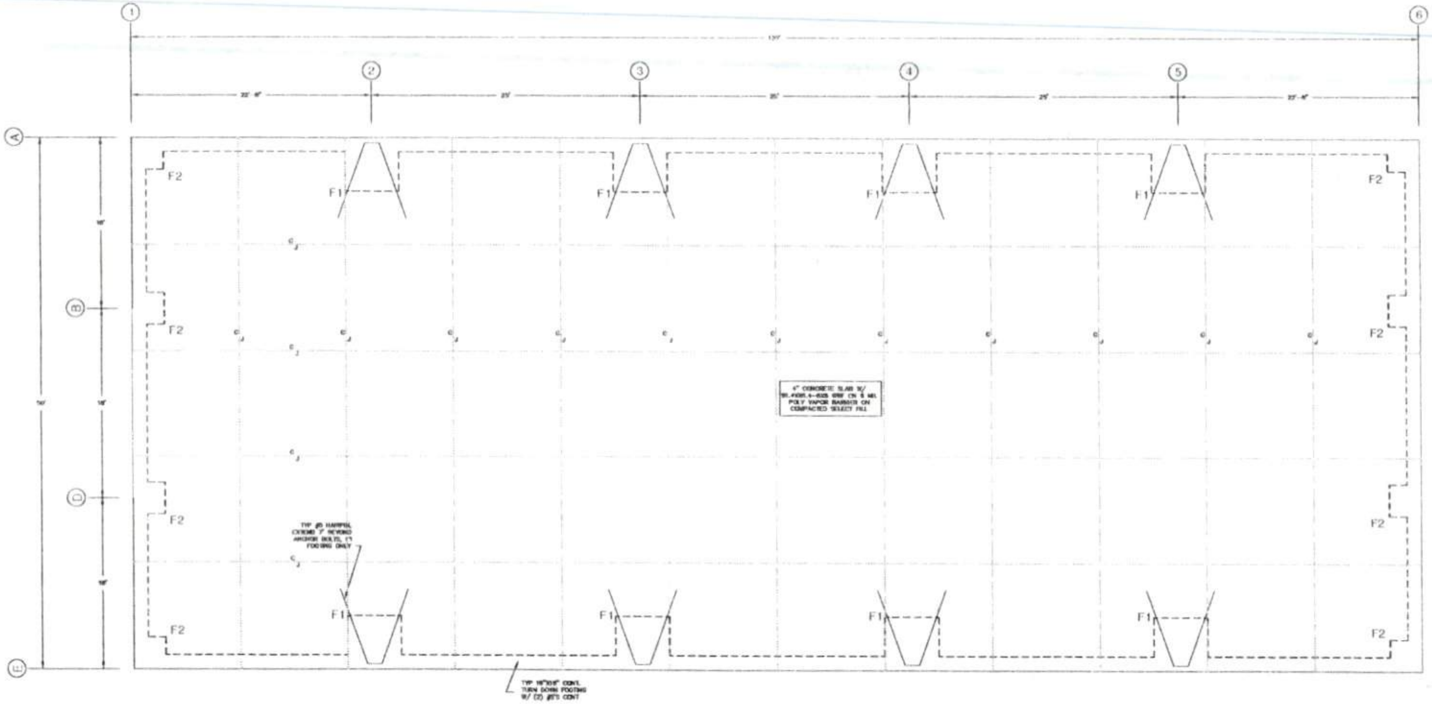
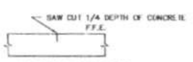


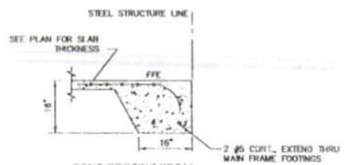
FOUNDATION PLAN

SCALE: 3/16"=1'-0"



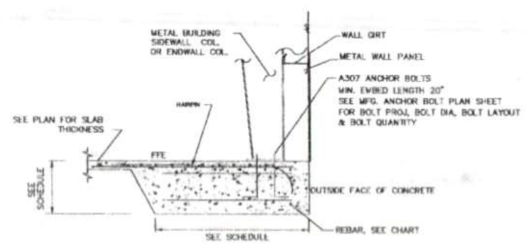
TYPICAL CONTROL JOINT

NTS



CONT. FOOTING DETAIL

NTS



TYPICAL FOOTING DETAIL

NTS

FOOTING	WIDTH	LENGTH	DEPTH	REINFORCEMENT
F1	5'-0"	5'-0"	2'-0"	(6) # 5. EA. WAY, TOP & BOTTOM
F2	3'-0"	3'-0"	2'-0"	(3) # 5. EA. WAY. BOTTOM

NOTES:

1. ALL CONCRETE TO HAVE MIN. 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI.
2. CONTRACTOR TO VERIFY ALL DIMENSIONS W/ METAL BUILDING MANUFACTURER'S DRAWINGS.
3. CONCRETE FOOTINGS AND SLAB TO BE PLACED ON COMPACTED SELECT MATERIAL MIN. 98% DENSITY - SELECT FILL PLACED & COMPACTED IN 6" LIFTS. ASSUMED SOIL BEARING CAPACITY = 1500 PSF.
4. CONSTRUCTION JOINTS TO BE SPACED MAX. 10' O/C.
5. SEE MANUFACTURER'S ANCHOR PLANS FOR ANCHOR BOLT LAYOUT, QUANTITY, SIZE & PROJECTION.
6. REINFORCING STEEL SHALL BE 60 KSI., ASTM A 615.
7. SEE MANUFACTURER'S DRAWINGS FOR FRAMED OPENING LOCATIONS.
8. CONT. REINFORCING STEEL SHALL HAVE 24 BAR DIA LAP OR MIN. 12".
9. CLEAR DISTANCE FOR ALL REINFORCING SHALL BE 3", UON.
10. PROVIDE CORNER BARS AT ALL FOOTING CORNERS. CORNER BARS SHALL HAVE THE SAME SIZE AND SPACING AS MAIN REINFORCING AND SHALL HAVE A MIN 48" LAP.
11. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A 185 & A 82.
12. FLOOR LIVE LOAD DESIGN = 125 PSF
13. FOUNDATION DESIGNED USING REACTIONS PROVIDED BY THE METAL BUILDING MFR AND IN ACCORDANCE TO THE 2018 NC BUILDING CODE FOR 121 ULTIMATE WIND SPEED.
14. FOOTING AND SLAB SHALL BE POURED MONOLITHICALLY.
15. FLATNESS AND LEVELNESS IN ACCORDANCE W/ ACI 117 FF25/FL20

SUMMIT DESIGN GROUP INC
STRUCTURAL • CIVIL • MECHANICAL • ELECTRICAL
PO BOX 995
BLADENBORO, NC 28320
910-876-0376

ENGINEER

DRAWING DATA	
DATE:	1/27/22
SCALE:	3/16" = 1'-0"
DRAWN BY:	RPA
DESIGNED BY:	RPA
CHECKED BY:	
PLOT SCALE:	N/A
CAD FILE NO:	S-1

ROCK SOLID

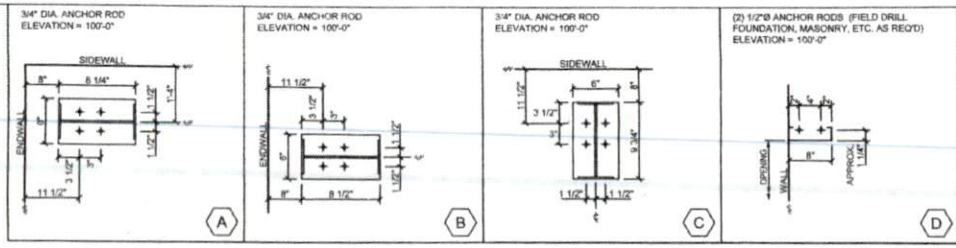
116 CROWNVIEW LANE, DUNN, NC

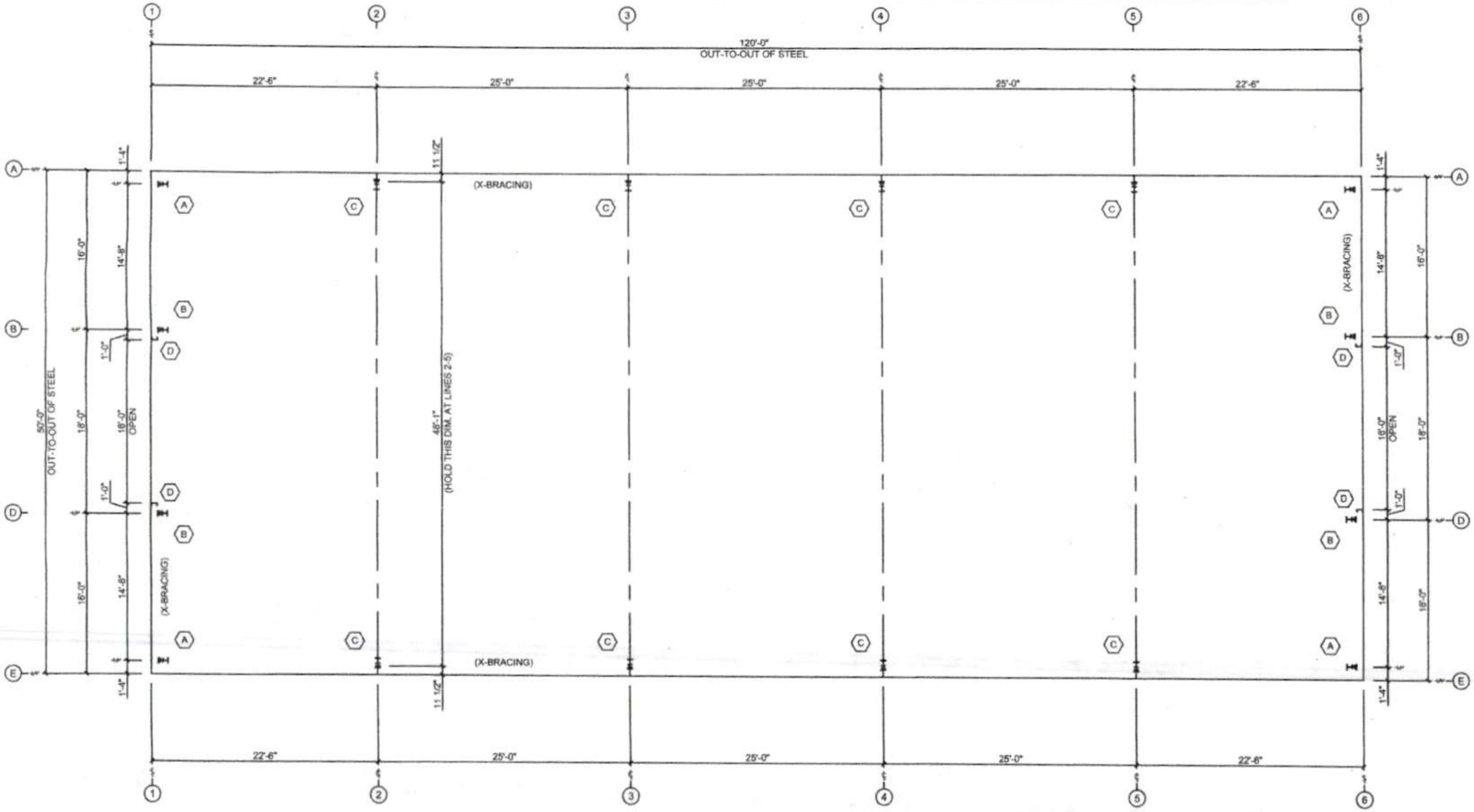
FOUNDATION PLAN

NO.	REVISION

SHEET

S-1





ANCHOR ROD PLAN

ANCHOR ROD PLAN GENERAL NOTES:

AN1. ANCHOR ROD EMBEDMENT LENGTH SHALL BE DETERMINED BY THE FOUNDATION ENGINEER.

AN2. METAL BUILDING MANUFACTURER IS NOT RESPONSIBLE FOR PROJECT FOUNDATION DESIGN. THE FOUNDATION DESIGN IS THE RESPONSIBILITY OF A REGISTERED PROFESSIONAL ENGINEER, FAMILIAR WITH LOCAL SITE CONDITIONS.

AN3. ANCHOR RODS, NUTS, FLAT WASHERS FOR ANCHOR RODS, EXPANSION BOLTS, AND CONCRETE / MASONRY EMBEDMENT PLATES ARE NOT BY METAL BUILDING MANUFACTURER.

AN4. THE ANCHOR ROD LOCATIONS PROVIDED BY METAL BUILDING MANUFACTURER SATISFY PERTINENT REQUIREMENTS FOR THE DESIGN OF THE MATERIALS SUPPLIED BY THE METAL BUILDING MANUFACTURER. IT IS THE RESPONSIBILITY OF THE FOUNDATION ENGINEER TO MAKE SURE THAT SUFFICIENT EDGE DISTANCE IS PROVIDED FOR ALL ANCHOR RODS IN THE DETAILS OF THE FOUNDATION DESIGN.

AN5. DRAWINGS ARE NOT TO SCALE. SEE DETAILS FOR COLUMN ORIENTATION.

AN6. THE ANCHOR ROD PLAN INDICATES WHERE THE ANCHOR RODS ARE TO BE PLACED, AS WELL AS THE FOOTPRINT OF THE METAL BUILDING. IT IS ESSENTIAL THAT THESE ANCHOR ROD PATTERNS BE FOLLOWED. IF THESE SETTINGS DIFFER FROM THE ARCHITECTURAL FOUNDATION PLANS, THE METAL BUILDING MANUFACTURER MUST BE CONTACTED IMMEDIATELY - BEFORE CONCRETE IS PLACED.

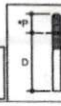
AN7. "SINGLE" CEE COLUMNS SHALL BE ORIENTED WITH THE "TOES" TOWARD THE LOW EAVE UNLESS NOTED OTHERWISE.

AN8. ALL DIMENSIONS ARE OUT TO OUT OF STEEL. IF CONCRETE NOTCH IS REQUIRED, THEN THE REQUIRED DIMENSION SHOULD BE ADDED TO OBTAIN THE OUT TO OUT OF CONCRETE DIMENSIONS.

AN9. FINISHED FLOOR ELEVATION = 100'-0", AND BOTTOM OF BASE PLATE = 100'-0", UNLESS NOTED OTHERWISE.

DESIGN ENGINEER:

DATE:



BOTTOM OF BASE PLATE

ANCHOR ROD SHAPE AND EMBEDMENT LENGTH "D" IS TO BE DETERMINED BY THE FOUNDATION ENGINEER.

ANCHOR RODS			
QTY.	DIA.	MATERIAL	PROJECTION (*P)
64	3/4"	F1554 GR 36	3"
	1"	F1554 GR 36	3"
	1-1/4"	F1554 GR 36	3-1/2"
	1-1/2"	F1554 GR 36	3-1/2"

Digitally signed by Andrew D. Johnson
Date: 2021.10.06 09:36:23-04'00'

KIRBY BUILDING SYSTEMS
A NUCOR Company

MBMA

IAS ACCREDITED

JOB NUMBER	K21L0812A
PROJECT NAME	ROCK SOLID
ADDRESS	116 CROWNVIEW LANE DUNN, NC 28334
CUSTOMER NAME	SOUTHEASTERN DESIGN AND CONSTRUCTION
DRAWING STATUS	FOR CONSTRUCTION
DRAWING TITLE	ANCHOR ROD PLAN
SHEET	AB1

NO	RELEASE / REVISION	DWN / CHK	ENG	DATE
0	ANCHOR ROD	DUG	CTT	09/28/21

GENERAL NOTES:

1. MATERIALS

	ASTM DESCRIPTION
STRUCTURAL STEEL PLATE	AS529 / A572 / A1011
HOT ROLLED MILLS SHAPES	A36 / A529 / A572 / A500
HSS ROUND	A500
HSS RECTANGULAR	A500
COLD FORM SHAPES	A653 / A1011
ROOF AND WALL SHEETING	A653 / A792
BOLTS	A307 / A325 / A490
CABLE	A475
RODS	A529 / A572

2. STRUCTURAL PRIMER NOTE:

SHOP COAT PRIMER IS INTENDED TO PROTECT THE STEEL FRAMING FOR A SHORT PERIOD OF TIME. STORAGE IN EXTREME COLD TEMPERATURES OR WINTER SNOW CONDITIONS, INCLUDING TRANSPORTATION ON SALTED OR CHEMICALLY TREATED ROADS WILL ADVERSELY AFFECT THE DURABILITY AND LONGEVITY OF THE PRIMER. THE COAT OF SHOP PRIMER DOES NOT PROVIDE THE UNIFORMITY OF APPEARANCE, OR THE DURABILITY AND CORROSION RESISTANCE OF A FIELD APPLIED FINISH COAT OF PAINT OVER A SHOP PRIMER. MINOR ABRASIONS TO THE SHOP COAT PRIMER CAUSED BY HANDLING, LOADING, SHIPPING, UNLOADING AND ERECTION ARE UNAVOIDABLE AND ARE NOT THE RESPONSIBILITY OF THE METAL BUILDING MANUFACTURER. METAL BUILDING MANUFACTURER IS NOT *RESPONSIBLE FOR* THE DETERIORATION OF THE PRIMER OR CORROSION THAT MAY RESULT FROM ATMOSPHERIC AND ENVIRONMENTAL CONDITIONS NOR THE COMPATIBILITY OF THE PRIMER TO ANY FIELD APPLIED COATING.

3. BUILDING ERECTION NOTES:

THE GENERAL CONTRACTOR AND/OR ERECTOR IS RESPONSIBLE TO SAFELY AND PROPERLY ERECT THE METAL BUILDING SYSTEM IN CONFORMANCE WITH THESE DRAWINGS, OSHA REQUIREMENTS, AND EITHER MBMA OR CSA S16 STANDARDS PERTAINING TO PROPER ERECTION. TEMPORARY SUPPORTS SUCH AS GUYS, BRACES, FALSEWORK, CRIBBING, OR OTHER ELEMENTS FOR ERECTION ARE TO BE DETERMINED, FURNISHED, AND INSTALLED BY THE ERECTOR. THESE SUPPORTS MUST SECURE THE STEEL FRAMING, OR PARTLY ASSEMBLED STEEL FRAMING, AGAINST LOADS COMPARABLE IN INTENSITY TO THOSE FOR WHICH THE STRUCTURE WAS DESIGNED IN ADDITION TO LOADS RESULTING FROM THE ERECTION OPERATION. SECONDARY WALL AND ROOF FRAMING (GIRTS, PURLINS, AND/OR JOISTS) ARE NOT DESIGNED TO FUNCTION AS A WORKING PLATFORM OR TO PROVIDE AS AN ANCHORAGE POINT FOR A FALL ARREST / SAFETY TIE OFF.

4. SPECIAL INSPECTION:

SPECIAL INSPECTIONS AND TESTING THAT MAY BE REQUIRED BY GOVERNMENTAL OR OTHER AUTHORITY DURING CONSTRUCTION AND/OR STEEL FABRICATION (COLLECTIVELY, "INSPECTIONS") ARE NOT THE RESPONSIBILITY OF NBG, AND TO THE EXTENT REQUIRED IT SHALL BE THE RESPONSIBILITY OF THE BUILDER AND/OR OWNER. IN THE EVENT INSPECTIONS ARE REQUIRED, THE BUILDER AND/OR OWNER SHALL EMPLOY A THIRD PARTY QUALITY ASSURANCE TESTING AGENCY APPROVED BY THE RELEVANT AUTHORITY. IF SUCH REQUIREMENTS ARE NOT SPECIFICALLY INCLUDED IN NBG SALES DOCUMENTS, NO INSPECTIONS BY NBG OR AT ANY NBG FACILITY SHALL BE MADE. ALL NBG FACILITIES ARE ACCREDITED BY IAS AC472.

5. A325 & A490 BOLT TIGHTENING REQUIREMENTS:

IT IS THE RESPONSIBILITY OF THE ERECTOR TO ENSURE PROPER BOLT TIGHTNESS IN ACCORDANCE WITH APPLICABLE REGULATIONS. FOR PROJECTS IN THE UNITED STATES SEE THE RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING A325 OR A490 BOLTS OR FOR PROJECTS IN CANADA, SEE THE CAN/CSA S16 LIMIT STATES DESIGN OF STEEL STRUCTURES FOR MORE INFORMATION.

THE FOLLOWING CRITERIA MAY BE USED TO DETERMINE THE BOLT TIGHTNESS (I.E., "SNUG-TIGHT" OR "FULLY-PRETENSIONED"), UNLESS REQUIRED OTHERWISE BY LOCAL JURISDICTION OR CONTRACT REQUIREMENTS:

A) ALL A490 BOLTS SHALL BE "FULLY-PRETENSIONED".
B) ALL A325 BOLTS IN PRIMARY FRAMING (RIGID FRAMES AND BRACING) MAY BE "SNUG-TIGHT", EXCEPT AS FOLLOWS: "FULLY-PRETENSION" A325 BOLTS IF:
   a) BUILDING SUPPORTS A CRANE SYSTEM WITH A CAPACITY GREATER THAN 5 TONS.
   b) BUILDING SUPPORTS MACHINERY THAT CREATES VIBRATION, IMPACT OR STRESS-REVERSALS ON THE CONNECTIONS. THE ENGINEER-OF-RECORD FOR THE PROJECT SHOULD BE CONSULTED TO EVALUATE FOR THIS CONDITION.
   c) THE PROJECT SITE IS LOCATED IN A HIGH SEISMIC AREA. FOR IBC-BASED CODES, "HIGH SEISMIC AREA" IS DEFINED AS "SEISMIC DESIGN CATEGORY" OF "D", "E", OR "F". SEE THE "BUILDING LOADS" SECTION OF THIS PAGE FOR THE DEFINED SEISMIC DESIGN CATEGORY FOR THIS PROJECT.
   d) ANY CONNECTION DESIGNATED IN THESE DRAWINGS AS "A325-SC" OR "SLIP-CRITICAL (SC)" CONNECTIONS MUST BE FREE OF PAINT, OIL, OR OTHER MATERIALS THAT REDUCE FRICTION AT CONTACT SURFACES. GALVANIZED OR LIGHTLY RUSTED SURFACES ARE ACCEPTABLE.

C) IN CANADA, ALL A325 AND A490 BOLTS SHALL BE "FULLY PRE-TENSIONED", EXCEPT FOR SECONDARY MEMBERS (PURLINS, GIRTS, OPENING FRAMING, ETC.) AND FLANGE BRACES.

SECONDARY MEMBER (PURLIN, GIRT, OPENING FRAMING, ETC.) AND FLANGE BRACE CONNECTIONS MAY ALWAYS BE "SNUG-TIGHT", UNLESS INDICATED OTHERWISE IN THESE DRAWINGS.

6. GENERAL DESIGN NOTES:

1) ALL STRUCTURAL *STEEL SECTIONS AND WELDED* PLATE MEMBERS ARE DESIGNED IN ACCORDANCE WITH ANSI/AISC 360 "SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS" OR THE CAN/CSA S16 "LIMIT STATES DESIGN OF STEEL STRUCTURES", AS REQUIRED BY THE SPECIFIED BUILDING CODE.
2) ALL WELDING OF STRUCTURAL STEEL IS BASED ON EITHER AWS D1.1 "STRUCTURAL WELDING CODE – STEEL" OR CAN/CSA W59 "WELDED STEEL CONSTRUCTION (METAL ARC WELDING)", AS REQUIRED BY THE SPECIFIED BUILDING CODE.
3) ALL COLD FORMED MEMBERS ARE DESIGNED IN ACCORDANCE WITH ANSI/AISI 100 OR THE CAN/CSA S136 "SPECIFICATIONS FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS", AS REQUIRED BY THE SPECIFIED BUILDING CODE.
4) ALL WELDING OF COLD FORMED STEEL IS BASED ON AWS D1.3 "STRUCTURAL WELDING CODE – SHEET STEEL" OR CAN/CSA W59 "WELDED STEEL CONSTRUCTION (METAL ARC WELDING)", AS REQUIRED BY THE SPECIFIED BUILDING CODE.
5) THIS MANUFACTURING FACILITY IS IAS AC-472 ACCREDITED AND CAN/CSA A660 AND W47.1 CERTIFIED (IF APPLICABLE) FOR THE DESIGN AND MANUFACTURING OF METAL BUILDING SYSTEMS.
6) IF JOISTS ARE INCLUDED WITH THIS PROJECT, THEY ARE SUPPLIED AS A PART OF THE SYSTEMS ENGINEERED METAL BUILDING AND ARE FABRICATED IN ACCORDANCE WITH THE REQUIREMENTS OF SECTION 1926.758 OF OSHA SAFETY STANDARDS FOR STEEL ERECTION DATED JANUARY 18, 2001.

THE DRAWINGS AND THE METAL BUILDING THEY REPRESENT ARE THE PRODUCT OF THE METAL BUILDING MANUFACTURER. THE REGISTERED PROFESSIONAL ENGINEER'S SEAL PERTAINS ONLY TO THE REQUIREMENTS LISTED HEREIN FOR THE MATERIALS DESIGNED AND SUPPLIED BY THE METAL BUILDING MANUFACTURER. THE REGISTERED PROFESSIONAL ENGINEER WHOSE SEAL APPEARS ON THESE DRAWINGS IS EMPLOYED OR ENGAGED BY THE METAL BUILDING MANUFACTURER AND DOES NOT SERVE AS OR REPRESENT THE PROJECT ENGINEER OF RECORD AND SHALL NOT BE CONSTRUED AS SUCH.

7. GLOSSARY OF ABBREVIATIONS:

A.B. = ANCHOR RODS
B.U. = BUILT-UP
BS = BOTH SIDES
DIA = DIAMETER
F.S. = FAR SIDE
FLG = FLANGE
GA. = GAUGE
H.S.B. = HIGH STRENGTH BOLTS
HT. = HEIGHT
LLV = LONG LEG VERTICAL
M.B. = MACHINE BOLTS
MAX = MAXIMUM
MBS = METAL BUILDING SUPPLIER
MIN = MINIMUM
N.S. = NEAR SIDE
N/A = NOT APPLICABLE
NIC = NOT IN CONTRACT
O.A.L. = OVERALL LENGTH
O.C. = ON CENTER
PL = PLATE
REQ'D = REQUIRED
REV. = REVISION
SIM = SIMILAR
SL = STEEL LINE
SLV = SHORT LEG VERTICAL
TBD = TO BE DETERMINED
TYP = TYPICAL
U.N.O. = UNLESS NOTED OTHERWISE

?? = PART MARK TO BE DETERMINED AND WILL BE UPDATED ON CONSTRUCTION DRAWINGS

KIRBY BUILDING SYSTEMS
124 KIRBY DRIVE
PORTLAND, TN 37148
PHONE: 615-325-4165

PROJECT BUILDING LOADS

CERTIFICATION EXTENDS ONLY FOR THE LOADS SPECIFIED ON KIRBY'S PURCHASE ORDER TO THE STRUCTURAL COMPONENTS OF THE BUILDING DESIGNED AND SUPPLIED BY KIRBY BUILDING SYSTEMS, IF ERECTED AS INDICATED. KIRBY'S CUSTOMER IS TO CONFIRM THAT THESE LOADS COMPLY WITH THE REQUIREMENTS OF THE LOCAL BUILDING DEPARTMENT. NOTE THAT KIRBY'S ENGINEER IS NOT ACTING AS THE ENGINEER OF RECORD FOR THIS CONSTRUCTION PROJECT. DESIGN LOADS HAVE BEEN APPLIED IN ACCORDANCE WITH THE FOLLOWING.

DESIGN CODE: NORTH CAROLINA (NCBC 2018)
ROOF LIVE LOAD: 20.00 psf
REDUCIBLE PER CODE
*** RISK CATEGORY:
II - STANDARD BUILDINGS

GROUND SNOW LOAD: 10.00 psf
SNOW EXP. FACTOR, Ce: 1.00
SNOW IMPORTANCE FACTOR, Is: 1.00

ULTIMATE DESIGN WIND SPEED: 121 mph (Vult)
NOMINAL DESIGN WIND SPEED: 94 mph (Vasd)
WIND EXPOSURE: C

DESIGN SUCTION / PRESSURE FOR WALL COMPONENTS AND CLADDING NOT DESIGNED OR PROVIDED BY KBS: + 30 PSF / - 40 PSF

*** FOR RISK CATEGORY I OR II BUILDINGS, IBC ALLOWS FOR SINGLE STORY BUILDINGS TO HAVE NO LIMIT FOR SEISMIC STORY DRIFT. PLEASE NOTE THAT ANY INTERIOR WALLS, PARTITIONS, CEILINGS, AND EXTERIOR WALLS SHOULD BE DETAILED (BY OTHERS) TO ACCOMMODATE THIS STORY DRIFT.

UL-90 : NO

SEISMIC INFORMATION: Ss: 0.183 S1: 0.085
DESIGN (Sds / Sd1) : 0.195/0.136
SITE CLASS: D
SEISMIC IMP. FACTOR, Ie: 1.00
SEISMIC DESIGN CATEGORY: C

ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE
BASIC SFRS: NOT DETAILED FOR SEISMIC

STATE: NC
COUNTY: HARNETT

NOTES:

1) COLLATERAL DEAD LOADS, UNLESS OTHERWISE NOTED, ARE ASSUMED TO BE UNIFORMLY DISTRIBUTED. WHEN SUSPENDED SPRINKLER SYSTEMS, LIGHTING, HVAC EQUIPMENT, CEILINGS, ETC., ARE SUSPENDED FROM ROOF MEMBERS, CONSULT THE M.B.S. IF THESE CONCENTRATED LOADS EXCEED 500 POUNDS (USING THE WEB MOUNT DETAIL), OR 200 POUNDS (USING THE FLANGE MOUNT DETAIL), OR IF INDIVIDUAL MEMBERS ARE LOADED SIGNIFICANTLY MORE THAN OTHERS.

2) THE DESIGN OF STRUCTURAL MEMBERS SUPPORTING GRAVITY LOADS IS CONTROLLED BY THE MORE CRITICAL EFFECT OF ROOF LIVE LOAD OR ROOF SNOW LOAD, AS DETERMINED BY THE APPLICABLE CODE.

3) ALL WELDING MUST BE PERFORMED BY AWS QUALIFIED WELDERS FOR THE WELDING PROCESSES AND POSITIONS TO BE USED. ALL WELDING AND WELD PREP MUST BE COMPLETED AND VISUALLY INSPECTED TO AWS ACCEPTANCE CRITERIA (TABLE 6.1) IN ACCORDANCE WITH THE APPLICABLE AWS STANDARD. WELD ELECTRODES USED FOR ALL FIELD WELD PROCESSES MUST BE SELECTED FROM TABLE 3.1 IN AWS D1.1 FOR GROUP II MATERIAL GREATER THAN OR EQUAL TO 0.125" THICK OR TABLE1.2 IN AWS D1.3 FOR MATERIAL LESS THAN 0.125" THICK AND ALL FILLER MATERIAL MUST HAVE A Fu OF 70 KSI.

4) ALL EXTERIOR COMPONENTS (WINDOWS, DOORS, ETC) MUST MEET WIND LOADING REQUIREMENTS FOR THE BUILDING CODE LISTED ABOVE OR MUST BE ADEQUATELY PROTECTED DURING A HIGH WIND EVENT. ALL GLAZING AND OTHER APPLICABLE OPENINGS IN WINDBORNE DEBRIS REGIONS MUST BE IMPACT-RESISTANT OR PROTECTED WITH AN IMPACT-RESISTANT COVERING. IMPACT RESISTANT MATERIALS MUST MEET THE LARGE AND/OR SMALL MISSILE TEST OF ASTM E 1996 AND ASTM E 1886.

BUILDING SPECIFIC LOADING INFORMATION

* DEAD LOAD: NORMAL WEIGHT OF METAL BUILDING COMPONENTS, NOT INCLUDING PRIMARY FRAMING, AS SUPPLIED BY THE MANUFACTURER

** Pm IS BASED ON THE MINIMUM ROOF SNOW LOAD CALCULATED PER BUILDING CODE OR THE CONTRACT-SPECIFIED ROOF SNOW LOAD, WHICHEVER IS GREATER. THIS VALUE, Pm, IS ONLY APPLIED IN COMBINATION WITH DEAD AND COLLATERAL LOADS. ROOF SNOW IN OTHER LOADING CONDITIONS IS DETERMINED PER THE SPECIFIED BUILDING CODE.

	ROOF DEAD	COLLATERAL DEAD		SNOW COEFFICIENT		SNOW LOAD		WIND		SEISMIC		
BLDG.	(psf)*	Pri (psf)	Sec (psf)	Ct	Cs	Ps (psf)	**Pm (psf)	Enclosure	GCpi	R	Cs	V (kips)
A	3.0	3.0	3.0	1.0	1.00	7.00	10.00	Enclosed	±0.18	3.00	0.085	3.1

CONTENTS

PRIMER

STRUCTURAL FRAMING: GP - GRAY PRIMER
WALL SECONDARY: GP - GRAY PRIMER
ROOF SECONDARY: GP - GRAY PRIMER

ROOF PANELS

TYPE: 24 Ga. STANDING SEAM 360 (SS3)
HIGH SYSTEM w/ THERMAL SPACERS
COLOR: GALVALUME PLUS (GM)

WALL PANELS

TYPE: 26 Ga. KIRBY WALL (KW1)
COLOR: SANDSTONE, SP (SS)

TRIM COLORS

ROOF LINE TRIM: POLAR WHITE, SP (PW)
DOWNSPOUTS: POLAR WHITE, SP (PW)
WALL CORNER TRIM: POLAR WHITE, SP (PW)
BASE TRIM: SANDSTONE, SP (SS)
FRAMED OPENING TRIM: SANDSTONE, SP (SS)

NOTE: ANY VARIANCE FROM THE PANEL TYPES OR COLORS LISTED HERE WILL BE NOTED ON THE ELEVATION DRAWINGS.

KIRBY BUILDING SYSTEMS
A NUCOR Company
(615) 325-4165
200 Whetstone Rd.
Swansea, SC 29160
COA # F-1470
IAS ACCREDITED

JOB NUMBER: K21L0812A
PROJECT NAME: ROCK SOLID
ADDRESS: 116 CROWNVIEW LANE
DUNN, NC 28334
CUSTOMER NAME: SOUTHEASTERN DESIGN AND CONSTRUCTION
DRAWING STATUS: FOR PERMITS ONLY **NOT FOR CONSTRUCTION**
SHEET: C1
DRAWING TITLE: COVERSHEET

NO	RELEASE / REVISION	DWN / CHK	ENG	DATE
0	ANCHOR ROD	DUG	CTT	09/29/21
1	PERMITS	DUG	CTT	10/04/21

Digitally signed by Andrew D. Johnson
Date: 2021.10.06 09:27:21-04'00'