# ERECTION NOTES

- 1. All bracing shown and provided by the Metal Building Provider (MBP) for this building is required and shall be installed by the erector as a permanent part of the structure ("Code of Standard Practice for Steel Buildings" in the ANSI/AISC 303-16; Section 7.10).
- 2. Temporary supports, such as guys, braces, falsework, cribbing or other elements required for the erection operation shall be determined and furnished by the erector ("Code of Standard Practice for Steel Buildings and Bridges" in the ANSI/AISC 303-16; Section 7.10.3).
- 3. Normal erection operations include the correction of minor misfits by moderate amounts of reaming, grinding, welding or cutting, and the drawing of elements into line through use of drift pins. Errors which require major changes in the member configuration are to be reported immediately to the Metal Building Provider by the customer to enable whoever is responsible either to correct the error or to approve the most efficient and economic method of correction to be used by others ("Code of Standard Practice for Steel Buildings and Bridges "in the ANSI/AISC 303-16; Section 7.14).
- Erection tolerances are set forth in the "Code of Standard Practice for Steel Buildings and Bridges" in the ANSI/AISC 303-16; Section 7.13 note that individual members are considered plump, level and aligned if the deviation does not exceed 1:500. Variations in finished overall dimensions of structure steel framing are deemed within the limits of good practice when they do not exceed the cumulative effect of rolling, fabricating, and erection tolerances.
- 4.1. When crane support systems are part of the metal building system erection tolerances Section 6.8. Erection Tolerances. 2018 MBMA Metal Building Systems manual shall apply. To achieve the required tolerances arouting of the columns and shimming of the runway beams may be required. The customer shall provide grout if required. The contractor erecting the runway beams is responsible for shimming, plumbing, and leveling of the runway system. When aligning the runway beams the alignment shall be with respect to the beam webs so that the center of the aligned rail is over the runway web.
- 5. As a general rule field welding is not used to assemble a metal building system. In cases where the drawings indicate field welding and in cases where approved corrections are to be made by field welding the following requirements shall be met; 5.1.
- welders must be qualified by an independent testing agency, with suitable documentation to AWS D1.1 Structural Welding Code -Steel or AWS D1.3 Structural Welding Code - Sheet as applicable, for the processes, positions, and materials involved. 5.2. All welds must be made in conformance to a documented and approved Welding Procedure Specification (WPS). All joints which
- are not pregualified must be supported by a certified Procedure Qualification Record (PQR) by an independent testing agency. 6. All documentation and records shall be the responsibility of the customer.
- 7. Any claims or shortages by buyer must be made to the Metal Building Provider within seven (7) working days after delivery, or such claims will be considered to have been waived by the customer and disallowed. All claims should be directed to the Metal Building Provider's Customer Service Department.
- 8. Claims for correction of alleged misfits will be disallowed unless the Metal Building Provider shall have received prior notice thereof and allowed reasonable inspection of such misfits. Ordinary inaccuracies of shop work shall not be construed as misfits. No part of the building may be returned or charges assessed for alleged misfits without prior approval from the Metal Building Provider.
- Neither the Metal Building Provider nor the customer will cut, drill or otherwise after their work, or the work of other trades to accommodate other trades unless such work is clearly specified in the contract documents. Whenever such work is specified the customer is responsible for furnishing complete information as to materials, size, location, and number of alterations prior to preparation of shop drawings ("Code of Standard Practice for Steel Buildings and Bridges" in the ANSI/AISC 303-16, Section 7.15).
- 10. The Metal Building Provider Field Modifications Policy: 10.1. The Metal Building Provider will only be responsible for the field-modified parts designed and approved by the Metal Building Provider's Customer Service Department.
- 10.2. Any field modifications designed by third parties may not be approved by the Metal Building Provider and may limit the Metal Building Provider's warranty and liability.
- The Metal Building Provider makes no warranty and hereby disclaims any responsibility with respect to the design, engineering, or construction of any field-modified parts performed by third parties.
- 11. WARNING SOME PANÉLS AND TRIM PARTS ARE FURNISHED WITH A PROTECTIVE PEEL-OFF FILM. PARTS PROVIDED WITH THIS FILM CANNOT BE EXPOSED TO SUNLIGHT WITHOUT FIRST REMOVING THE FILM. THIS FILM MUST BE REMOVED PRIOR TO INSTALLATION. FILM MUST ALSO BE REMOVED FROM ALL NON EXPOSED PARTS WITHIN SIX MONTHS FROM FILM APPLICATION OR IRREPARABLE DAMAGE WILL OCCUR TO THE SURFACE CLAIMS WILL NOT BE ACCEPTED FOR THIS ISSUE.

### RESPONSIBILITIES

- 1. The Metal Building Provider Customer, hereafter referred to as the "customer," obtains and pays for all building permits, licenses, public assessments, paving or utility pro rata, utility connections, occupancy fees and other fees required by any governmental authority or utility in connection with the work provided for in the Contract Documents. The customer provides at his expense all plans and specifications required to obtain a building permit. it is the customer's responsibility to ensure that all plans and specifications comply with the applicable requirements of any governing building authorities.
- 2. The customer is responsible for identifying all applicable building codes, zoning codes, or other regulations applicable to the Construction Project, including the Metal Building system.
- 3. It is the responsibility of the customer to interpret all aspects of the End User's specifications and incorporate the appropriate specifications, design criteria, and design loads into the Order Documents submitted to the Metal Building Provider.
- 4. It is the responsibility of the Metal Building Provider to furnish the metal building system to meet the specifications including the design criteria and design loads incorporated by the Contractor into the Order Documents. The Metal Building Provider is not responsible for making an independent determination of any local codes or any other requirements not part of the Order Document.
- The Metal Building Provider's standard specifications apply unless stipulated otherwise in the Contract Documents. The Metal Building Provider design, fabrication, quality criteria, standards, practice, methods and tolerances shall govern the work any other interpretations to the contrary not with standing. it is understood by both parties that the customer is responsible for clarifications of inclusions or exclusions from the Architectural plans.
- 6. In case of discrepancies between the Metal Building Provider's structural steel plans and plans for other trades, the Metal Building Provider's shall govern ("Code of Standard Practice for Steel Buildings and Bridges" in the AISC 303-16; Section 3.3).
- 7. The customer is responsible for overall project coordination. All interface, compatibility and design considerations concerning any materials not furnished by the Metal Building Provider and the Metal Building Provider's steel system are to be considered and coordinated by the customer. Specific design criteria concerning this interface between materials must be furnished by the customer before release for fabrication or the Metal Building Provider's assumptions will govern.
- 8. Foundations, anchor rods, and anchor rod embedment are designed, furnished, and set by the customer in accordance with an approved drawing. Dimensional accuracy shall satisfy the requirements of Section 7.5 1 of "Code of Standard Practice for Steel Buildings and Bridges" in the AISC 303-16.
- 9. All other embedded items or connection materials between the structural steel and the work of other trades are located and set by the customer in accordance with approved location on erection drawings. Accuracy of these items must satisfy the erection tolerance requirements
- 10. The Metal Building Provider does not investigate the influence of the metal building system on existing buildings or structures. The End Customer assures that such buildings and structures are adequate to resist snow drifts, wind loads, or other conditions as a result of the presence of the metal building system.

### GENERAL SPECIFICATIONS

- 1. Wall and liner panels are an integral part of the structural system. Unauthorized removal of panels or cutting panels for framed openings not shown is prohibited.
- Oil-canning, a perceived waviness inherent to light gauge metal, may exist. This condition does not affect the structural integrity or the finish of the panel, and therefor is not a cause for rejection.
- 3. The Metal Building Provider's red-oxide and gray-oxide primer are designed for short term field protection from exposure to ordinary atmospheric conditions. Primed steel which is stored in the field pending erection should be kept free of the ground, and so positioned as to minimize water-holding pockets, dust, mud, and other contamination of the primer film. Repairs of damage to primed surfaces and/or removal of foreign material due to transportation (e.g. road salt, de-icing chemicals and other substances encountered during transportation that may accelerate deterioration of the primer or corrosion of the underlying steel), improper field storage, or site conditions are not the responsibility of the Metal Building Provider. (MBMA, 2018 MBSM, Section 4.2.4)
- 4. All bolts are 1/2" x 1-1/4" A307 unless noted. Refer to the erection drawings for specific framing connections and the cross-section(s) for main frame connections.
- 5. Unless noted otherwise on the frame cross section(s), all bolted joints with ASTM F3125 Grade A325 bolts are specified as snug-tightened joints in accordance with the specification for Structural Joints Using High-Strength Bolts, June 11, 2020. Installation Inspection requirements for Snug-Tight Bolts (Specification for Structural joints, Section 9.1) is suggested.
- 6. Unless noted otherwise, all bolted connections are designed as bearing type connections with bolt threads not excluded from the shear plane.
- 7. Any type of suspended or load inducing system(s) is prohibited if zero collateral and zero sprinkler loads are designated on the contract. This would include lights, duct work, piping, and insulation types other than 3" standard duty fiberglass blanket insulation, etc.

BUILDING DESIGN CODES Building Code: North Carolina Hot—rolled version: Cold—formed version:	Building <u>Code 2018</u> (IBC 15) <u>AISC 360-10</u> <u>AISI S100-12</u>
GENERAL LOADS Dead Load: Roof Collateral Load: Sprinkler Load: Roof Live Load: Tributary Live Load Reduction: Rainfall Intensity:	<u>2.00</u> psf <u>1.00</u> psf (Misc.) <u>0.00</u> psf <u>20.00</u> psf <u>YES</u> <u>6.54</u> in/hr (5-minute duration 5-year recurrence)
WIND LOAD Wind Load (3—sec gust) Vult: Vasd:	117_ mph 91 mph
V service: Exposure Factor: Wind Condition: Internal Pressure Coefficient : Edge Zone Width:	
SNOW LOAD Ground Snow Load : Roof Snow Load : Importance Factor: Exposure Factor: Thermal Factor: Slope Factor:	<u>15.00</u> psf <u>12.60</u> psf <u>1.00</u> <u>1.00</u> <u>1.20</u> <u>1.00</u>
DEFLECTION CRITERIA Main Frames Horizontal: <u>H/60</u> Main Frames Vertical: <u>L/180</u> Bearing Frame Rafter: <u>L/180</u> Endwall Columns: <u>L/180</u>	Roof Panels: <u>L/60</u> Purlins: <u>L/180</u> Wall Panels: <u>L/60</u> Girts: <u>L/90</u>

For components.claddings and MWFRS, deflections involving wind are based on 10 year serviceability wind pressures.

ISMIC LOAD
Risk Category:
Seismic Importance Factor :
Structural Response Acceleration (Ss):
Structural Response Acceleration(S1):
Site Class:
Design Spectral Response (Sds):
Design Spectral Response (Sd1):
Seismic Design Category:

Wind Frame Horizontal : H/60

Framing Direction: Structural Syst:

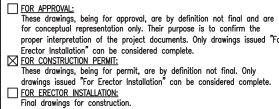
Response Modification Factor : Deflection Amplification Sesimic Response Coefficient (Cs): Design Base Shear V : Analysis Procedure:

ROOF PANEL Profile: <u>Super Span X</u> UL580 Class 90: <u>Yes</u> Clip Type if Standing Seam: _	Gauge: NO	26	_ Cold
WALL PANEL Profile: <u>Super Span X</u>	Gauge:	26	_ Cold
WAINSCOT PANEL Profile: <u>Super Span X</u>	Gauge:	26	_ Cold
PRIMARY FRAMING Built-Up & Hot-Rolled:	Gray	Oxide	Prime
SECONDARY FRAMING			

SECUNDART FRAMING	
Purlins, Eave Struts:	Pre-Galvanized
Girts, Light Gage Columns:	Pre-Galvanized
Light Gage Jambs & Headers:	Pre-Galvanized
Base Angle Finish:	Pre-Galvanized
-	

Hot-Dip Galvanizing conforms to the ASTM A123 specification Pre-Galvanized members conform to the ASTM A653. Grade 50. Coating G-90 specification.

The rigid frame at line 1&11 are designed as a non-expandable rigid frame. Corresponding frame reactions are calculated based upon actual tributary area.





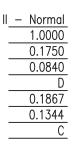
# APPROVAL SPECIFICATIONS

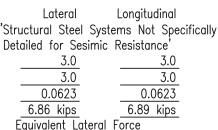
- 1. Approval of the Metal Building Provider drawings and/or calculations indicate that the Metal Building Provider has correctly interpreted the contact requirements. This approval constitutes the customer acceptance of the Metal Building Provider design, concepts, assumptions, and loadings.
  - 2. Failure to respond to clouded areas and areas to verify may result in additional costs and/or schedule delays for which the Metal Building Provider will not be responsible.
  - 3. Any changes made after the Metal Building Provider's customer has signed and returned the Metal Building Provider drawings and/or calculations and the project is released for fabrication shall be billed to the Metal Building Provider customer including material, engineering, and other costs. An additional fee may be charged if the project must be moved in the fabrication and/or the shipping schedule.
  - 4. It is the responsibility of the customer to field verify all existing conditions prior to fabrication.
- 5. It is imperative that any changes to these drawings:
- 5.1. Be made in contrasting ink. 5.2. Be legible and unambiguous.
- 5.3. Have all instances of changes clearly indicated.
- 6. A dated signature, in the designated areas, is required on all pages. The signature must be from the person authorized on the
- contract or a person authorized, in writing, by the Metal Building Provider customer. 7. The Metal Building Provider reserves the right to resubmit drawings with extensive or complex changes required to avoid misfabrication.
- This may impact the delivery schedule. 8. Any changes noted on the drawings not in conformance with the terms and requirements of the contract between the Metal Building
- Provider and its customer are not binding on the Metal Building Provider unless subsequently acknowledged and agreed to in writing by change order or separate documentation.
- 9. Waiving the approval process by designating the order "For Production" supercedes notes 1,2,5,6, and 8 in this section, and constitutes the customer acceptance of the Metal Building Provider's design, concepts, assumptions, and loadings.

DRAWING SCHEDULE								
DWG NO.	ISSUE	DATE	DESCRIPTION					
C1	P1	01.30.24	COVER SHEET					
F1	0	01.30.24	ANCHOR BOLT PLAN & DETAILS					
F2	0	01.30.24	ANCHOR BOLT REACTIONS					
F3	0	01.30.24	ANCHOR BOLT REACTIONS					
P1	P1	01.30.24	RIGID FRAME ELEVATION					
P2	P1	01.30.24	RIGID FRAME ELEVATION					
P3	P1	01.30.24	RIGID FRAME ELEVATION					
W1	P1	01.30.24	PORTAL FRAME ELEVATION					
W2	P1	01.30.24	PORTAL FRAME ELEVATION					
E1	P1	01.30.24	ROOF FRAMING PLAN					
E2	P1	01.30.24	ROOF SHEETING PLAN					
E3	P1	01.30.24	ENDWALL FRAME & SHEETING ELEVATION					
E4	P1	01.30.24	ENDWALL FRAME & SHEETING ELEVATION					
E5	P1	01.30.24	SIDEWALL FRAME & SHEETING ELEVATION					
E6	P1	01.30.24	SIDEWALL FRAME & SHEETING ELEVATION					
E7	P1	01.30.24	BUILDING SECTIONS					
D1	P1	01.30.24	STANDARD DETAILS PAGE					
D2	P1	01.30.24	STANDARD DETAILS PAGE					
D3	P1	01.30.24	STANDARD DETAILS PAGE					

TRIM COLOR:	
FL GUTTER: SMP Steel Gray	GAUGE: 26
FL RAKE: SMP Steel Gray	GAUGE: 26
CORNER: SMP Steel Gray	GAUGE: 26
ACCESSORY: SMP Steel Gray	GAUGE: 26
DOWNSPOUTS: SMP Steel Gray	GAUGE: 26
BASE: SMP Steel Gray	GAUGE: 26
WAINSCOT: SMP Steel Gray	GAUGE: 26

anels:	L/60
:	L/180
anels:	L/60
	L/90





lor: SMP Ash Gray

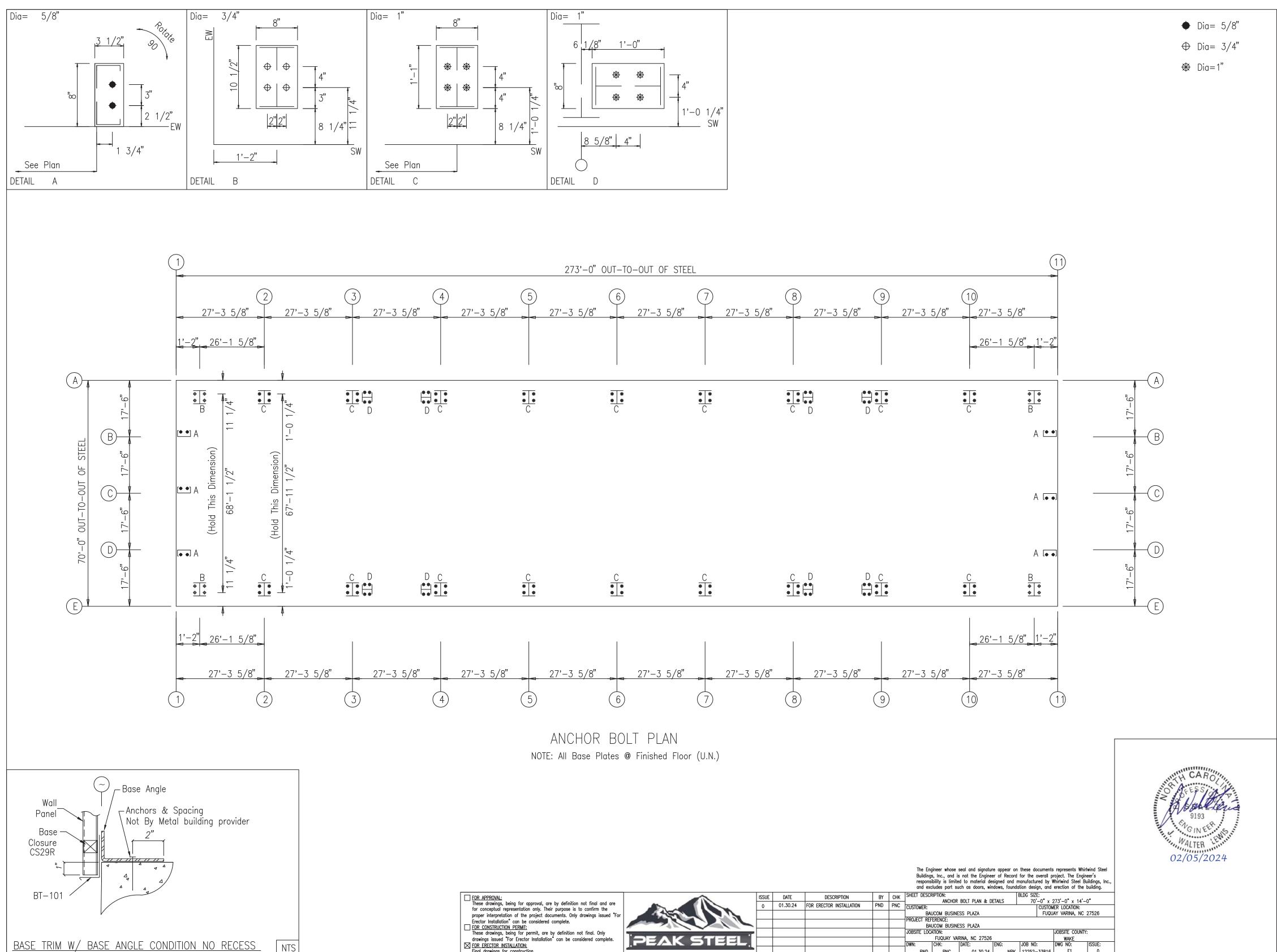
lor: SMP Hawaiian Blue

lor: SMP Steel Gray

### The Engineer whose seal and signature appear on these documents represents Whirlwind Steel Buildings, Inc., and is not the Engineer of Record for the overall project. The Engineer's responsibility is limited to material designed and manufactured by Whirlwind Steel Buildings, Inc. and excludes part such as doors, windows, foundation design, and

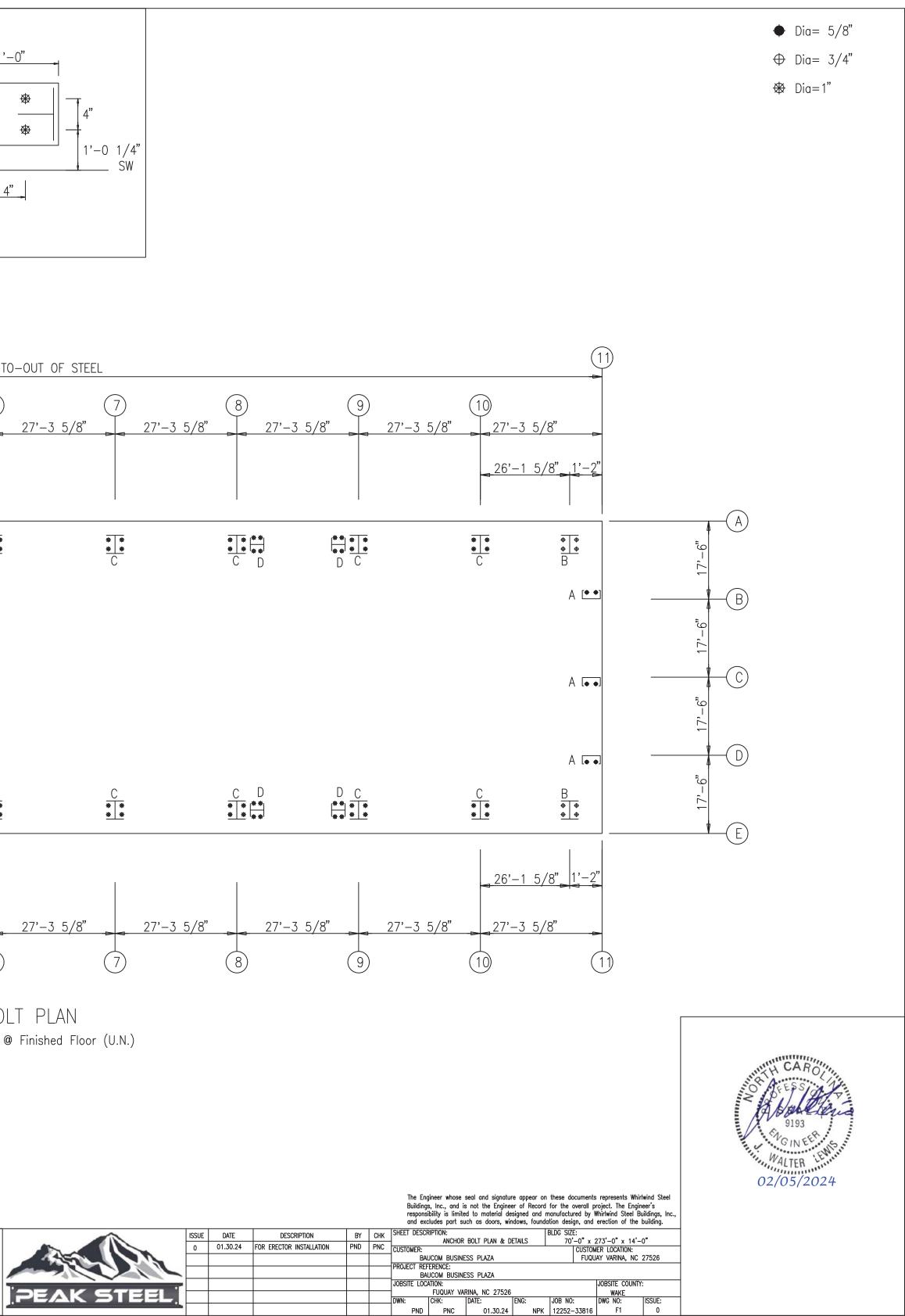
							addo part ou	on do dooro, wii	aono, rounaa	don dobign, and		building.	
	ISSUE	DATE	DESCRIPTION	BY	СНК	SHEET DESCR			f	BLDG SIZE:	مر روند م	<b>0</b> "	1
	P1	01.30.24	FOR CONSTRUCTION PERMIT	PND	PNC	COVER SHEET				70'-0" x 273'-0" x 14'-0" CUSTOMER LOCATION:			
						BAUCOM BUSINESS PLAZA PROJECT REFERENCE:				FUQUAY VARINA, NC 27526			
						1	JCOM BUSINE	SS PLAZA					
A REAL PROPERTY AND A REAL						JOBSITE LOCA				JOBSITE COUNTY:			
V GTEEL I							FUQUAY VARINA, NC 27526				WAKE		
AN DIEEL.						DWN:	CHK:	DATE:	ENG:	JOB NO:	DWG NO:	ISSUE:	٦.
						PND	PNC	01.30.24	NPK	12252-33816	C1	P1	





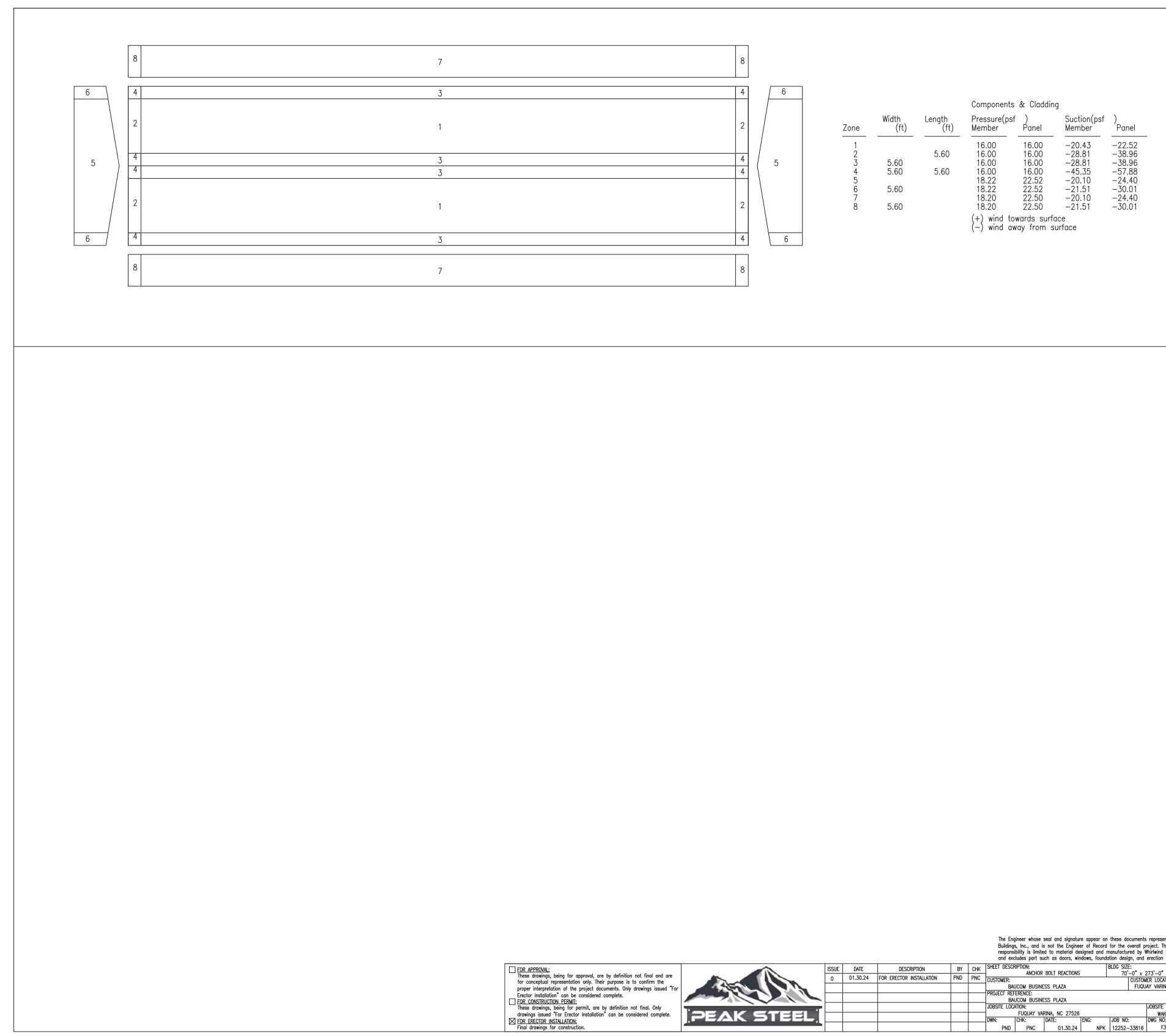
BASE TRIM W/ BASE ANGLE CONDITION NO RECESS

FOR ERECTOR INSTALLATION: Final drawings for construction



ENDWALL COLUMN: BASIC COLUMN REACTIONS (k )	RIGID FRAME: BASIC COLUMN REACTIONS (K )	NOTES FOR REACTIONS
WindWindSeisFrmColDeadPressSuctLongLineLineVertHorzHorzVert1B0.1-2.62.80.0	Frame ColumnCollateralLiveSnowWind_Left1-Wind_Right1LineLineHorizVertHorizVertHorizVertHorizVert1*A1.21.80.40.54.56.04.76.3-6.7-10.4-7.1-8.4	<ol> <li>All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.</li> <li>Positive reactions are as shown in the sketch. Foundation loads are in</li> </ol>
$ \begin{vmatrix} 1 & C & 0.1 & -3.0 & 3.3 & 0.0 \\ 1 & D & 0.1 & -2.6 & 2.8 & 0.0 \\ 11 & D & 0.1 & -2.6 & 2.8 & 0.0 \end{vmatrix} $	1*       E       -1.2       1.8       -0.4       0.5       -4.5       6.0       -4.7       6.3       7.1       -8.4       6.7       -10.4         Frame       Column      Wind_Left2-       -Wind_Right2-      Wind_Long1-      Wind_Long2-       -Seismic_Left       Seismic_Right	<ol> <li>Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.</li> <li>Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.</li> </ol>
$\begin{bmatrix} 11 & B & 0.1 & -2.6 & 2.8 & 0.0 \\ \hline & & & & & \\ \hline & & & & & \\ \hline & & & &$	1*       A       -4.0       -6.7       -4.4       -4.7       -6.2       -10.3       -7.0       -8.1       -0.2       -0.1       0.2       0.1         1*       E       4.4       -4.7       4.0       -6.7       7.0       -8.1       6.2       -10.3       -0.2       0.1       0.2       -0.1	<ul> <li>4. Loading conditions are:</li> <li>1 0.6Dead+0.6Wind_Left1</li> <li>2 0.6Dead+0.6Wind_Right1</li> </ul>
ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES Column_Reactions(k)	FrameColumn-MIN_SNOWF1UNB_SL_L-F1UNB_SL_R-LineHorizVertHorizVert1*A5.67.54.06.21*E-5.67.5-4.03.7	<ul> <li>3 Dead+Collateral+MIN_SNOW</li> <li>4 0.6Dead+0.6Wind_Right2+0.6Wind_Suction</li> <li>5 0.6Dead+0.6Wind_Pressure+0.6Wind_Long2L</li> </ul>
Frm Col Load Hmax V Load Hmin V Bolt(in) Base_Plate(in) Elev. Line Line Id H Vmax Id H Vmin Qty Dia Width Length Thick (in)	Frame Column —————Dead—————Collateral— ————Live—————Snow—————Wind_Left1— —Wind_Right1— Line Line Horiz Vert	6 Dead+0.6Wind_Right2+0.6Wind_Suction
1       B       4       1.7       0.1       5       -1.5       0.1       2       0.625       3.500       8.000       0.250       0.0         1       C       4       2.0       0.1       5       -1.8       0.1       2       0.625       3.500       8.000       0.250       0.0	2*       A       2.0       2.9       0.8       1.0       9.0       11.5       9.5       12.0       -10.7       -15.7       -11.4       -13.1         2*       E       -2.0       2.9       -0.8       1.0       -9.0       11.5       -9.5       12.0       11.4       -13.1       10.7       -15.7	BUILDING BRACING REACTIONS <u> </u>
6         2.0         0.1           1         D         4         1.7         0.1         5         -1.5         0.1         2         0.625         3.500         8.000         0.250         0.0           6         1.7         0.1         5         -1.5         0.1         2         0.625         3.500         8.000         0.250         0.0		Loc Line Line Horz Vert Horz Vert Wind Seis Note (h)
11       D       4       1.7       0.1       5       -1.5       0.1       2       0.625       3.500       8.000       0.250       0.0         11       C       4       2.0       0.1       5       -1.8       0.1       2       0.625       3.500       8.000       0.250       0.0	Frame Column -MIN_SNOW F2UNB_SL_L- F2UNB_SL_R- Line Line Horiz Vert Horiz Vert Horiz Vert 2* A 11.3 14.3 8.1 11.8 8.1 7.0	F_SW E 3,4 (a) 8,9 (a) R_EW 11 (h)
6         2.0         0.1           11         B         4         1.7         0.1         5         -1.5         0.1         2         0.625         3.500         8.000         0.250         0.0           6         1.7         0.1         5         -1.5         0.1         2         0.625         3.500         8.000         0.250         0.0	2* E -11.3 14.3 -8.1 7.0 -8.1 11.8 Frame ColumnDeadCollateralLiveSnowWind_Left1Wind_Right1-	B_SW A 8,9 3,4 (a) (a) Wind bent in bay
RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES	Line Line Horiz Vert Horiz Vert Horiz Vert Horiz Vert Horiz Vert Horiz Vert Horiz Vert 3* A 2.0 2.9 0.8 1.0 9.0 11.5 9.5 12.0 -10.7 -15.7 -11.4 -13.1 3* E -2.0 2.9 -0.8 1.0 -9.0 11.5 -9.5 12.0 11.4 -13.1 10.7 -15.7	(h)Rigid frame at éndwall
Column_Reactions(k ) Frm Col Load Hmax V Load Hmin V Bolt(in) Base_Plate(in) Elev. Line Line Id H Vmax Id H Vmin Qty Dia Width Length Thick (in)	Frame Column ——Wind_Left2— —Wind_Right2— ——Wind_Long1— ——Wind_Long2— —Seismic_Left Seismic_Right Line Line Horiz Vert 3* A —5.3 —8.5 —5.9 —5.9 —10.3 —15.5 —11.3 —12.8 —0.3 —0.1 0.3 0.1	Reactions for seismic represent shear force, Eh         ANCHOR BOLT SUMMARY (GRADE 36)
1*     A     3     7.1     9.8     2     -3.5     -4.0     4     0.750     8.000     10.50     0.375     0.0	3*       E       5.9       -5.9       5.3       -8.5       11.3       -12.8       10.3       -15.5       -0.3       0.1       0.3       -0.1         Frame       Column       -MIN_SNOW       F3UNB_SL_L-       F3UNB_SL_R-         Line       Line       Horiz       Vert       Horiz       Vert	Qty Locate Dia Proj (in) Type (in)
1*       E       1       3.5       -4.0       3       -7.1       9.8       4       0.750       8.000       10.50       0.375       0.0         3       -7.1       9.8       2       3.3       -5.2       1*       Frame lines:       1       11	3*       A       11.3       14.3       8.1       11.8       8.1       7.0         3*       E       -11.3       14.3       -8.1       7.0       -8.1       11.8	<ul> <li>◆ 12 Endwall 5/8" F1554 2.50</li> <li>◆ 16 Frame 3/4" F1554 3.00</li> <li>◆ 72 Frame 1" F1554 3.50</li> <li>◆ 32 Portal Frame 1" F1554 3.50</li> </ul>
RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES	2*         Frame lines:         2         5         6         7         10           3*         Frame lines:         3         4         8         9	✿ 32 Portal Frame 1" F1554 3.50
	FRAME LINES: 1 2 3 4 5 6 7 8 9 10 11	
$\begin{array}{ c c c c c c c c c c c c c c c c c c c$	A COLUMN LINE	Frame Col Wind Wind Line Line Press Suct Horz Horz
2*       E       1       5.6       -6.1       3       -14.1       18.2       4       1.000       8.000       13.00       0.500       0.0         3       -14.1       18.2       2       5.2       -7.7       4       1.000       8.000       13.00       0.500       0.0         2*       Frame lines:       2       5       6       7       10		1 A -1.08 K 1.28 K 1 E -1.08 K 1.28 K 11 A -1.08 K 1.28 K
RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES		11 E -1.08 K 1.28 K
Frm Col Load Hmax V Load Hmin V Bolt(in) Base_Plate(in) Elev. Line Line Id H Vmax Id H Vmin Qty Dia Width Length Thick (in)		
3*       A       3       14.1       18.2       2       -5.6       -6.1       4       1.000       8.000       13.00       0.500       0.0         3*       E       1       5.6       -6.1       3       -14.1       18.2       4       1.000       8.000       13.00       0.500       0.0		
3*       E       1       5.6       -6.1       3       -14.1       18.2       4       1.000       8.000       13.00       0.500       0.0         3*       Frame lines:       3       4       8.2       5.2       -7.7       7       1.000       1.000       1.000       0.500       0.0		
	PORTAL FRAME REACTIONS ± Reactions	
	Image: Wall in the set of t	Thick 0.375
	$ \begin{bmatrix} & & & & & & & & & & & & & & & & & & $	0.375 0.375 0.375
	Image: Normal system         B_SW         A         9         1.2         1.2         0.9         0.8         4         1.000         8.000         12.000           Image: V         Image: Normal system         A         8         1.2         1.2         0.9         0.8         4         1.000         8.000         12.000           Image: Normal system         A         8         1.2         1.2         0.9         0.8         4         1.000         8.000         12.000           Image: B_SW         A         4         1.2         1.2         0.9         0.8         4         1.000         8.000         12.000           Image: B_SW         A         3         1.2         1.2         0.9         0.8         4         1.000         8.000         12.000	0.375 0.375 0.375 0.375
	D_3W A 5 1.2 1.2 0.9 0.0 4 1.000 0.000 12.000	
GENERAL NOTES THREADED ANCHOR BOLT		CAROUNA CARO
1. All anchor bolts (by others) to have nuts and flat washers.         2. All anchor bolts are designed to full S.A.E. diameters with cut threads. No substitutions are allowed.         BASE         PLATE		About teris
3. The Metal Building Provider is not responsible for the design, materials and workmanship of the foundation. Anchor bolt plans prepared by the Metal Building Provider are intended to show only location, diameter, and projection of anchor bolts required to attach the Metal Building System to		WALTER WALTER
to the Builder the loads imposed by the Metal Building System on the foundation. It is the responsibility of the End Customer to ensure that adequate provisions are made for specifying bolt embedment, bearing angles, tie rods, and/or other associated items embedded in the concrete foundation.	Buildings, I responsibilit and exclud	eer whose seal and signature appear on these documents represents Whirlwind Steel Inc., and is not the Engineer of Record for the overall project. The Engineer's ity is limited to material designed and manufactured by Whirlwind Steel Buildings, Inc., tes part such as doors, windows, foundation design, and erection of the building.
The roas, and/or other associated items empedded in the concrete foundation, as well as foundation design for the loads imposed by the Metal Building System, other imposed loads, and the bearing capacity of the soil and other conditions of the building site. This is typically the responsibility of the Design Professional or Engineer of Record, which is another reason that their involvement in the Construction Project from the outset is highly recommended.	R APPROVAL:       ISSUE       DATE       DESCRIPTION       BY       CHK       SHEET DESCRIPT         se drawings, being for approval, are by definition not final and are conceptual representation only. Their purpose is to confirm the per interpretation of the project documents. Only drawings issued "For lestendation" can be considered complete       ISSUE       DATE       DESCRIPTION       BY       CHK       SHEET DESCRIPT         0       01.30.24       FOR ERECTOR INSTALLATION       PND       PNC       CUSTOMER: BAUCCO         0	ANCHOR BULL REACTIONS         70 - 0 x 273 - 0 x 14 - 0           CUSTOMER LOCATION:         CUSTOMER LOCATION:           DM BUSINESS PLAZA         FUQUAY VARINA, NC 27526
PLATES.	R CONSTRUCTION PERMIT: se drawings, being for permit, are by definition not final. Only wings issued "For Erector Installation" can be considered complete.	OM BUSINESS PLAZA DN: JOBSITE COUNTY: UQUAY VARINA, NC 27526 WAKE HK: DATE: ENG: JOB NO: DWG NO: ISSUE:
Design Professional or Engineer of Record, which is another reason that their involvement in the Construction Project from the outset is highly recommended. (2012 MBMA Metal Building Systems Manual, Section 3.2.2)       PLATE. ADJUSTMENTS       pro         4. The projection is based from the bottom of the base plate. Adjustments       PLATE. ADJUSTMENTS       pro	conceptual representation only. Their purpose is to confirm the pre-interpretation of the project documents. Only drawings issued "For constallation" can be considered complete. <u>         CONSTRUCTION PERMIT:</u> se drawings, being for permit, are by definition not final. Only	CUSTOMER LOCATION: FUQUAY VARINA, NC 27526 ENCE: OM BUSINESS PLAZA DN: UQUAY VARINA, NC 27526 WAKE

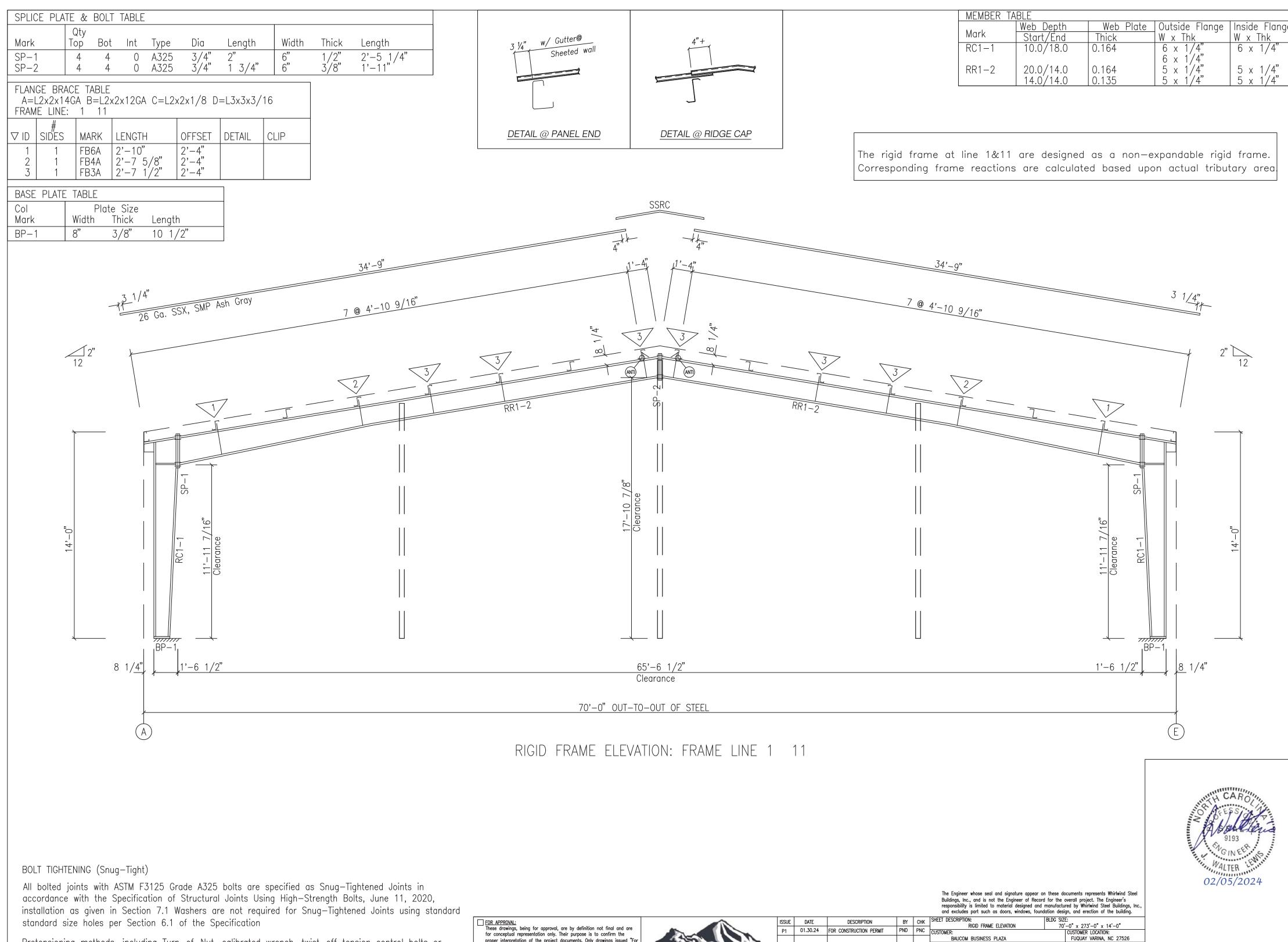




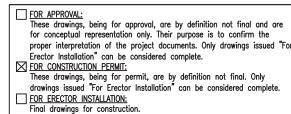


# The Engineer whose seal and signature appear on these documents represents Whirlwind Steel Buildings, Inc., and is not the Engineer of Record for the overall project. The Engineer's responsibility is limited to material designed and manufactured by Whirlwind Steel Buildings, Inc., and excludes part such as doors windows foundation design and erection of the building.

						and exc	cludes part si	uch as doors, wir	ndows, found	ation design, and	erection of the	building.
	ISSUE	DATE	DESCRIPTION	BY	СНК	SHEET DESCR		BOLT REACTIONS		BLDG SIZE:	273'-0" x 14'	0"
		01.30.24	FOR ERECTOR INSTALLATION	PND	PNC	CUSTOMER:	ANCHUK	DULI REACTIONS			VER LOCATION:	-0
AST AND							JCOM BUSINE	ESS PLAZA		FUQU	JAY VARINA, NC	27526
						PROJECT REF	ERENCE: UCOM BUSINI					
						JOBSITE LOC		LOO FLAZA			JOBSITE COUNT	Y:
AK STEEL						FUQUAY VARINA, NC 27526 WAKE						
						DWN:	CHK:	DATE:	ENG:	JOB NO:	DWG NO:	ISSUE:
						PND	PNC	01.30.24	I NPK	12252-33816	F3	0



Pretensioning methods, including Turn-of-Nut, calibrated wrench, twist-off tension control bolts or direct tension indicator are not required. Installation inspection requirements for Snug-Tight Bolt is found in Section 9.1 of the Specification.





STEE

MEMBER TA	BLE			
	Web Depth	Web Plate	Outside Flange	Inside Flange
Mark	Start/Énd	Thick	W x Thk	W x Thk
RC1-1	10.0/18.0	0.164	6 x 1/4"	6 x 1/4"
			6 x 1/4"	,
RR1-2	20.0/14.0	0.164	5 x 1/4"	5 x 1/4"
	14.0/14.0	0.135	5 x 1/4"	5 x 1/4"

ROJECT REFERENCE:

JOBSITE LOCATION:

PND

BAUCOM BUSINESS PLAZA

PNC

FUQUAY VARINA, NC 27526 CHK: DATE: |I

01.30.24

JOBSITE COUNTY:

ISSUE:

P1

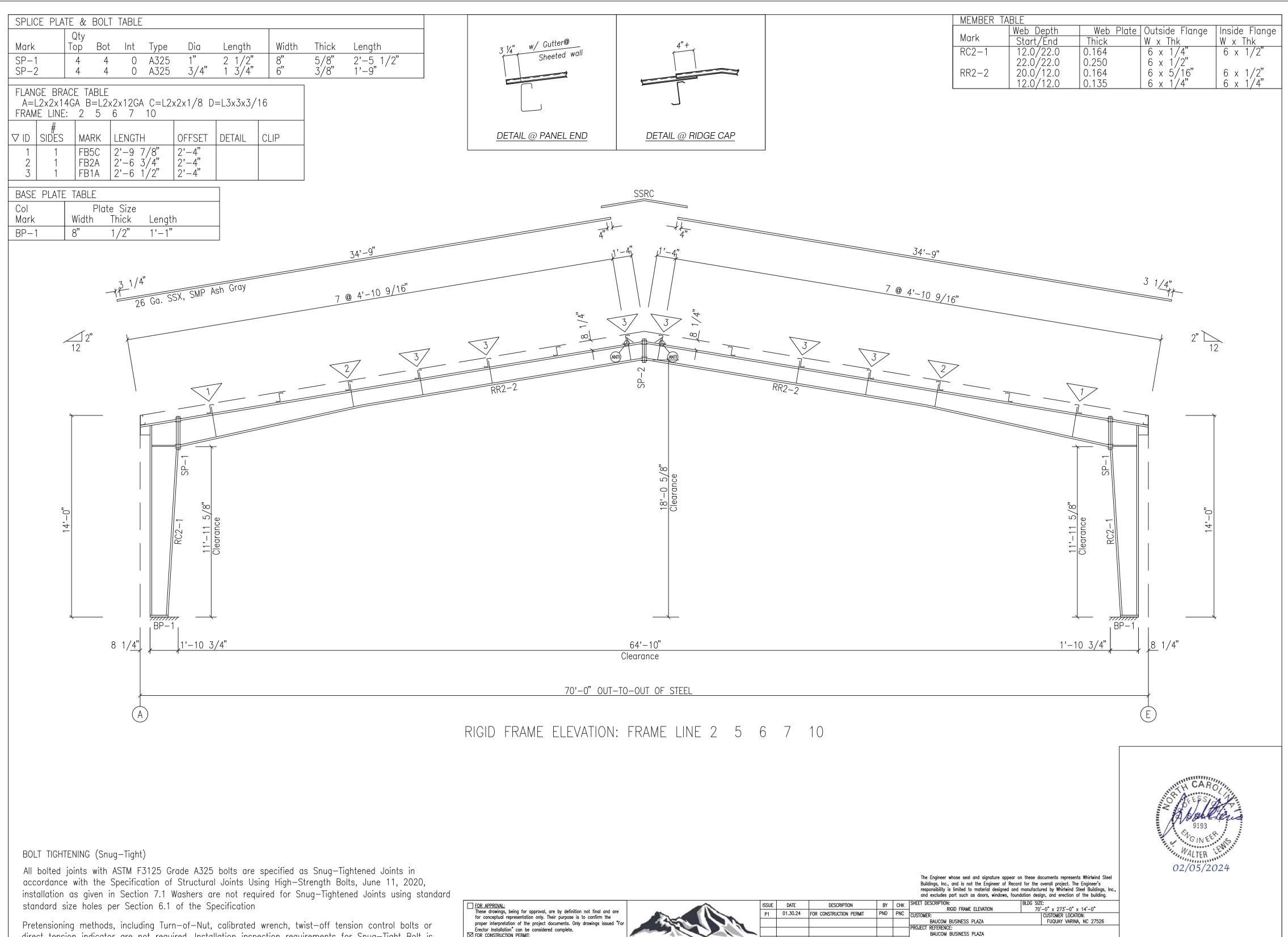
WAKE DWG NO:

P1

JOB NO:

NPK 12252-33816

ENG:



direct tension indicator are not required. Installation inspection requirements for Snug-Tight Bolt is found in Section 9.1 of the Specification.

FOR CONSTRUCTION PERMIT: These drawings, being for permit, are by definition not final. Only drawings issued "For Erector Installation" can be considered complete.

FOR ERECTOR INSTALLATION: Final drawings for construction



STEE

JOBSITE LOCATION:

CHK:

DWN

FUQUAY VARINA, NC 27526

DATE:

JOBSITE COUNTY:

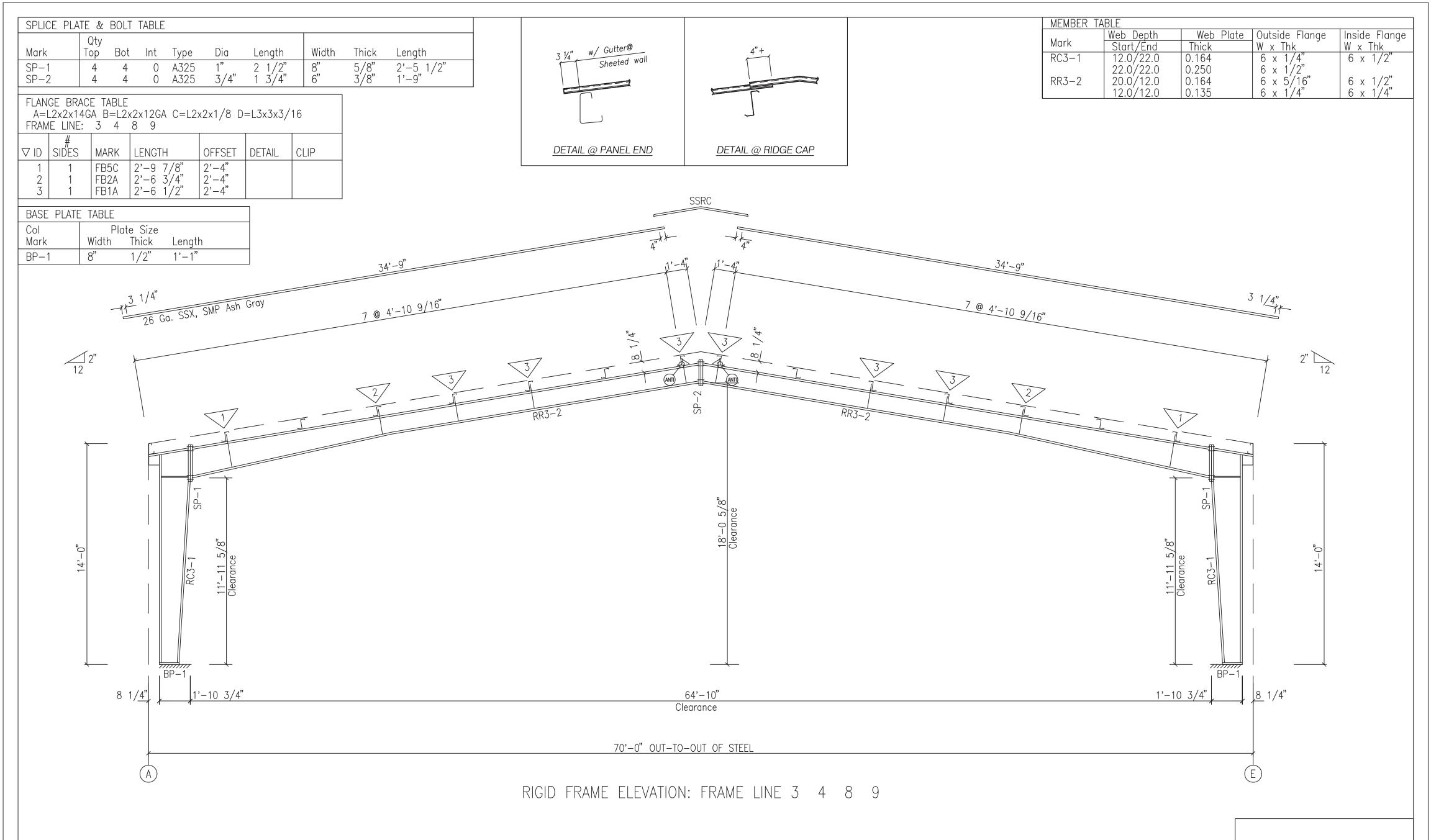
ISSUE:

P1

WAKE DWG NO:

JOB NO:

ENG: I: CHK: DATE: ENG: JOB NO: DWG NO: PND PNC 01.30.24 NPK 12252-33816 P2



# BOLT TIGHTENING (Snug-Tight)

All bolted joints with ASTM F3125 Grade A325 bolts are specified as Snug-Tightened Joints in accordance with the Specification of Structural Joints Using High-Strength Bolts, June 11, 2020, installation as given in Section 7.1 Washers are not required for Snug-Tightened Joints using standard standard size holes per Section 6.1 of the Specification

Pretensioning methods, including Turn-of-Nut, calibrated wrench, twist-off tension control bolts or direct tension indicator are not required. Installation inspection requirements for Snug-Tight Bolt is found in Section 9.1 of the Specification.

 ☐ FOR APPROVAL: These drawings, being for approval, are by definition not final and are for conceptual representation only. Their purpose is to confirm the proper interpretation of the project documents. Only drawings issued "Fo Erector Installation" can be considered complete.
 ○ FOR CONSTRUCTION PERMIT: These drawings, being for permit, are by definition not final. Only drawings issued "For Erector Installation" can be considered complete.

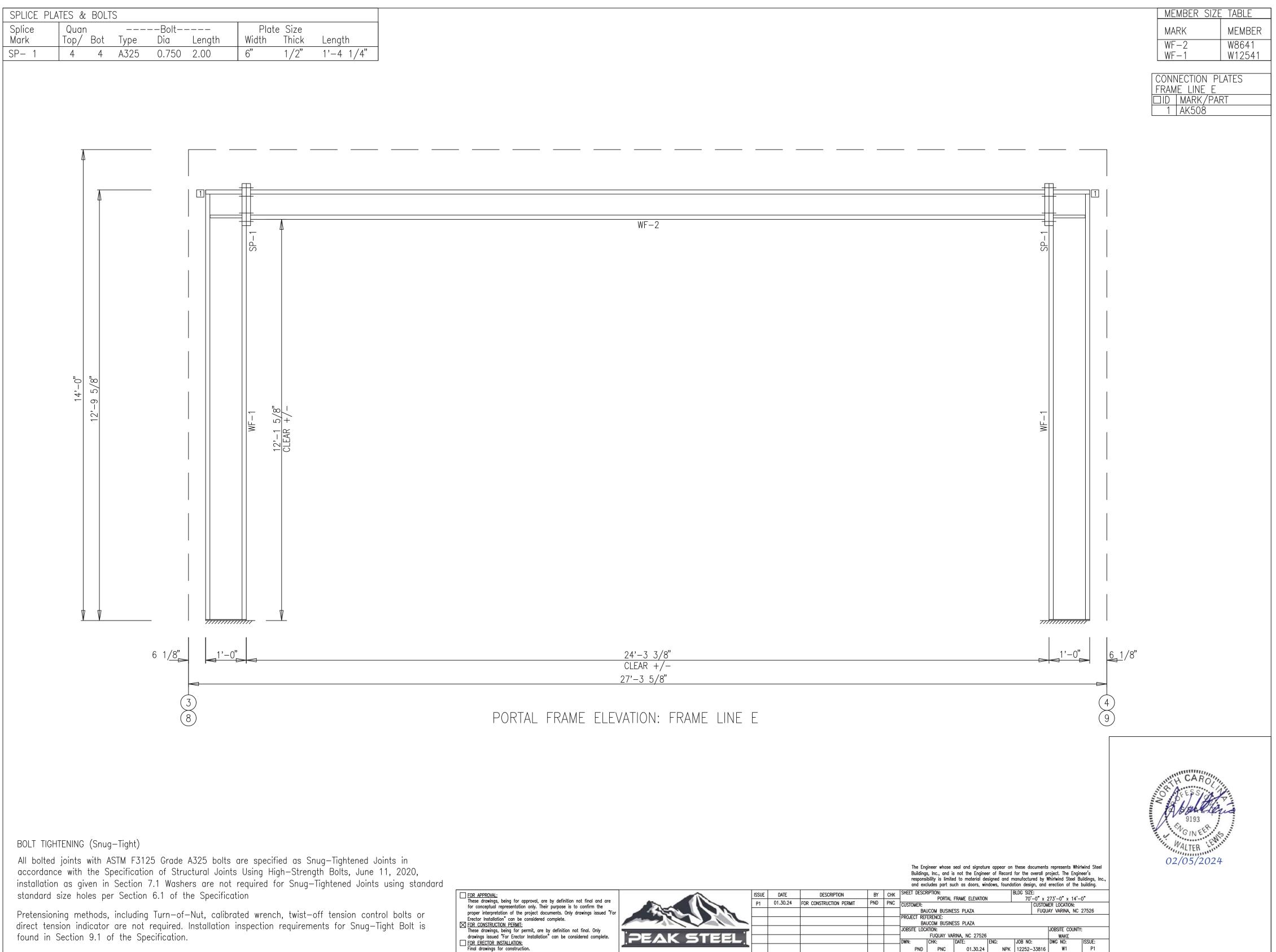
FOR ERECTOR INSTALLATION: Final drawings for construction

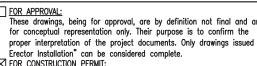


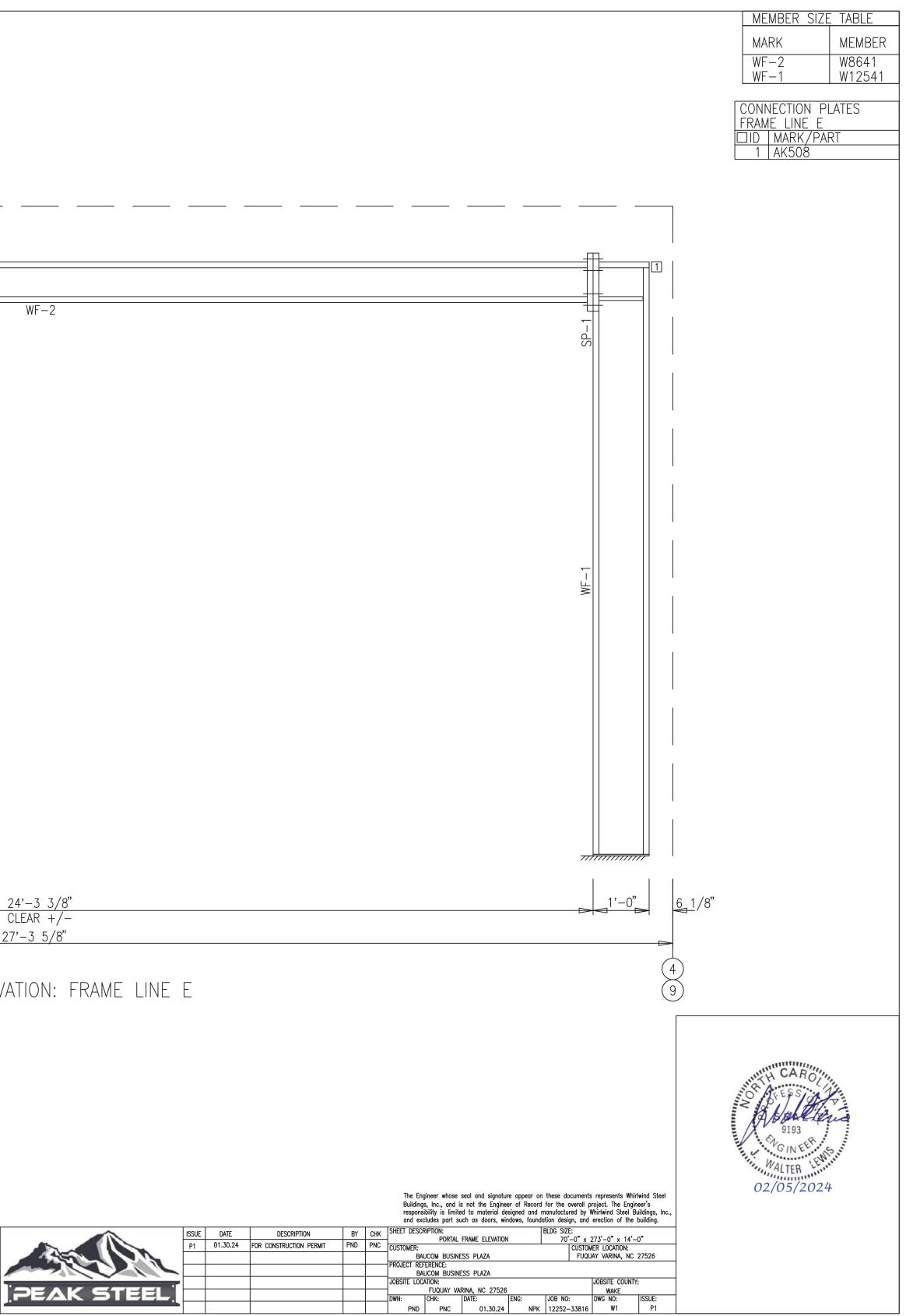


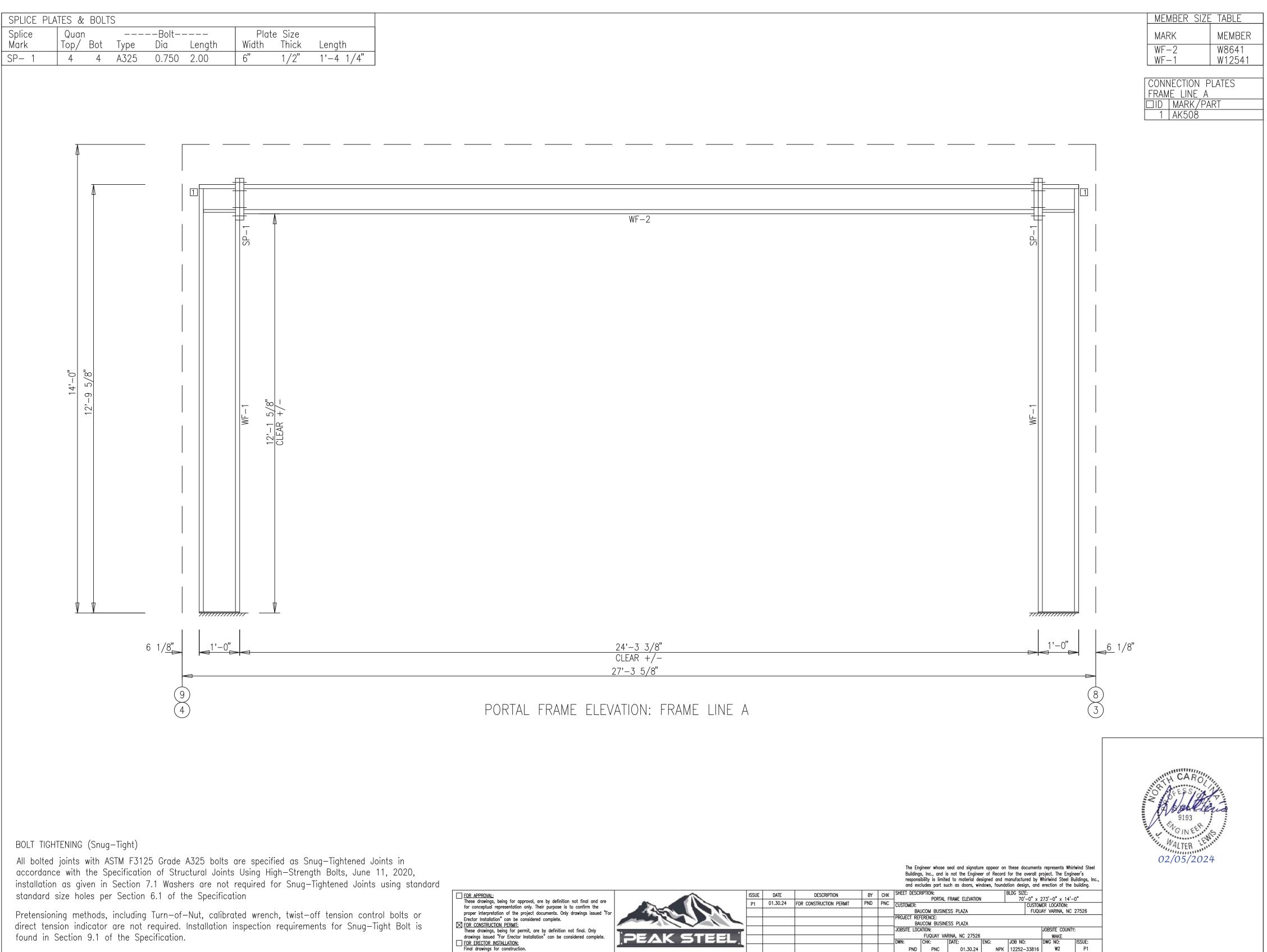
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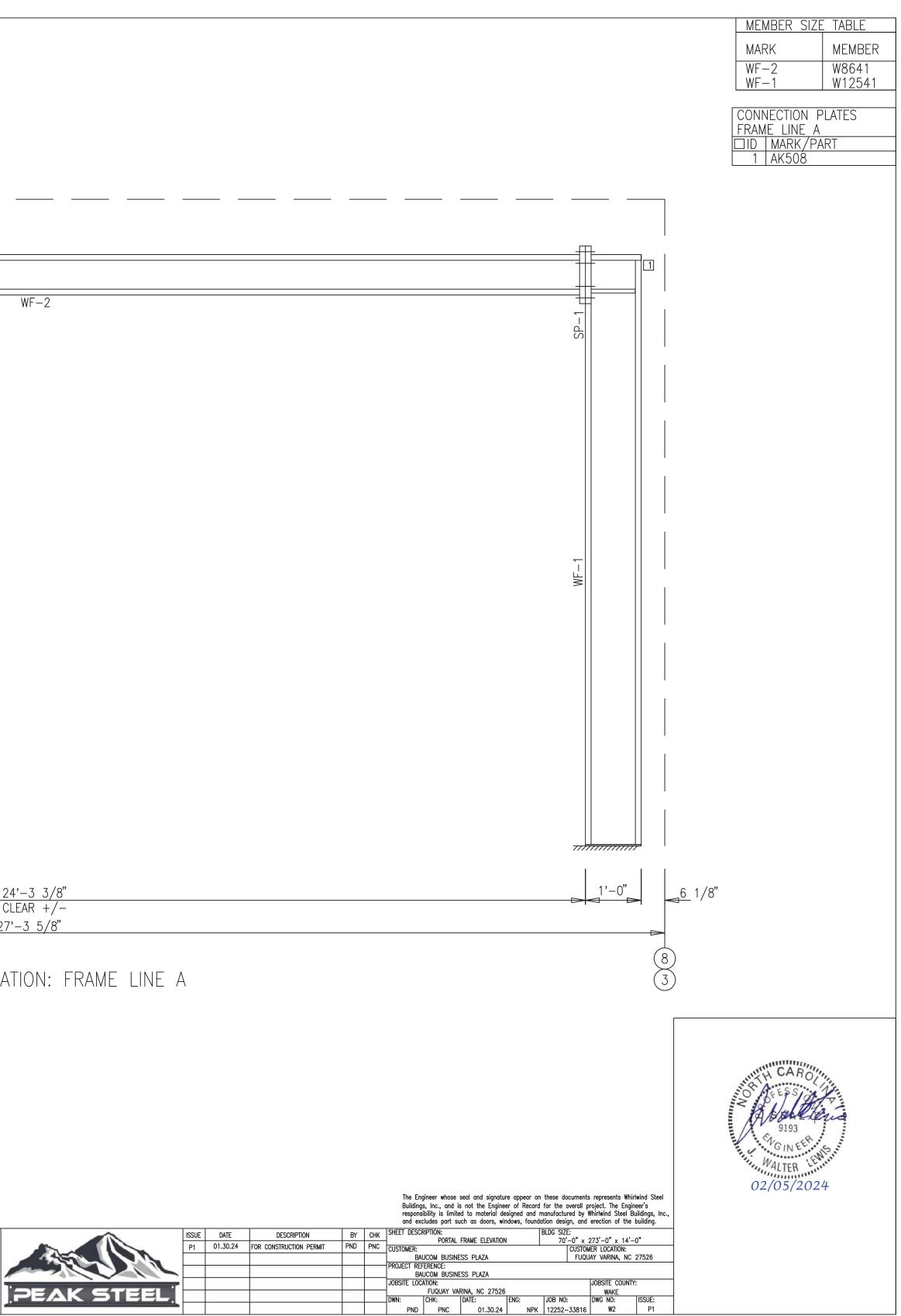
						and e	excludes part s	uch as doors, wir	ndows, founda	tion design, and	erection of the	building.
	ISSUE	DATE	DESCRIPTION	BY	СНК	SHEET DES		RAME ELEVATION	1	BLDG SIZE:	273'-0" x 14'-	٥"
	P1	01.30.24	FOR CONSTRUCTION PERMIT	PND	PNC	CUSTOMER		CAME ELEVATION			IER LOCATION:	0
							BAUCOM BUSIN	ESS PLAZA		FUQL	JAY VARINA, NC	27526
						PROJECT F						
						JOBSITE LO	BAUCOM BUSIN	ESS PLAZA			JOBSITE COUNTY	
AK ETEEL I								RINA. NC 27526			WAKE	•
AK STEEL						DWN:	CHK:	DATE:	ENG:	JOB NO:	DWG NO:	ISSUE:
						I PNE	PNC	01.30.24	NPK	12252-33816	P3	P1

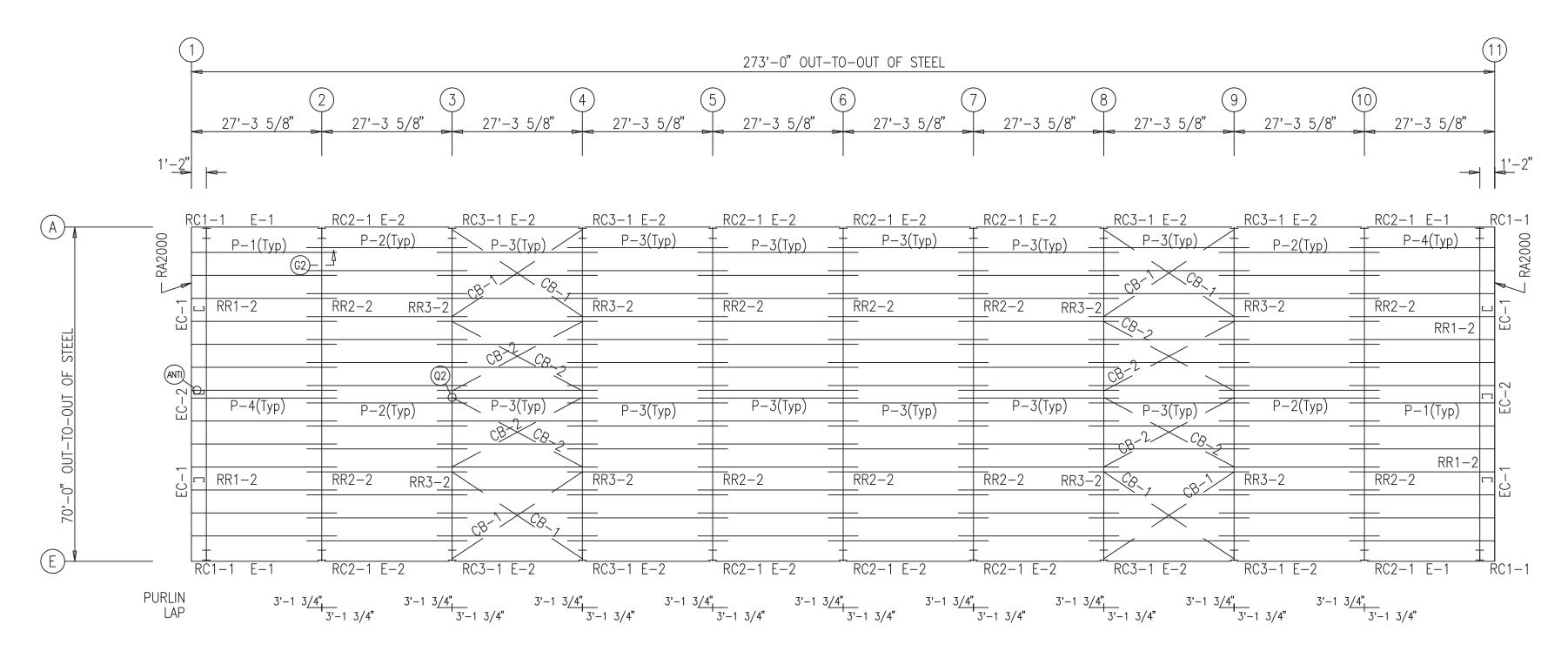






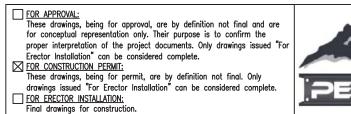






ROOF FRAMING PLAN

UL580, CLASS 90 CONST. NUMBER 167

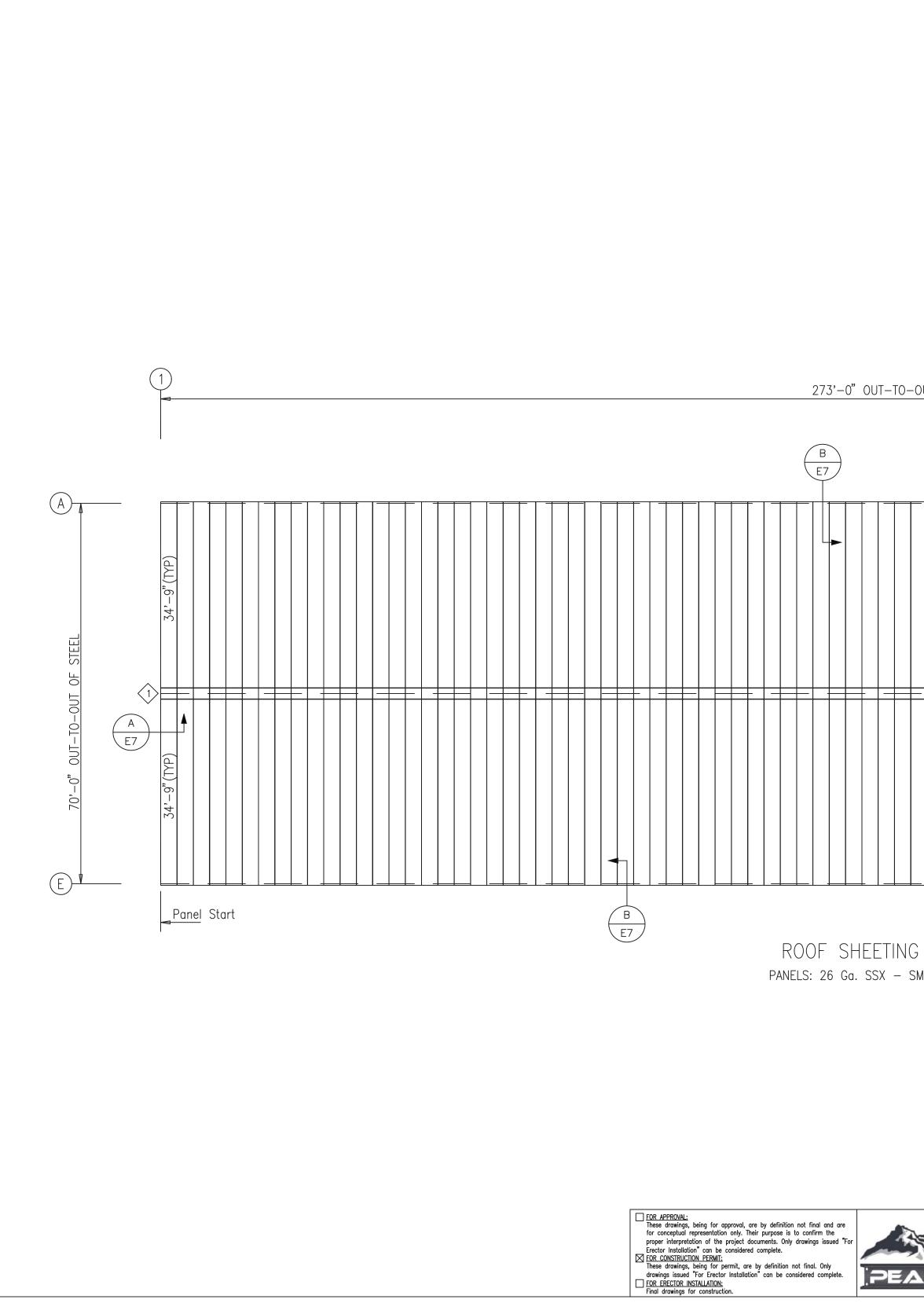


MEMBER	TABLE
ROOF PLA	N
MARK	PART
P-1	8X25Z12
P-2	8X25Z14
P-3	8X25Z16
P-4	8X25Z12
E-1	8ES142
E-2	8ES142
CB-1	0.25_CBL
CB-2	0.25_CBL



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						und exc	ciudes part su	cii us uoors, wir	idows, iounidu	uon design, and	erection of the	bullang.	
	ISSUE	DATE	DESCRIPTION	BY	СНК	SHEET DESCH		AMING PLAN	E	BLDG SIZE:	273'-0" x 14'-	0"	]
	P1	01.30.24	FOR CONSTRUCTION PERMIT	PND	PNC	CUSTOMER:		AMING FLAN			VER LOCATION:	0	-
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						PROJECT REF		00 01 171					
							UCOM BUSINE	SS PLAZA					-
································						JOBSITE LOC					JOBSITE COUNTY	:	
AK STEEL I						-		RINA, NC 27526			WAKE		
						DWN:	CHK:	DATE:	ENG:	JOB NO:	DWG NO:	ISSUE:	
						PND	PNC	01.30.24	NPK	12252-33816	E1	P1	



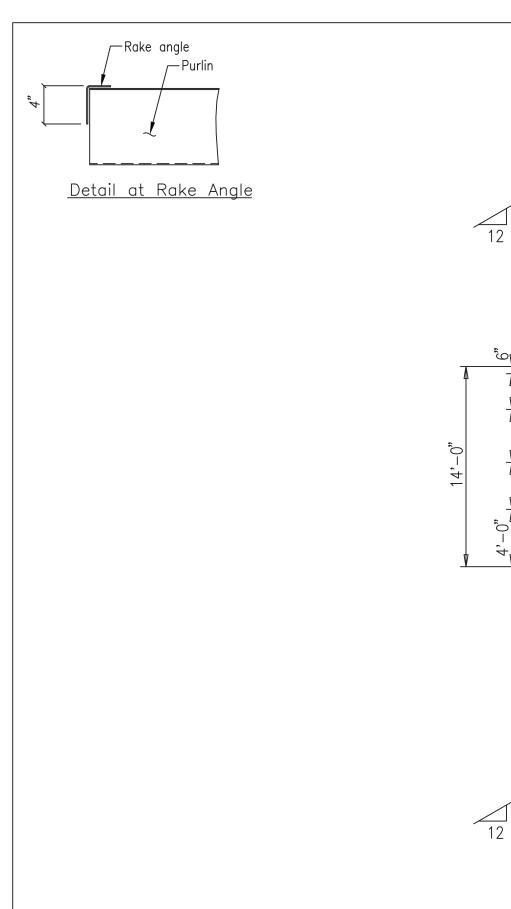


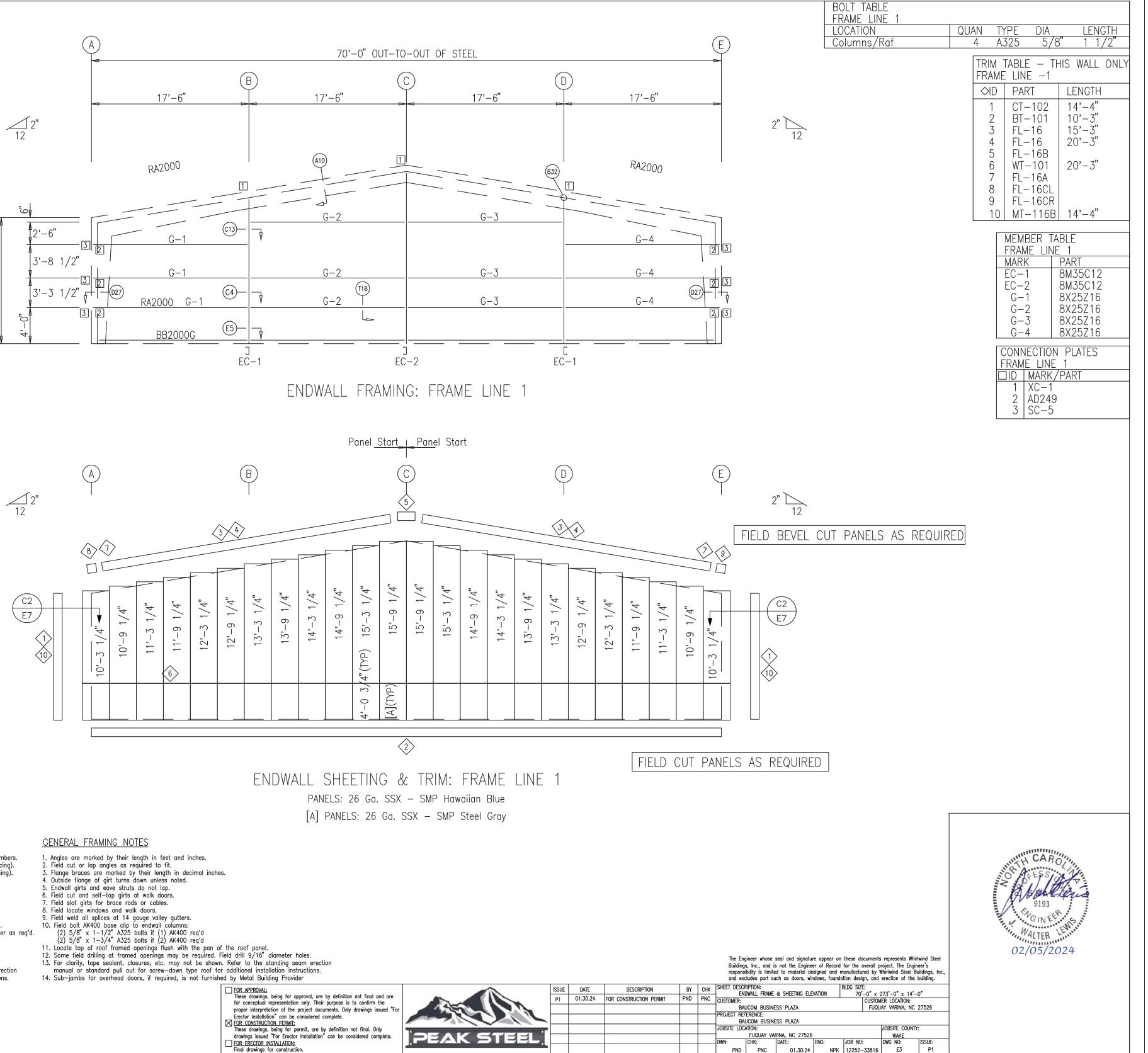
	ROOF       SHEETING       TRIM       TABLE         ◇ID       PART       LENGTH         1       SSRC30       3'-0"
UT OF STEEL	
PLAN IP Ash Gray	

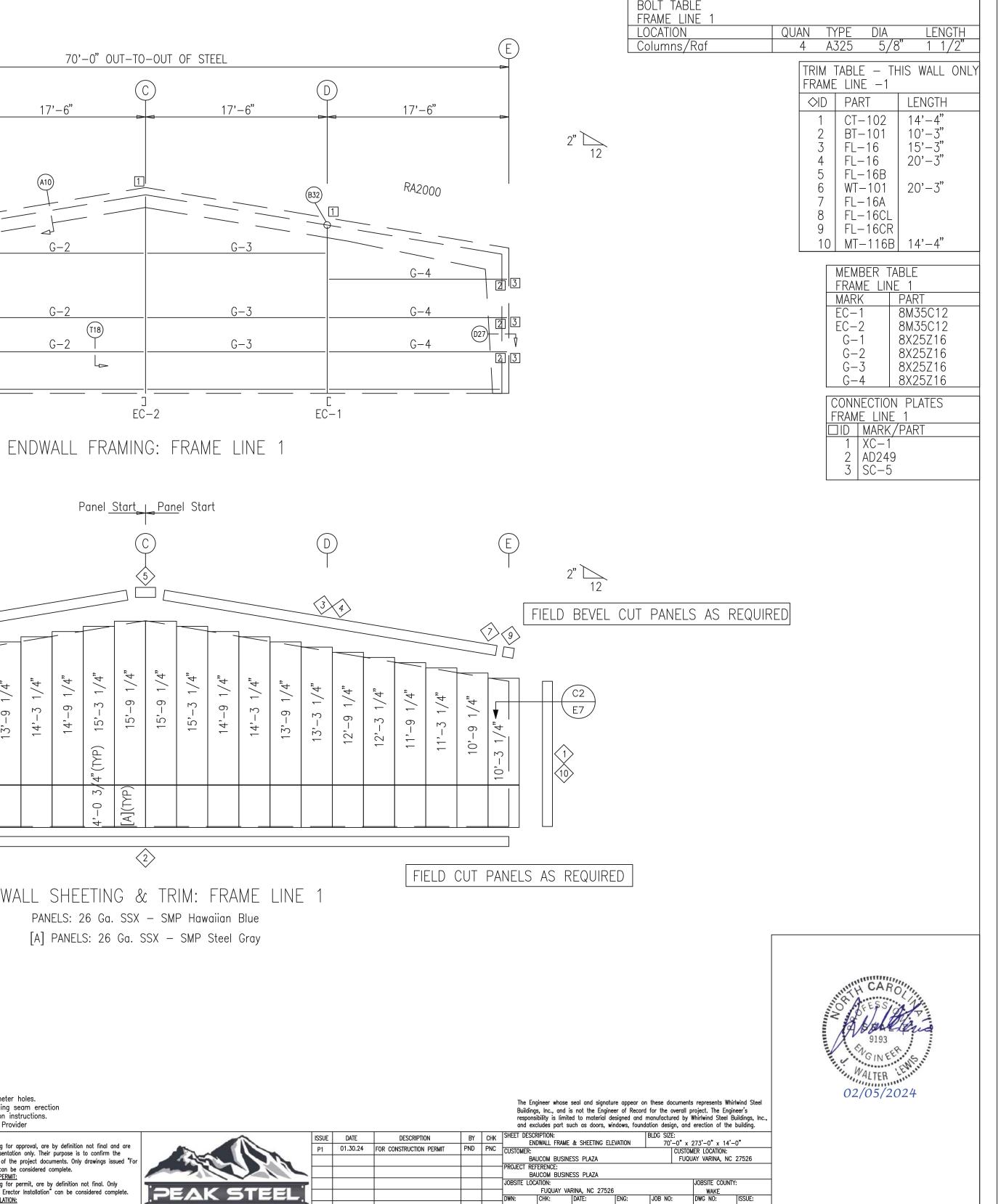


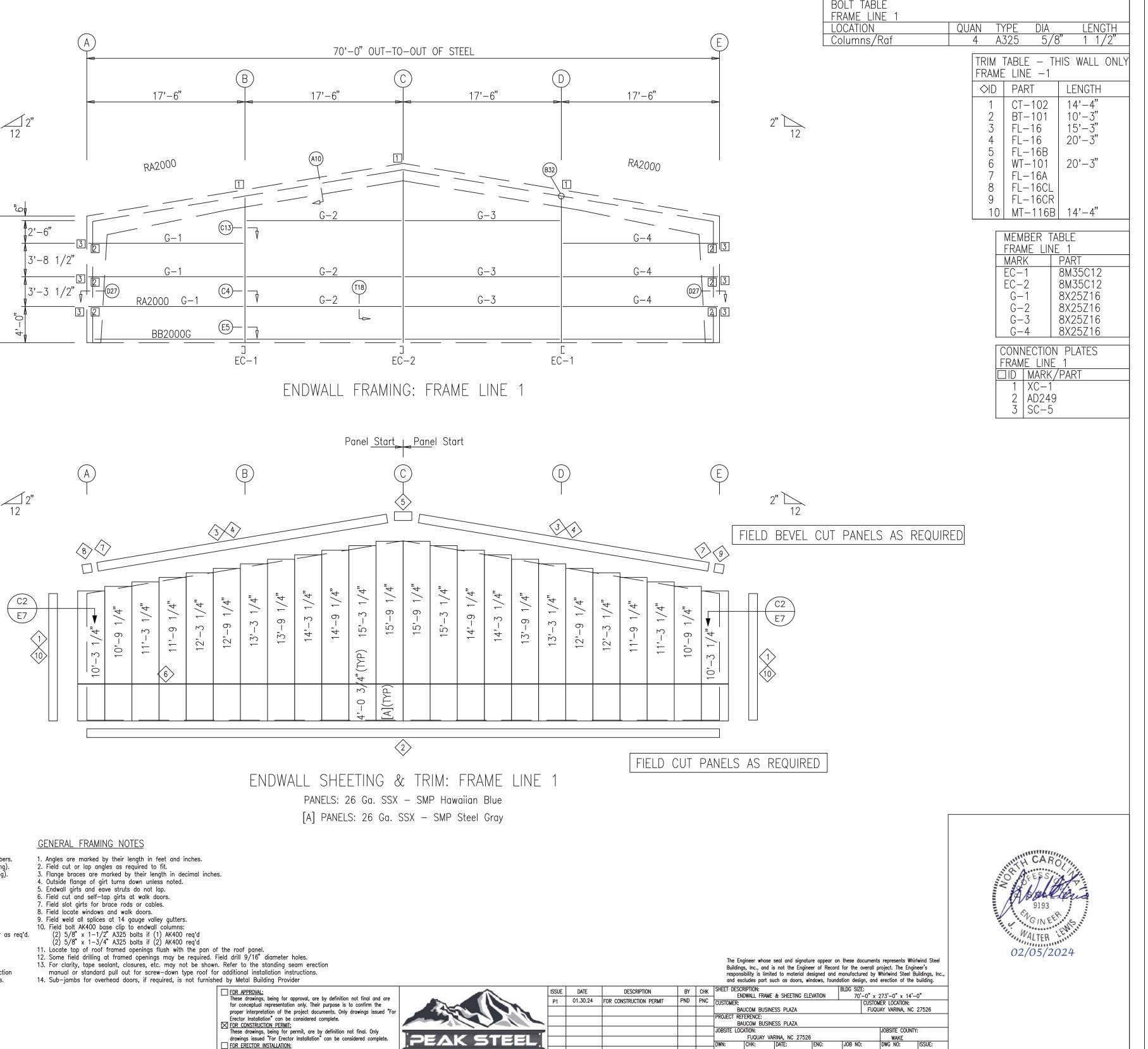
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ISSUE         DATE         DESCRIPTION         BY         CHK         SHEET DESCRIPTION: ROOF SHEETING PLAN         BLDG SIZE: 70'-0" x 273'-0" x 14'-0"           P1         01.30.24         FOR CONSTRUCTION PERMIT         PND         PNC         CUSTOMER:         CUSTOMER LOCATION:							ana exc	ciuaes part su	ich as aoors, wir	idows, tounda	tion design, and	erection of the	building.	
P1 01.30.24 FOR CONSTRUCTION PERMIT PND PNC CUSTOMER: CUSTOMER LOCATION:		ISSUE	DATE	DESCRIPTION	BY	СНК	SHEET DESCR		HEFTING PLAN	E		273'_0" v 14'-	-0"	]
		P1	01.30.24	FOR CONSTRUCTION PERMIT	PND	PNC	CUSTOMER:	11001 31					-0	1
BAUCOM BUSINESS PLAZA FUQUAY VARINA, NC 27526							BAU	JCOM BUSINE	ESS PLAZA		FUQ	UAY VARINA, NC	27526	
PROJECT REFERENCE:														1
BAUCOM BUSINESS PLAZA									ESS PLAZA					
JOBSITE LOCATION: JOBSITE COUNTY:							JOBSITE LOC/					JOBSITE COUNT	Y:	
	AK STEEL I						-							
DWN: CHK: DATE: ENG: JOB NO: DWG NO: ISSUE:	AR DIEEL.						DWN:	CHK:	DATE:	ENG:	JOB NO:	DWG NO:	ISSUE:	
PND PNC 01.30.24 NPK 12252-33816 E2 P1							PND	PNC	01.30.24	NPK	12252-33816	E2	P1	



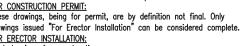


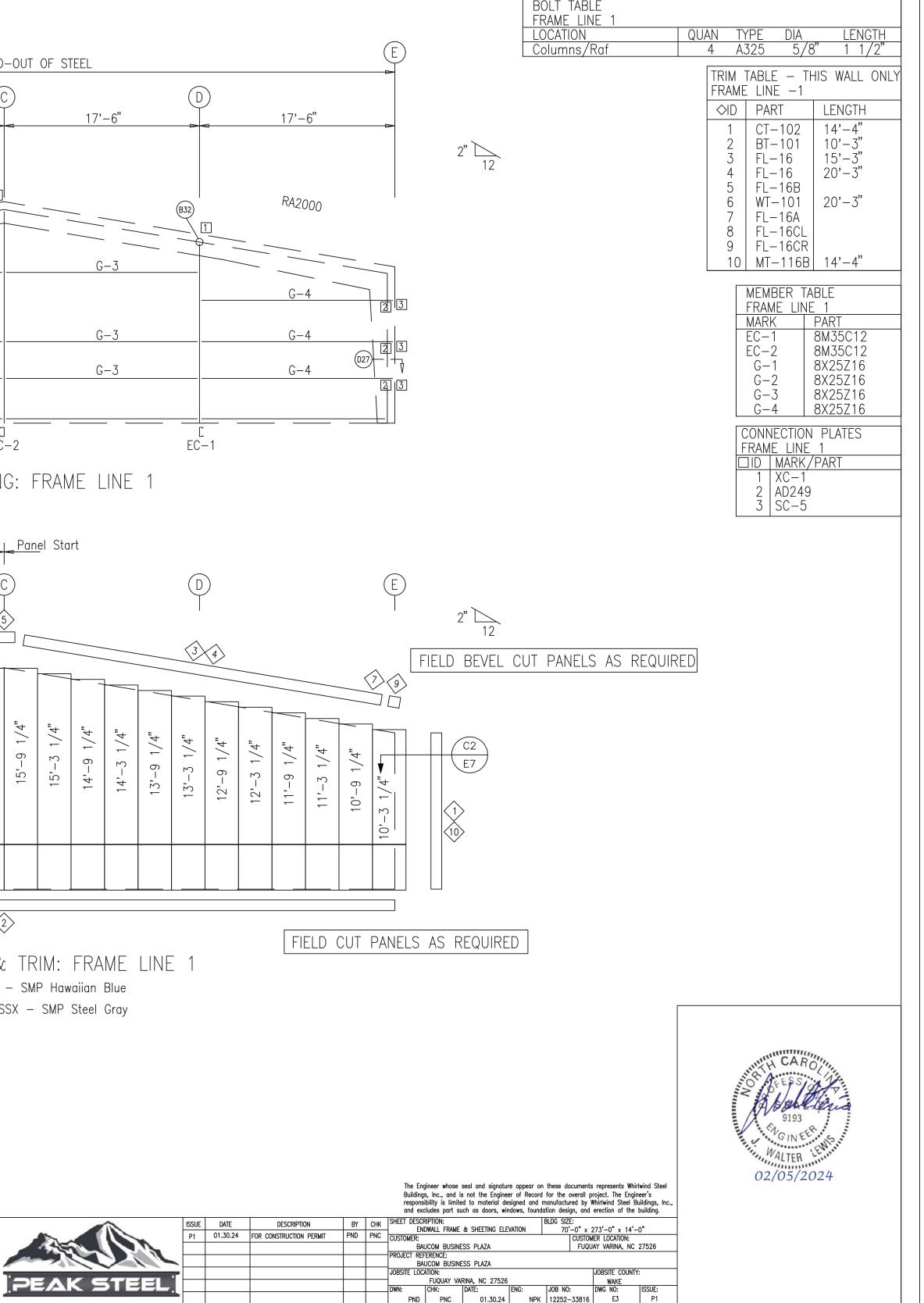


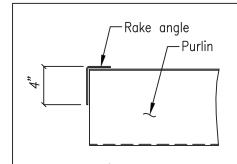


## GENERAL SHEETING & TRIM NOTES

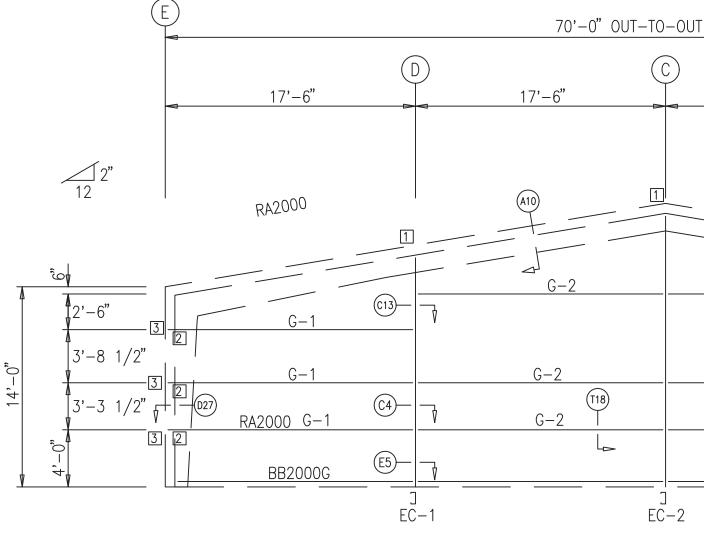
- 1. Refer to erection drawings for rake angle locations.
- 2. Roof member screws are at 12" o.c. Eave end lap and peak screws are as shown. 3. Wall member screws are at 6" o.c. at the base member and 12" o.c. at all remaining members.
- 4. Roof stitch screws are located at each member with two between members (20" max. spacing).
- 5. Wall stitch screws are located at each member with one between members (20" max. spacing).
- 6. Skylight stitch screws are at 6" o.c. 7. Start endwall panels at centerline of bldg. unless noted
- 8. Gutter, rake, & eave trim lap 2". All other trims lap 1".
- 9. Field cut or lap panels as required to fit.
- 10. Field cut panels for all openings.
- 11. Pop rivet gutter counterflashing to wall panel on 3'-0 centers and caulk all laps. 12. Gutter support strap spacing: Super Span 3'-0, Super Seam 4'-0, Weather Lok-16 2'-8".
- 13. Corner and/or peak boxes are not furnished with special rake or gutter profiles. Field miter as reg'd.
- 14. Downspout straps are located 6" from base and at every girt location.
- 15. Hot-rolled or built-up members must be pre-drilled before attaching members screws.
- 16. Metal shavings must be swept from the roof each day to avoid surface rusting. 17. Windows and louvers must be installed before sheeting the walls.
- 18. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.



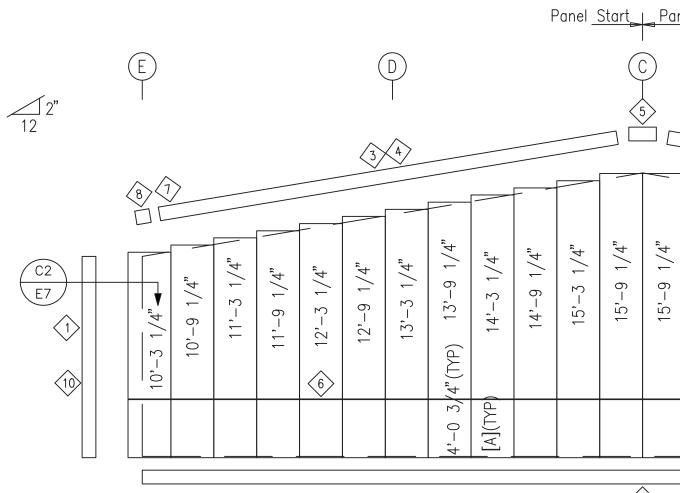




<u>Detail at Rake Angle</u>







# GENERAL SHEETING & TRIM NOTES

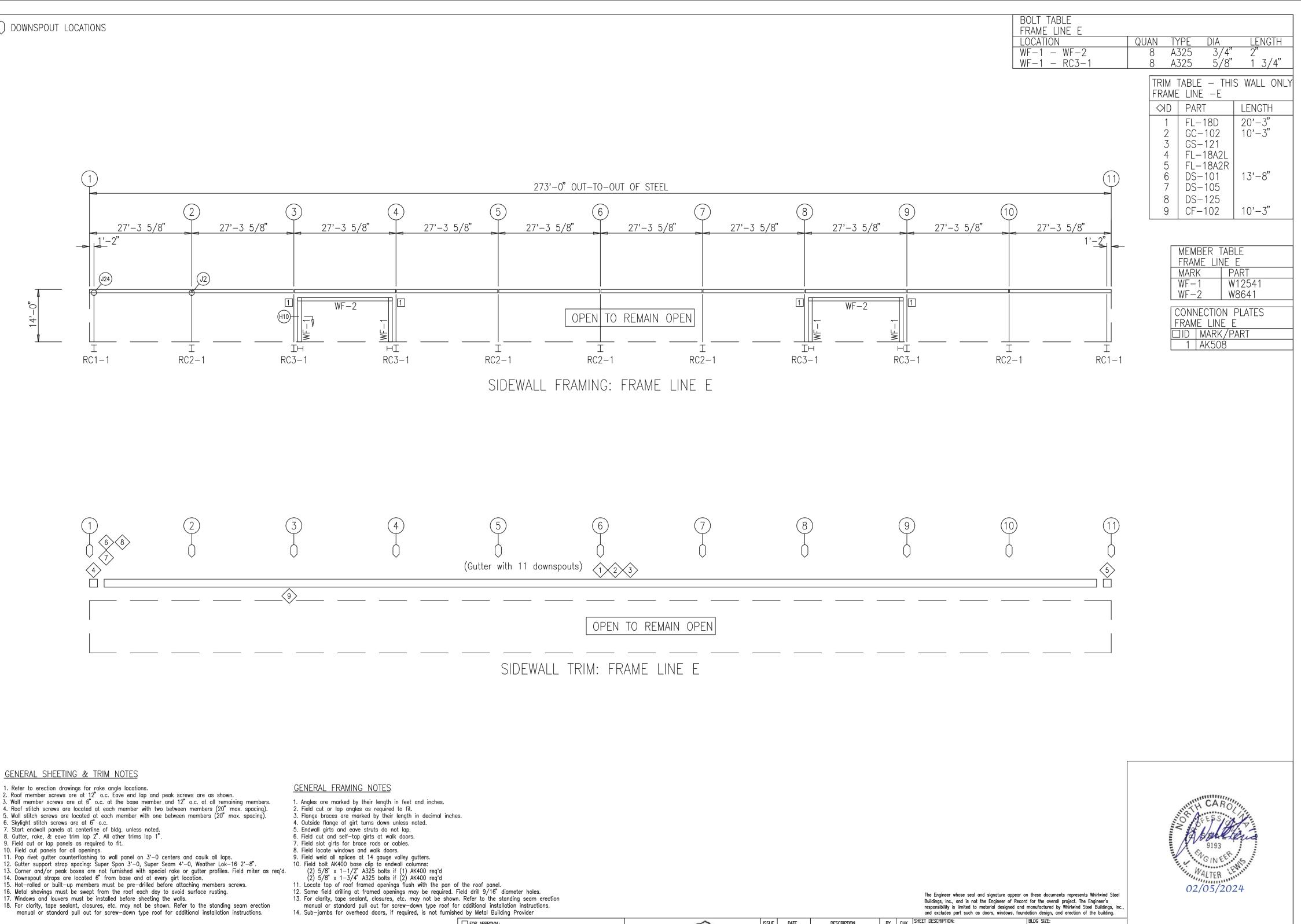
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- 14. Downspout straps are located 6" from base and at every girt location. 15. Hot-rolled or built-up members must be pre-drilled before attaching members screws.
- 16. Metal shavings must be swept from the roof each day to avoid surface rusting.
- 17. Windows and louvers must be installed before sheeting the walls.
- 18. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.

# GENERAL FRAMING NOTES

- 1. Angles are marked by their length in feet and inches.
- 2. Field cut or lap angles as required to fit. 3. Flange braces are marked by their length in decimal inches.
- 4. Outside flange of girt turns down unless noted.
- 5. Endwall girts and eave struts do not lap.
- 6. Field cut and self—tap girts at walk doors. '. Field slot girts for brace rods or cables.
- 8. Field locate windows and walk doors.
- 9. Field weld all splices at 14 gauge valley gutters.
- 10. Field bolt AK400 base clip to endwall columns:
- (2)  $5/8" \times 1-1/2"$  A325 bolts if (1) AK400 req'd (2)  $5/8" \times 1-3/4"$  A325 bolts if (2) AK400 req'd
- 11. Locate top of roof framed openings flush with the pan of the roof panel.
- 12. Some field drilling at framed openings may be required. Field drill 9/16" diameter holes.
- 13. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection
- manual or standard pull out for screw-down type roof for additional installation instructions. 14. Sub-jambs for overhead doors, if required, is not furnished by Metal Building Provider
  - FOR APPROVAL:
  - These drawings, being for approval, are by definition not final and are for conceptual representation only. Their purpose is to confirm the proper interpretation of the project documents. Only drawings issued "Fo
  - Erector Installation" can be considered complete. FOR CONSTRUCTION PERMIT: These drawings, being for permit, are by definition not final. Only drawings issued "For Erector Installation" can be considered complete.
  - FOR ERECTOR INSTALLATION: Final drawings for construction

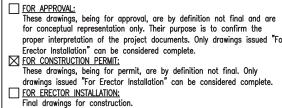


																				BOLT T FRAME LOCATIO Column	<u>LINE 11</u> DN		QUAN 4		PE DI 25 (		ENGTH 1/2"
			70'	_0" ^I		)-OUT	UF C	TEEI									A	$\mathbf{O}$					F	FRAME	LINE –	11	ALL ONLY
		17	′′́−6"				UF 3	17'	-6"		B	)		17'-	-6"		A							3	PART CT-102 BT-10 FL-16 FL-16		-4" -3" -3"
1					1						(B32)	1		RA2	000				2" 12					6 7 8 9	FL-16 WT-10 FL-16 FL-16 FL-16 MT-11	1 20'- A CL CR	
			G-2					G	-3															 M	IEMBER RAME L	TABLE	
7											_			G-	-4			3						N E E	<u>IARK</u> C-1 C-2	PART 8M350 8M350	C12
			<u>G-2</u> G-2	(T18)				<u> </u>	<u>-3</u>					<u> </u>		(		3							G-1 G-2 G-3	8X252 8X252 8X252	16 16
			0-2					G-	- <u></u>					0-4	-			3							<u>G-4</u> ONNECT RAME LI	<u>8X257</u> 0N PLAT NE 11	
] EC-1					EC	] ] ;2					EC-	1				·										<u>K/PART</u> 1	
	EN	IDW <i>A</i>	ALL	FRAI	MINC	G: F	RAM	EL	NE	11															3   SC-	-5	
			F	anel <u>(</u>	Start	Pan	i <u>e</u> l Sto	art																			
											B	)					A										
4						5> 					<	3/4							2" 12								
								]											FIELD BI	EVEL CL	JT PANEL	S AS	REQUI	IRED			
1/4"	9 1/4"	1/4"	1/4"	1/4"	9 1/4"	9 1/4"	5 1/4"	1/4"	1/4"	1/4"	1/4"	/4"	1/4"	1/4"	,4" /	4"		]									
13'-3	13'-	14'-3	14'-9	15'–3	15'-9	15'-9	15'-3	14'-9	14'-3	13'-9	13'-3	12'-9 1	12'-3 1	11'-9 1,	11'-3 1/	10'-9 1/4"			E7								
	(/4"(TYP)																10'-3										
	4'-0 3	[A](TYP																									
						2>																					
ΓN	DWAI	(	SHFF	TINC	<u>,</u> &r	TRI	M·	FRAM	JF I	INF	11					FIE	ELD CU	JT	PANELS AS	REQUIF	RED						
nel. 6" diameta	er holes.	PAN [A]	PANEL	26 Ga.	SSX	– SM	P Hav	vaiian	Blue								<b>Ъ.</b> С.	ninces	vhose seal and signature ap	nnege on three do	umente reproceste "	hidwind Charl		HION MANNAME	СА	ROIL ROIL S	
istallation Building Pr		3.	tion act f	and en-						ISSU			DESCR			′ СНК	Building respons and exe SHEET DESCI	s, Inc., ibility is cludes p RIPTION:	and is not the Engineer of imited to material designed part such as doors, windows	f Record for the ov ed and manufacture s, foundation design BLDG SIZE: 1 70'-	verall project. The Eng ad by Whirlwind Steel n, and erection of th 0" x 273'-0" x 14	gineer's Buildings, Inc., e building.					
pretation of allation" can RUCTION PER	or approval, a tation only. Th the project do be considered <u>MIT:</u> or permit, are ector Installatio <u>ION:</u> ruction.	cuments. 0 d complete.	nly drawings	issued "For		EA	K	STI			01.30.2	4 FOR	CONSTRUCT	TION PERMIT	PND	PNC	CUSTOMER: BA PROJECT REF	UCOM B FERENCE: UCOM B ATION: FUQUA CHK:	BUSINESS PLAZA :: BUSINESS PLAZA AY VARINA, NC 27526 DATE: ENG		CUSTOMER LOCATION: FUQUAY VARINA, N JOBSITE COUN WAKE DWG NO:	C 27526					



- 14. Downspout straps are located 6" from base and at every girt location.

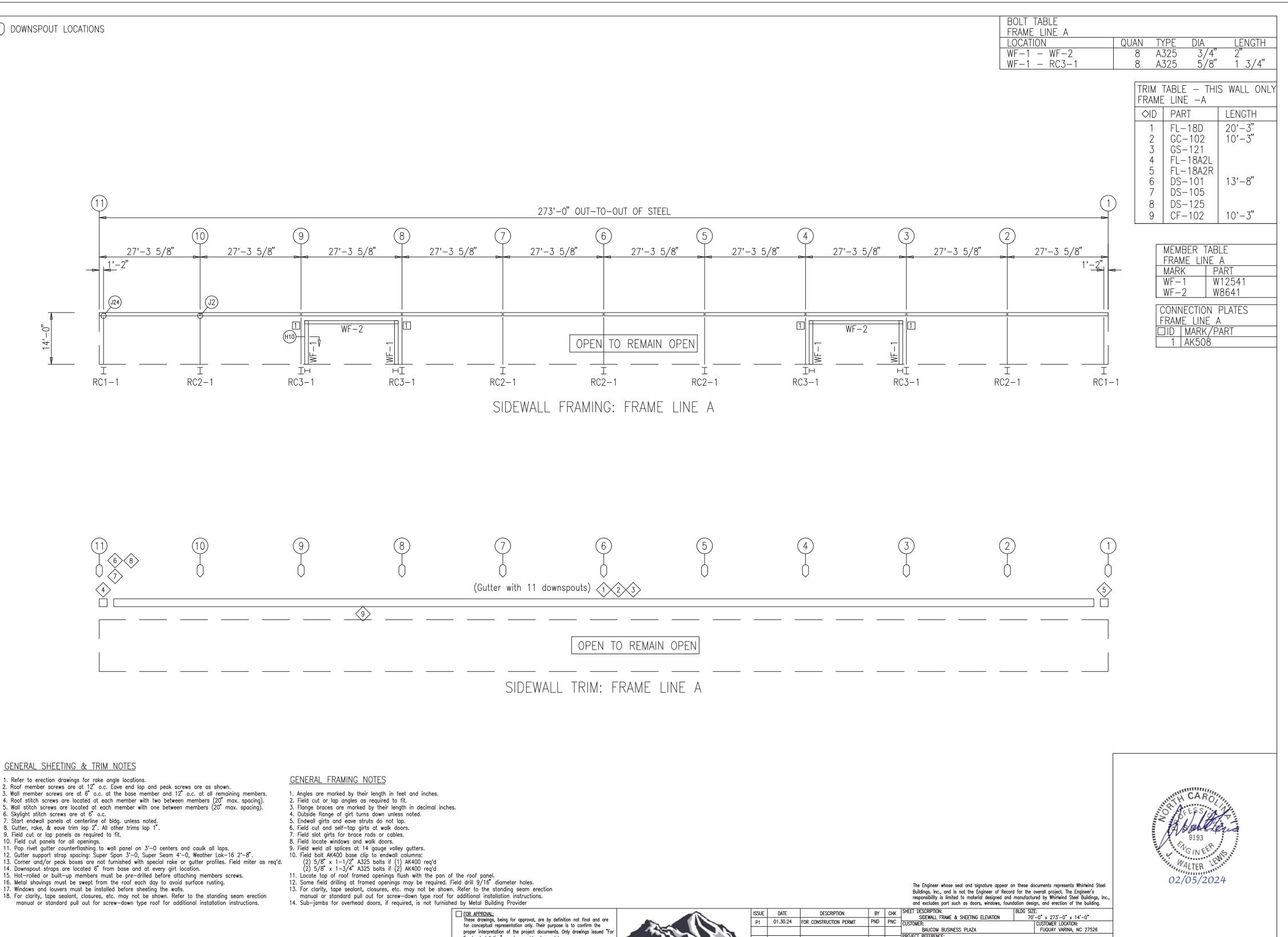
- 18. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.





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- 14. Downspout straps are located 6" from base and at every girt location.

- 18. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection

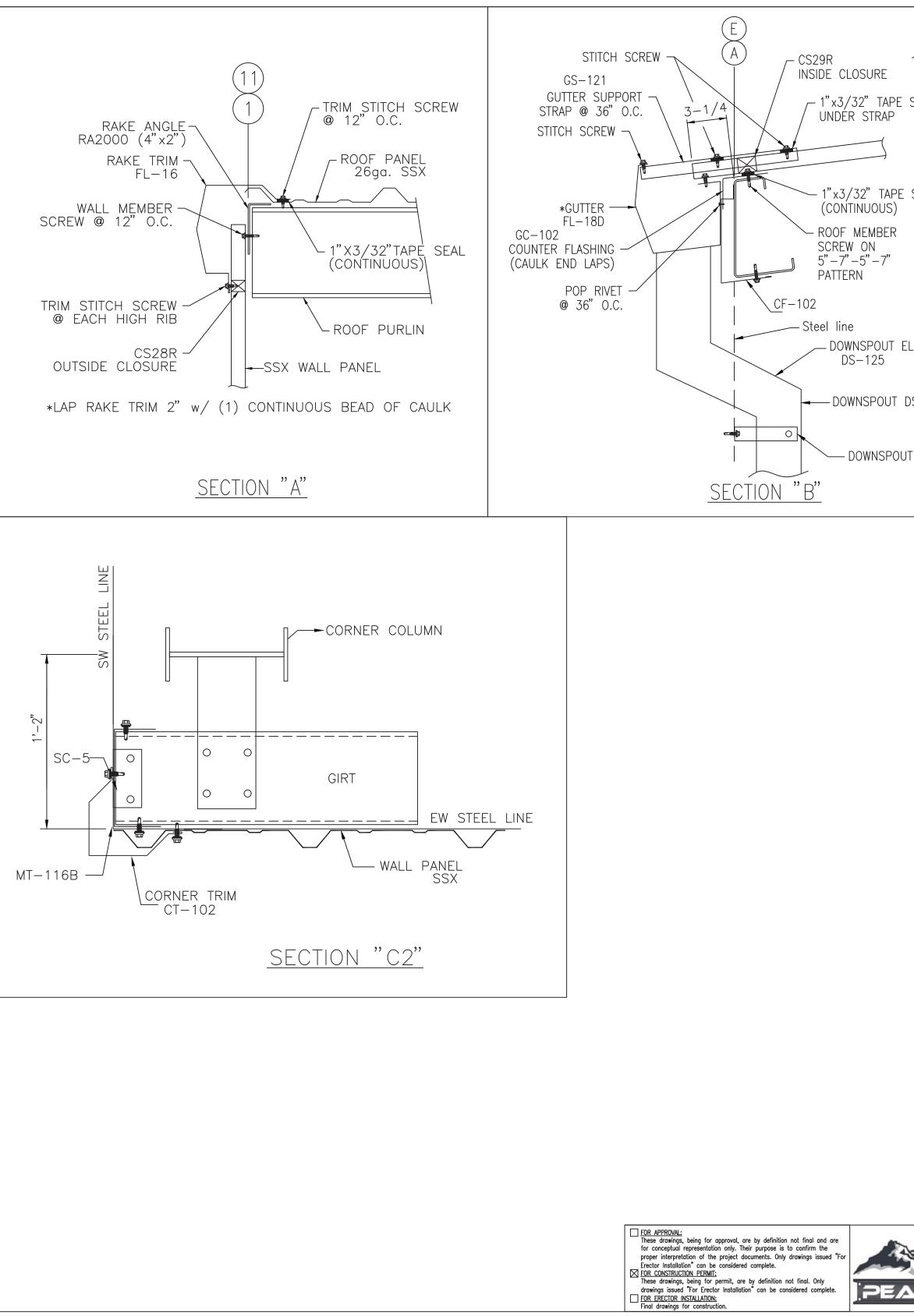
FOR ERECTOR INSTALLATION: Final drawings for construction.

Erector Installation" can be considered complete. FOR CONSTRUCTION PERMIT: These drawings, being for permit, are by definition not final. Only drawings issued "For Erector Installation" can be considered complete.



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ROJECT REFERENCE: BAUCOM BUSINESS PLAZA JOBSITE LOCATION: JOBSITE COUNTY: FUQUAY VARINA, NC 27526 WAKE DWG NO: JOB NO: ISSUE: CHK: DATE: ENG: 01.30.24 NPK 12252-33816 PND PNC E6 P1

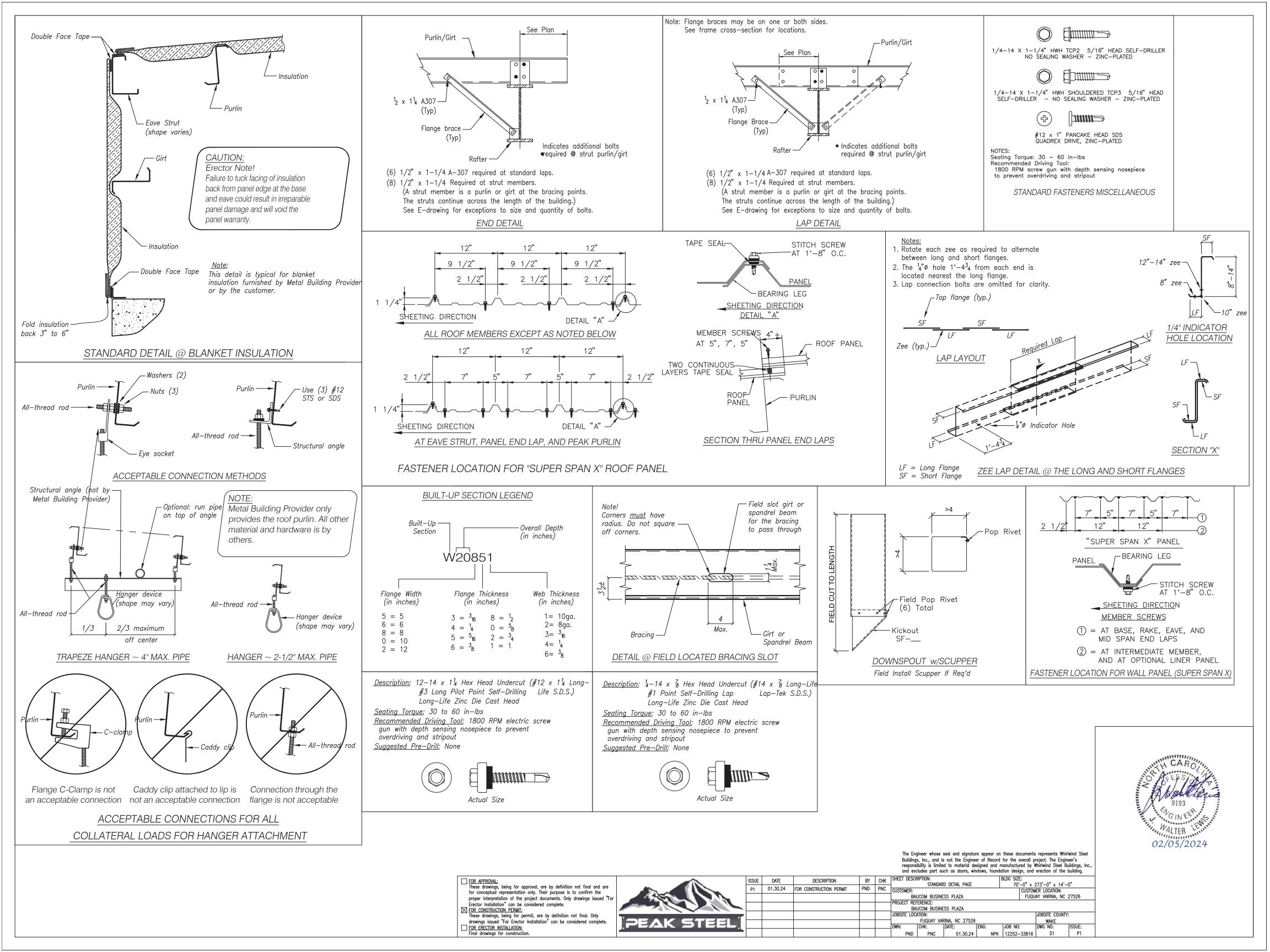


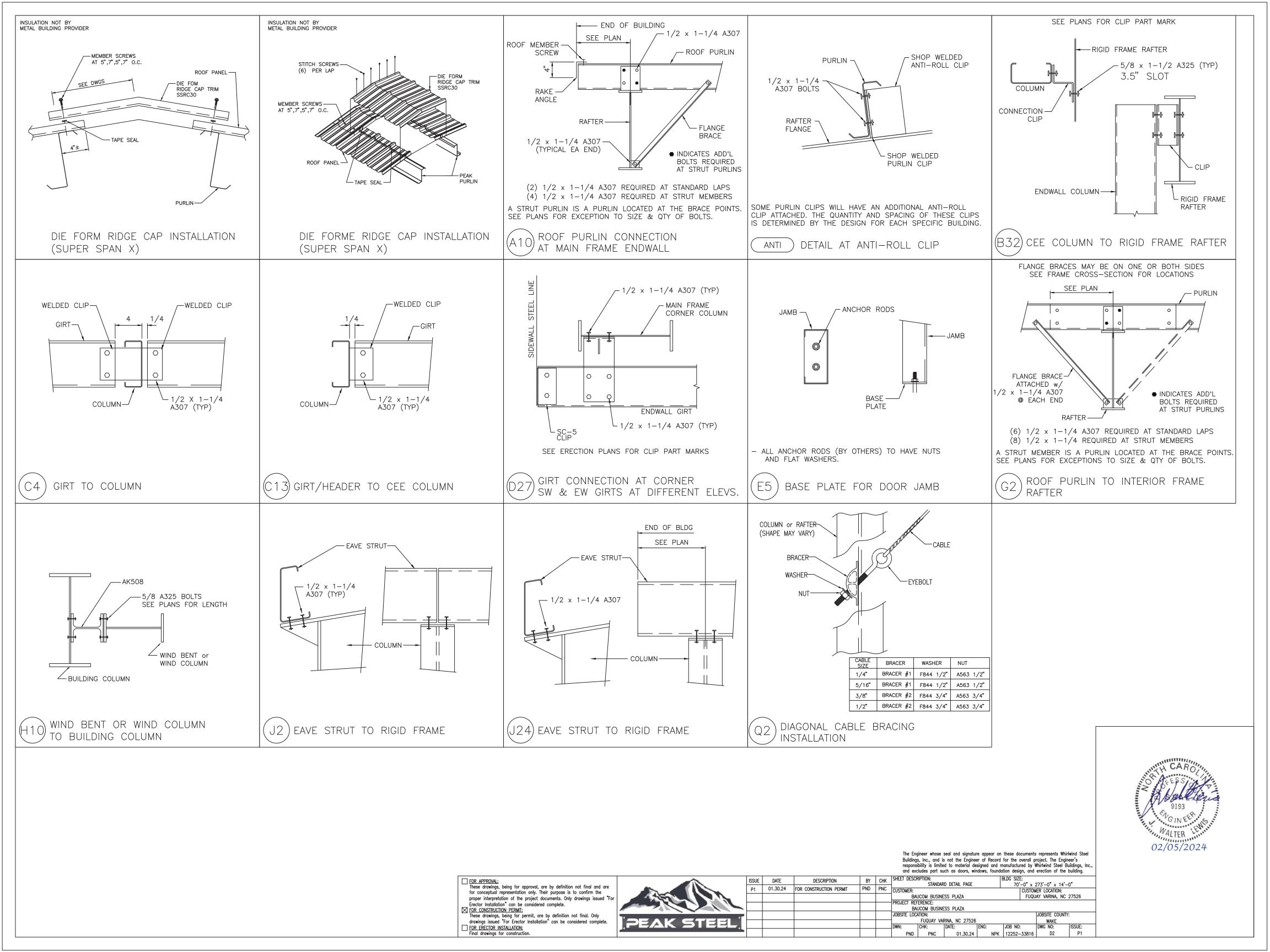
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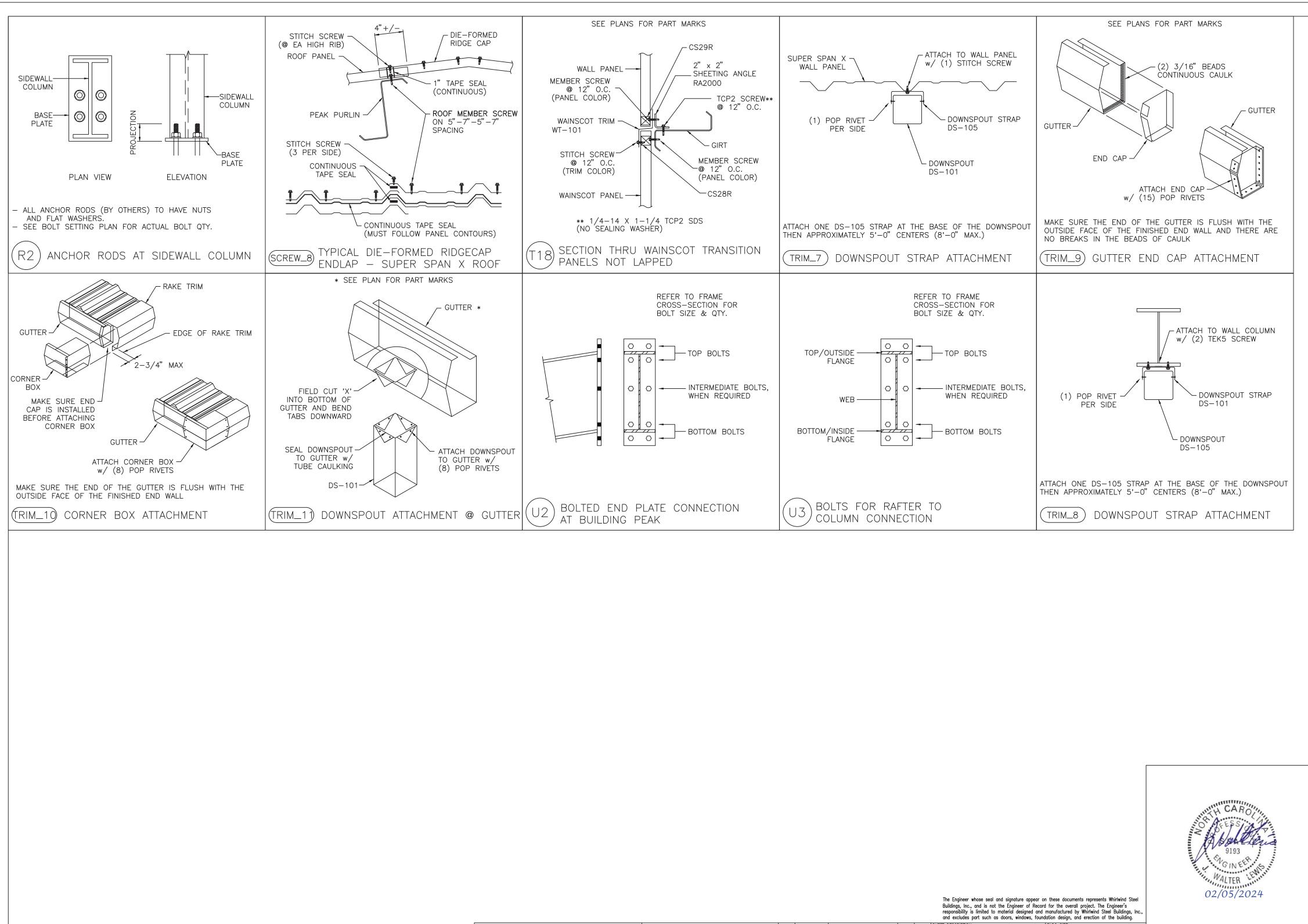


### The Engineer whose seal and signature appear on these documents represents Whirlwind Steel Buildings, Inc., and is not the Engineer of Record for the overall project. The Engineer's responsibility is limited to material designed and manufactured by Whirlwind Steel Buildings, Inc., and excludes part such as doors, windows, foundation design, and erection of the building.

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FOR APPROVAL:
These drawings, being for approval, are by definition not final and are
for conceptual representation only. Their purpose is to confirm the
proper interpretation of the project documents. Only drawings issued "Fo
Erector Installation" can be considered complete.
FOR CONSTRUCTION PERMIT:
These drawings, being for permit, are by definition not final. Only
drawings issued "For Erector Installation" can be considered complete.
FOR ERECTOR INSTALLATION:
Final drawings for construction.



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