

# GENERAL NOTES:

1. MATERIALS	ASTM DESCRIPTION
STRUCTURAL STEEL PLATE	A529 / A572 / A1011
HOT ROLLED MILL SHAPES	A36 / A529 / A500
HHS ROUND	A500
HHS RECTANGULAR	A500
COLD FORM SHAPES	A653 / A1011
ROOF AND WALL SHEETING	A653 / A792
BOLTS	A307 / A325 / A490
CABLE	A475
RODS	A529 / A572

## 2. STRUCTURAL PRIMER NOTE:

SHOP COAT PRIMER IS INTENDED TO PROTECT THE STEEL FRAMING FOR A SHORT PERIOD OF TIME. STORAGE IN EXTREME COLD TEMPERATURES OR WINTER SNOW CONDITIONS, INCLUDING TRANSPORTATION ON SALTED OR CHEMICALLY TREATED ROADS WILL ADVERSELY AFFECT THE DURABILITY AND LONGEVITY OF THE PRIMER. THE COAT OF SHOP PRIMER DOES NOT PROVIDE THE UNIFORMITY OF APPEARANCE, OR THE DURABILITY AND CORROSION RESISTANCE OF A FIELD APPLIED FINISH COAT OF PAINT OVER A SHOP PRIMER. MINOR ABRASIONS TO THE SHOP COAT PRIMER CAUSED BY HANDLING, LOADING, SHIPPING, UNLOADING AND ERECTION ARE UNAVOIDABLE AND ARE NOT THE RESPONSIBILITY OF THE METAL BUILDING MANUFACTURER. METAL BUILDING MANUFACTURER IS NOT RESPONSIBLE FOR THE DETERIORATION OF THE PRIMER OR CORROSION THAT MAY RESULT FROM ATMOSPHERIC AND ENVIRONMENTAL CONDITIONS NOR THE COMPATIBILITY OF THE PRIMER TO ANY FIELD APPLIED COATING.

## 3. BUILDING ERECTION NOTES:

THE GENERAL CONTRACTOR AND/OR ERECTOR IS RESPONSIBLE TO SAFELY AND PROPERLY ERECT THE METAL BUILDING SYSTEM IN CONFORMANCE WITH THESE DRAWINGS, OSHA REQUIREMENTS, AND EITHER MBMA OR CSA S16 STANDARDS PERTAINING TO PROPER ERECTION. TEMPORARY SUPPORTS SUCH AS GUYS, BRACES, FALSEWORK, CRIBBING OR OTHER ELEMENTS FOR ERECTION ARE TO BE DETERMINED, FURNISHED AND INSTALLED BY THE ERECTOR. THESE SUPPORTS MUST SECURE THE STEEL FRAMING, OR PARTLY ASSEMBLED STEEL FRAMING, AGAINST LOADS COMPARABLE IN INTENSITY TO THOSE FOR WHICH THE STRUCTURE WAS DESIGNED IN ADDITION TO LOADS RESULTING FROM THE ERECTION OPERATION. SECONDARY WALL AND ROOF FRAMING (PURLINS, GIRTS AND/OR JOIST) ARE NOT DESIGNED TO FUNCTION AS A WORKING PLATFORM OR TO PROVIDE AS AN ANCHORAGE POINT FOR A FALL ARREST /SAFETY TIE OFF.

## 4. SPECIAL INSPECTION:

SPECIAL INSPECTIONS AND TESTING THAT MAY BE REQUIRED BY GOVERNMENTAL OR OTHER AUTHORITY DURING CONSTRUCTION AND/OR STEEL FABRICATION (COLLECTIVELY, "INSPECTIONS") ARE NOT THE RESPONSIBILITY OF THE PEMB MANUFACTURER, AND TO THE EXTENT REQUIRED IT SHALL BE THE RESPONSIBILITY OF THE OWNER AND/OR THE OWNER'S REPRESENTATIVE. IN THE EVENT INSPECTIONS ARE REQUIRED, THE OWNER AND/OR THE OWNER'S REPRESENTATIVE SHALL EMPLOY A THIRD PARTY QUALITY ASSURANCE TESTING AGENCY APPROVED BY THE RELEVANT AUTHORITY. IF SUCH REQUIREMENTS ARE NOT SPECIFICALLY INCLUDED IN THE PEMB MANUFACTURER'S SALES DOCUMENTS, NO INSPECTIONS BY THE PEMB MANUFACTURER OR AT THE PEMB MANUFACTURER'S FACILITY SHALL BE MADE. THE PEMB MANUFACTURER'S FACILITIES ARE ACCREDITED BY IAS AC472.

## 5. A325 & A490 BOLT TIGHTENING REQUIREMENTS:

IT IS THE RESPONSIBILITY OF THE ERECTOR TO ENSURE PROPER BOLT TIGHTNESS IN ACCORDANCE WITH APPLICABLE REGULATIONS. FOR PROJECTS IN THE UNITED STATES, SEE THE RSCC SPECIFICATION FOR STRUCTURAL JOINTS USING A325 OR A490 BOLTS OR FOR PROJECTS IN CANADA, SEE THE CAN/CSA S16 LIMIT STATES DESIGN OF STEEL STRUCTURES FOR MORE INFORMATION.

THE FOLLOWING CRITERIA MAY BE USED TO DETERMINE THE BOLT TIGHTNESS (I.E., "SNUG-TIGHT" OR "FULLY-PRETENSIONED"), UNLESS REQUIRED OTHERWISE BY LOCAL JURISDICTION OR CONTRACT REQUIREMENTS:

- ALL A490 BOLTS SHALL BE "FULLY-PRETENSIONED".
- ALL A325 BOLTS IN PRIMARY FRAMING (RIGID FRAMES AND BRACING) MAY BE "SNUG-TIGHT", EXCEPT AS FOLLOWS: "FULLY-PRETENSION" A325 BOLTS IF:
  - BUILDING SUPPORTS A CRANE SYSTEM WITH A CAPACITY GREATER THAN 5 TONS.
  - BUILDING SUPPORTS MACHINERY THAT CREATES VIBRATION, IMPACT, OR STRESS-REVERSALS ON THE CONNECTIONS. THE ENGINEER-OF-RECORD FOR THE PROJECT SHOULD BE CONSULTED TO EVALUATE FOR THIS CONDITION.
  - THE PROJECT SITE IS LOCATED IN A HIGH SEISMIC AREA. FOR IBC-BASED CODES, "HIGH SEISMIC AREA" IS DEFINED AS "SEISMIC DESIGN CATEGORY" OF 'D', 'E', OR 'F'. SEE THE "BUILDING LOADS" SECTION ON THIS PAGE FOR THE DEFINED SEISMIC DESIGN CATEGORY FOR THIS PROJECT.
  - ANY CONNECTION DESIGNATED IN THESE DRAWINGS AS "A325-SC". "SLIP-CRITICAL (SC)" CONNECTIONS MUST BE FREE OF PAINT, OIL, OR OTHER MATERIALS THAT REDUCE FRICTION AT CONTACT SURFACES. GALVANIZED OR LIGHTLY-RUSTED SURFACES ARE ACCEPTABLE.
- IN CANADA, ALL A325 AND A490 BOLTS SHALL BE "FULLY-PRETENSIONED", EXCEPT FOR SECONDARY MEMBERS (PURLINS, GIRTS, OPENING FRAMING, ETC.) AND FLANGE BRACES.

SECONDARY MEMBERS (PURLINS, GIRTS, OPENING FRAMING, ETC.) AND FLANGE BRACE CONNECTIONS MAY ALWAYS BE "SNUG-TIGHT", UNLESS INDICATED OTHERWISE IN THESE DRAWINGS.

## 6. GENERAL DESIGN NOTES:

- ALL STRUCTURAL STEEL SECTIONS AND WELDED PLATE MEMBERS ARE DESIGNED IN ACCORDANCE WITH ANSI/AISC 360 "SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS" OR THE CAN/CSA S16 "LIMIT STATES DESIGN OF STEEL STRUCTURES", AS REQUIRED BY THE SPECIFIED BUILDING CODE.
- ALL WELDING OF STRUCTURAL STEEL IS BASED ON EITHER AWS D1.1 "STRUCTURAL WELDING CODE - STEEL" OR CAN/CSA W59 "WELDED STEEL CONSTRUCTION (METAL ARC WELDING)", AS REQUIRED BY THE SPECIFIED BUILDING CODE.
- ALL COLD FORMED MEMBERS ARE DESIGNED IN ACCORDANCE WITH ANSI/AISI S100 OR CAN/CSA S136 "SPECIFICATIONS FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS", AS REQUIRED BY THE SPECIFIED BUILDING CODE.
- ALL WELDING OF COLD FORMED STEEL IS BASED ON AWS D1.3 "STRUCTURAL WELDING CODE - SHEET STEEL" OR CAN/CSA W59 "WELDED STEEL CONSTRUCTION (METAL ARC WELDING)", AS REQUIRED BY THE SPECIFIED BUILDING CODE.
- ALL NUCOR BUILDING GROUP FACILITIES ARE IAS AC-472 ACCREDITED FOR DESIGN AND FABRICATION OF METAL BUILDING SYSTEMS. FOR PROJECTS IN CANADA, DESIGN AND FABRICATION ARE DONE ONLY IN FACILITIES THAT ARE ALSO CAN/CSA A660 AND W47.1 CERTIFIED.
- IF JOISTS ARE INCLUDED WITH THIS PROJECT, THEY ARE SUPPLIED AS A PART OF THE SYSTEMS ENGINEERED METAL BUILDING AND ARE FABRICATED IN ACCORDANCE WITH THE REQUIREMENTS OF SECTION 1926.758 OF THE OSHA SAFETY STANDARDS FOR STEEL ERECTION, DATED JANUARY 18, 2001.
- COLUMN BASE PLATES ARE DESIGNED NOT TO EXCEED THE ALLOWABLE BEARING STRESS OF CONCRETE THAT HAS A MINIMUM COMPRESSIVE STRENGTH OF 3000 P.S.I. AT 28 DAYS.

# BUILDING INFORMATION

## PRIMER COLORS

PRIMARY PRIMER COLOR: RED SECONDARY PRIMER COLOR: RED

## ROOF SHEETING

TYPE: S3P GAUGE: 24 FINISH: GALVALUME CLIP TYPE: TALL  
 THERMAL BLOCKS: YES EPS FOAM SPACER: NO ROOF LINE TRIM, PAINTED: POLAR WHITE/GALVALUME  
 YES  NO  DOWNSPOUTS PAINTED: POLAR WHITE GUTTERS PAINTED: POLAR WHITE  
 YES  NO  INSULATION 4 INCH (NOT BY MBS)  
 YES  NO  PIPE JACKS, SIZE: \_\_\_\_\_ QUANTITY: \_\_\_\_\_  
 YES  NO  RIDGE VENTS, 10'-0" LONG X 9" THROAT. QUANTITY: \_\_\_\_\_  
 YES  NO  ROOF FRAMED OPENINGS, SEE ROOF FRAMING PLAN FOR SIZES  
 YES  NO  COMPOSITE N/A DECK, TYPE: \_\_\_\_\_ GAUGE: \_\_\_\_\_ FINISH: \_\_\_\_\_

## WALL SHEETING

TYPE: A3P GAUGE: 26 FINISH: FOX GRAY  
 CORNER TRIM, PAINTED: FOX GRAY BASE TRIM, PAINTED: BURNISHED SLATE  
 YES  NO  WALKDOORS, QUANTITY: 2 (3070) PAINTED: WHITE  
 YES  NO  WINDOWS, QUANTITY: \_\_\_\_\_ PAINTED: \_\_\_\_\_  
 YES  NO  INSULATION 4.38 INCH (NOT BY MBS)

## WALL FRAMED OPENINGS

YES  NO  FRAMED OPENING TRIM, PAINTED: POLAR WHITE (SEE DRAWING ELEVATIONS)

## BUILDING OPTIONS

YES  NO  LINER PANELS  
 YES  NO  TRANSLUCENT PANELS  
 WALL: \_\_\_\_\_  
 ROOF: 2  
 INSULATED PANELS? YES  NO   
 YES  NO  EAVE EXTENSION  
 YES  NO  RAKE EXTENSION  
 YES  NO  CANOPY  
 YES  NO  PARTITION WALLS  
 YES  NO  WAINSCOT  
 YES  NO  FASCIA  
 YES  NO  PARAPET  
 YES  NO  CRANES (SEE CRANE PLAN FOR ADDITIONAL INFORMATION)  
 YES  NO  MEZZANINE (SEE MEZZANINE PLAN FOR ADDITIONAL INFORMATION)

FOR OCCUPANCY (RISK) CATEGORY I OR II, IBC PROVISIONS INDICATE THAT SINGLE-STORY BUILDINGS SHALL HAVE "NO DRIFT LIMIT" PROVIDED THAT INTERIOR WALLS, PARTITIONS, CEILINGS, AND EXTERIOR WALL SYSTEMS HAVE BEEN DESIGNED TO ACCOMMODATE THE SEISMIC STORY DRIFTS. INTERIOR WALLS, PARTITIONS, CEILINGS, OR EXTERIOR WALL SYSTEMS NOT PROVIDED BY THE METAL BUILDING MANUFACTURER SHALL BE DESIGNED AND DETAILED BY OTHERS TO ACCOMMODATE THE SEISMIC STORY DRIFTS. SEISMIC DRIFT VALUES MAY BE OBTAINED FROM THE METAL BUILDING MANUFACTURER.

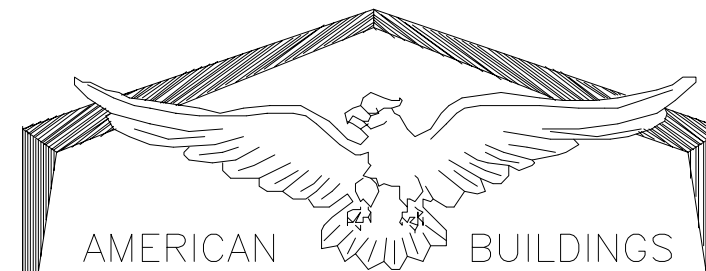
THIS BUILDING SYSTEM DESIGN IS BASED ON UNIFORMLY APPLYING THE CONTRACT-SPECIFIED LIVE LOAD AND ROOF SNOW LOAD. IN ADDITION, THE DESIGN IS BASED ON APPLYING A CODE-DEFINED LIVE LOAD (INCLUDING APPLICABLE REDUCTIONS) AND A CODE-DEFINED SNOW LOAD (BASED ON CONTRACT-SPECIFIED GROUND SNOW) FOR ALL PARTIAL LOADING AND UNBALANCED SNOW LOAD CONDITIONS.

IF SNOW GUARDS OR OTHER DEVICES INTENDED TO HOLD SNOW AND/OR ICE ACCUMULATION ON THE ROOF SYSTEM ARE TO BE USED ON THIS PROJECT, THEY MUST BE INSTALLED UNDER THE GUIDANCE OF THE PROJECT "ENGINEER OF RECORD" (EOR), NOT THE METAL BUILDING MANUFACTURER, SO AS NOT TO EXCEED THE DESIGN ROOF SNOW LOAD ON THIS PROJECT.

THE DRAWINGS AND THE METAL BUILDING THEY REPRESENT ARE THE PRODUCT OF THE METAL BUILDING MANUFACTURER. THE REGISTERED PROFESSIONAL ENGINEER'S SEAL PERTAINS ONLY TO THE REQUIREMENTS LISTED HEREIN FOR THE MATERIALS DESIGNED AND SUPPLIED BY THE METAL BUILDING MANUFACTURER. THE REGISTERED PROFESSIONAL ENGINEER WHOSE SEAL APPEARS ON THESE DRAWINGS IS EMPLOYED OR ENGAGED BY THE METAL BUILDING MANUFACTURER AND DOES NOT SERVE AS OR REPRESENT THE PROJECT ENGINEER OF RECORD AND SHALL NOT BE CONSTRUED AS SUCH.

## 7. GLOSSARY OF ABBREVIATIONS:

A.B. = ANCHOR BOLTS	MAX = MAXIMUM	REQ'D = REQUIRED
BS = BOTH SIDES	M.B. = MACHINE BOLTS	REV. = REVISION
B.U. = BUILT-UP	MBS = METAL BUILDING SUPPLIER	SIM = SIMILAR
DIA = DIAMETER	TBD = TO BE DETERMINED	SL = STEEL LINE
FLG = FLANGE	N/A = NOT APPLICABLE	N.S. = NEAR SIDE
F.S = FAR SIDE	NIC = NOT IN CONTRACT	M.N. = MINIMUM
GA. = GAUGE	SLV = SHORT LEG VERTICAL	TYP = TYPICAL
H.S.B. = HIGH STRENGTH BOLTS	O.A.L. = OVERALL LENGTH	PL = PLATE
HT. = HEIGHT	O.C. = ON CENTER	
LLV = LONG LEG VERTICAL	U.N.O. = UNLESS NOTED OTHERWISE	
PEMB = PRE-ENGINEERED METAL BUILDING MANUFACTURER		
?? = PART MARK TO BE DETERMINED AND WILL BE UPDATED ON CONSTRUCTION DRAWINGS		



# BUILDING LOADS

DESIGN CODE: NORTH CAROLINA (NCBC 2018)

ROOF LIVE LOAD: 20.00 PSF MBMA OCC. CLASS: II

LIVE LOAD REDUCIBLE Yes \_\_\_\_\_

GROUND SNOW LOAD: 15.00 PSF SNOW EXP. FACTOR, Ce: 1.00

SNOW IMPORTANCE FACTOR, Is: 1.00

WIND: 115 / 89 MPH  
 (Vult) / (Vosd)

C & C PRESSURES (PSF): 22 / -29

EXPOSURE: B

UL 90 NO

L3P Roof-Const. No.161 ; L3P Roof w/ Translucent Panel-Const. No.167  
 S3P Roof-Const. No.552 ; S3P Roof w/ Translucent Panel-Const. No.590 ;  
 Composite CFR Roof-Const. No.552A ; N/A Roof-Const. No. \_\_\_\_\_

SEISMIC INFORMATION Ss: 0.173 S1: 0.083

Design Sds/Sd1: 0.185 / 0.133 Site Class: D

Seismic Imp. Factor: 1.00 Seismic Design Category: B

Analysis Procedure: Equivalent Lateral Force Method

Basic SFRS: Not Detailed for Seismic

## NOTES:

1) COLLATERAL DEAD LOADS, UNLESS OTHERWISE NOTED, ARE ASSUMED TO BE UNIFORMLY DISTRIBUTED. WHEN SUSPENDED SPRINKLER SYSTEMS, LIGHTING, HVAC EQUIPMENT, CEILINGS, ETC., ARE SUSPENDED FROM ROOF MEMBERS, CONSULT THE M.B.S. IF THESE CONCENTRATED LOADS EXCEED 500 POUNDS (USING THE WEB MOUNT DETAIL) OR 200 POUNDS (USING THE FLANGE MOUNT DETAIL), OR IF INDIVIDUAL MEMBERS ARE LOADED SIGNIFICANTLY MORE THAN OTHERS.

2) THE DESIGN OF STRUCTURAL MEMBERS SUPPORTING GRAVITY LOADS IS CONTROLLED BY THE MORE CRITICAL EFFECT OF ROOF LIVE LOAD OR ROOF SNOW LOAD, AS DETERMINED BY THE APPLICABLE CODE.

3) Pm IS BASED ON THE MINIMUM ROOF SNOW LOAD CALCULATED PER BUILDING CODE OR THE CONTRACT SPECIFIED SNOW LOAD, WHICHEVER IS GREATER. THIS VALUE, Pm, IS ONLY APPLIED IN COMBINATION WITH THE DEAD AND COLLATERAL LOADS. ROOF SNOW IN OTHER LOADING CONDITIONS IS DETERMINED PER THE SPECIFIED BUILDING CODE.

BUILDING	
ROOF DEAD (PSF):	3.0
PRI. COL. (PSF):	3.0
SEC. COL. (PSF):	3.0
SNOW Ct:	1.0
SNOW Cs:	1.00
ROOF SNOW Ps (PSF):	10.50
ROOF SNOW Pm (PSF):	15.00
WIND ENCLOSURE:	Closed
GCpi:	+/-0.18
SEISMIC R:	3.00
SEISMIC Cs:	0.062
BASE SHEAR (KIPS):	2.2

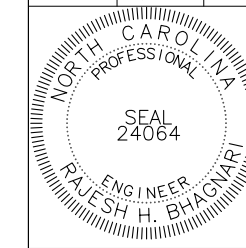
## DRAWING INDEX

COVERSHEET C1  
 ANCHOR BOLT DRAWINGS F1, F2  
 COLUMN BASE REACTIONS R1  
 STRUCTURAL/SHEETING DRAWINGS E1 - E8  
 DETAILS \_\_\_\_\_

DATE	ISSUE	ANCHOR BOLTS	PERMITS
5/31/2022			
5/31/2022			

Engineering Performed By:  
 Nucor Corporation  
 200 Whetstone Rd.  
 Swansea, SC 29460  
 COA# F-1470

PROJECT NAME  
**CDT**  
 62 PROGRESS DR, FUQUAY VARINA, NC 27526  
 CUSTOMER NAME  
**VINCENT BARBOUR AND COMPANY**  
 FUQUAY VARINA, NC 27526-6864  
 JOB NUMBER  
**A22B0392A**

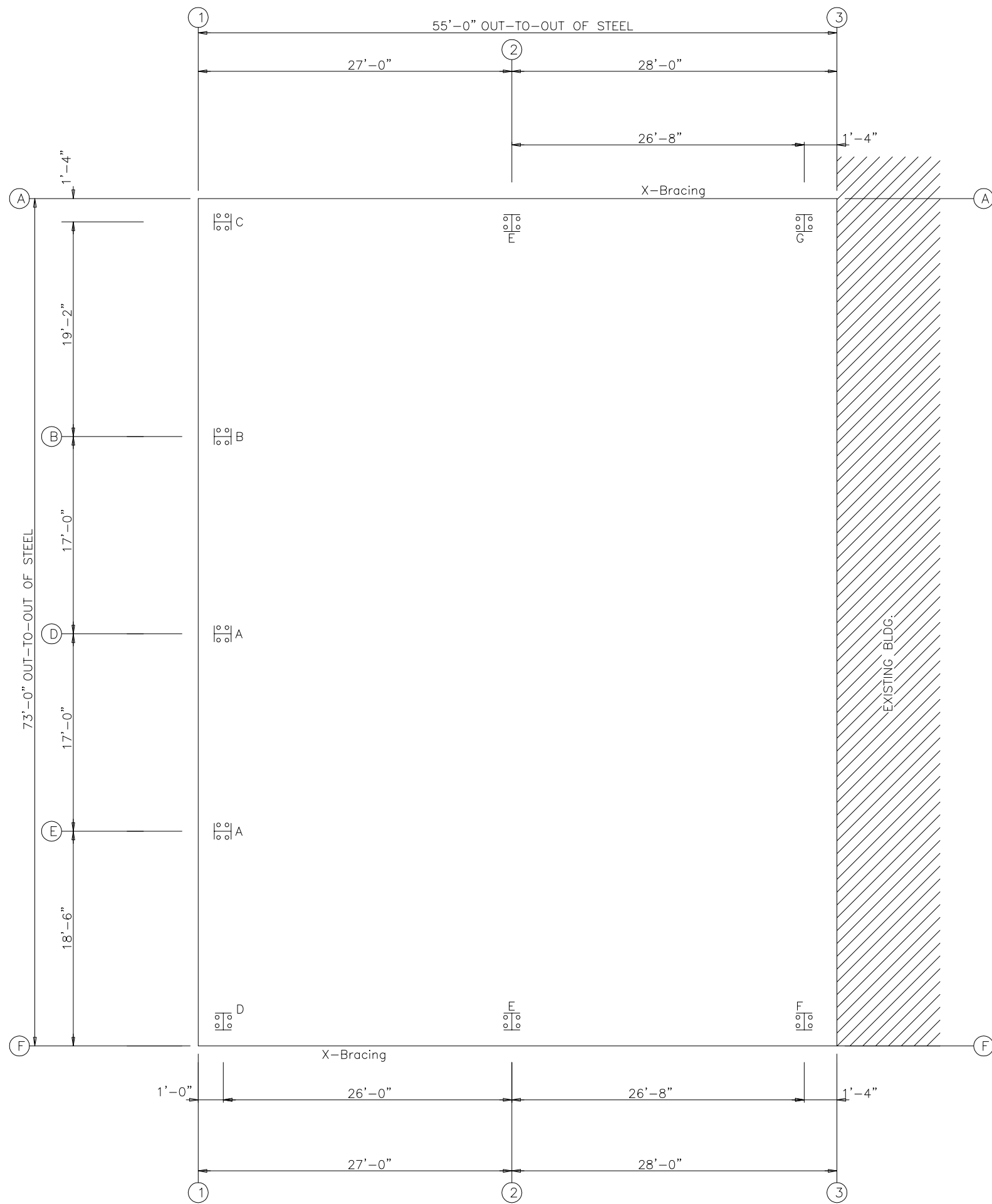


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SHEET TITLE

SHEET

C1 OF 1



ANCHOR BOLT PLAN  
 NOTE: All Base Plates @ 100'-0" (U.N.)

ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type	Proj (in)
20	Endwall	3/4"	F1554	3.00
16	Frame	3/4"	F1554	3.00

ANCHOR BOLT PLAN

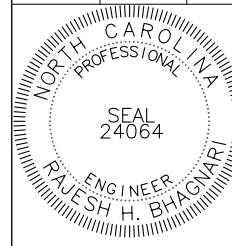
GENERAL NOTES

1. THE SPECIFIED ANCHOR ROD DIAMETER ASSUMES F1554 GRADE 36 UNLESS NOTED OTHERWISE. ANCHOR ROD MATERIAL OF EQUAL DIAMETER MEETING OR EXCEEDING THE STRENGTH REQUIREMENTS SET FORTH ON THESE DRAWINGS MAY BE UTILIZED AT THE DISCRETION OF THE FOUNDATION DESIGN ENGINEER. ANCHOR ROD EMBEDMENT LENGTH SHALL BE DETERMINED BY THE FOUNDATION DESIGN ENGINEER.
2. METAL BUILDING MANUFACTURER IS NOT RESPONSIBLE FOR PROJECT FOUNDATION DESIGN. THE FOUNDATION DESIGN IS THE RESPONSIBILITY OF A REGISTERED PROFESSIONAL ENGINEER, FAMILIAR WITH LOCAL SITE CONDITIONS.
3. ANCHOR RODS, NUTS, FLAT WASHERS FOR ANCHOR RODS, EXPANSION BOLTS, AND CONCRETE/MASONRY EMBEDMENT PLATES ARE NOT BY METAL BUILDING MANUFACTURER.
4. THE ANCHOR ROD LOCATIONS PROVIDED BY METAL BUILDING MANUFACTURER SATISFY PERTINENT REQUIREMENTS FOR THE DESIGN OF THE MATERIALS SUPPLIED BY THE METAL BUILDING MANUFACTURER. IT IS THE RESPONSIBILITY OF THE FOUNDATION ENGINEER TO MAKE CERTAIN THAT SUFFICIENT EDGE DISTANCE IS PROVIDED FOR ALL ANCHOR RODS IN THE DETAILS OF THE FOUNDATION DESIGN.
5. DRAWINGS ARE NOT TO SCALE. SEE DETAILS FOR COLUMN ORIENTATION.
6. THE ANCHOR ROD PLAN INDICATES WHERE THE ANCHOR RODS ARE TO BE PLACED AS WELL AS THE FOOTPRINT OF THE METAL BUILDING. IT IS ESSENTIAL THAT THESE ANCHOR ROD PATTERNS BE FOLLOWED. IF THESE SETTINGS DIFFER FROM THE ARCHITECTURAL FOUNDATION PLANS, THE METAL BUILDING MANUFACTURER MUST BE CONTACTED IMMEDIATELY - BEFORE CONCRETE IS PLACED.
7. "SINGLE" CEE COLUMNS SHALL BE ORIENTED WITH THE "TOES" TOWARD THE LOW EAVE UNLESS NOTED OTHERWISE.
8. ALL DIMENSIONS ARE OUT TO OUT OF STEEL. IF CONCRETE NOTCH IS REQUIRED THEN THE REQUIRED DIMENSION SHOULD BE ADDED TO OBTAIN THE OUT TO OUT OF CONCRETE DIMENSIONS.
9. FINISHED FLOOR ELEVATION = 100'-0" BOTTOM OF BASE PLATE = 100'-0" UNLESS NOTED OTHERWISE.

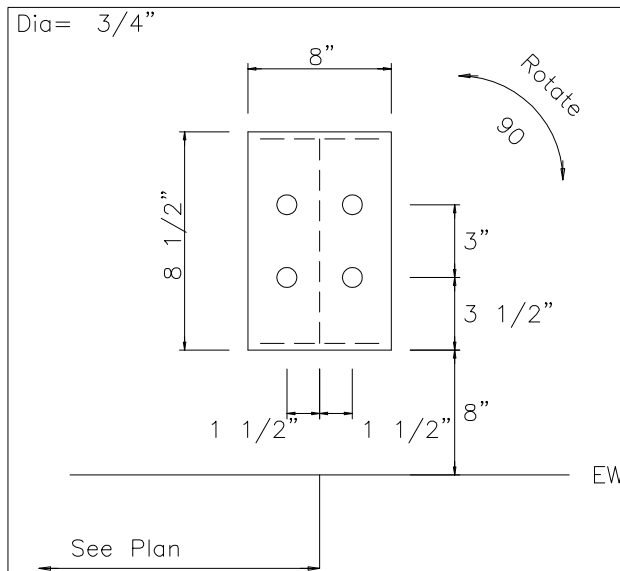
ISSUE	DATE
ANCHOR BOLTS	5/31/2022

Engineering Performed By:  
 Nucor Corporation  
 200 Whetstone Rd.  
 Swansea, SC 29460  
 COA# F-1470

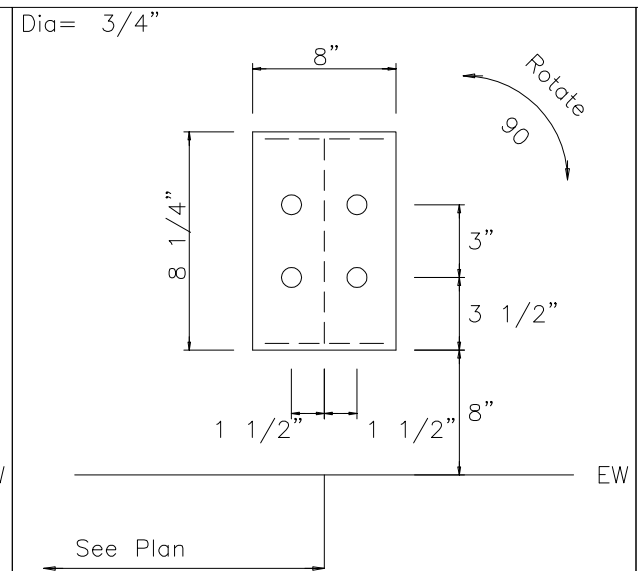
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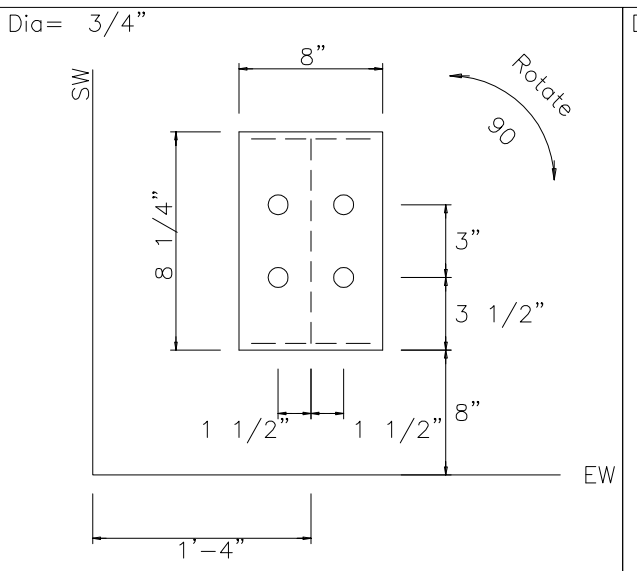
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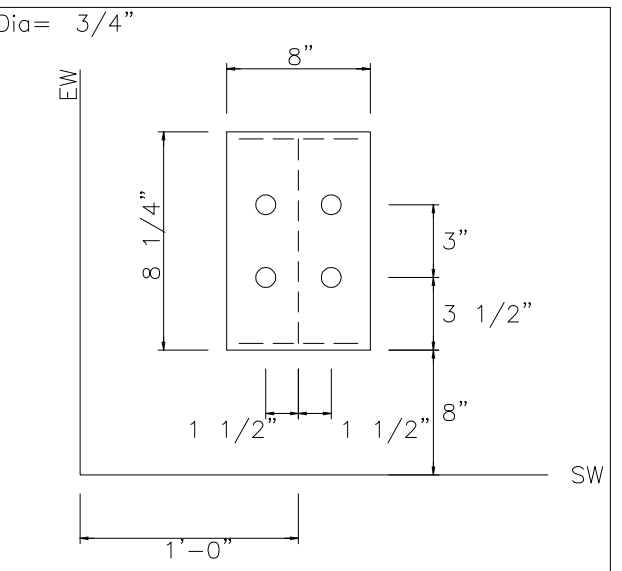
DETAIL A



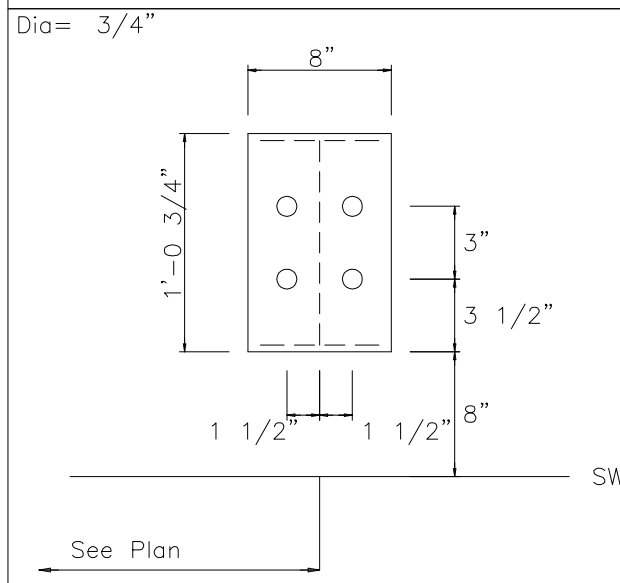
DETAIL B



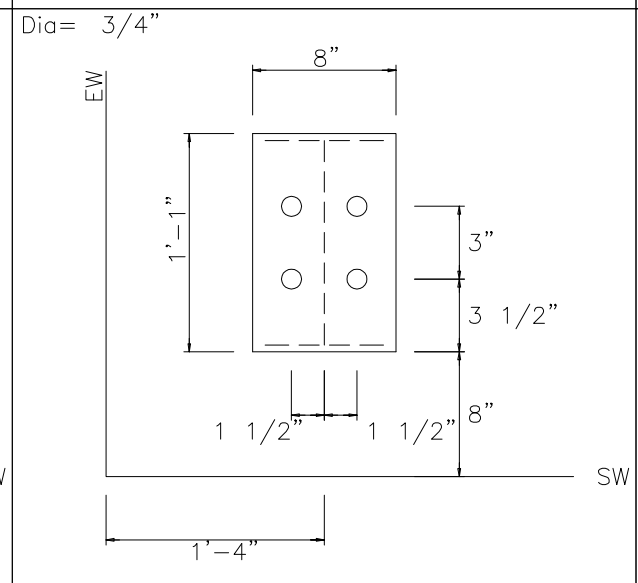
DETAIL C



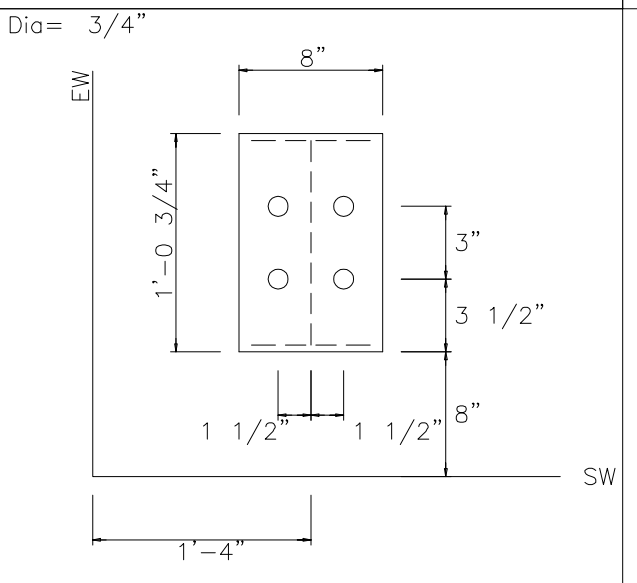
DETAIL D



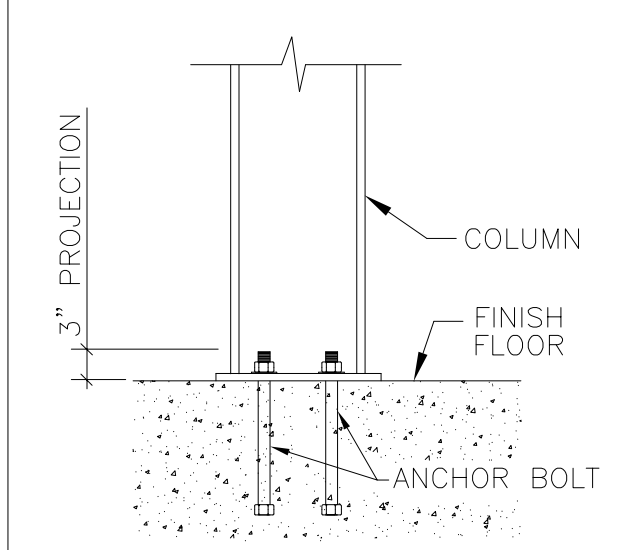
DETAIL E



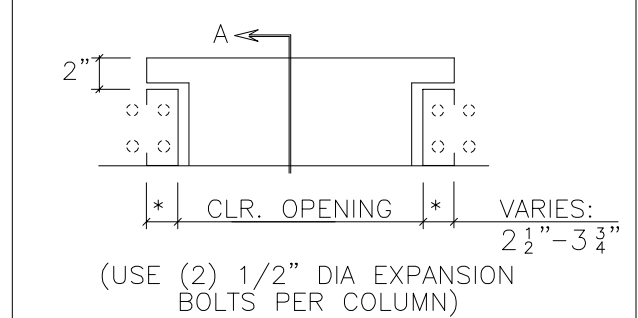
DETAIL F



DETAIL G



TYPICAL COLUMN BASE PLATE DETAIL



SECTION A  
(RECOMMENDED)  
TYPICAL OVERHEAD DOOR  
FRAMED OPENING

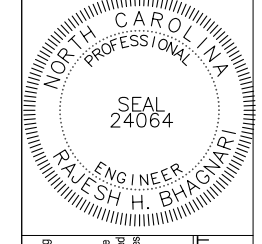
FOUNDATION DESIGN NOTES:

1. THE ORIENTATION OF THE ANCHOR BOLT DETAILS SHOWN ON THIS PAGE MAY NOT COINCIDE WITH THE ACTUAL COLUMN ORIENTATION SHOWN ON THE ANCHOR BOLT DRAWING. PLEASE REFERENCE THE SIDEWALL (SW) AND ENDWALL (EW) STEEL LINES SHOWN ON THE ANCHOR BOLT DETAILS WITH THE ANCHOR BOLT PLAN DURING LAYOUT OF COLUMN AND ANCHOR BOLT LOCATIONS.
2. COLUMN BASE PLATES MAY HAVE MORE HOLES THAN ARE REQUIRED DUE TO PRODUCTION LIMITATIONS. PLEASE FOLLOW ANCHOR BOLT DETAILS FOR QUANTITY OF ANCHOR BOLTS REQUIRED. EXTRA BASE PLATE HOLES DO NOT NEED INFILLED PER THE MBS DESIGN SPECIFICATIONS.

ISSUE	ANCHOR BOLTS	DATE	5/31/2022
CHK	AB	ENG	RHB
DRN	LC	PE	

Engineering Performed By:  
Nucor Corporation  
200 Whetstone Rd.  
Swansea, SC 29460  
COA# F-1470

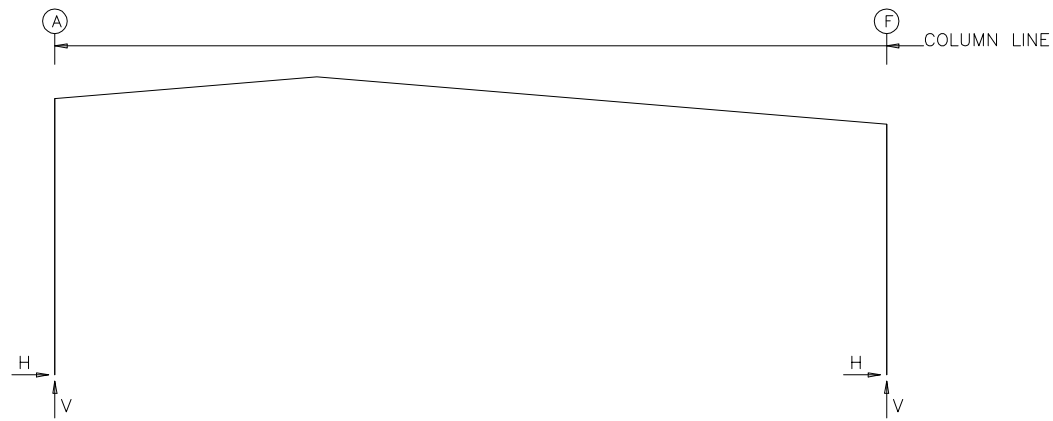
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SHEET  
F2 OF 2

FRAME LINES: 2 3



RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Qty	Bolt Dia	Base_Plate Width	Base_Plate Length	Base_Plate Thick	Elev. (in)
2	A	4	0.750	8.000	12.75	0.500	0.0
2	F	4	0.750	8.000	12.75	0.500	0.0

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Qty	Bolt Dia	Base_Plate Width	Base_Plate Length	Base_Plate Thick	Elev. (in)
3	A	4	0.750	8.000	12.75	0.375	0.0
3	F	4	0.750	8.000	13.00	0.375	0.0

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k )

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Snow Vert	Wind Left1 Vert	Wind Right1 Vert	Wind Left2 Vert	Wind Right2 Vert	Wind Press Horiz	Wind Suct Horiz	Wind Long2 Vert	Seis Left Vert
1	A	0.8	0.4	2.4	1.3	-2.8	-1.6	-1.8	-0.5	-2.2	2.6	-1.8	0.0
1	B	1.6	1.0	6.3	3.4	-6.5	-5.0	-4.4	-2.9	-4.2	4.6	-4.6	0.0
1	D	1.4	0.7	4.3	2.2	-2.6	-5.3	-1.0	-3.8	-3.7	4.1	-5.3	0.0
1	E	1.7	0.9	5.8	3.1	-4.0	-6.7	-1.8	-4.5	-3.6	3.9	-6.7	0.0
1	F	0.7	0.3	2.1	1.1	-1.3	-2.4	-0.5	-1.5	-2.6	3.0	-2.7	0.1

Frm Line	Col Line	Seis Right Vert	-MIN_SNOW- Horiz	E1UNB_SL_L- Vert	E1UNB_SL_R- Vert	E1PAT_LL_1- Horiz	E1PAT_LL_2- Horiz	E1PAT_LL_3- Horiz
1	A	0.1	0.0	1.9	0.0	0.3	0.0	2.4
1	B	-0.1	0.0	4.8	0.0	4.9	0.0	2.1
1	D	0.0	0.0	3.1	0.0	0.1	0.0	3.1
1	E	0.0	0.0	4.2	0.0	1.1	0.0	3.0
1	F	0.0	0.0	1.7	0.0	0.3	0.0	1.2

Frm Line	Col Line	E1PAT_LL_4- Horiz	E1PAT_LL_5- Horiz
1	A	0.0	-0.1
1	B	0.0	2.2
1	D	0.0	5.6
1	E	0.0	2.2
1	F	0.0	-0.2

BUILDING BRACING REACTIONS

Wall Loc	Col Line	± Reactions(k )				Panel_Shear (lb/ft)	Note
		Wind Horiz	Wind Vert	Seismic Horiz	Seismic Vert	Wind	Seis
L_SW	1						(i)
F_SW	F	1,2	7.3	5.4	1.4	1.0	
R_SW	3						(h)
B_SW	A	3,2	7.7	6.1	1.4	1.1	

(h)Rigid frame at endwall  
(i)Bracing in roof to rigid frame

RIGID FRAME: BASIC COLUMN REACTIONS (k )

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Snow Horiz	Snow Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert
2	A	2.6	5.7	2.0	3.9	8.2	15.5	7.2	13.5	-15.6	-21.9	-1.3	-15.1
2	F	-2.6	5.5	-2.0	3.9	-8.2	15.4	-7.2	13.5	-1.2	-11.8	18.1	-24.7

Frame Line	Column Line	Wind_Left2 Horiz	Wind_Left2 Vert	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	Seismic_Left Horiz	Seismic_Left Vert	Seismic_Right Horiz	Seismic_Right Vert
2	A	-14.0	-12.6	-0.1	-5.8	-3.3	-18.5	-6.4	-18.4	-1.2	-0.7	1.2	0.7
2	F	-2.9	-2.4	17.1	-15.4	3.8	-15.2	5.6	-21.5	-1.3	0.7	1.3	-0.7

Frame Line	Column Line	MIN_SNOW Horiz	MIN_SNOW Vert	F1UNB_SL_L Horiz	F1UNB_SL_L Vert	F1UNB_SL_R Horiz	F1UNB_SL_R Vert
2	A	10.2	19.3	4.6	14.3	7.0	9.8
2	F	-10.2	19.3	-4.6	6.0	-6.9	13.5

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Snow Horiz	Snow Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert
3	A	1.1	3.0	0.8	1.7	3.1	6.7	2.7	5.9	-6.6	-11.2	-1.9	-8.9
3	F	-1.1	3.1	-0.8	1.7	-3.1	6.7	-2.7	5.9	0.0	-6.7	7.8	-12.9

Frame Line	Column Line	Wind_Left2 Horiz	Wind_Left2 Vert	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	Seismic_Left Horiz	Seismic_Left Vert	Seismic_Right Horiz	Seismic_Right Vert
3	A	-6.0	-7.2	-1.4	-4.9	-1.5	-10.1	-3.2	-10.0	-0.4	-0.3	0.4	0.3
3	F	-0.6	-2.7	7.3	-8.8	1.8	-8.0	2.6	-12.0	-0.5	0.3	0.5	-0.3

Frame Line	Column Line	MIN_SNOW Horiz	MIN_SNOW Vert	F2UNB_SL_L Horiz	F2UNB_SL_L Vert	F2UNB_SL_R Horiz	F2UNB_SL_R Vert
3	A	3.8	8.5	1.8	6.2	2.6	4.3
3	F	-3.8	8.4	-1.8	2.6	-2.6	5.9

ENDWALL COLUMN: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Qty	Bolt Dia	Base_Plate Width	Base_Plate Length	Base_Plate Thick	Elev. (in)
1	A	4	0.750	8.000	8.250	0.375	0.0
1	B	4	0.750	8.000	8.250	0.375	0.0
1	D	4	0.750	8.000	8.500	0.375	0.0
1	E	4	0.750	8.000	8.500	0.375	0.0
1	F	4	0.750	8.000	8.250	0.375	0.0

GENERAL NOTES

- ALL LOADING CONDITIONS ARE EXAMINED. THE MAXIMUM AND MINIMUM HORIZONTAL (H) AND VERTICAL (V) REACTIONS AND THE CORRESPONDING VERTICAL (V) OR HORIZONTAL (H) REACTIONS ARE REPORTED.
- REACTIONS ARE PROVIDED BY LOAD CASE IN ORDER TO AID THE FOUNDATION ENGINEER IN DETERMINING THE APPROPRIATE LOAD FACTORS AND COMBINATIONS TO BE USED WITH EITHER WORKING STRESS OR ULTIMATE STRENGTH DESIGN METHODS. WIND LOAD CASES ARE GIVEN FOR EACH PRIMARY WIND DIRECTION.
- FOR ASCE7-10 AND LATER BASED BUILDING CODES, THE UNFACTORED LOAD CASE REACTIONS DUE TO WIND ARE GENERATED USING THE ULTIMATE DESIGN WIND SPEED (VuIt).
- POSITIVE (+) REACTIONS ARE AS SHOWN ABOVE. FOUNDATION LOADS ARE IN OPPOSITE DIRECTIONS.
- BRACING REACTIONS ARE IN THE PLANE OF THE BRACE WITH THE HORIZONTAL REACTION (H) ACTING AWAY FROM THE BRACED BAY AND THE VERTICAL REACTION (V) ACTING DOWNWARD.

\*\*\*\*\* RIGID FRAME LOAD CASE ABBREVIATIONS: \*\*\*\*\*

Wind\_L1/Wind\_R1: LATERAL WIND FROM THE LEFT/RIGHT, CASE 1  
 Wind\_L2/Wind\_R2: LATERAL WIND FROM THE LEFT/RIGHT, CASE 2  
 Wind\_Ln1/Wind\_Ln2: LONGITUDINAL WIND, CASE 1/2  
 Seismic\_L/Seismic\_R: LATERAL SEISMIC LOAD FROM LEFT/RIGHT  
 LWIND#\_L/E/LWIND#\_R#: LONGITUDINAL WIND EDGE ZONES  
 F#UNB\_SL\_L/F#UNB\_SL\_R: UNBALANCED ROOF SNOW WITH WIND FROM LEFT/RIGHT  
 F#PAT\_LL #/F#PAT\_SL #: PARTIAL LIVE/SNOW LOADING FOR CONTINUOUS BEAM SYSTEMS

\*\*\*\*\* ENDWALL COLUMN LOAD CASE ABBREVIATIONS: \*\*\*\*\*

Collat: COLLATERAL LOAD  
 Rafter Wind\_L/Rafter Wind\_R: LATERAL WIND FROM THE LEFT/RIGHT  
 Brace Wind\_L/Brace Wind\_R: LATERAL WIND FROM THE LEFT/RIGHT  
 Wind\_P/Wind\_S: LONGITUDINAL WIND PRESSURE/SUCTION ON COLUMNS  
 Wind\_Ln: LONGITUDINAL WIND SUCTION ON ROOF  
 Seis\_L/Seis\_R: LATERAL SEISMIC LOAD FROM LEFT/RIGHT  
 E#UNB\_SL\_L/E#UNB\_SL\_R: UNBALANCED ROOF SNOW WITH WIND FROM LEFT/RIGHT  
 E#PAT\_LL #/E#PAT\_SL #: PARTIAL LIVE/SNOW LOADING FOR CONTINUOUS BEAM SYSTEMS

DATE	ISSUE	CHK	ENG	PE
5/31/2022		AB	RHB	
		LC		

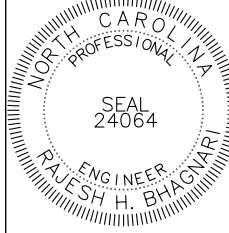
Engineering Performed By:  
 Nucor Corporation  
 200 Whetstone Rd.  
 Swansea, SC 29460  
 COA# F-1470

PROJECT NAME  
**CDT**  
 62 PROGRESS DR, FUQUAY VARINA, NC 27526

CUSTOMER NAME  
**VINCENT BARBOUR AND COMPANY**  
 FUQUAY VARINA, NC 27526-6864

JOB NUMBER  
**A22B0392A**

SHEET TITLE

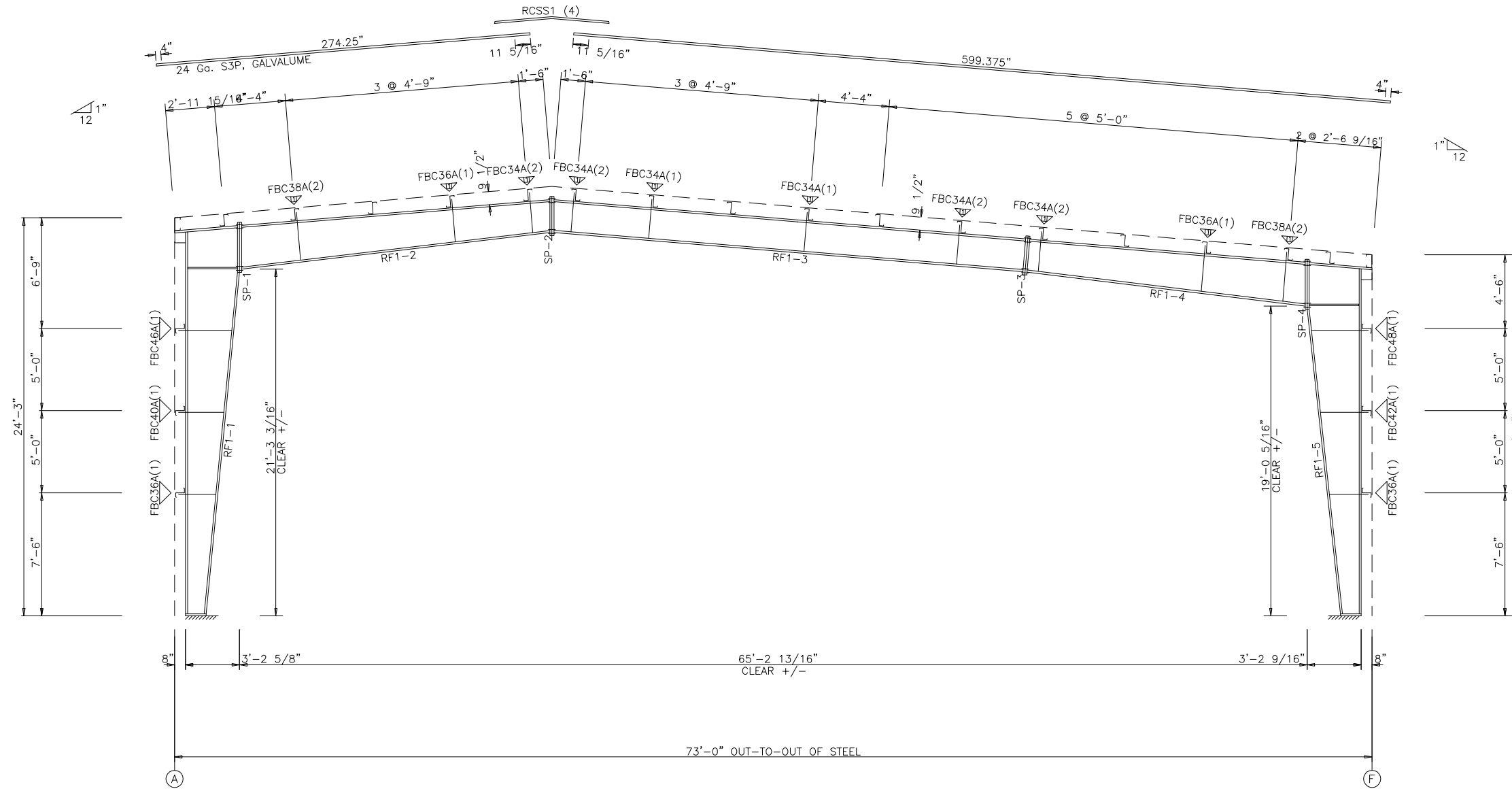


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SHEET  
**R1 OF 1**

SPLICE PLATE & BOLT TABLE									
Mark	Qty		Int	Type	Dia	Length	Width	Thick	Length
	Top	Bot							
SP-1	4	4	0	A325	0.875	3.00	6"	3/4"	3'-1"
SP-2	4	4	0	A325	0.625	2.25	6"	3/8"	2'-3 1/2"
SP-3	4	4	0	A325	0.625	2.25	6"	3/8"	2'-3 1/4"
SP-4	4	4	0	A325	0.875	3.00	6"	5/8"	3'-0 3/4"

CONNECTION PLATES	
ID	Mark/Part
1	FBL&N01



RIGID FRAME ELEVATION: FRAME LINE 2

GENERAL NOTES

1. INDICATES FLANGE BRACING LOCATIONS. (1) = ONE SIDE; (2) = TWO SIDES.
2. IF FLANGE BRACING IS REQUIRED ON BOTH SIDES OF AN EXPANDABLE RIGID FRAME, THE OPPOSITE SIDE FLANGE BRACES WILL HAVE TO BE INSTALLED AT THE TIME OF FUTURE EXPANSION. THESE FLANGE BRACES HAVE BEEN PROVIDED, AS REQUIRED, FOR THIS FUTURE CONDITION.
3. RIGID FRAMES SHALL HAVE 50% OF THEIR BOLTS INSTALLED AND TIGHTENED ON BOTH SIDES OF THE WEB ADJACENT TO EACH FLANGE BEFORE THE HOISTING EQUIPMENT IS RELEASED.
4. INTERIOR COLUMN METAL TAG IS ORIENTED TOWARD THE LOW EAVE OF THE BUILDING.

ISSUE	PERMITS	DATE
1		5/31/2022

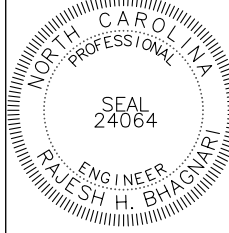
Engineering Performed By:  
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 200 Whetstone Rd.  
 Swansea, SC 29460  
 COA# F-1470

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SHEET TITLE

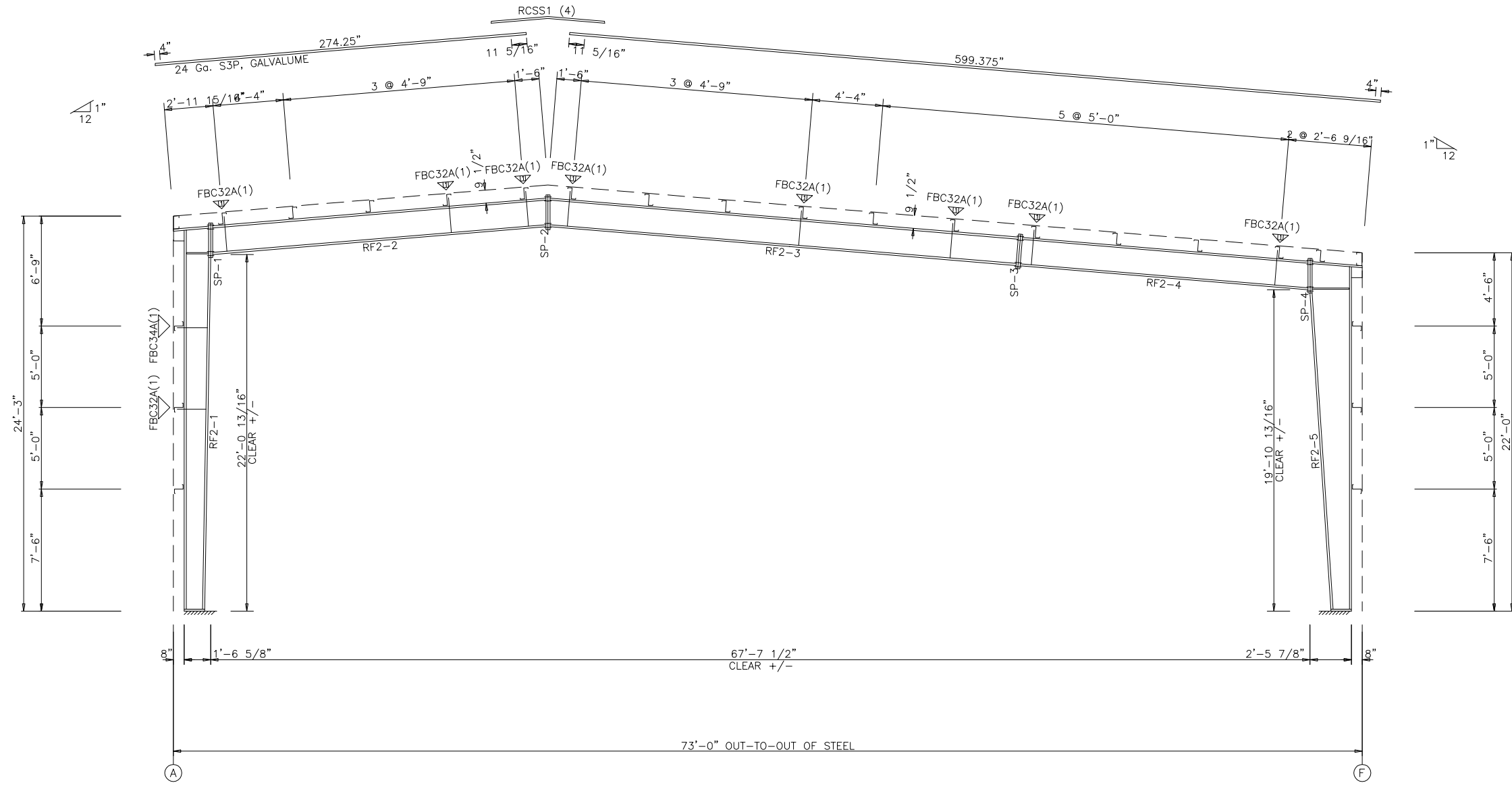


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SHEET  
**E1 OF 8**

SPLICE PLATE & BOLT TABLE									
Mark	Qty		Int	Type	Dia	Length	Width	Thick	Length
	Top	Bot							
SP-1	4	4	0	A325	0.750	3.00	6"	3/4"	2'-0 7/8"
SP-2	4	4	0	A325	0.625	2.25	6"	3/8"	2'-0 3/8"
SP-3	4	4	0	A325	0.625	2.25	6"	3/8"	2'-0 1/4"
SP-4	4	4	0	A325	0.625	2.25	6"	5/8"	2'-0 1/4"

CONNECTION PLATES	
ID	Mark/Part
1	FBL&N01



RIGID FRAME ELEVATION: FRAME LINE 3

**GENERAL NOTES**

1. INDICATES FLANGE BRACING LOCATIONS. (1) = ONE SIDE; (2) = TWO SIDES.
2. IF FLANGE BRACING IS REQUIRED ON BOTH SIDES OF AN EXPANDABLE RIGID FRAME, THE OPPOSITE SIDE FLANGE BRACES WILL HAVE TO BE INSTALLED AT THE TIME OF FUTURE EXPANSION. THESE FLANGE BRACES HAVE BEEN PROVIDED, AS REQUIRED, FOR THIS FUTURE CONDITION.
3. RIGID FRAMES SHALL HAVE 50% OF THEIR BOLTS INSTALLED AND TIGHTENED ON BOTH SIDES OF THE WEB ADJACENT TO EACH FLANGE BEFORE THE HOISTING EQUIPMENT IS RELEASED.
4. INTERIOR COLUMN METAL TAG IS ORIENTED TOWARD THE LOW EAVE OF THE BUILDING.

ISSUE	PERMITS	CHK	ENG	PE	DATE
		LC	RHB		5/31/2022

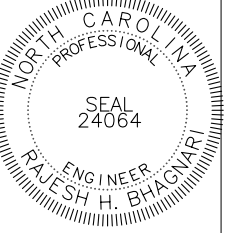
Engineering Performed By:  
 Nucor Corporation  
 200 Whetstone Rd.  
 Swansea, SC 29460  
 COA# F-1470

PROJECT NAME  
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 FUQUAY VARINA, NC 27526-6864

JOB NUMBER  
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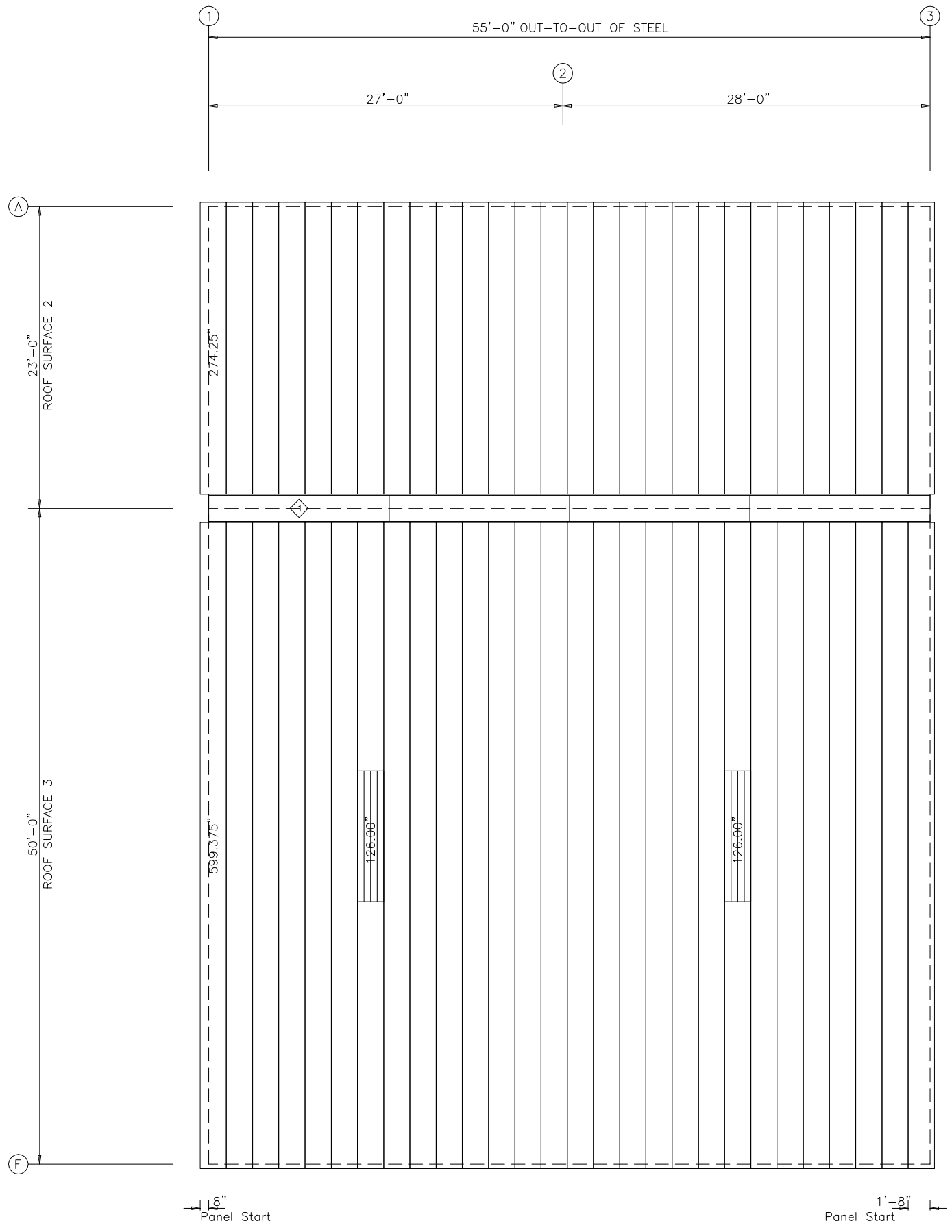
SHEET TITLE



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SHEET  
**E2 OF 8**





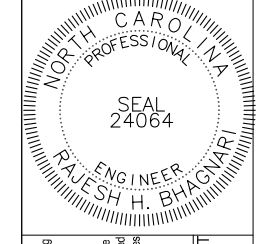
TRIM TABLE		
ROOF PLAN		
◇ ID	PART	LENGTH
1	RCSS1	182.000

ROOF SHEETING PLAN  
 PANELS: 24 Ga. S3P - GALVALUME

ISSUE	DWN	CHK	ENG	PE	DATE
PERMITS	LC	AB	RHB		5/31/2022

Engineering Performed By:  
 Nucor Corporation  
 200 Whetstone Rd.  
 Swansea, SC 29460  
 COA# F-1470

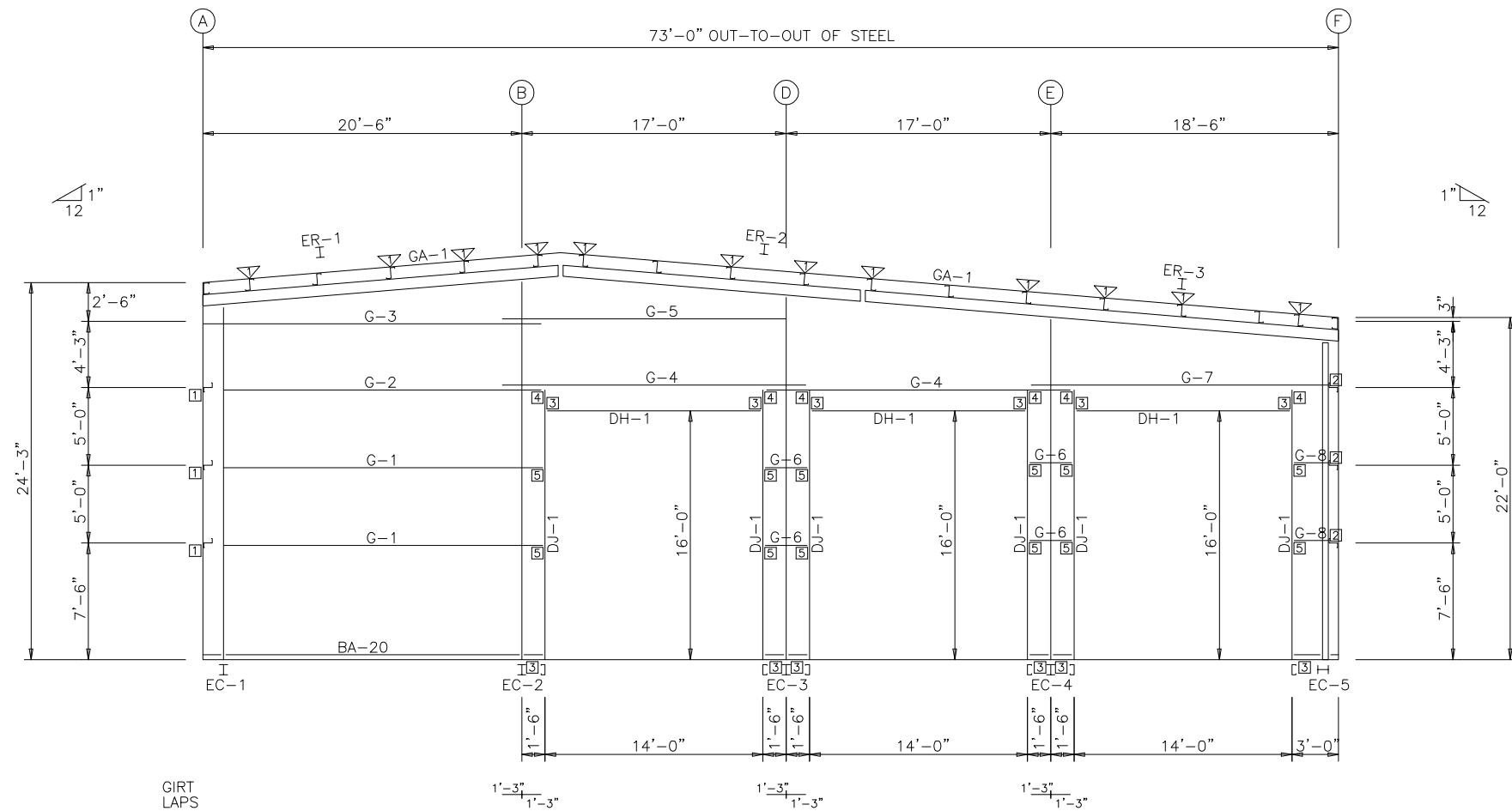
PROJECT NAME  
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 CUSTOMER NAME  
 VINCENT BARBOUR AND COMPANY  
 FUQUAY VARINA, NC 27526-6864  
 JOB NUMBER  
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SHEET  
**E4 OF 8**





ENDWALL FRAMING: FRAME LINE 1

BOLT TABLE FRAME LINE 1				
LOCATION	QUAN	TYPE	DIA	LENGTH
ER-1/ER-2	4	A325	1/2"	2"
ER-2/ER-3	4	A325	1/2"	2"
Columns/Raf	4	A325	1/2"	2"

TRIM TABLE FRAME LINE 1		
ID	PART	LENGTH
1	NoTrim	Use Drop
2	FCRA2	182.000
3	TRU1	182.000
4	RSF1	182.000
5	TRUECL	8.130
6	TRCU1-	18.938
7	TRPBB1	7.500
8	TRUECR	8.130
9	CCA121	121.000
10	JTD097	97.000
11	JTD121	121.000
12	CCA169	169.000
13	HTA172	172.000

FLANGE BRACE TABLE FRAME LINE 1			
▽ ID	#	MARK	CLIP
1	1	FBC30	FBL&N01

CONNECTION PLATES FRAME LINE 1	
ID	MARK/PART
1	GCC03
2	GCC02
3	HCJ01&bh
4	JCT01
5	FOC94&bh

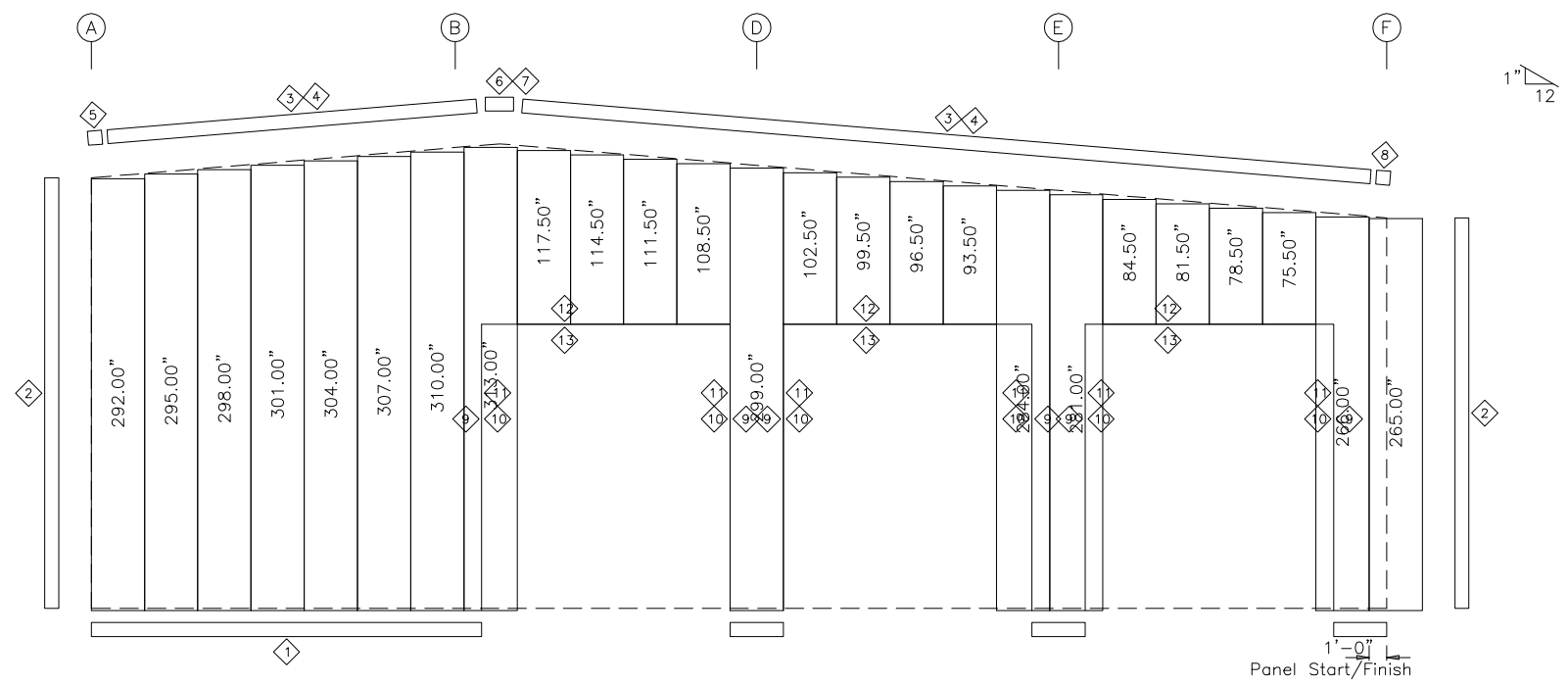
MEMBER TABLE FRAME LINE 1		
MARK	PART	LENGTH
EC-1	W8x10	274.813
EC-2	W8x10	293.938
EC-3	W8x18	282.000
EC-4	W8x18	265.000
EC-5	W8x10	247.625
ER-1	W8x18	276.688
ER-2	W8x18	233.813
ER-3	W8x18	368.000
DJ-1	F08C060	205.750
DH-1	F08C060	168.000
G-1	08Z060	252.500
G-2	08Z060	252.500
G-3	08Z060	260.750
G-4	08Z060	234.000
G-5	08Z060	221.250
G-6	08Z060	30.000
G-7	08Z060	228.500
G-8	08Z060	24.500

ENDWALL FRAMING PLAN

GENERAL NOTES

- STD. ROD/CABLE SIZES PER PART PREFIX ARE:  

RD05- = 5/8" ROD	CA02- = 1/4" CABLE
RD06- = 3/4" ROD	CA03- = 3/8" CABLE
RD07- = 7/8" ROD	CA04- = 1/2" CABLE
RD08- = 1" ROD	
RD09- = 1 1/8" ROD	
RD10- = 1 1/4" ROD	
- ROD/CABLE BRACING THAT OCCURS IN FLUSH OR INSET GIRT CONDITIONS WILL REQUIRE FIELD SLOTTING OF GIRT WEBS TO ALLOW FOR BRACING.
- FRAMED OPENINGS WHICH ARE FIELD LOCATED WILL REQUIRE FIELD CUTTING OF GIRTS AND SHEETING.
- THIS DRAWING IS NOT TO SCALE.



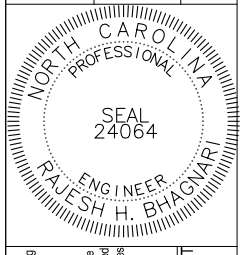
ENDWALL SHEETING & TRIM: FRAME LINE 1

PANELS: 26 Co. A3P - FOX GRAY-SP

DATE	ISSUE	PERMITS	CHK	ENG	PE
5/31/2022			AB	RHB	

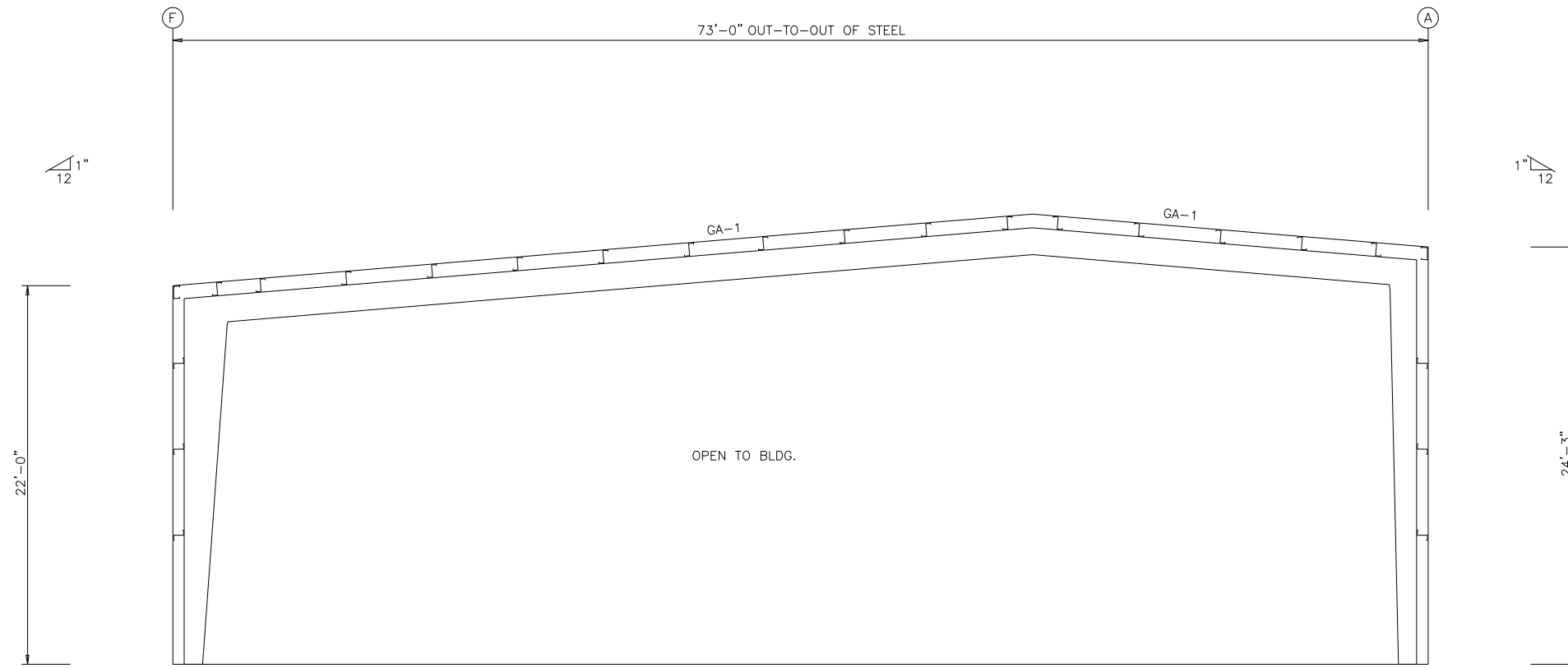
Engineering Performed By:  
 Nucor Corporation  
 200 Whetstone Rd.  
 Swansea, SC 29460  
 COA# F-1470

PROJECT NAME  
**CDT**  
 62 PROGRESS DR, FUQUAY VARINA, NC 27526  
 CUSTOMER NAME  
**VINCENT BARBOUR AND COMPANY**  
 FUQUAY VARINA, NC 27526-6864  
 JOB NUMBER  
 A22B0392A

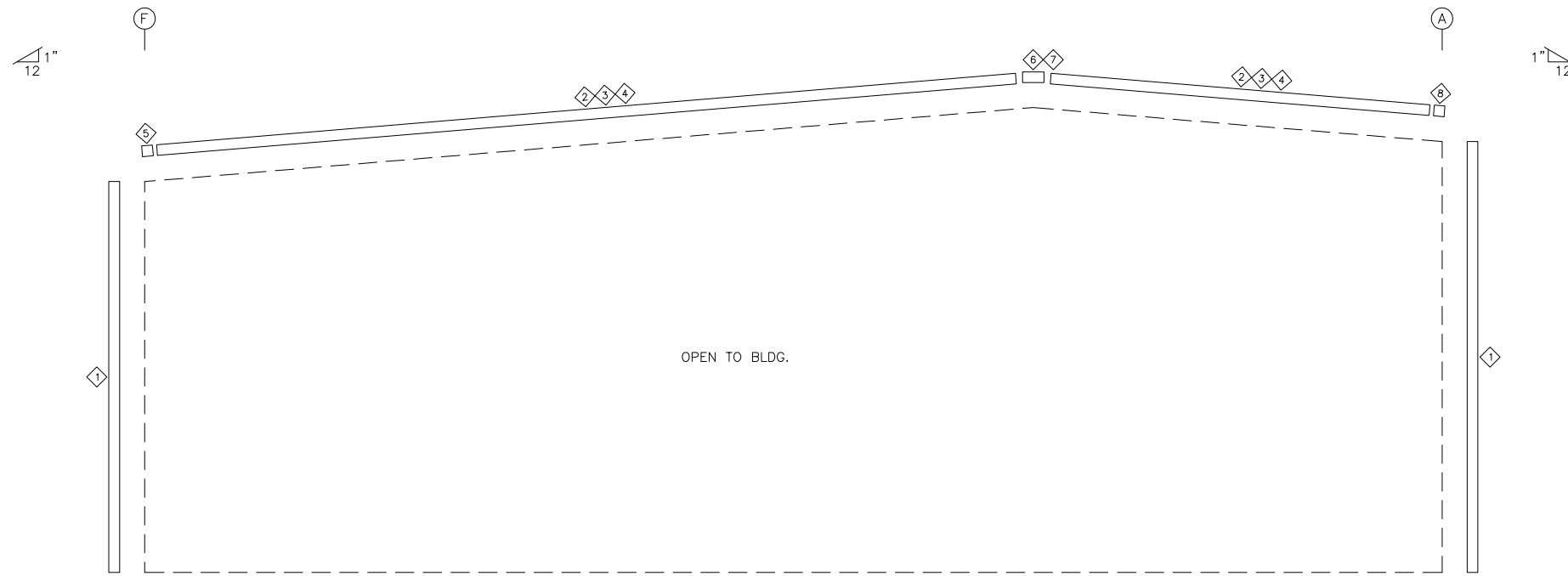


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SHEET  
**E5 OF 8**



ENDWALL FRAMING: FRAME LINE 3



ENDWALL SHEETING & TRIM: FRAME LINE 3

TRIM TABLE		
FRAME LINE 3		
◇ ID	PART	LENGTH
1	FCRA2	182.000
2	RSF1	182.000
3	TRU1	182.000
4	FJ2	182.000
5	TRUECL	8.130
6	TRPBB1	7.500
7	TRCU1	18.938
8	TRUECR	8.130

### ENDWALL FRAMING PLAN

#### GENERAL NOTES

- STD. ROD/CABLE SIZES PER PART PREFIX ARE:  

RD05- = 5/8" ROD	CA02- = 1/4" CABLE
RD06- = 3/4" ROD	CA03- = 3/8" CABLE
RD07- = 7/8" ROD	CA04- = 1/2" CABLE
RD08- = 1" ROD	
RD09- = 1 1/8" ROD	
RD10- = 1 1/4" ROD	
- ROD/CABLE BRACING THAT OCCURS IN FLUSH OR INSET GIRT CONDITIONS WILL REQUIRE FIELD SLOTTING OF GIRT WEBS TO ALLOW FOR BRACING.
- FRAMED OPENINGS WHICH ARE FIELD LOCATED WILL REQUIRE FIELD CUTTING OF GIRTS AND SHEETING.
- THIS DRAWING IS NOT TO SCALE.

ISSUE	DATE	ENG	CHK	DWN	LC	AB	RHB	PE
PERMITS	5/31/2022							

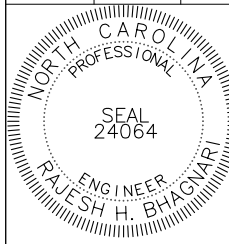
Engineering Performed By:  
 Nucor Corporation  
 200 Whetstone Rd.  
 Swansea, SC 29460  
 COA# F-1470

PROJECT NAME  
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 62 PROGRESS DR, FUQUAY VARINA, NC 27526

CUSTOMER NAME  
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 FUQUAY VARINA, NC 27526-6864

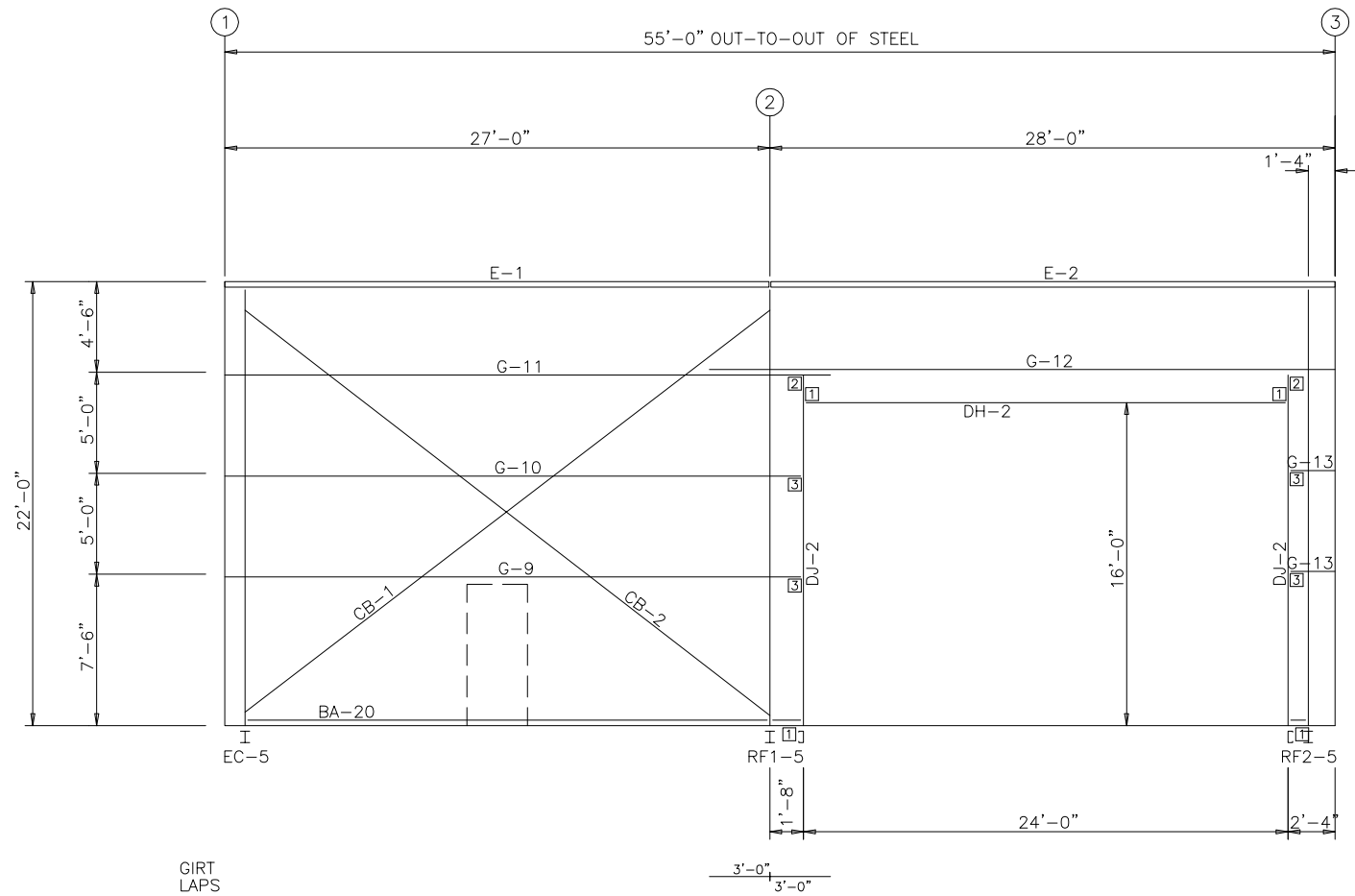
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SHEET TITLE

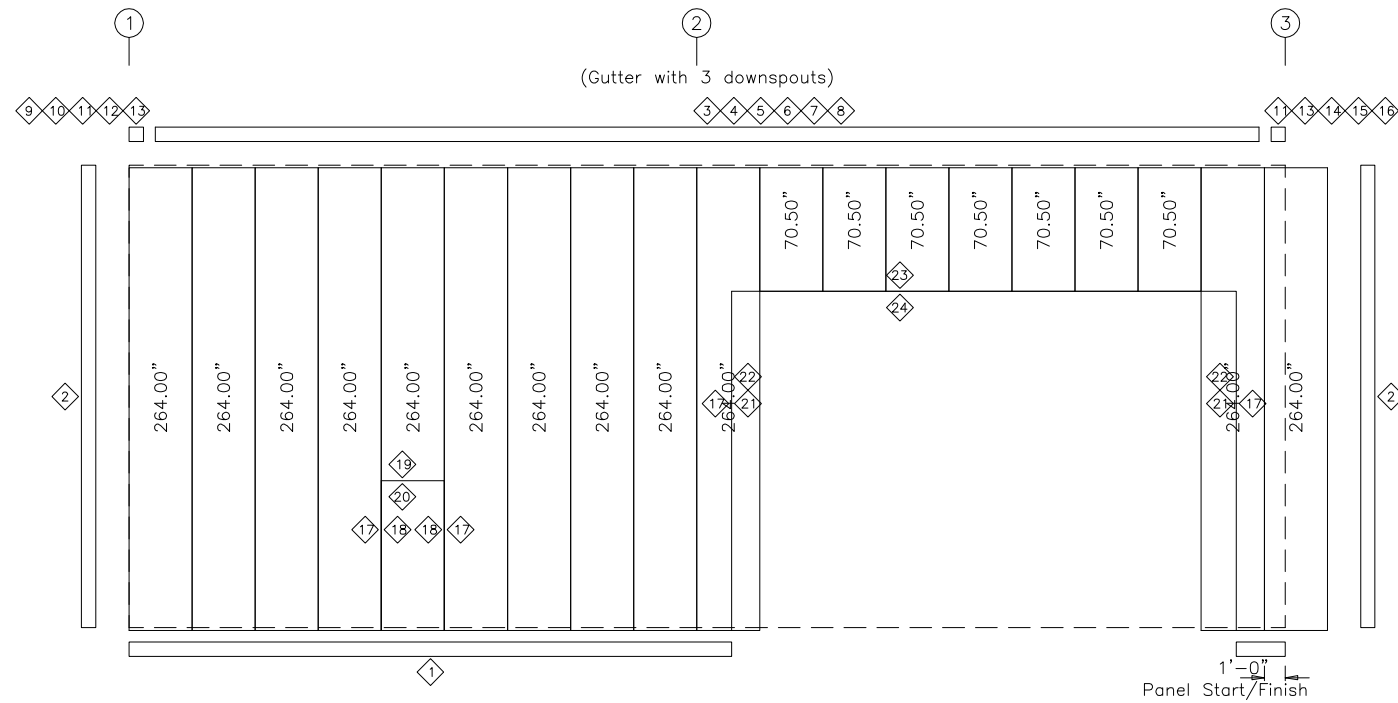


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SHEET  
**E6 OF 8**



SIDEWALL FRAMING: FRAME LINE F



SIDEWALL SHEETING & TRIM: FRAME LINE F

PANELS: 26 Ga. A3P - FOX GRAY-SP

TRIM TABLE FRAME LINE F		
◇ ID	PART	LENGTH
1	NoTrim	Use Drop
2	FCRA2	182.000
3	TGT1	182.000
4	TFEC1	182.000
5	CGB1	5.940
6	GC-A	9.940
7	DELETE	182.000
8	DELETE	182.000
9	GE1R	9.250
10	TCB1R	14.310
11	CGB1	Use Drop
12	GC-A	Use Drop
13	TCGC	Use Drop
14	GE1L	9.250
15	TCB1L	14.310
16	GC-A	Use Drop
17	CCA121	121.000
18	JTD087	87.000
19	CCA121	Use Drop
20	HTA044	44.000
21	JTD097	97.000
22	JTD121	121.000
23	CCA145	145.000
24	HTA148	148.000

CONNECTION PLATES FRAME LINE F	
□ ID	MARK/PART
1	HCJ01&bh
2	JCT01
3	FOC94&bh

MEMBER TABLE FRAME LINE F		
MARK	PART	LENGTH
DJ-2	F08C089	205.750
DH-2	F08C060	288.000
E-1	95E099	323.625
E-2	95E099	335.625
G-9	08Z089	340.750
G-10	08Z075	340.750
G-11	08Z060	359.750
G-12	08Z060	371.750
G-13	08Z060	24.750
CB-1	RD05-	403.000
CB-2	RD05-	406.000

SIDEWALL FRAMING PLAN

GENERAL NOTES

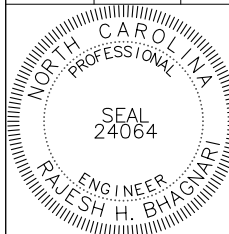
- STD. ROD/CABLE SIZES PER PART PREFIX ARE:  

RD05- = 5/8" ROD	CA02- = 1/4" CABLE
RD06- = 3/4" ROD	CA03- = 3/8" CABLE
RD07- = 7/8" ROD	CA04- = 1/2" CABLE
RD08- = 1" ROD	
RD09- = 1 1/8" ROD	
RD10- = 1 1/4" ROD	
- ROD/CABLE BRACING THAT OCCURS IN FLUSH OR INSET GIRTS WILL REQUIRE FIELD SLOTTING OF GIRTS TO ALLOW FOR BRACING.
- FRAMED OPENINGS WHICH ARE FIELD LOCATED WILL REQUIRE FIELD CUTTING OF GIRTS AND SHEETING.
- THIS DRAWING IS NOT TO SCALE.

ISSUE	PERMITS	DATE
		5/31/2022

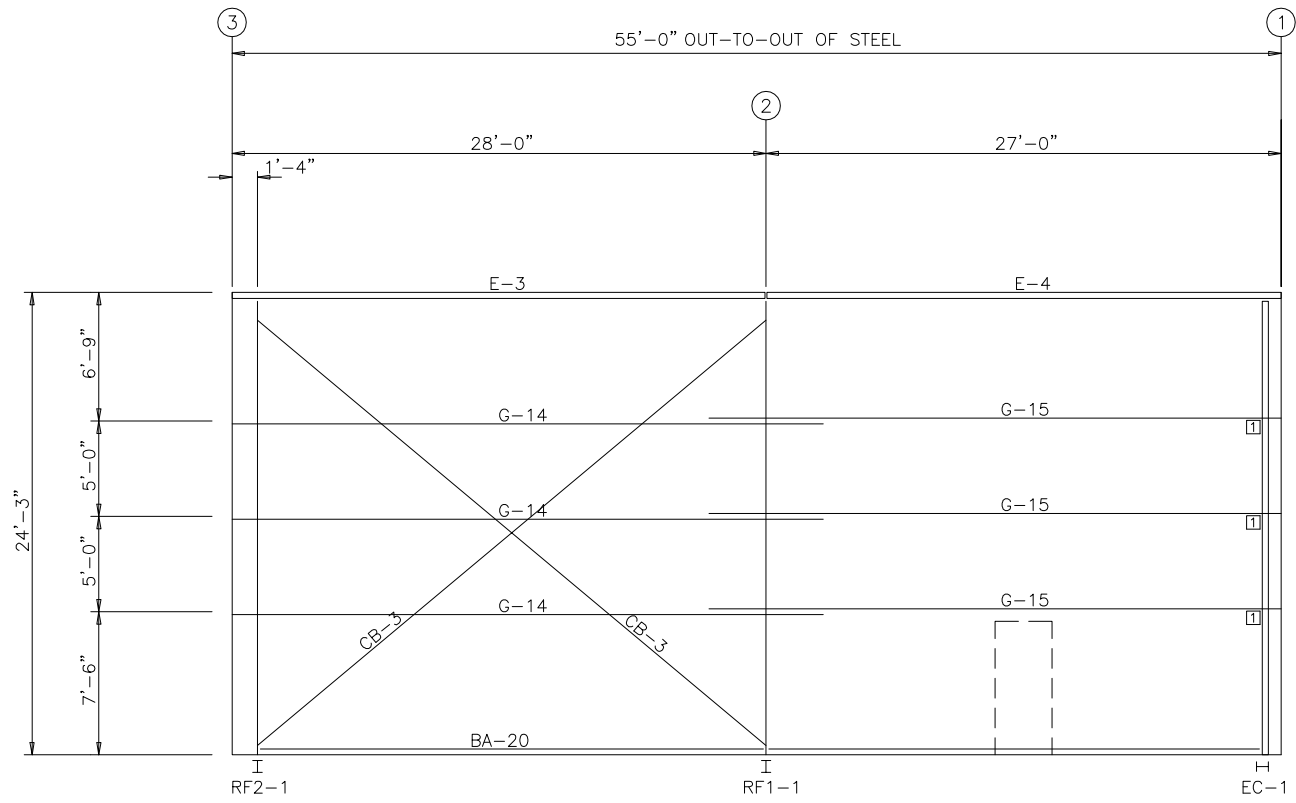
Engineering Performed By:  
 Nucor Corporation  
 200 Whetstone Rd.  
 Swansea, SC 29460  
 COA# F-1470

PROJECT NAME  
**CDT**  
 62 PROGRESS DR, FUQUAY VARINA, NC 27526  
 CUSTOMER NAME  
**VINCENT BARBOUR AND COMPANY**  
 FUQUAY VARINA, NC 27526-6864  
 JOB NUMBER  
**A22B0392A**



This seal pertains only to the materials designed and supplied by the Metal Building Manufacturer. The drawings and the metal building are the property of the Metal Building Manufacturer. The registered professional engineer whose seal appears on these drawings is employed by the Metal Building Manufacturer and does not serve as or represent the project engineer of record and shall not be construed as such.

SHEET  
**E7 OF 8**



SIDEWALL FRAMING: FRAME LINE A

TRIM TABLE FRAME LINE A		
◇ ID	PART	LENGTH
1	NoTrim	Use Drop
2	FCRA2	182.000
3	TGT1	182.000
4	TFEC1	182.000
5	CGB1	5.940
6	GC-A	9.940
7	DELETE	182.000
8	DELETE	182.000
9	GE1R	9.250
10	TCB1R	14.310
11	CGB1	Use Drop
12	GC-A	Use Drop
13	TCGC	Use Drop
14	GE1L	9.250
15	TCB1L	14.310
16	GC-A	Use Drop
17	CCA121	121.000
18	JTD087	87.000
19	CCA121	Use Drop
20	HTA044	44.000

MEMBER TABLE FRAME LINE A		
MARK	PART	LENGTH
E-3	95E099	335.625
E-4	95E099	323.625
G-14	08Z060	371.750
G-15	08Z060	359.750
CB-3	RD05-	426.000

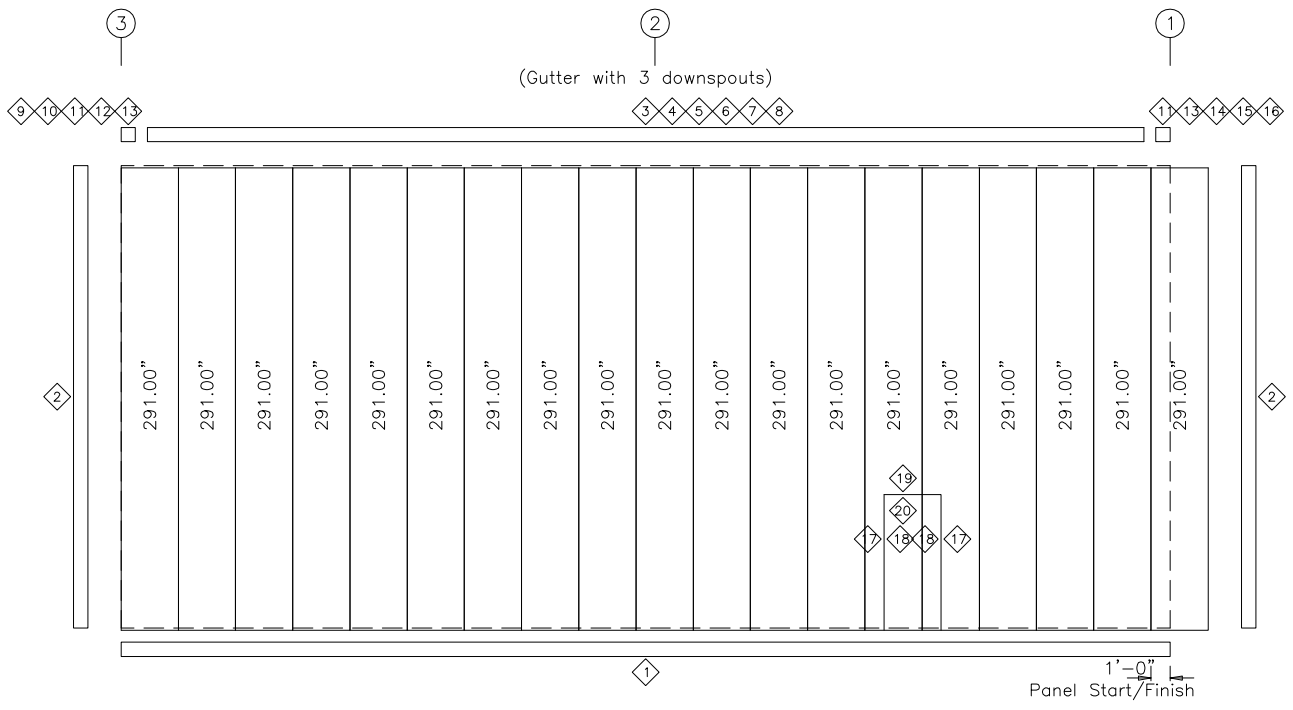
CONNECTION PLATES FRAME LINE A	
◇ ID	MARK/PART
1	GCC03

SIDEWALL FRAMING PLAN

GENERAL NOTES

- STD. ROD/CABLE SIZES PER PART PREFIX ARE:  

<u>ROD</u>	<u>CABLE</u>
RD05- = 5/8" ROD	CA02- = 1/4" CABLE
RD06- = 3/4" ROD	CA03- = 3/8" CABLE
RD07- = 7/8" ROD	CA04- = 1/2" CABLE
RD08- = 1" ROD	
RD09- = 1 1/8" ROD	
RD10- = 1 1/4" ROD	
- ROD/CABLE BRACING THAT OCCURS IN FLUSH OR INSET GIRT CONDITIONS WILL REQUIRE FIELD SLOTTING OF GIRT WEBS TO ALLOW FOR BRACING.
- FRAMED OPENINGS WHICH ARE FIELD LOCATED WILL REQUIRE FIELD CUTTING OF GIRTS AND SHEETING.
- THIS DRAWING IS NOT TO SCALE.

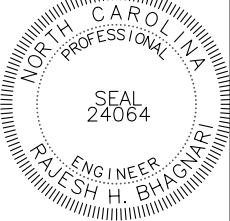


SIDEWALL SHEETING & TRIM: FRAME LINE A  
PANELS: 26 G. A3P - FOX GRAY-SP

ISSUE	PERMITS	DATE
		5/31/2022

Engineering Performed By:  
Nucor Corporation  
200 Whetstone Rd.  
Swansea, SC 29460  
COA# F-1470

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SHEET  
E8 OF 8