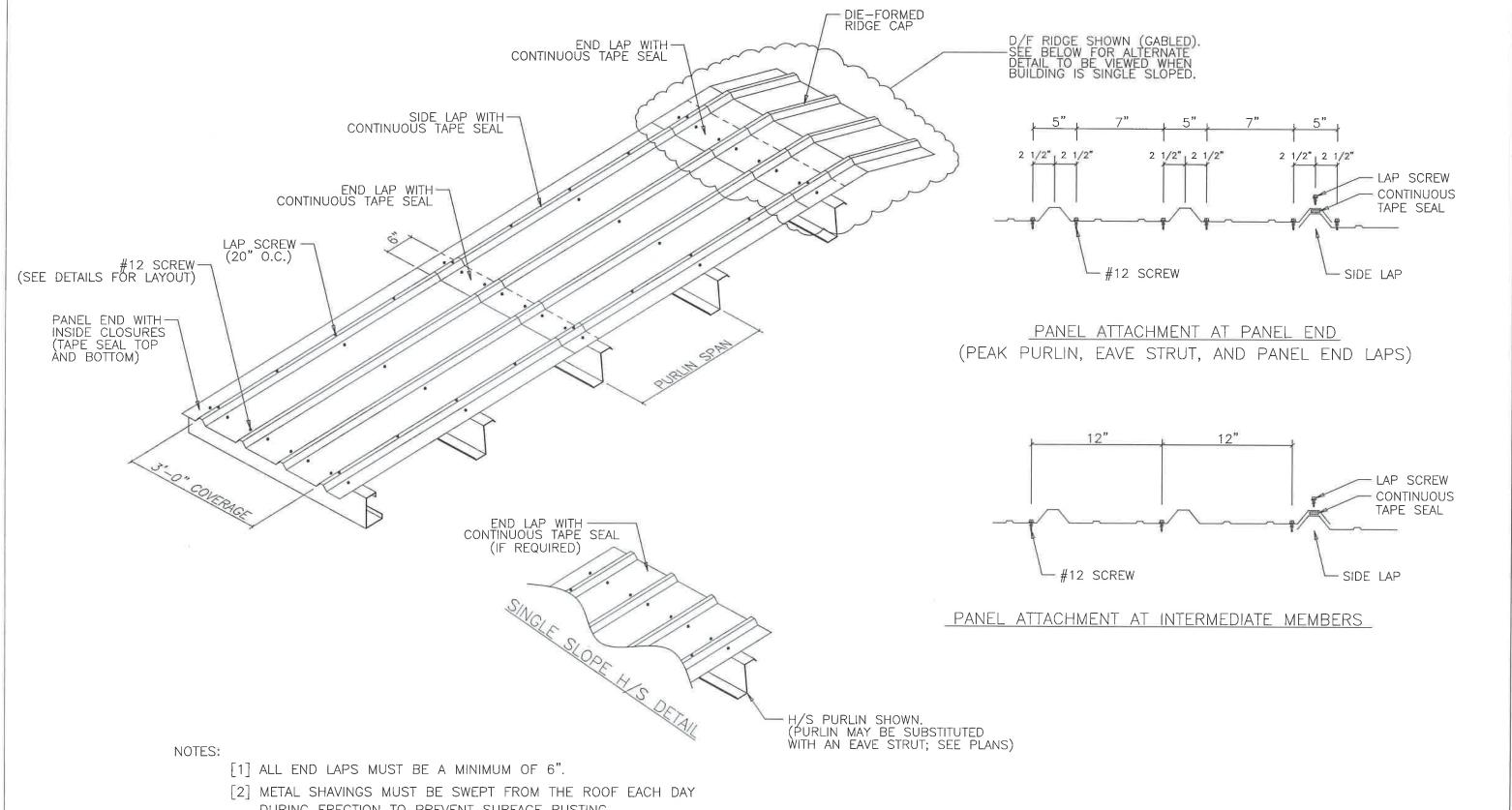
6 R JAMB 1'-10"





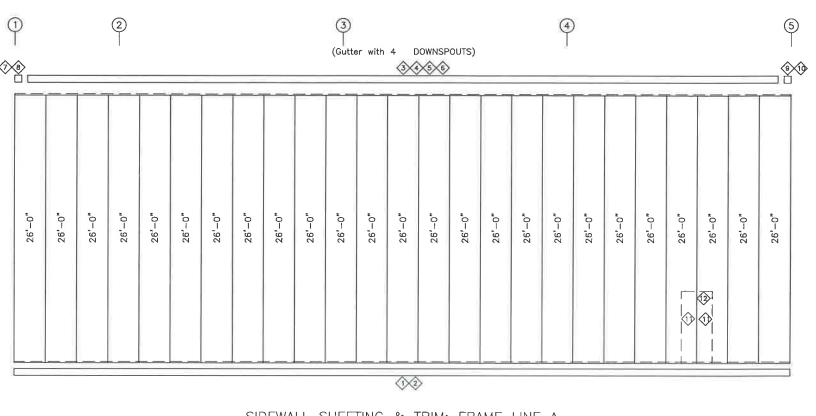
= U.L. 90 SCREW PATTERN @ HATCHED IN AREAS WITHIN DIMENSIONS SHOWN ON ROOF SHEETING PLAN.

ISSU	JE	DET	СНК	DATE
STEELCOR BUILD	NGS			
CUSTOWER: ERWIN HANGAR				
JOB NO: 8245		DATE:	/26/2	23
ERWIN, NC 283	39			
ROOF PANELS &C	TRIM			
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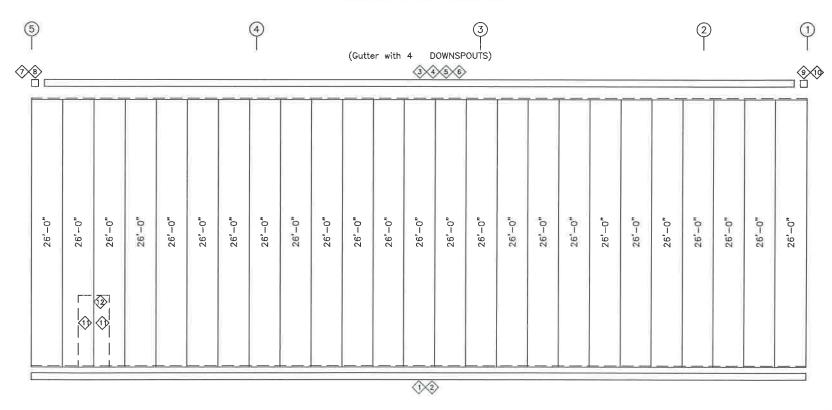


- DURING ERECTION TO PREVENT SURFACE RUSTING.
- [3] TAPE SEAL MUST BE APPLIED WITH NO GAPS OR BREAKS.
- [4] #12 SCREWS ARE USED TO ATTACH THE PANEL TO THE PURLINS. #14 LAP SCREWS ARE USED AT THE PANEL-TO-PANEL ATTACHMENTS. ALL FASTENERS ARE SELF-DRILLING.

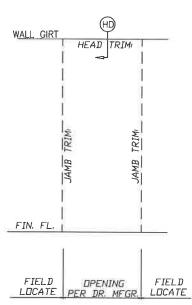
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STEELCOR BUILD	INGS			
ERWIN HANGAR				
JOB NO:		DATE:		
8245		10	/26/	23
LOCATIONS				
ERWIN, NC 283	339			
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ROOF PANEL DE	TAILS			
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SIDEWALL SHEETING & TRIM: FRAME LINE A
PANELS: 26 GA. PBR - SADDLE TAN

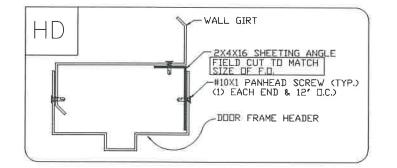


SIDEWALL SHEETING & TRIM: FRAME LINE I
PANELS: 26 GA. PBR - SADDLE TAN



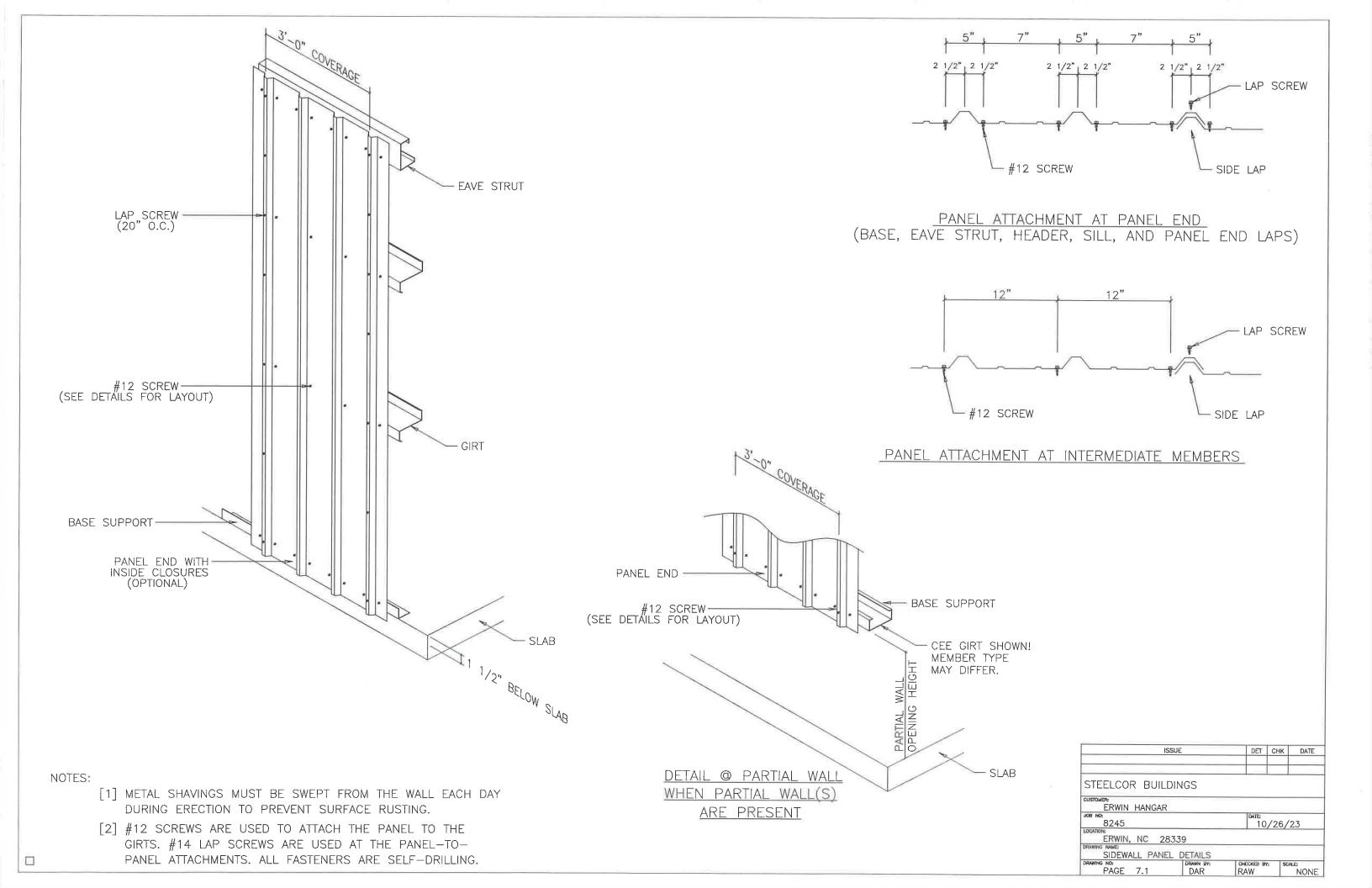
3070 TRIM KIT

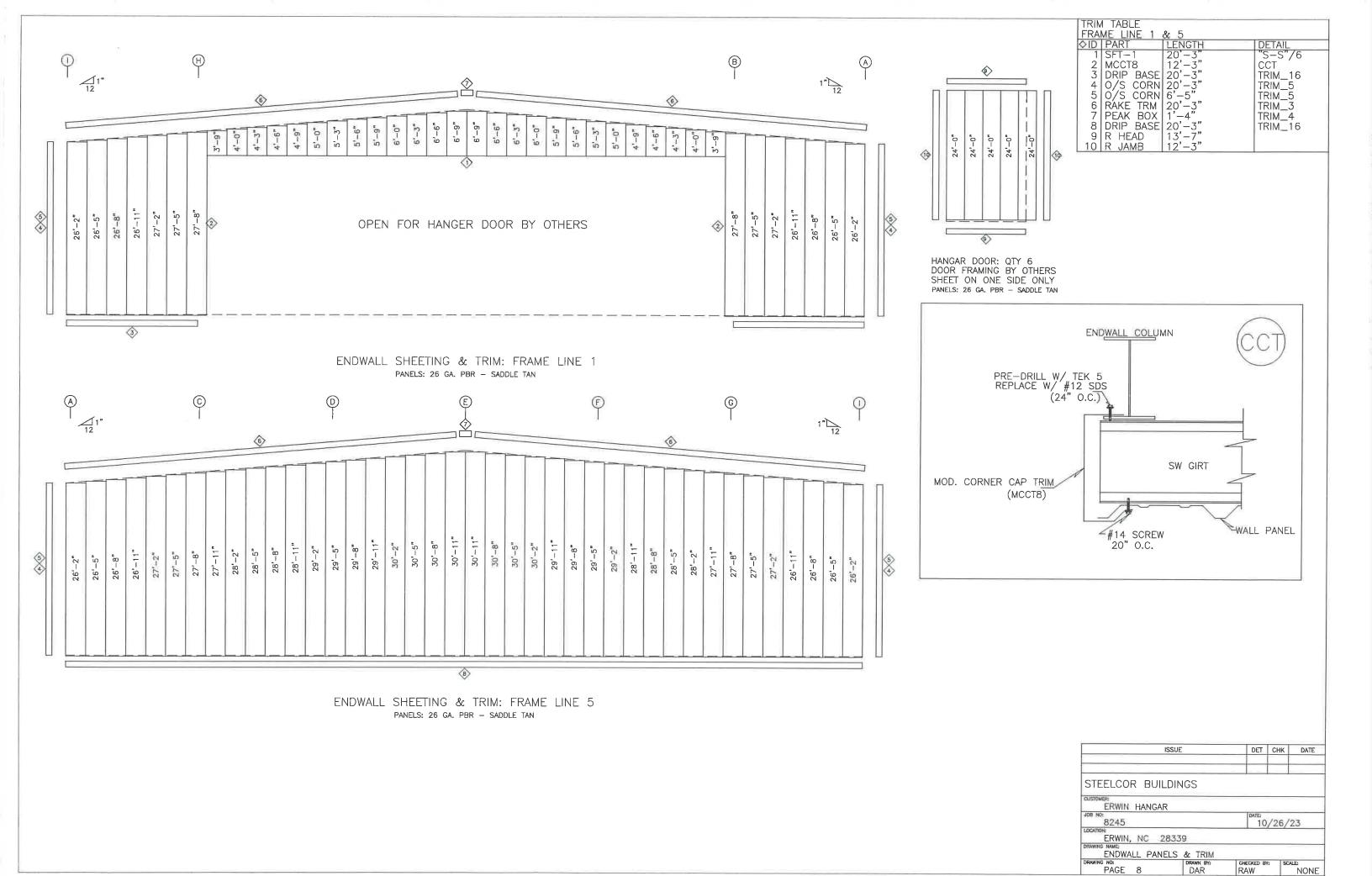
- 1. FIELD CUT AND WORK GIRTS, PANELS, AND TRIM AS REQUIRED.
- 2. REFER TO DETAIL PAGES FOR APPLICABLE TRIM DETAILS.(DETAIL PAGE 5.1)

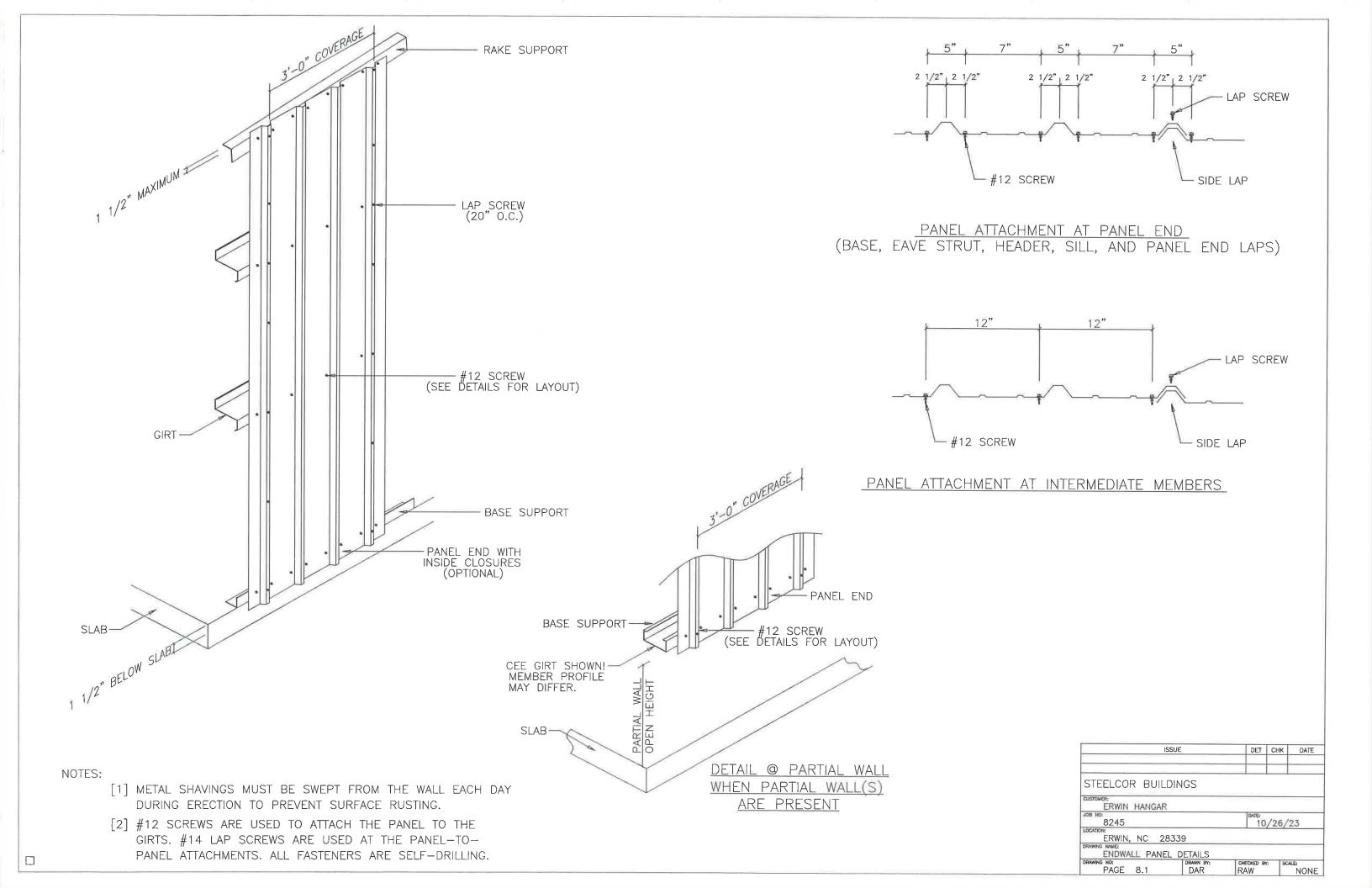


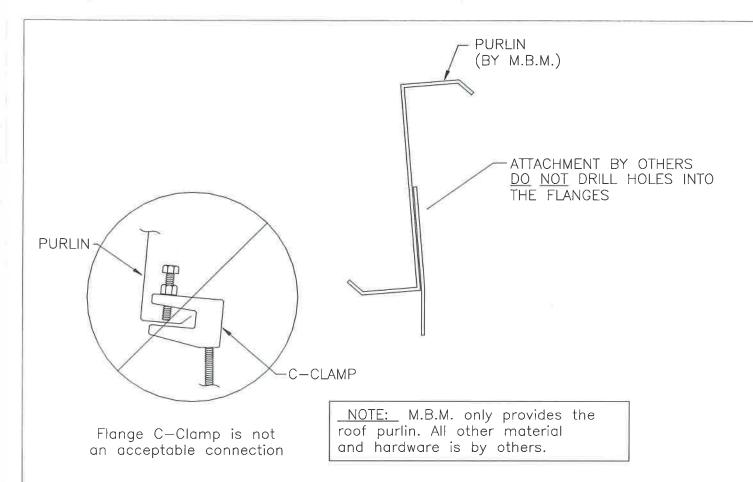
TRIM TABLE	& I	
FRAME LINE A OID PART 1 DRIP BASE 2 DRIP BASE 3 GUTTER 4 GUTTER 5 EAVE TRM 6 EAVE TRM 7 GUTEND L 8 CORBOX L 9 GUTEND R	LENGTH 20'-3"	DETAIL TRIM_16 TRIM_16 TRIM_1 TRIM_1 TRIM_1 TRIM_120 TRIM_120 TRIM_2 TRIM_2 TRIM_2 TRIM_2 TRIM_2
10 CORBOX R 11 R JAMB 12 R HEAD	1'-0" 7'-3" 3'-3"	TRIM_2 TRIM_8 TRIM_61

ISSUE		DET	СНК	DATE	
TEELCOR BUILD	INGS	94/			
ERWIN HANGAR					
8245			10/26/23		
ERWIN, NC 283	339				
WING NAME: SIDEWALL PANEL	S & TRIM				
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Recommended Connection Detail

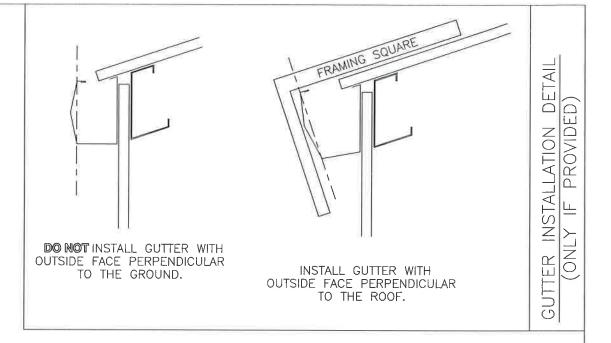
NOTE

MANY FACTORS BEYOND THE CONTROL OF THE METAL BUILDING SUPPLIER AFFECT THE ABILITY OF A PURLIN TO SAFELY SUPPORT HANGING LOADS COMBINED WITH OTHER REQUIRED ROOF LOADS. DUE TO THE VARIABLES INVOLVED IN HANGING LOADS AND THEIR ATTACHMENTS TO THE PURLINS, THE METAL BUILDING SUPPLIER CANNOT ASSURE THAT THE PURLINS FOR A PARTICULAR BUILDING PROJECT CAN SAFELY SUPPORT THE MAXIMUM ALLOWABLE HANGING LOADS IN COMBINATION WITH OTHER ROOF LOADS.

IT IS THE RESPONSIBILITY OF THE HANGER SYSTEM INSTALLER TO COORDINATE WITH THE ENGINEER OF RECORD FOR THE OVERALL PROJECT TO ENSURE A SAFE HANGING LOAD INSTALLATION. THE METAL BUILDING ENGINEER IS NOT THE ENGINEER OF RECORD FOR THE OVERALL PROJECT. WITHOUT SPECIFIC CERTIFICATION FOR INDIVIDUAL HANGING LOADS, THE NET EFFECTS OF APPLIED HANGER LOADS INSTALLED ON A PARTICULAR PURLIN SHALL NOT EXCEED THE NET EFFECTS OF THE CERTIFIED UNIFORMLY APPLIED DESIGN COLLATERAL LOAD.

HANGING LOADS SHOULD NOT BE APPLIED TO THE PURLIN LIP. WHERE PERMISSIBLE, THE BEST PRACTICE FOR HANGING LOADS IS TO ATTACH TO THE PURLIN WEB USING A BOLT AND NUT, OR SELF—DRILLING SCREWS.

HANGING UNIFORM LOADS SUCH AS SPRINKLER MAINS OR HVAC EQUIPMENT SHOULD BE DISTRIBUTED OVER SEVERAL PURLINS, AND SHOULD NEVER EXCEED THE COLLATERAL LOAD ALLOWANCE FOR THE ROOF SYSTEM. FOR UNIFORM LOADS THAT RUN PARALLEL TO THE PURLINS, IT MAY BE NECESSARY TO USE TRANSVERSE SUPPORT CHANNELS(A.KA. TRAPEZE BEAMS) ATTACHED TO THE WEBS OR FLANGES OF ADJACENT PURLINS TO SPREAD THE LOAD BETWEEN TWO OR MORE PURLINS. IN SUCH CASES, CONTACT THE BUILDING MANUFACTURER OR A LOCAL PROFESSIONAL ENGINEER PRIOR TO ATTEMPTING TO HANG LOADS FROM THE PURLINS



ISSUE DET CHK DATE

STEELCOR BUILDINGS

CUSTOMER:
ERWIN HANGAR

JOB NO:
8245

LOCATION:
ERWIN, NC 28339

DRAWING NAME:
SPECIAL DETAILS

DRAWN BY: CHECKED BY: SCALE:
PAGE 9 DAR RAW NONE

