NEW FACILITY:

BUILDING A

LAKESIDE STORAGE - ANGIER

STRUCTURAL PLANS FOR SELF STORAGE FACILITY

ANGIER, NORTH CAROLINA

STRUCTURAL DRAWING SCHEDULE			
SHEET NO.	SHEET NAME	ORIGINAL DATE	RE-ISSUE DATE
SN1	COVER SHEET	07-07-2022	---
SN2	SPECIAL INSPECTIONS	07-07-2022	---
S1.1	FOUNDATION PLAN	07-07-2022	---
S2.1	ROOF FRAMING PLAN	07-07-2022	---
S2.1a	ROOF PLAN	07-07-2022	---
S2.4	ROOF DETAILS	07-07-2022	---
S3.1	ELEVATIONS	07-07-2022	---
S4	FOUNDATION DETAILS	07-07-2022	---
S5	FRAMING DETAILS	07-07-2022	---
S6	FRAMING DETAILS	07-07-2022	---

BASE SHEAR SCHEDULE				
	WIND BASE SHEAR[1]		SEISMIC BASE SHEAR[2]	
	Vx	Vy	Vx	Vy
BUILDING A	22.7 K	17.4 K	2.4 K	2.4 K
BUILDING B	22.7 K	17.4 K	2.4 K	2.4 K
BUILDING D	51.6 K	4.0 K	1.4 K	1.4 K

1. WIND BASE SHEAR INCLUDES A 0.6 WIND FACTOR.
2. SEISMIC BASE SHEAR INCLUDES A 0.7 SEISMIC FACTOR.

STRUCTURAL DESIGN DATA SHEET:

RISK CATEGORY:
II

IMPORTANCE FACTORS:
I seismic 1.0
I snow 1.0

DEAD LOADS:
ROOF 5 psf
ELEVATED FLOOR 60 psf

LIVE LOADS:
ROOF 20 psf
FLOOR 125 psf

SNOW LOAD:
Pg 15 psf

WIND LOAD:
Basic Wind Speed 120 MPH
Exposure Category C

SEISMIC LOAD:
Spectral Response
Ss 0.176
S1 0.084
Sds 0.188
Sd1 0.134
Seismic Design Category C
Seismic Site Class D - Default
Structural System Light framed walls w/ Steel Sheets
R-Factor 6.5
Analysis Procedure Equivalent Lateral Force

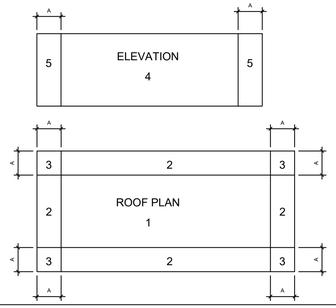
SEISMIC ANCHORAGE OF NON-STRUCTURAL COMPONENTS:
SEISMIC ANCHORING NOT REQUIRED

LATERAL DESIGN CONTROL:
X-Direction WIND
Y-Direction WIND

SOIL BEARING PROPERTIES:
Allowable Bearing Capacity = 2000 psf

WIND LOAD SCHEDULE					
COMPONENTS & CLADDING	ROOF WIND LOAD			WALL WIND LOADS	
	ROOF AREA			WALL AREA	
	1	2	3	4	5
PRESSURE (PSF)	+10.2	+10.2	+10.2	+27.5	+27.5
SUCTION (PSF)	-27.1	-36.4	-43.8	-30.0	-35.8

1. CORNER DISTANCE, A=15 FEET, ROOF = 50 SF, WALL = 20 S.F. C&C



COLD-FORMED STEEL:

1. ALL MEMBERS SHALL CONFORM TO THE AISI "SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS," NAS-01 AND SHALL BE OF THE TYPE AND SIZE AS INDICATED ON THE PLANS. ALL STRUCTURAL MEMBERS SHALL MEET THE REQUIREMENTS OF 2007 A.I.S.I. GENERAL PROVISIONS. STRUCTURAL MEMBER MATERIAL IS EITHER ASTM A653-06 GR 55 OR A1011-04 HSLAS GR. 55 CI-L. ALL MEMBERS SHALL BE ZINC COATED MEETING ASTM A1003, G-60 OR EQUAL.
2. THE PHYSICAL AND STRUCTURAL PROPERTIES AS LISTED BY BUILDING VENDOR SHALL BE THE MINIMUM PERMITTED FOR FRAMING MEMBERS. WE HAVE ASSUMED SSMA LISTED SIZES OR EQUIVALENT SUBSTITUTIONS MUST BE SUBMITTED THROUGH SHOP DRAWINGS AND APPROVED PRIOR TO CONSTRUCTION BY THE ENGINEER.
3. FASTENING OF COMPONENTS SHALL BE WITH SELF-DRILLING SCREWS OR WELDING IN COMPLIANCE WITH C1513. SCREWS AND WELDS SHALL BE OF SUFFICIENT SIZE TO ENSURE THE STRENGTH OF THE CONNECTION. ALL SCREWS SHALL NOT BE LESS THAN 3/4" O.C. OR FROM EDGE. ALL WELDS SHALL BE TOUCHED-UP WITH ZINC-RICH PAINT. U.N.O. ALL SCREW ATTACHMENTS SHALL BE #12 OR BETTER.
4. ALL POWER-ACTUATED FASTENERS (PAF) SHALL BE 0.177" DIA., U.N.O.
5. STRUCTURAL MATERIAL IS NOT DESIGNED TO BE PUNCHED. IF MATERIAL IS PUNCHED, CONSULT EOR FOR REMEDIATION.
6. TOP AND BOTTOM TRACKS SHALL BE THE SAME DEPTH AND GAGE, ALL TRACKS SHALL BE CONNECTED TO SUPPORTS WITH (2) FASTENERS OR PAFs AT EACH 30" O.C., MAXIMUM.
7. U.N.O, FLANGES SHOULD 2-1/2".
8. SPLICES IN FRAMING COMPONENTS, OTHER THAN RUNNER TRACK, SHALL NOT BE PERMITTED.
9. TEMPORARY BRACING, WHERE REQUIRED, SHALL BE PROVIDED UNTIL ERECTION IS COMPLETE.
10. ALL FRAMING COMPONENTS SHALL BE CUT SQUARELY FOR ATTACHMENT TO PERPENDICULAR MEMBERS OR, AS REQUIRED, FOR AN ANGULAR FIT AGAINST ABUTTING MEMBERS.
11. PROVIDE ADDITIONAL STUDS, WHEN NECESSARY, TO RESIST VERTICAL COMPONENTS OF LOADS.
12. THE QUANTITY OF STUDS AT HEADER OPENINGS SHALL BE MINIMUM AMOUNT OF STUDS DISPLACED DUE TO OPENING WITH HALF ON EACH SIDE OF OPENING.
13. MULTIPLE STUDS AT STUD PACKS SHALL BE ATTACHED AT (2) ROWS, STAGGERED WITH #10 TEKS SCREWS AT 24" O.C., IN A BACK-TO-BACK CONFIGURATION. WHEN FLANGE-TO-FLANGE IS REQUIRED GUSSET PLATES OR TRACKS SHALL BE INSTALLED AT THE ABOVE MENTIONED SPACING.
14. STUDS SHALL BE INSTALLED SO THE ENDS ARE POSITIONED AGAINST THE INSIDE OF THE RUNNER TRACK WEB PRIOR TO FASTENING AND SHALL BE ATTACHED TO BOTH FLANGES OF THE UPPER AND LOWER RUNNER TRACKS.
15. PROVIDE STIFFENERS IN HEADERS AT EACH POINT LOAD AND AT BEARING LOCATIONS, AS DESIGNATED ON PLANS.
16. ATTACH ALL CONNECTION PER PLANS OR AS DETAILED AND NOTED IN MANUFACTURER TECHNICAL MANUALS, PROVIDE SCREW OR POWDER ACTUATED FASTENER (PAF) ATTACHMENTS AS SPECIFIED.
17. LAYOUTS AS INDICATED ON PLANS IS FOR GRAPHICAL REPRESENTATION PURPOSES ONLY. ACTUAL STUD LOCATIONS MUST BE SUBMITTED WITH SHOP DRAWINGS.

STRUCTURAL STEEL:

1. ALL STRUCTURAL STEEL WORK SHALL CONFORM TO THE A.I.S.C. "STEEL CONSTRUCTION MANUAL" 360-05.
2. STRUCTURAL STEEL SHALL BE ASTM A-992.
3. STRUCTURAL TUBES SHALL BE ASTM A500, GRADE B.
4. STEEL FRAMING CONNECTIONS SHALL BE BOLTED OR WELDED. BOLTS SHALL BE 3/4" DIAMETER MINIMUM AND SHALL BE ASTM A-325-N U.N.O., SNUG TIGHT ALL CONNECTIONS.
5. ANCHOR BOLTS SHALL BE ASTM F1554 HEADED BOLTS. MINIMUM ANCHOR BOLT EMBEDMENT LENGTH SHALL BE 12 BOLT DIAMETERS U.N.O. CLEAN ANCHOR BOLTS OF ALL GREASE, DIRT, ETC., BEFORE INSTALLATION.
6. WELDS SHOWN ON THE STRUCTURAL DRAWINGS ARE THE MINIMUM REQ'D BY DESIGN, THE FABRICATOR'S DRAWINGS SHALL SHOW WELDS AND THEY SHALL CONFORM TO A.W.S. SPECIFICATIONS. ALL WELDING SHALL BE DONE WITH E-70 SERIES ELECTRODES.
7. PAINT ALL STRUCTURAL STEEL WITH ONE COAT OF RED OXIDE RUST-INHIBITIVE PRIMER 2.5 MILS IN THICKNESS. THE COMPATABILITY OF PRIMER AND ANY TOP COAT SHALL BE VERIFIED BEFORE ANY PAINTING IS PERFORMED. TOUCH-UP ALL EXPOSED METAL AFTER FIELD INSTALLATION. ALL STRUCTURAL STEEL WHICH IS EXPOSED TO THE ELEMENTS SHALL RECEIVE TWO COATS OF EXTERIOR ENAMEL WHICH IS COMPATIBLE TO THE PRIMED SURFACE.
8. THE SHOP DRAWINGS SHALL INCLUDE COMPLETE DETAILS AND SCHEDULES FOR FABRICATION AND ASSEMBLY OF STRUCTURAL STEEL MEMBERS. SUBMIT FOUR PRINTS OF EACH DRAWING. REPRODUCTION OF STRUCTURAL DRAWINGS FOR SHOP DRAWINGS IS NOT PERMITTED. CONTRACTOR TO REVIEW AND STAMP DRAWINGS PRIOR TO SUBMISSION TO THE EOR.

DESIGN AND CODE INFORMATION:

1. ALL CONSTRUCTION SHALL CONFORM TO THE 2018 NORTH CAROLINA BUILDING CODE AND ASCE 7-10.
2. VERIFY EXISTING CONDITIONS AND NOTIFY ARCHITECT OF ANY CONDITIONS WHICH DO NOT COMPLY WITH PLANS AND SPECIFICATIONS. STRUCTURAL DRAWINGS MUST BE WORKED WITH ARCHITECTURAL DWGS.
3. THE DESIGN ADEQUACY, SAFETY OF ERECTION BRACING, SHORING, TEMPORARY SUPPORTS, ETC. IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
4. FOR LOCATION OF MISCELLANEOUS ITEMS (SUCH AS INSERTS, ETC.) AFFECTING STRUCTURAL WORK, SEE ARCHITECTURAL, MECHANICAL, PLUMBING AND ELECTRICAL DRAWINGS.
5. THIS PROJECT CONTAINS A SERIES OF DETAILS CONSIDERED "TYPICAL DETAILS", THESE SHALL APPLY AT ALL SITUATIONS THAT ARE THE SAME OR SIMILAR AS THESE DETAILS. THESE "TYPICAL DETAILS" SHALL APPLY WHETHER OR NOT THEY ARE INDICATED OR CUT AT EACH LOCATION.
6. USE OF STRUCTURAL DRAWINGS FOR SHOP DRAWINGS IS NOT PERMITTED. CONTRACTOR TO REVIEW AND STAMP DRAWINGS ACCORDINGLY PRIOR TO SUBMITTING TO THE ENGINEER. THE OMISSION OF ITEMS FROM SHOP DRAWINGS SHALL NOT RELIEVE CONTRACTOR OF RESPONSIBILITY OF FURNISHING AND INSTALLING ITEMS REGARDLESS OF WHETHER SHOP DWGS. HAVE BEEN REVIEWED AND APPROVED.

FOUNDATION NOTES:

1. FOUNDATION DESIGN IS BASED UPON ASSUMED SOIL VALUES. CONTRACTOR/OWNER SHALL VERIFY PRIOR TO CONSTRUCTION.
2. FOOTINGS ARE DESIGNED TO BEAR ON UNIFORM SUITABLE SOIL CAPABLE OF SUPPORTING 2000 PSF.
3. THE SOIL BEARING CAPACITY AND CONSISTENCY SHALL BE VERIFIED FOR THE BUILDING LIMITS BY A REGISTERED GEOTECHNICAL ENGINEER WHEN FOUNDATION EXCAVATIONS HAVE BEEN CARRIED DOWN TO THE PROPOSED ELEVATIONS. THE BOTTOM OF ALL EXTERIOR FOOTINGS SHALL BE -1'-4" MINIMUM BELOW FINISHED GRADE. (U.N.O.)
4. WHERE FOOTING EXCAVATIONS ARE TO REMAIN OPEN AND MAY BE EXPOSED TO RAINFALL, THE EXCAVATIONS SHALL BE UNDERCUT AND A 3" THICK MUD MAT OF 2000 PSI CONCRETE SHALL BE PLACED OR CLEAN SHALL BE PLACED IN THE BOTTOM TO PROTECT THE BEARING SOILS.
5. WHERE FOOTING STEPS ARE NECESSARY, THEY SHALL BE NO STEEPER THAN 1 VERTICAL TO 2 HORIZONTAL, UNLESS SHOWN OTHERWISE ON PLANS.

REINFORCED CONCRETE:

1. ALL CONCRETE WORK SHALL CONFORM TO THE "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE," (ACI 318, 05)
2. REINFORCING STEEL SHALL BE DEFORMED BARS ASTM A-615 (GRADE 60)
3. FOUNDATIONS AND SLAB-ON-GRADE COMPRESSIVE STRENGTH AT 28 DAYS SHALL BE 3000 P.S.I. (SEE CIVIL DRAWINGS FOR SITE CONCRETE] KEEP COPY OF CONC. TEST REPORTS ON SITE AT ALL TIMES.
4. WALL AND ELEVATED SLAB COMPRESSIVE STRENGTH AT 28 DAYS SHALL BE 4000 P.S.I. (SEE CIVIL DRAWINGS FOR SITE CONCRETE] KEEP COPY OF CONC. TEST REPORTS ON SITE AT ALL TIMES
5. LAP SPLICES FOR #5 REINFORCING BARS SHALL BE 36" MIN., AND #6 REINFORCING BARS SHALL BE 43" MIN., UNLESS SUBMITTED AND APPROVED OTHERWISE.
6. CLEAR CONCRETE COVER FOR REINFORCING STEEL:
   WALLS: 3" CAST AGAINST GROUND; 2" FORMED EDGES
   FOOTINGS: 2" FORMED EDGES; 3" CAST AGAINST GROUND
   SLAB ON GRADE: MID-HEIGHT OF SLAB
6. THE LONGITUDINAL REINFORCING STEEL IN WALLS AND FOOTINGS SHALL BE CONTINUOUS AROUND CORNERS. SEE TYPICAL DETAILS.
7. SLUMP LIMIT IS 5 INCHES FOR CONCRETE WITH VERIFIED SLUMP OF 2 TO 4 INCHES BEFORE ADDING HIGH-RANGE WATER-REDUCING ADMIXTURE OR PLASTICIZING ADMIXTURE, PLUS OR MINUS 1 INCH
8. AIR CONTENT: 6 PERCENT, PLUS OR MINUS 1.5 PERCENT AT POINT OF DELIVERY FOR 3/4-INCH NOMINAL MAXIMUM AGGREGATE SIZE. EXCEPTION TROWEL-FINISHED FLOOR SHALL NOT EXCEED 3 PERCENT.
9. MAXIMUM COARSE-AGGREGATE SIZE: 3/4 INCH NOMINAL.
10. PORTLAND CEMENT: ASTM C 150/C 150M, TYPE I.
11. COLD-WEATHER PLACEMENT: COMPLY WITH ACI 306.1.
12. HOT-WEATHER PLACEMENT: COMPLY WITH ACI 301.
13. DESIGN, ERECT, SHORE, BRACE, AND MAINTAIN FORMWORK, ACCORDING TO ACI 301, TO SUPPORT VERTICAL, LATERAL, STATIC, AND DYNAMIC LOADS, AND CONSTRUCTION LOADS THAT MIGHT BE APPLIED, UNTIL STRUCTURE CAN SUPPORT SUCH LOADS. PLACE FORMWORK SO CONCRETE MEMBERS AND STRUCTURES ARE OF SIZE, SHAPE, ALIGNMENT, ELEVATION, AND POSITION INDICATED, WITHIN TOLERANCE LIMITS OF ACI 117. CHAMFER EXTERIOR CORNERS AND EDGES OF PERMANENTLY EXPOSED CONCRETE
14. BEFORE PLACING CONCRETE, VERIFY THAT INSTALLATION OF FORMWORK, REINFORCEMENT, AND EMBEDDED ITEMS IS COMPLETE AND THAT REQUIRED INSPECTIONS ARE COMPLETED. DEPOSIT CONCRETE CONTINUOUSLY IN ONE LAYER OR IN HORIZONTAL LAYERS OF SUCH THICKNESS THAT NO NEW CONCRETE IS PLACED ON CONCRETE THAT HAS HARDENED ENOUGH TO CAUSE SEAMS OR PLANES OF WEAKNESS. IF A SECTION CANNOT BE PLACED CONTINUOUSLY, PROVIDE CONSTRUCTION JOINTS AS INDICATED. DEPOSIT CONCRETE TO AVOID SEGREGATION. CONSOLIDATE PLACED CONCRETE WITH MECHANICAL VIBRATING EQUIPMENT ACCORDING TO ACI 301.

CONCRETE MASONRY:

1. CONCRETE MASONRY SHALL CONFORM TO THE NATIONAL CONCRETE MASONRY ASSOCIATION SPECIFICATIONS, AND HAVE A DENSITY OF125 P.C.F, AND SHALL HAVE A MINIMUM PRISM STRENGTH (F'm) OF 1500 P.S.I.
2. GROUT FOR FILLING CONCRETE MASONRY CELLS SHALL CONFORM TO STANDARD SPECIFICATIONS FOR "GROUT FOR MASONARY", ASTM C-476-02, AND SHALL HAVE A COMPRESSIVE PRISM STRENGTH (F'm) OF 3000 P.S.I. AT 28 DAYS. THE SLUMP SHALL BE BETWEEN 9" AND 11". WHERE THE MINIMUM DIMENSION OF ANY CONTINUOUS VERTICAL CELL IS 3" OR LESS, USE FINE GROUT, OTHERWISE USE COARSE (PEA GRAVEL) GROUT.
3. MORTAR FOR CONCRETE MASONRY SHALL BE TYPE "S" AND SHALL CONFORM TO ASTM C-270-04.
4. GROUT PROCEDURES AND REBAR INSTALLATION SHALL PER ASTM ACI 530 1-99. LAP SPLICES FOR REINFORCING BARS SHALL BE 24" MIN., U.N.O.
5. BRICK LINTELS - SEE SCHEDULE ON STRUCTURAL "S" SHEETS
6. ALL METAL BRICK TIES FOR BRICK VENEER SHALL BE A 2-PIECE, 3/16" DIAMETER ADJUSTABLE TIE, SPACED AT EACH STUD LOCATION, 24" O.C. (MAX) HORIZONTALLY, AND 16" O.C. VERTICALLY. METAL TIES SHALL BE EMBEDDED AT LEAST 2" INTO THE BRICK WYTHE. TIE MUST BE HOT DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A153, CLASS B. IN ADDITION, TIES SHOULD NOT HAVE MECHANICAL PLAY IN EXCESS OF 0.05" AND SHOULD NOT DEFORM OVER 0.05" FOR 100 LB LOAD IN EITHER TENSION OR COMPRESSION. METAL TIES SHOULD BE INSTALLED WITH 1/4-14 FASTENERS

HAUSER-CREECH, INC.
PROJECT #: 22-10X-00X

NORTH CAROLINA PROFESSIONAL ENGINEER SEAL 035814 MICHAEL GABRIEL HAUSER
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BUILDING A
LAKESIDE STORAGE - ANGIER
ANGIER, NC

ISSUE DATE: 07.07.2022

REV	DATE

PROJECT DATA & SPECS

SN1

STATEMENT OF SPECIAL INSPECTIONS:

Project Name: LAKESIDE STORAGE - ANGIER

Building Permit Number: _____

Project Address: 5556 NC-210, Angier, North Carolina, 27501

The following information is being submitted in accordance with the Special Inspection provisions of the International Building Code. Attached is the Schedule of Special Inspections (SSI) required for this project.

The Special Inspection program outlined herein does not relieve the Contractor or any other entity of contractual duties, including quality control, quality assurance or safety. The contractor is soley responsible for construction means, methods and job site safety.

Respectfully submitted,
The Structural Engineer of Record

Signature: _____ Date: 7/7/2022

SCHEDULE OF SPECIAL INSPECTIONS:

Project Name: LAKESIDE STORAGE - ANGIER
Construction divisions which require inspections for this project are as follows:

INSPECTION TASK	CONTINUOUS (C) OR PERIODIC (P) INSPECTIONS		SPECIAL INSPECTIONS FIRM	NOTES & SCOPE
	C	P		
1. VERIFICATION OF SOILS (Table 1704.7)				
Verify materials below shallow Foundations are adequate to achieve the design bearing capacity.		P	Testing Agency (TA)	Testing Agency shall provide soils report
Verify excavations are extended to proper depth.		P	Testing Agency (TA)	
Perform Classification and testing of compacted fill materials.		P	Testing Agency (TA)	
Verify use of proper materials, densities and lift thickness during placement and compaction of compacted fill.	C		Testing Agency (TA)	
Prior to placement of compacted fill, observe sub-grade and verify that site has been prepared properly.		P	Testing Agency (TA)	
2. REINFORCED CONCRETE (Table 1704.4)				
Inspection of reinforcing steel, including prestressing tendons, and placement. ACI 318:3.5, 7.1-7.7		P	Testing Agency (TA)	ACI 318: 3.5,7.1-7.7 IBC: 1913.4
Verifying use of required design mix: ACI 318: Ch. 4, 5.2-5.4		P	Testing Agency (TA)	ACI 318: Ch. 4, 5.2-5.4 IBC: 1904.2.2, 1913.2, 1913.3
At the time fresh concrete is sampled to fabricate specimens for strength tests, slump, air content, and temperature of concrete.	C		Testing Agency (TA)	ASTM C 172, C 31 ACI: 318: 5.6, 5.8 IBC: 1913.10

SCHEDULE OF SPECIAL INSPECTIONS (Continued):

Project Name: 6917 NC 55 HIGHWAY
Construction divisions which require inspections for this project are as follows:

INSPECTION TASK	CONTINUOUS (C) OR PERIODIC (P) INSPECTIONS		SPECIAL INSPECTIONS FIRM	NOTES & SCOPE
	C	P		
3. STRUCTURAL STEEL (Table 1704.3)				
Material verification of high strength bolts, nuts and washers.		P	Special Inspector (SI)	AISC 360, A3.3
Inspection of high strength bolting, snug tight joints		P	Special Inspector (SI)	AISC 360, M2.5 IBC 1704.3.3
Material verification of structural steel.		P	Special Inspector (SI)	Fabricator's bill of materials verification is acceptable.
All field welding.		P	Special Inspector (SI)	AWS D1.1 IBC 1704.3.1
4. RETAINING WALLS (Table 1704.12)				
Inspect all retaining walls over 5 feet in height per NCSBC.		P	Testing Agency (TA)	
5. MASONRY (Table 1704.4)				
As masonry construction begins, the following shall be verified to ensure compliance: (A) Proportions of site mixed mortar. (B) Construction of mortar joints. (C) Location of reinforcement and connectors.		P	Testing Agency (TA)	ACI 318: 3.5,7.1-7.7 IBC: 1913.4
The inspection program shall verify: (A) Size and location of structural elements. (B) Size, grade, type of reinforcement. (C) Protection of masonry during cold weather (temperature below 40 degrees F) or hot weather (temperature above 90 degrees F)		P	Testing Agency (TA)	Sec. 2108.9.2.11, Item 2, Sec. 2104.3, 2104.4, ACI Sec. 1.15.4, 2.1.2, Sec, 1.12, Sec 2.1.8.6, 2.1.8.6.2, ACI 3.3G, Art 2.4,3.4, Art 1.8
Prior to grouting, the following shall be verified to ensure compliance: (A) Grout space is clean. (B) Placement of reinforcement and connectors. (C) Proportions of site-prepared grout. (D) Construction of mortar joints		P	Testing Agency (TA)	Sec. 1.12, Art. 3.2D, Art 3.4, Art. 2.6B, Art. 3.3B
Grout Placement shall be verified to ensure compliance with code and construction provisions.		P	Testing Agency (TA)	Art. 3.5

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SPECIAL INSPECTIONS
SN2

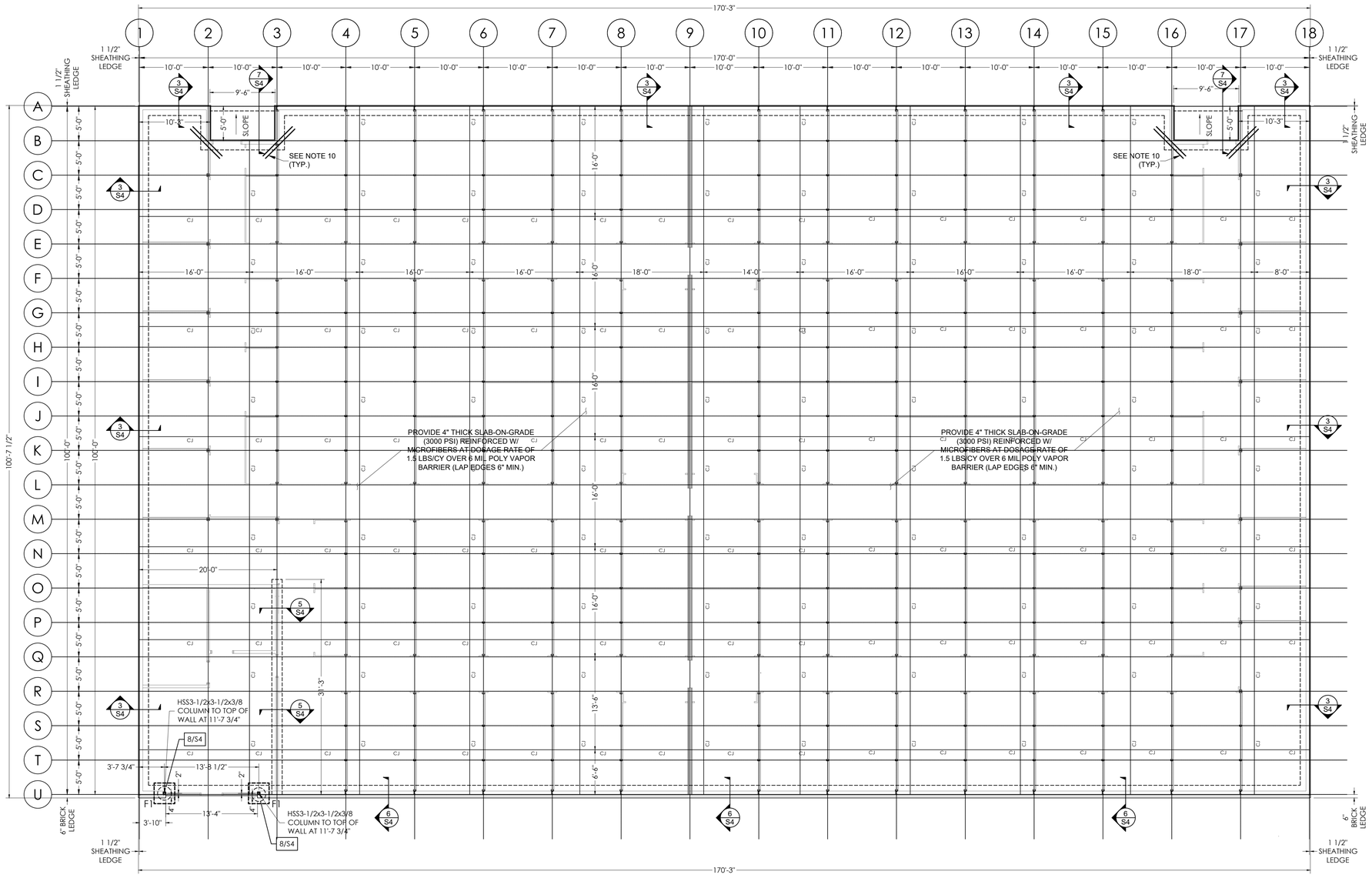
FOUNDATION NOTES:

1. PROVIDE COMPACTED BUILDING PAD (95% MIN COMPACTION). CONTRACTOR MUST VERIFY WITH GEOTECHNICAL ENGINEER AND SPECIAL INSPECTOR ONSITE IF MOISTURE CONTENT IN SOILS WARRANTS 4" POROUS BASE UNDER SLAB (CLEAN NO. 57 STONE, SAND, OR EQUIVALENT).
2. ALL DIMENSIONS REFERENCED TO SLAB EDGE, AND CENTERLINE OF INTERIOR BEARING WALLS. SEE ARCHITECTURAL PLANS FOR DIMENSIONS NOT SHOWN. VERIFY DIMENSIONS PRIOR TO CONSTRUCTION.
3. MIN. TOP OF EXTERIOR FTG. = F.F.E. -SEE PLAN.
4. SEE DETAIL 1/S4 FOR SLAB CONTROL JOINTS (CJ). ALTERNATE LAYOUT PLANS MAY BE SUBMITTED FOR APPROVAL.
5. PROVIDE POSITIVE DRAINAGE FROM ALL PERIMETER WALLS.
6. SEE DETAILS AND SCHEDULES FOR FOOTING SIZES AND REINFORCING.
7. PROVIDE 1'-6" MINIMUM DISTANCE BETWEEN THE NEW ANCHOR BOLTS AND THE CONCRETE EDGE, EXPANSION JOINT, CONTROL JOINT, MIS-ALIGNED/ABANDONED BOLT HOLE.
8. PROVIDE DRAINAGE FOR EXPOSED EARTH SURROUNDED BY FOOTINGS UNTIL SLAB IS POURED.
9. ALL CONCRETE FOOTINGS AND SLABS SHALL HAVE A MINIMUM DESIGN STRENGTH OF F'c=3000 PSI.
10. PROVIDE (2) 6'-0" LONG #5 BARS AT RE-ENTRANT CORNERS, PLACE AT MID-DEPTH OF SLAB, ONE IN EACH DIRECTION.
11. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING THE SERVICES OF A QUALIFIED TESTING LABORATORY TO PERFORM ALL COMPACTION TESTING
12. FOOTING STEP LOCATIONS ARE BASED ON THE SITE CIVIL DRAWINGS AND SHOULD BE VERIFIED BY THE CONTRACTOR PRIOR TO CONSTRUCTION.

ABBREVIATIONS:

A.	COLUMN
EX.	EXISTING
S.O.G.	SLAB ON GRADE
T.O.S.	TOP OF STEEL
T.O.P.	TOP OF PARAPET
T.O.M.	TOP OF MASONRY
O.C.	ON CENTERS SPACING
T+B	TOP AND BOTTOM
F.F.E.	FINISH FLOOR ELEVATION
TYP.	TYPICAL
DEMO.	DEMOLITION
CONT.	CONTINUOUS
CMU	CONCRETE MASONRY UNIT
STD.	STANDARD
XS.	EXTRA STRONG
XXS.	DOUBLE EXTRA STRING
GALV.	GALVANIZED

FOOTING SCHEDULE		
TYPE	SIZE	REBAR
F1	3'-0"-x3'-0"x1'-0"	(3) #5 BARS (2'-6" LONG) E.W., B



BUILDING A FOUNDATION PLAN
SCALE: 1/8" = 1'-0"

HAUSER-CREECH, INC.
PROJECT #: 22-10X-00X

NORTH CAROLINA REGISTERED SEAL 035814 ENGINEER MICHAEL GABRIEL HAUSER
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FOUNDATION PLAN

S1.1

FRAMING NOTES:

1. MAXIMUM ZEE JOIST SPACING IS INDICATED ON THE PLANS. SPACE JOIST AT ACCESS DOORS TO ALLOW FOR PROPER INSTALLATION. REFER TO ARCHITECTURAL PLANS FOR LOCATIONS.
2. MATERIAL SUPPLIER SHALL VERIFY ALL DIMENSIONS, LAYOUTS AND COORDINATE WITH BEARING WALL AND BEAM LOCATIONS. SUBMIT SHOP DRAWINGS FOR APPROVAL. ALTERNATE LAYOUT PLANS MAY BE SUBMITTED FOR APPROVAL.
3. REFER TO FOUNDATION PLAN FOR DIMENSIONS AND TO ARCHITECTURAL PLANS FOR DIMENSIONS NOT SHOWN.
4. SEE DETAIL 1/S2.3 FOR ROOF PANEL SIZE AND ATTACHMENT..
5. VERIFY LOCATIONS AND AMOUNTS OF ALL HEADERS.
6. METAL STUD WALL SHOP DRAWINGS SHALL PROVIDED FOR REVIEW AND APPROVAL.
7. STUD SPACING SHALL NOT EXCEED 60" O.C. ON UPPER LEVEL (OR SINGLE STORY BUILDING) AND 30" ON LOWER LEVEL. ADDITIONALLY POINTS LOADS FROM STUDS ARE DESIGNED TO STACK FROM FLOOR-TO-FLOOR. CONTACT EOR IF STUDS DO NOT ALIGN.
8. STUD WALL SIZES AND CONNECTIONS DIFFERING FROM THOSE SHOWN ON THESE PLANS MAY BE SUBMITTED FOR APPROVAL. PROVIDED THE ALTERNATES ARE PROVIDED IN THE FORM OF A SIGNED AND SEALED SHOP DRAWING BY A LICENSED PROFESSIONAL. NOTE THAT ANY PARTS OMITTED FROM THESE PLANS SHALL BE CONSIDERED THE DESIGNATED ENGINEER RESPONSIBILITY THROUGH SHOP DRAWINGS.
9. EXTERIOR WALL PANELS REQUIRE MID-HEIGHT WALL GIRT OR BRACING AT THIRD POINTS FOR SUPPORT. SEE DETAIL 2 ON S6
10. SEE DETAIL 3 ON S6 FOR PARTITION WALL INTERSECTION W/ BEARING WALL.

BUILDING 1 - LIGHT GAGE METAL STUD SCHEDULE

LOCATION	STUD HEIGHT	SIZE	SPACING	LATERAL BRACING LOCATIONS
FIRST FLOOR EXTERIOR WALLS - METAL PANELS	VARIES	4Cx2 1/2x16GA (50 KSI)	60" MAX.	60" O.C. BRACING
FIRST FLOOR INTERIOR BEARING WALLS	VARIES	4Cx2 1/2x16GA (50 KSI)	60" MAX.	SHEATHED ONE SIDE

CLADDING SCHEDULE

PANEL TYPE	LOCATION	MATERIAL	GIRT/PURLIN - WALL/ROOF PANEL BRACE SPACING
U PANEL BY VENDOR	INTERIOR WALL	29 GA.	INTERIOR STUD SPACING = 5.0 FT O.C
R PANEL BY VENDOR	EXTERIOR WALL	26 GA.	CORNER ZONE = 5.0 FT O.C INTERIOR ZONE = 5.0 FT O.C.
24 GA. STANDING SEEM BY VENDOR	ROOF	24 GA.	CORNER ZONE = 5.0 FT O.C. PERIMETER ZONE = 5.0 FT O.C INTERIOR ZONE = 5.0 FT O.C

1. SUBMIT VENDOR CUT SHEETS/SHOP DRAWING INFORMATION FOR APPROVAL.
2. SEE MANUFACTURER REQUIREMENTS FOR INSTALLATION COMPONENTS AND TRIM COMPONENTS TO RESIST CLADDING PRESSURES

LINTEL SCHEDULE

SIZE	NOTES
L3-1/2x3-1/2x5/16	UP TO 4'-0" OPENINGS
L4x4x3/8	4'-0" TO 6'-0" OPENINGS
L6x4x3/8 (LLV)	6'-0" TO 8'-0" OPENINGS
L7x4x7/16 (LLV)	8'-0" TO 10'-0" OPENINGS
CONTACT EOR	OPENINGS > 10'-0"

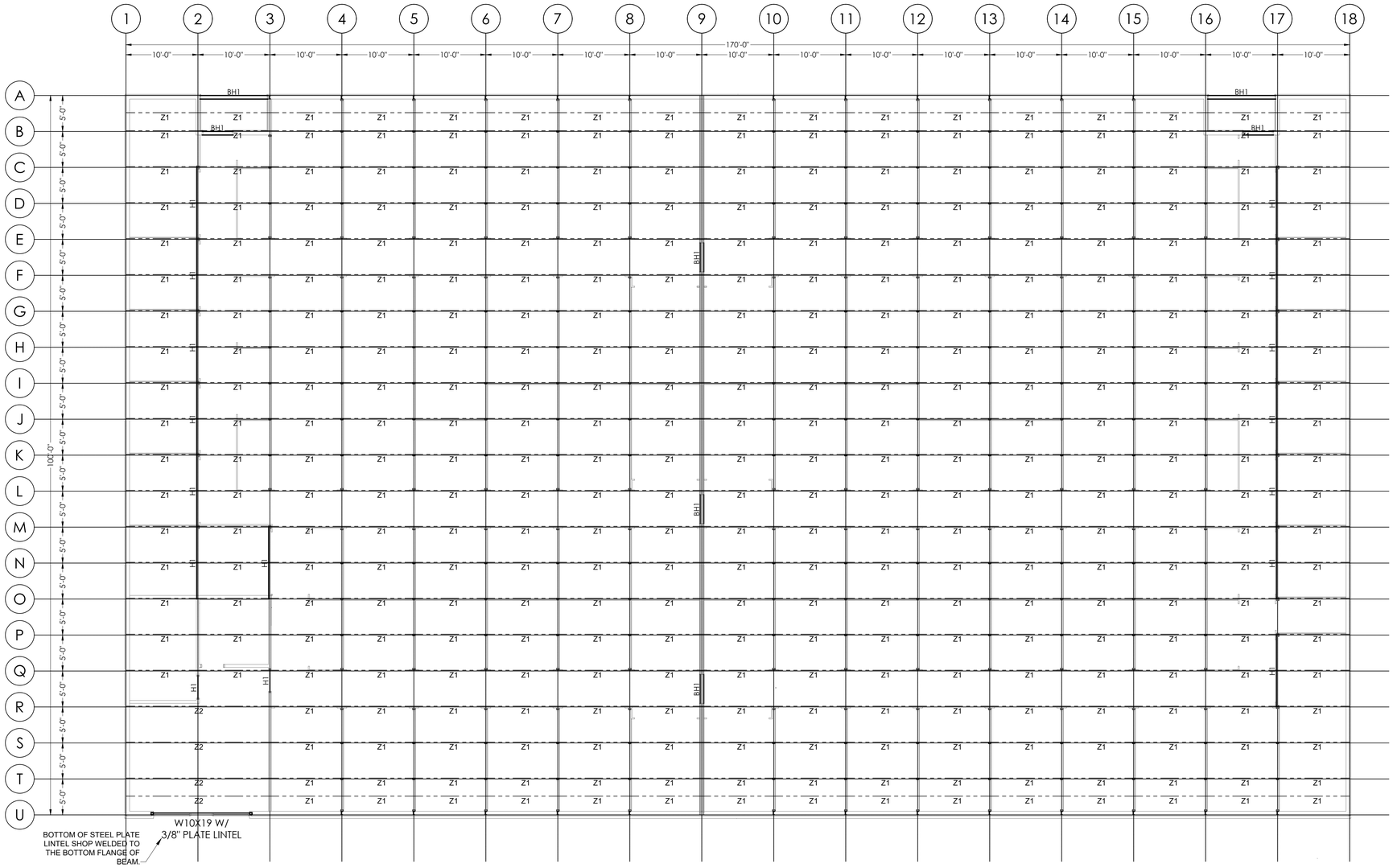
1. NO EXPANSION JOINTS MAY BE POSITIONED ON EITHER SIDE OF OPENING OF ABOVE OPENING. LINTEL IS DESIGNED WITH ARCHING AFFECT OF MASONRY ACCOUNTED.

2. FOR OPENINGS UP TO 8'-0" PROVIDE 6" BEARING ON EACH SIDE. FOR OPENING 8'-0" TO 10'-0", PROVIDE 8" BEARING ON EACH SIDE.

3. NO CONCENTRATED LOADS SHALL BE INSTALLED ABOVE LINTELS. IE, AWNING CONNECTIONS, ARCH FEATURES ETC.

LIGHT GAGE HEADER AND PURLIN SCHEDULE

LABEL	SIZE	MATERIAL	NOTES
H1	SINGLE 8Cx3-1/2x14GA	50 KSI	SEE DETAILS 5 AND 6 ON S5
DH1	DOUBLE 6Cx2x14GA (50 KSI) W/ CRIPPLE STUDS AT 24" O.C.	50 KSI	SEE DETAIL 4 ON S6
DH2	DOUBLE 12Cx3-1/2x12GA (50 KSI) W/ CRIPPLE STUDS AT 24" O.C.	50 KSI	SEE DETAIL 4 ON S6
DH3	DOUBLE 8Cx2-1/2x16GA (50 KSI) W/ CRIPPLE STUDS AT 24" O.C.	50 KSI	SEE DETAIL 4 ON S6
BH1	DOUBLE 6Cx2x16GA (50 KSI) W/ T&B TRACK TO FORM BOX HEADER	50 KSI	SEE DETAILS 5 AND 6 ON S6
BH2	DOUBLE 6Cx2x14GA (50 KSI) W/ T&B TRACK TO FORM BOX HEADER	50 KSI	SEE DETAILS 5 AND 6 ON S6
Z1	4"x2 1/2"x16 GA Zee Purlins SEE SHOP DRAWINGS	50 KSI	SEE DETAILS 2,3,4, AND 5 ON S5
Z2	12"x3 1/2"x14 GA Zee Purlins SEE SHOP DRAWINGS	50 KSI	SEE DETAILS 2,3,4, AND 5 ON S5



N

BUILDING A ROOF FRAMING PLAN

SCALE: 1/8" = 1'-0"

HAUSER-CREECH, INC.
PROJECT #: 22-10X-00X

NORTH CAROLINA PROFESSIONAL ENGINEER SEAL 035814 MICHAEL GABRIEL HAUSER
MICHAEL GABRIEL HAUSER
NORTH CAROLINA PE NO. 035814

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BUILDING A
LAKESIDE STORAGE - ANGIER
ANGIER, NC

ISSUE DATE: 07.07.2022

REV	DATE

ROOF FRAMING PLAN

S2.1

FRAMING NOTES:

1. MAXIMUM ZEE JOIST SPACING IS INDICATED ON THE PLANS. SPACE JOIST AT ACCESS DOORS TO ALLOW FOR PROPER INSTALLATION. REFER TO ARCHITECTURAL PLANS FOR LOCATIONS.
2. MATERIAL SUPPLIER SHALL VERIFY ALL DIMENSIONS, LAYOUTS AND COORDINATE WITH BEARING WALL AND BEAM LOCATIONS. SUBMIT SHOP DRAWINGS FOR APPROVAL. ALTERNATE LAYOUT PLANS MAY BE SUBMITTED FOR APPROVAL.
3. REFER TO FOUNDATION PLAN FOR DIMENSIONS AND TO ARCHITECTURAL PLANS FOR DIMENSIONS NOT SHOWN.
4. SEE DETAIL 1/S2.3 FOR ROOF PANEL SIZE AND ATTACHMENT..
5. VERIFY LOCATIONS AND AMOUNTS OF ALL HEADERS.
6. METAL STUD WALL SHOP DRAWINGS SHALL PROVIDED FOR REVIEW AND APPROVAL.
7. STUD SPACING SHALL NOT EXCEED 60" O.C. ON UPPER LEVEL (OR SINGLE STORY BUILDING) AND 30" ON LOWER LEVEL. ADDITIONALLY POINTS LOADS FROM STUDS ARE DESIGNED TO STACK FROM FLOOR-TO-FLOOR. CONTACT EOR IF STUDS DO NOT ALIGN.
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BUILDING 1 - LIGHT GAGE METAL STUD SCHEDULE

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CLADDING SCHEDULE

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2. SEE MANUFACTURER REQUIREMENTS FOR INSTALLATION COMPONENTS AND TRIM COMPONENTS TO RESIST CLADDING PRESSURES

LINTEL SCHEDULE

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LIGHT GAGE HEADER AND PURLIN SCHEDULE

LABEL	SIZE	MATERIAL	NOTES
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DH2	DOUBLE 12Cx3-1/2x12GA (50 KSI) W/ CRIPPLE STUDS AT 24" O.C.	50 KSI	SEE DETAIL 4 ON S6
DH3	DOUBLE 8Cx2-1/2x16GA (50 KSI) W/ CRIPPLE STUDS AT 24" O.C.	50 KSI	SEE DETAIL 4 ON S6
BH1	DOUBLE 6Cx2x16GA (50 KSI) W/ T&B TRACK TO FORM BOX HEADER	50 KSI	SEE DETAILS 5 AND 6 ON S6
BH2	DOUBLE 6Cx2x14GA (50 KSI) W/ T&B TRACK TO FORM BOX HEADER	50 KSI	SEE DETAILS 5 AND 6 ON S6
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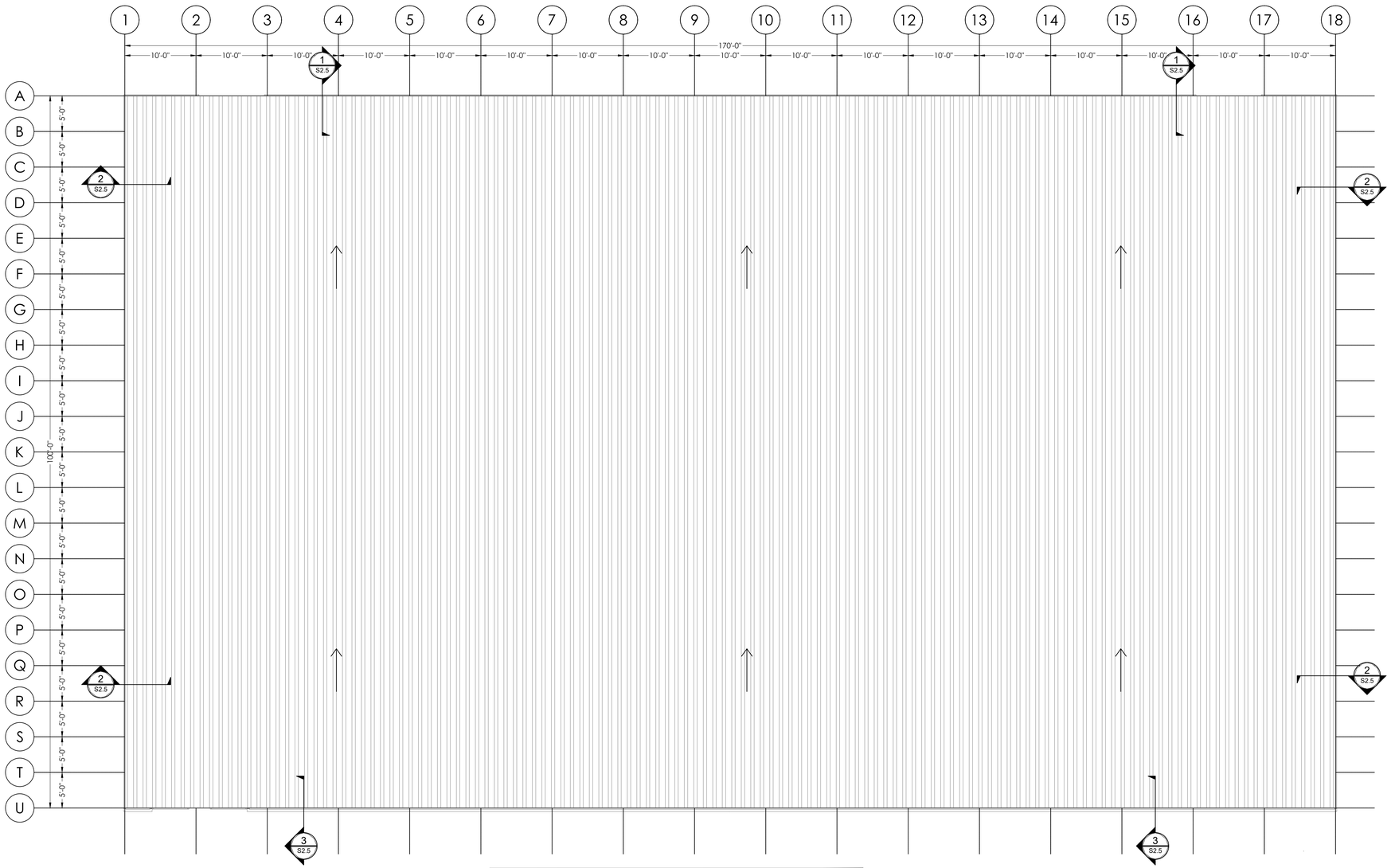
Z2	12"x3 1/2"x14 GA Zee Purlins SEE SHOP DRAWINGS	50 KSI	SEE DETAILS 2,3,4, AND 5 ON S5



N

BUILDING A ROOFING PLAN

SCALE: 1/8" = 1'-0"

HAUSER-CREECH, INC.
PROJECT #: 22-10X-00X

NORTH CAROLINA PROFESSIONAL SEAL 035814 ENGINEER MICHAEL GABRIEL HAUSER

MICHAEL GABRIEL HAUSER
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RALEIGH, NC 27606

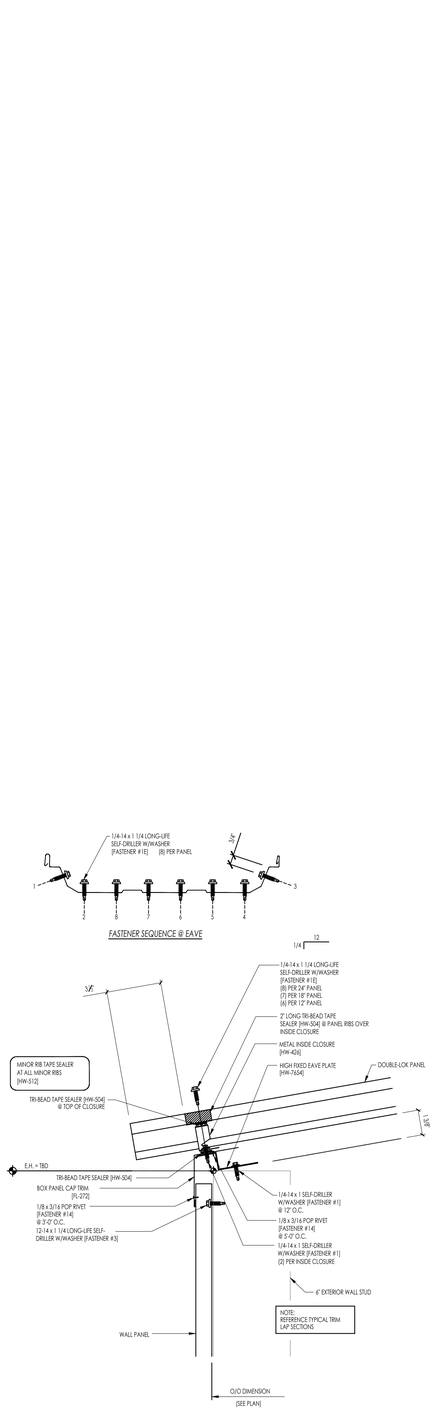
BUILDING A
LAKESIDE STORAGE - ANGIER
ANGIER, NC

ISSUE DATE: 07.07.2022

REV	DATE

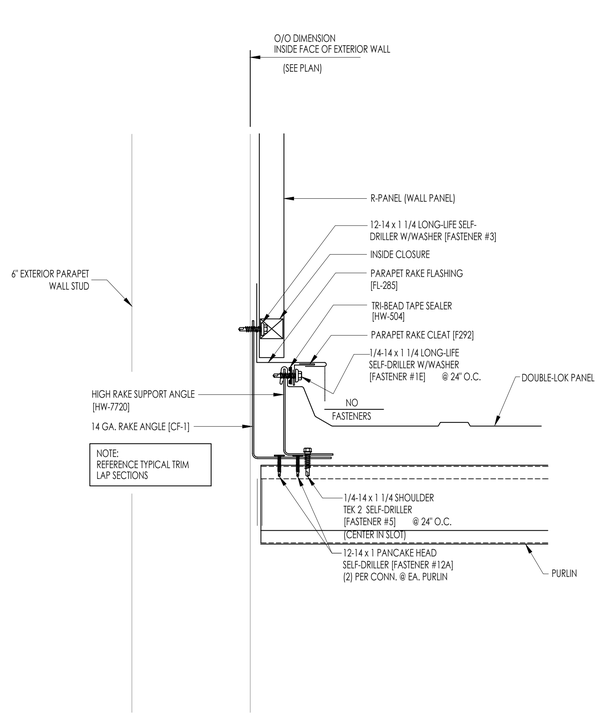
ROOFING PLAN

S2.1a



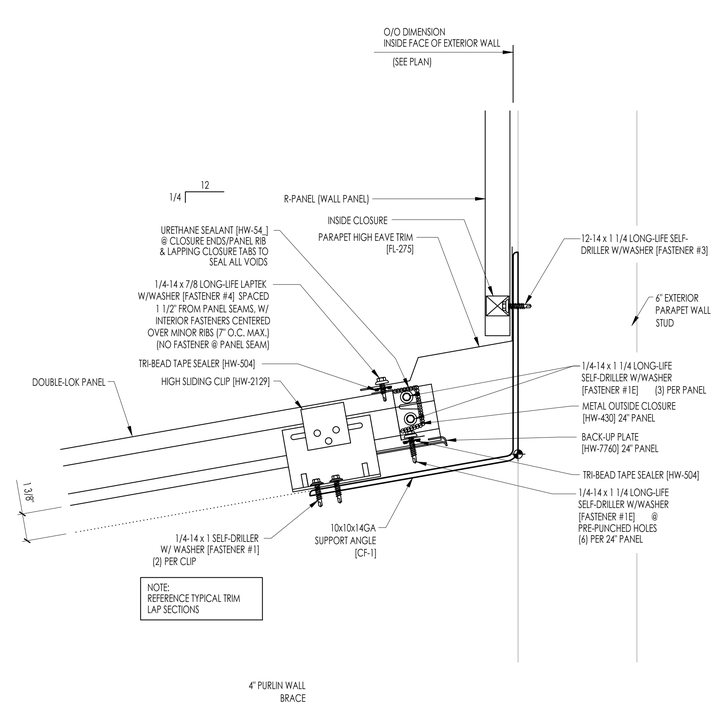
1 S2.4 LOW EAVE NO GUTTER SECTION

SCALE: N/A



2 S2.4 PARAPET RAKE TRIM SECTION

SCALE: N/A



3 S2.4 PARAPET HIGH EAVE TRIM SECTION

SCALE: N/A

HAUSER-CREECH, INC.
PROJECT #: 22-10X-00X

NORTH CAROLINA
PROFESSIONAL
SEAL
035814
ENGINEER
MICHAEL GABRIEL HAUSER

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NORTH CAROLINA PE NO. 035814

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LAKESIDE STORAGE - ANGIER
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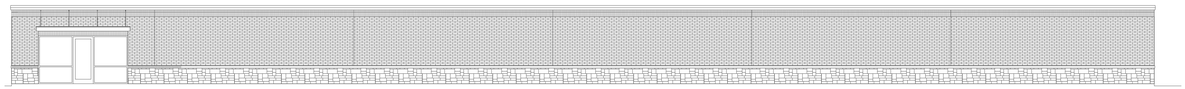
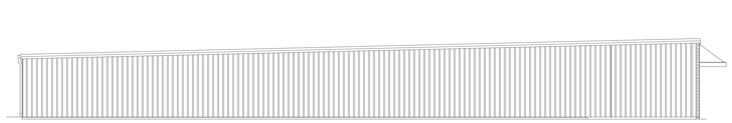
ROOF DETAILS

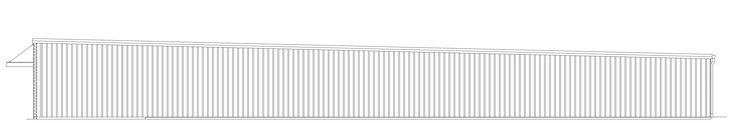
S2.4



FRONT ELEVATION



LEFT SIDE ELEVATION



RIGHT SIDE ELEVATION

REAR ELEVATION

HAUSER-CREECH, INC.
PROJECT #: 22-10X-00X

NORTH CAROLINA PROFESSIONAL SEAL 035814 ENGINEER MICHAEL GABRIEL HAUSER

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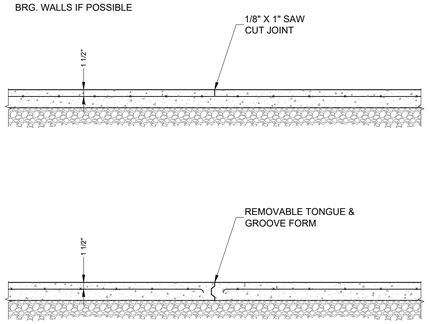
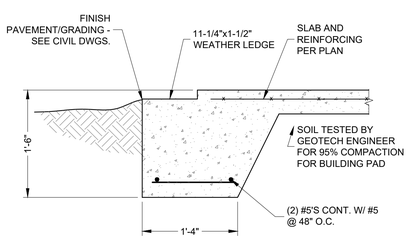
BUILDING A ELEVATIONS

S3.1

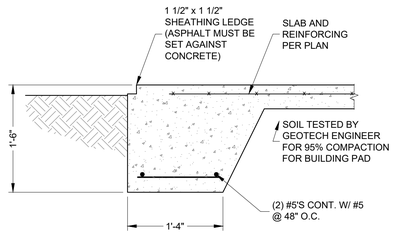
NOTE:
MAXIMUM JOINT SPACING SHALL
BE 20 FT. IN EACH DIRECTION
UNLESS SHOWN OTHERWISE ON PLAN
LOCATED UNDER NON-LOAD
BRG. WALLS IF POSSIBLE



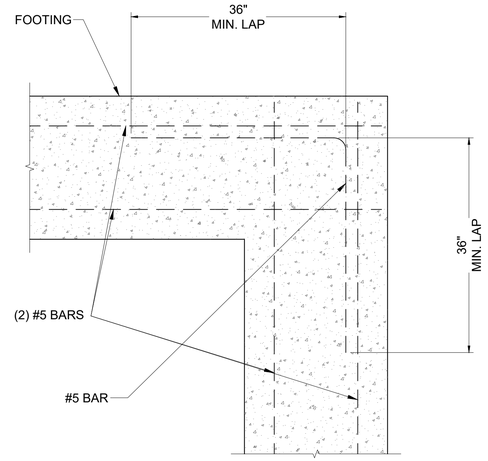
1 SLAB ON GRADE JOINTS
SCALE: NONE



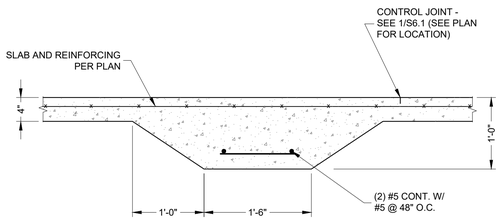
2 SECTION @ EXTERIOR WALL (WEATHER LEDGE)
SCALE: NONE



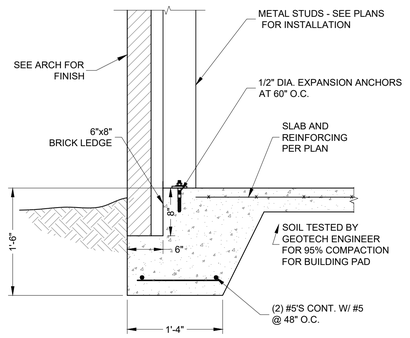
3 SECTION @ SHEATHING NOTCH
SCALE: NONE



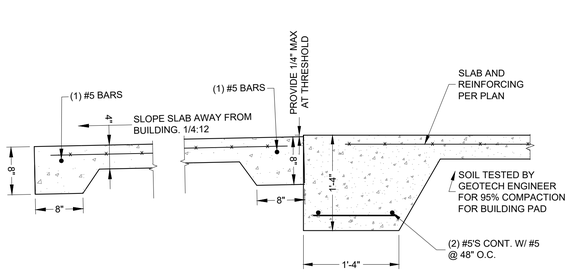
4 CONTINUITY CORNERS - ALL BUILDING CORNERS
SCALE: NONE



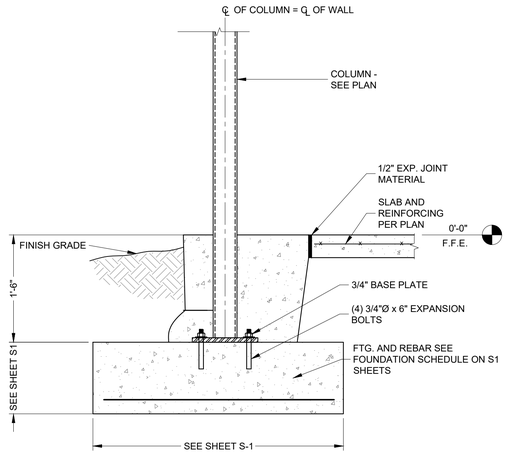
5 THICKENED SLAB
SCALE: NONE



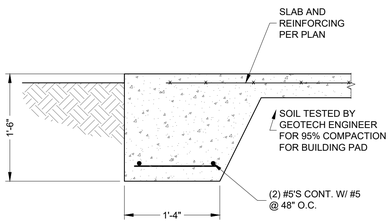
6 SECTION @ EXTERIOR WALL (BRICK LEDGE)
SCALE: NONE



7 EXTERIOR ENTRY/ALCOVE PAD
SCALE: NONE



8 EXTERIOR STEEL COLUMN
SCALE: NONE



9 EXTERIOR AT TURN-DOWN
SCALE: NONE

HAUSER-CREECH, INC.
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NORTH CAROLINA
PROFESSIONAL
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ANGIER, NC

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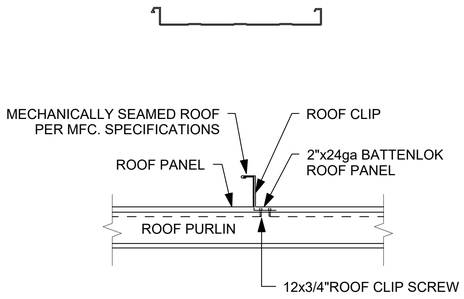
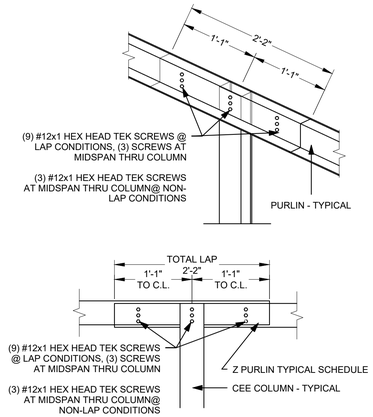
FOUNDATION DETAILS

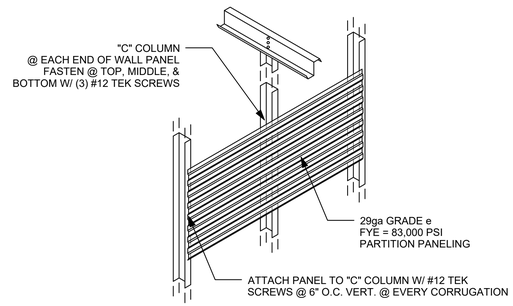
S4



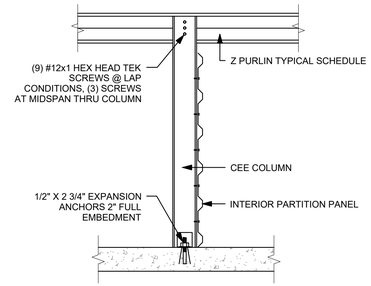
1 STANDING SEAM ROOF PANEL DETAIL
SCALE: NONE



2 PURLIN CONNECTION/LAP
SCALE: NONE



3 PURLIN SUPPORT WALL FRAMING
SCALE: NONE



4 PURLIN SUPPORT WALL CONNECTION
SCALE: NONE

5 PURLIN SUPPORT - HEADER CONNECTION
SCALE: NONE

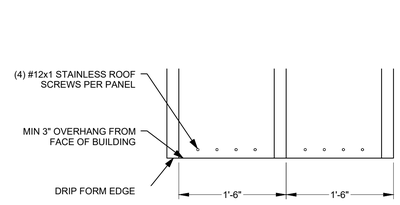
6 PURLIN SUPPORT - HEADER CONNECTION
SCALE: NONE

7 DETAIL NOT USED
SCALE: NONE

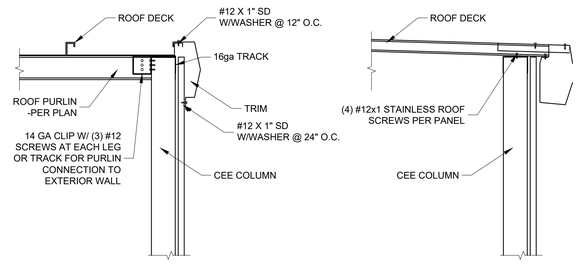
8 DETAIL NOT USED
SCALE: NONE

9 DETAIL NOT USED
SCALE: NONE

10 DETAIL NOT USED
SCALE: NONE



11 ROOF PANEL CONNECTION AT EAVE
SCALE: NONE



12 ROOF CONNECTION AT EXTERIOR
SCALE: NONE

HAUSER-CREECH, INC.
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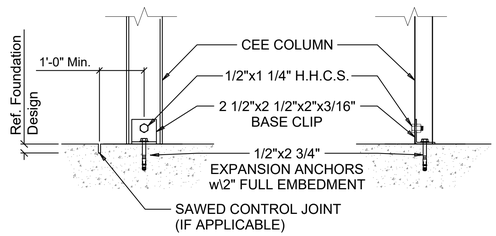
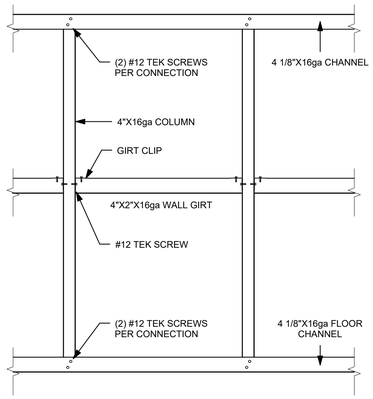
FRAMING DETAILS

S5



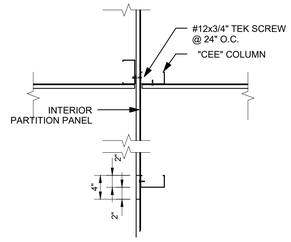
1 FIRST FLOOR - LATERAL WALL BRACING

SCALE: NONE



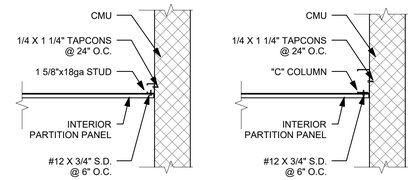
2 SECTION

SCALE: NONE



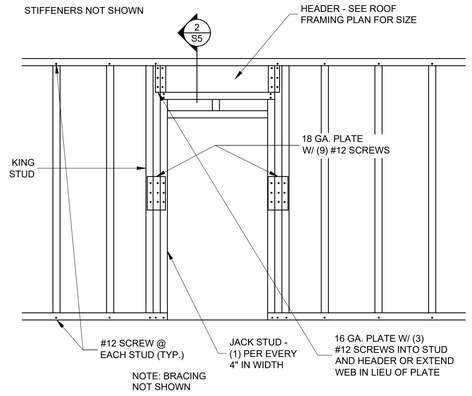
3 PARTITION WALL INTERSECTION W/ BEARING WALL

SCALE: NONE



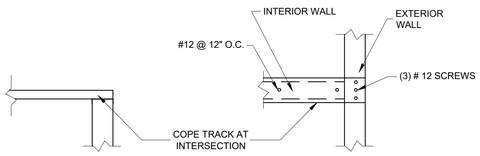
4 METAL WALL @ SHAFT

SCALE: NONE



5 DOUBLE HEADER - COMPOSITE SLAB LATERAL SUPPORT

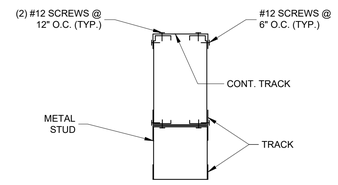
SCALE: NONE



ELEVATION VIEW PLAN VIEW

INTERSECTION OF INTERIOR AND EXTERIOR WALLS

ATTACH SHEATHING AND WALLBOARD TO STUDS @ 7" O.C. U.N.O.
PROVIDE DETAIL AT WALL BETWEEN EACH UNIT



6 PURLIN SUPPORT - HEADER CONNECTION

SCALE: NONE

HAUSER-CREECH, INC.
PROJECT #: 22-10X-00X

NORTH CAROLINA PROFESSIONAL SEAL 035814 ENGINEER MICHAEL GABRIEL HAUSER

MICHAEL GABRIEL HAUSER
NORTH CAROLINA PE NO. 035814

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ANGIER, NC

ISSUE DATE: 07.07.2022

REV	DATE

FRAMING DETAILS

S6