

GENERAL NOTES

- THE CONTRACTOR SHALL VERIFY DIMENSIONS AND SITE CONDITIONS BEFORE STARTING WORK AND THE DESIGNER SHALL BE NOTIFIED IMMEDIATELY OF ANY DISCREPANCIES. IN NO CASE SHALL DIMENSIONS BE SCALED FROM PLANS, SECTIONS, OR DETAILS ON THESE DRAWINGS.
- ALL OMISSIONS AND CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND OR SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER BEFORE PROCEEDING WITH ANY WORK SO INVOLVED.
- NO STRUCTURAL MEMBER SHALL BE CUT FOR PIPES, DUCTS, ETC., UNLESS NOTED.
- THE CONTRACTOR SHALL DETERMINE THE LOCATION OF EXISTING UTILITY SERVICES IN THE AREA TO BE EXCAVATED PRIOR TO BEGINNING OF EXCAVATION.
- ALL WORKMANSHIP AND MATERIALS SHALL CONFORM TO THE REQUIREMENTS OF THE 2018 EDITION OF THE "NC STATE BUILDING CODE".
- THESE DRAWINGS REPRESENT THE FINISHED STRUCTURE AND DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR IS RESPONSIBLE FOR ALL TEMPORARY BRACING, SHORING, AND SUPPORT NECESSARY TO ACHIEVE THE FINISHED STRUCTURE.

FOUNDATION NOTES:

1. MAXIMUM DESIGN SOIL PRESSURE:	CODE MINIMUM: 2,000	PSF
CONTINUOUS FOOTINGS:	2,000	PSF
FAD FOOTINGS:	2,000	PSF

2. SEE SOILS REPORT BY: N/A
PROJECT NO.: N/A
DATED: N/A

3. ALL FOOTINGS TO BE A MINIMUM OF: 12" BELOW NATURAL GRADE
12" BELOW FINISHED GRADE

- SOILS COMPACTION AND SITE PREPARATION TO BE IN ACCORDANCE WITH SOILS REPORT (AS APPLICABLE). IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY SOIL BEARING CAPACITY.
- FINISH EXCAVATION FOR FOUNDATION SHALL BE NEAT AND TRUE TO LINE WITH LOOSE MATERIAL REMOVED FROM EXCAVATION.
- THE FOOTING EXCAVATIONS SHALL BE KEPT FREE FROM LOOSE MATERIAL AND BEFORE ANY FOOTING CONCRETE IS PLACED, SHALL BE CHECKED AND APPROVED BY CONTRACTOR FOR COMPLIANCE WITH THE REQUIREMENTS.
- SIDE OF FOUNDATION MAY BE FOUDED AGAINST STABLE EARTH (U.O.N.). CONTRACTOR SHALL PROTECT ALL UTILITY LINES, ETC., ENCOUNTERED DURING EXCAVATION AND BACKFILLING.
- CONTRACTOR TO BRACE OR PROTECT ALL RETAINING WALLS FROM LATERAL LOADS UNTIL SUPPORTING FLOORS, WALLS AND/OR SLABS ARE COMPLETELY IN PLACE AND HAVE BEEN SHEATHED PER PLAN OR ATTAINED FULL STRENGTH.
- FOOTING BACKFILL AND UTILITY TRENCH BACKFILL WITHIN BUILDING AREA SHALL BE MECHANICALLY COMPACTED IN LAYERS TO THE APPROVAL OF THE GEOTECHNICAL ENGINEER, AS APPLICABLE. FLOODING WILL NOT BE PERMITTED.
- ALL CONCRETE AND MASONRY FOUNDATION WALLS ARE TO BE CONSTRUCTED IN ACCORDANCE W/ NC BUILDING CODE SECTION 1907, ACI 318, ACI 332, NCMA TR-68-A, OR ACE 530/ASCE5/TMS 402. FOUNDATION WALLS MAY BE STEPPED AND FRAMED W/ 2x6 @ 16" O.C. KNEE WALLS WHERE GRADE FERMENTS.

MASONRY NOTES:

- CONCRETE MASONRY WALLS SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF $f'm = 1500 \text{ PSI}$.
- CONCRETE MASONRY UNITS SHALL BE MINIMUM LIGHTWEIGHT UNITS CONFORMING TO ACI 530/ASCE 5/TMS 402, WITH MAX LINEAR SHRINKAGE OF 0.06% (1300 PSI MINIMUM).
- MORTAR SHALL BE TYPE "M" OR "S", CONFORMING TO IRC SECTION R-607 AND TO ASTM C-270.
- ALL GROUT SHALL ATTAIN A MINIMUM COMPRESSIVE STRENGTH OF 2,000 PSI AT 28 DAYS. GROUT SHALL BE PROPORTIONED PER IRC TABLE R-607.1 AND WITH SUFFICIENT WATER FOR POURING WITHOUT SEGREGATION OF GROUT CONSTITUENTS.
- ALL CELLS CONTAINING REINFORCING STEEL OR EMBEDDED ITEMS AND ALL CELLS IN RETAINING WALLS AND WALLS BELOW GRADE SHALL BE SOLID GROUTED UNLESS OTHERWISE NOTED ON PLANS.
- ALL HORIZONTAL REINFORCEMENT SHALL BE PLACED IN BOND BEAM OR LINTEL BEAM UNITS.
- WHEN GROUTING IS STOPPED FOR ONE HOUR OR LONGER, HORIZONTAL CONSTRUCTION JOINTS SHALL BE FORMED BY STOPPING THE GROUT FOUR 1/2" BELOW TOP OF THE UPPERMOST UNIT.
- ALL BOND BEAM BLOCK SHALL BE "DEEP CUT" UNITS.
- PROVIDE INSPECTION AND CLEANOUT HOLES AT BASE OF VERTICAL CELLS HAVING GROUT LIFTS IN EXCESS OF 4'-0" OF HEIGHT.
- ALL GROUT SHALL BE CONSOLIDATED WITH A MECHANICAL VIBRATOR.
- ANCHOR BOLTS MUST BE SET WITH TEMPLATES AND HELD IN PLACE PRIOR TO GROUTING. PROVIDE AT LEAST ONE INCH OF GROUT BETWEEN ANCHOR BOLT AND MASONRY.
- SPECIAL INSPECTION IS REQUIRED FOR $f'm \geq 1500 \text{ PSI}$.

CONCRETE NOTES:

- CONCRETE IN ALL WORK SHALL HAVE 3,000 PSI ULTIMATE COMPRESSIVE STRENGTH AT 28 DAYS.
- CEMENT SHALL CONFORM TO ASTM C-15, TYPE I OR TYPE II.
- AGGREGATES FOR NORMAL WEIGHT CONCRETE SHALL CONFORM TO ASTM C-33. AGGREGATES FOR SHOTCRETE/GUNITE SHALL NOT EXCEED 3/4". READY MIX CONCRETE SHALL BE MIXED AND DELIVERED IN ACCORDANCE WITH ASTM C-94-R1.
- ADMIXTURE MAY BE USED WITH THE PRIOR APPROVAL OF THE ENGINEER. ADMIXTURE (COMPLYING WITH ASTM A494) USE TO INCREASE THE WORKABILITY OF THE CONCRETE SHALL NOT BE CONSIDERED TO REDUCE THE SPECIFIED MINIMUM CEMENT (CALCIUM CHLORIDE SHALL NOT BE USED).
- WATER SHALL BE CLEAN, FREE FROM DELETERIOUS AMOUNT OF ACIDS, ALKALIS OR ORGANIC MATERIALS.
- SLUMPS: THE MAXIMUM SLUMP SHALL NOT EXCEED 5". DURING TEMPERATURES ABOVE 80°F, MAXIMUM OF 6" SLUMP IS PERMISSIBLE PROVIDED THE DESIGN IS REVISED ACCORDINGLY BY THE TESTING LABORATORY, AS APPLICABLE. MEASURE SLUMP IN ACCORDANCE WITH METHOD OF TEST FOR SLUMP OF PORTLAND CEMENT CONCRETE ASTM C143.
- IF APPLICABLE, 3/4" DEEP CONTROL JOINTS ARE TO BE SAWCUT TO SUBDIVIDE ALL FLOOR SLABS ON GRADE INTO APPROXIMATELY SQUARE AREAS OF 400 SQ FT OR LESS. CONTRACTOR IS RESPONSIBLE FOR ADJUSTING OR ADDING CONTROL JOINTS AS NECESSARY.

REINFORCING STEEL NOTES:

- STEEL REINFORCEMENT SHALL BE: GR 40 = #4 & SMALLER
ASTM A615 GR. 60 = #5 & LARGER
ASTM A185 = WELDED WIRE FABRIC
- REINFORCING DETAILING AND PLACING SHALL BE IN ACCORDANCE WITH THE CONCRETE REINFORCING STEEL INSTITUTE "MANUAL OF STANDARD PRACTICE" LATEST EDITION.
- ALL REINFORCING STEEL, ANCHOR BOLTS, DOWNELS, AND INSERTS SHALL BE WELL SECURED IN POSITION PRIOR TO PLACING CONCRETE.
- REINFORCING STEEL SHALL BE PROVIDED WITH THE FOLLOWING AMOUNTS OF CONCRETE COVER:
FOOTINGS (CONC. DEPOSITED AGAINST EARTH).....3"
CONC. SURFACE (FORMED) EXPOSED TO EARTH OR WEATHER
#6 THROUGH #8 BARS:2"
#5 & SMALLER:1 1/2"
CON. NOT EXPOSED TO EARTH OR WEATHER:
SLABS, WALLS & JOISTS:
#4 & #8 BARS:1 1/2"
#1 BAR & SMALLER: 3/4"
BEAMS, COLUMNS:
PRIMARY REINFORCEMENT TIES STRIPPUS, SPIRALS:1 1/2"

STRUCTURAL WORK NOTES:

- STRUCTURAL WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE AISC SPECIFICATION FOR DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL BUILDINGS, LATEST EDITION.
- ALL STEEL SHAPES, PLATES AND BARS SHALL CONFORM TO ASTM A36 (U.O.N.).
- USE GRADE 50, A572 OR DUEL CERTIFIED FOR WIDE FLANGE BEAMS AND COLUMNS.
- ALL STRUCTURAL TUBING SHALL CONFORM TO ASTM-A500, GRADE "B", $F_y \geq 46 \text{ ksi}$
- ALL STEEL PIPES SHALL CONFORM TO ASTM-A53 GRADE-B.
- MACHINE BOLTS, ANCHOR BOLTS, SHALL CONFORM TO ASTM A-307 (U.O.N.).
- NUTS FOR MACHINE BOLTS SHALL CONFORM TO ASTM A563, HEX GRADE "A".
- HIGH STRENGTH BOLTS SHALL CONFORM TO ASTM A325.
- NUTS FOR HIGH STRENGTH BOLTS SHALL BE HEAVY HEX, GRADE "C" CONFORMING WITH ASTM A563.
- ALL WELDING SHALL CONFORM TO A.I.W.S. STANDARDS AND SHALL BE PERFORMED BY CERTIFIED WELDERS. WELDS IDENTIFIED AS REQUIRING CONTINUOUS SPECIAL INSPECTION NEED NOT HAVE SPECIAL INSPECTION WHEN THE WELDING IS PERFORMED IN AN APPROVED FABRICATOR'S SHOP. APPROVED FABRICATORS MUST BE APPROVED IN ACCORDANCE WITH LOCAL BUILDING OFFICIAL (AS REQUIRED), PRIOR TO START OF WORK. THE SPECIAL INSPECTOR SHALL CHECK WELDER QUALIFICATIONS AND VERIFY THAT WELDING PROCEDURE SPECIFICATIONS HAVE BEEN APPROVED AND ARE APPLICABLE TO THE WORK INTENDED. THE SPECIAL INSPECTOR SHALL CONTINUOUSLY OBSERVE THE WELDING WHILE BEING PERFORMED.
- ALL WELDING SHALL CONFORM TO IBC REQUIREMENTS.
- ALL WELDS TO BE E70XX.
- ALL FIELD WELDS TO BE SPECIAL INSPECTED. U.O.N. ON PLANS.
- ALL FULL PENETRATION GROOVE WELDS FOR DUCTILE MOMENT FRAME MEMBERS SHALL BE ULTRASONICALLY INSPECTED BY AN APPROVED TESTING AGENCY AND SHALL CONFORM TO THE LATEST EDITION OF AWS D1 SECTIONS 5 AND 6.
- WELDED CONNECTION OF RIGID FRAME BEAMS SHALL NOT BE TIGHTENED UNTIL FLANGE WELDS ARE COMPLETE ALONG EACH LINE OF COLUMN AT EACH LINE OF FLOOR.
- HIGH STRENGTH BOLTING OF MAIN MEMBERS:
A. NON-RIGID FRAME CONNECTIONS LISTED BELOW SHALL HAVE ASTM A325N OR ASTM A490N HIGH STRENGTH BOLTS U.O.N.
1. BEAMS AND GIRDERS TO COLUMNS
2. BEAMS TO GIRDERS
3. BEAMS TO BEAMS
B. NON-RIGID FRAME CONNECTIONS (GIRDER TO COLUMNS): ASTM A325NC OR ASTM A490NC HIGH STRENGTH BOLTS U.O.N.
C. OTHER HIGH STRENGTH A325 BOLTS TO BE TESTED OCCASIONALLY AS DIRECTED BY THE STRUCTURAL ENGINEER.
- CONTRACTOR SHALL PROVIDE ADEQUATE WEATHERPROOFING OF ALL STEEL COMPONENTS THAT ARE EXPOSED TO EXTERIOR CONDITIONS AS NEEDED.

PREFABRICATED WOOD TRUSSES AND WOOD JOIST NOTES:

- PREFABRICATED WOOD TRUSSES (BY OTHERS) SHALL BE GANG NAILED TRUSSES.
 - PREFABRICATED WOOD JOISTS SHALL BE "BLI" BY BLUENIX CORPORATION (ESR 1262) EQUAL OR BETTER.
 - ALL TRUSSES/JOISTS SUPPORTING MECHANICAL EQUIPMENT SHALL BE PROPERLY DESIGNED BY TRUSS AND/OR JOIST MANUFACTURER.
 - WOOD TRUSSES AND/OR JOISTS SHALL BE DESIGNED FOR THE FOLLOWING LOADS:
ROOF:
D.L. = 16 PSF
*L.L. = 20 PSF
FLOORS:
D.L. = 15 PSF
*L.L. = 50 PSF OFFICES
L.L. = 80 PSF CORRIDORS
L.L. = 40 PSF ATTIC
- *LIVE LOADS REDUCIBLE PER CODE (U.O.N.). CONSIDERATION SHOULD BE TAKEN FOR ADDITIONAL LOADS DUE TO MECHANICAL UNITS, PARTITIONS, TILE/STONE FINISHES, ETC.
- TIE ROOF TRUSSES TO EXTERIOR TOP PLATES, PERIMETER BEAMS, AND ALL INTERIOR BEARING POINTS WHERE UPLIFT IS INDICATED ON TRUSS PROFILE DRAWINGS (BY OTHERS) USING PROPER SIMPSON TIE-DOWN (U.O.N.).

WOOD NOTES:

- ALL WOOD FRAMING SHALL BE AS FOLLOWS (U.O.N.):
A. ROOF RAFTERS & CEILING JOISTS:
NO.1/NO.2 SPRUCE PINE FIR (SPF) WITH THE FOLLOWING DESIGN PROPERTIES:
Fb = 175 PSI Fv = 135 PSI E = 1.4x10⁶ PSI
B. FLOOR JOISTS:
NO.2 SOUTHERN YELLOW PINE (SYP) WITH THE FOLLOWING DESIGN PROPERTIES:
Fb = 875 PSI Fv = 175 PSI E = 1.4x10⁶ PSI
- WOOD GRADES (U.O.N.):
A. FOR HORIZONTAL MEMBERS:
JOISTS & RAFTERS GRADE: NO. 2
BEAMS & STRINGERS GRADE: NO. 2 (U.O.N.)
FURLINS GRADE: NO. 1
SUB-FURLING:
2x4 GRADE: NO. 1
2x6 GRADE: NO. 2
LEDGERS & NAILERS GRADE: NO. 2
HEADERS GRADE: NO. 2 (U.O.N.)
B. FOR VERTICAL MEMBERS TOP & BOTTOM PLATES: MATCH VERTICAL MEMBERS
GRADE: NO. 2 MIN (U.O.N.)
4X POST GRADE: NO. 2
6X POST GRADE: NO. 2
STUDS: STUD OR BETTER, 9'-0" MAX (U.O.N.)
- FRAMING IN CONTACT WITH CONCRETE OR MASONRY, OR MEMBERS EXPOSED TO WEATHER SHALL BE NO. 2 SOUTHERN YELLOW PINE (SYP), TREATED IN ACCORDANCE WITH AWPA C22 WITH THE FOLLOWING DESIGN PROPERTIES:
Fb = 1050 PSI Fv = 55 PSI E = 1.6x10⁶ PSI
9/11 AND LEDGER BOLTS SHALL BE PLACED 12" MAX FROM END ENDS AND NOTCHES AND SPACED AT 6" O.C. MAX, U.O.N. (2 BOLTS MIN/PIECE OF STUD)
- ALL PLYWOOD AND OSB SHALL BE CERTIFIED AS CONFORMING TO U.S. PRODUCTS STANDARD PS-2-92 BY A CERTIFICATION AGENCY APPROVED BY THE NATIONAL EVALUATION SERVICES INC. OR I.C.C.
- ALL BOLT HEADS AND NUTS BEARING ON WOOD SHALL HAVE WASHERS. ALL BOLT HOLES IN WOOD SHALL BE DRILLED 1/16" MAXIMUM DIAMETER LARGER THAN THE NOMINAL BOLT DIAMETER.
- PROVIDE JOIST(S) UNDER ALL PARALLEL NON-BEARING PARTITIONS AND SOLID BLOCKING UNDER ALL PERPENDICULAR NON-BEARING PARTITIONS.
- ALL FRAMING ANCHORS, POST CAPS, COL. BASES, ETC. NOTED ARE MANUFACTURED BY SIMPSON OR APPROVED EQUAL. OTHER HARDWARE COMPANIES (E.I. ACS, USF) MAY BE SUBSTITUTED PROVIDED ALL PRODUCTS HAVE A CURRENT ICC-ES REPORT AND EQUIVALENT LOAD CAPACITIES. USE COMMON NAILS AS SPECIFIED BY MANUFACTURER.
- PLYWOOD FLOOR SHEATHING SHALL BE GLUED TO FLOOR JOISTS WITH ONE CONTINUOUS BEAD OF AN ADHESIVE COMPOUND CONFORMING TO ASTM D 3024 AND IN ACCORDANCE WITH THE MANUFACTURER'S DIRECTIONS.
- CUTTING, NOTCHING OR DRILLING OF BEAMS OR JOISTS SHALL BE PERMITTED ONLY AS DETAILED OR APPROVED BY THE ENGINEER.
- BOLTS IN WOOD SHALL NOT BE LESS THAN 1 DIAMETERS FROM THE END AND 4 DIAMETERS FROM THE EDGE OF THE MEMBER (U.O.N.).
- MOISTURE CONTENT OF WOOD AT TIME OF PLACING SHALL NOT EXCEED 19%.
- ALL NAILS SHALL BE COMMON NAILS (U.O.N.).
- PROVIDE SOLID BLOCKING TO GIRDERS AND/OR FOUNDATION BENEATH POINT LOADS AS DENOTED BY:
- SECURE LOAD BEARING HEADERS TO EACH JACK STUD W/ (4) 8d NAILS. BEAM/HEADER SUPPORTS REQUIRING MORE THAN (1) JACK ARE DENOTED BY: 2 (WHERE 2 JACKS ARE REQ'D FOR EXAMPLE). KING STUDS AT EACH END OF THE OPENING SHALL BE 1/2 THE NUMBER OF STUDS INTERRUPTED BY THE OPENING, TYP (U.O.N.).
- OVERFRAME ROOF W/ FLAT 2x10 PLATES W/ (2) 16d COMMON TO RAFTERS/TRUSSES AT FALSE VALLEYS.

PREFABRICATED WOOD BEAM NOTES:

- PREFABRICATED WOOD BEAMS SHALL BE 'ON-CENTER' LVL & AS INDICATED ON PLANS, MANUFACTURED BY BLUENIX, U.O.N (EQUIVALENT OR BETTER SUBSTITUTE IS ALLOWED) W/ MULTI-PLY MEMBERS BUILT-UP PER MFR REQUIREMENTS.
ALLOWABLE DESIGN STRESSES:
(LVL) Fb = 3,100 PSI
E = 2.1 x 10⁶ PSI
Fv = 285 PSI


**2018 APPENDIX B
BUILDING CODE SUMMARY FOR ALL COMMERCIAL PROJECTS
STRUCTURAL DESIGN**

DESIGN LOADS: IMPORTANCE FACTORS:	SNOW (S): 1.0 SEISMIC (IE): 1.0
LIVE LOADS:	FLOOR: 50PSF OFFICE 80PSF CORRIDOR ATTIC: 40PSF ROOF: 20PSF REDUCIBLE
GROUND SNOW LOAD:	15 PSF
WIND LOADS:	ULTIMATE WIND SPEED: 115 MPH (ASCE-7) EXPOSURE CATEGORY B
SEISMIC DESIGN CATEGORY: <input type="checkbox"/> A <input checked="" type="checkbox"/> B <input type="checkbox"/> C <input type="checkbox"/> D PROVIDE THE FOLLOWING SEISMIC DESIGN PARAMETERS: RISK CATEGORY (TABLE 1604.5) <input type="checkbox"/> I <input checked="" type="checkbox"/> II <input type="checkbox"/> III <input type="checkbox"/> IV SPECTRAL RESPONSE ACCELERATION Ss: 0.11g S1: 0.083g SITE CLASSIFICATION (ASCE 7) <input type="checkbox"/> A <input type="checkbox"/> B <input type="checkbox"/> C <input checked="" type="checkbox"/> D <input type="checkbox"/> E <input type="checkbox"/> F BASIC STRUCTURAL SYSTEM: <input checked="" type="checkbox"/> BEARING WALL <input type="checkbox"/> DUAL W/SPECIAL MOMENT FRAME <input type="checkbox"/> BUILDING FRAME <input type="checkbox"/> DUAL W/ INTERMEDIATE R/C OR SPECIAL STEEL <input type="checkbox"/> MOMENT FRAME <input type="checkbox"/> INVERTED PENDULUM ANALYSIS PROCEDURE <input type="checkbox"/> SIMPLIFIED <input checked="" type="checkbox"/> EQUIVALENT LATERAL FORCE <input type="checkbox"/> DYNAMIC ARCHITECTURAL, MECHANICAL, COMPONENTS ANCHORED <input checked="" type="checkbox"/> YES <input type="checkbox"/> NO LATERAL DESIGN CONTROL: <input type="checkbox"/> EARTHQUAKE <input checked="" type="checkbox"/> WIND	
SOIL BEARING CAPACITIES: FIELD TEST (PROVIDE COPY OF TEST REPORT) <u>N/A</u> PSF PRESUMED BEARING CAPACITY <u>2,000</u> PSF FILE SIZE, TYPE, AND CAPACITY <u>N/A</u>	

STRUCTURAL ABBREVIATIONS


A.B.	ANCHOR BOLT	JST.	JOIST
ABV.	ABOVE	JT.	JOINT
ADD'L	ADDITIONAL	K	KIPS(1000)
ADJ.	ADJACENT	LAT.	LATERAL
ALUM	ALUMINUM	LB.(#)	FOUND(S)
APA.	AMERICAN PLYWOOD ASSOCIATION	L.B.	LAG BOLTS
ALT.	ALTERNATE	L.F.	LINEAR FEET(FOOT)
APPRX.	APPROXIMATELY	LGTH.	LENGTH
ARCH.	ARCHITECTURAL	LLH	LONG LEG HORIZ.
#	AND	L.G.	LONG(TUDINAL)
	AT	LLV	LONG LEG VERT.
BLK	BELOW	LT.WT.	LIGHT WEIGHT
B.F.	BRACED FRAME	MAS.	MASONRY
BLDG.	BUILDING	MAT'L	MATERIAL
BLK	BLOCK	MAX.	MAXIMUM
BLKG.	BLOCKING	M.B.	MACHINE BOLT
BM	BEAM	MEZZ.	MEZZANINE
B.N.	BOUNDARY NAILING	MFR.	MANUFACTURER
BNDRY.	BOUNDARY	MISC.	MISCELLANEOUS
B.O.F.	BOTTOM OF FOOTING	MIN.	MINIMUM
BRDG.	BRIDGE(ING)	MTL.	METAL
BRG.	BEARING	NO.(#)	NUMBER
BTTM.	BOTTOM	N.S.	NEAR SIDE
BTWN.	BETWEEN	N.T.S.	NOT TO SCALE
BS.	BOTH SIDES	O.C.	ON CENTER
CAMB.(C)	CAMBER(ED)	O.D.	OUTSIDE DIAMETER
CE.	CARBON EQUIVALENT	PAR.	PARALLEL
CANT.	CANTILEVER(ED)	P/C	PREGAST
C.F.	CUBIC FEET(FOOT)	PERP.	PERPENDICULAR
C.I.P.	CAST IN PLACE	PL	PLATE
@	CENTER LINE	PLY.	PLYWOOD
CLG.	CEILING	P.S.F.	POUNDS PER SQUARE FOOT
CLR.	CLEAR	P.S.I.	POUNDS PER SQUARE INCH
COL.	COLUMN	P-T.	PRESSURE TREATED
CONG.	CONCRETE	QTY.	QUANTITY
CONN.	CONNECTION	RAD(R)	RADIUS
CONST.	CONSTRUCTION	R.C.P.	REINFORCED CONCRETE PIPE
CONT.	CONTINUOUS	REF.	REFERENCE
CTS.K.	COUNTERSINK	R.F.	RIGID FRAME
CTR.	CENTER(ED)	REINF.	REINFORCEMENT(ING)
C.Y.	CUBIC YARD	REQ'D	REQUIRED
d	PENNY(NAILS)	R.O.	ROUGH OPENING
DBL.	DOUBLE	SCH.	SCHEDULE
DEPT.	DEPARTMENT	SHT.	SHEET
DIA.	DIAMETER	SIM.	SIMILAR
DIAPH.	DIAPHRAGM	SKW.	SKEW(ED)
DN.	DOWN	SP.	SOUTHERN PINE
DP.	DEEP	SFC.	SPACE(ING)
DWG.	DRAWING(S)	SFF.	SPRUCE PINE FIR
DWL.	DOWEL(S)	SPEC.	SPECIFICATION(S)
EA.	EACH	SQ.	SQUARE
E.F.	EACH FACE	STD.	STANDARD
E.J.	EXPANSION JOINT	STGR.	STAGGERED
EL.	ELEVATION	STIF.	STIFFENER(S)
ELEV.	ELEVATION	STIR.	STIRRUP(S)
EMBD.	EMBED(MENT)	STL	STEEL
E.N.	EDGE NAIL	STRUC.	STRUCTURAL
ENG.	ENGINEER	SUSP.	SUSPENSION
EQ.	EQUAL	SYMM.	SYMMETRICAL
EXP.	EXPANSION	T&B	TOP AND BOTTOM
FAB.	FABRICATION	T&G	TONGUE AND GROOVE
FDN.	FOUNDATION	TEMP.	TEMPERATURE
FIN.	FINISHED	THK.	THICKNESS
FLG.	FLANGE	THRD.	THREADED
FLR.	FLOOR	TMPRY.	TEMPORARY
F.N.	FIELD (FACE NAIL)	T.N.	TOE NAIL
F.O.C	FACE OF CONCRETE	T.O.S.	TOP OF SHEATHING
F.O.M.	FACE OF MASONRY	T.O.W.	TOP OF WALL
F.O.S.	FACE OF STUD	T.S.	TOP OF STEEL
F.O.W.	FACE OF WALL	TRANSV.	TRANSVERSE
FRM.	FRAME(ING)	TYP.	TYPICAL
F.S.	FAR SIDE	U.O.N.	UNLESS OTHERWISE NOTED
FT.(')	FOOT(FEET)	VERT.(V)	VERTICAL
FTG.	FOOTING	(W)	WIDE(WIDTH)
GA.	GAUGE	W/	WITH
GALV.	GALVANIZE(D)	WD.	WOOD
GB.	GRADE BEAM	W.P.	WORK POINT
GLB.	GLUED LAMINATED BEAM	W.P.J.	WEAKENED PLANE JOINT
GRD.	GRADE	W.S.	WELDED STUD(S)
GYPBD	GYPSUM WALLBOARD	WT.	WEIGHT
HD	HOLD DOWN	W/F	WELDED WIRE FABRIC
HDR.	HEADER	X-STG	EXTRA STRONG
HGR.	HANGER	XX-STG	DOUBLE EXTRA STRONG
HORIZ.(H)	HORIZONTAL	YD.	YARD
H8B	HIGH STRENGTH BOLTS		
HT.	HEIGHT		
I.D.	INSIDE DIAMETER		
I.E.	INVERT ELEVATION		
IN.(")	INCHES		

REVISIONS	BY



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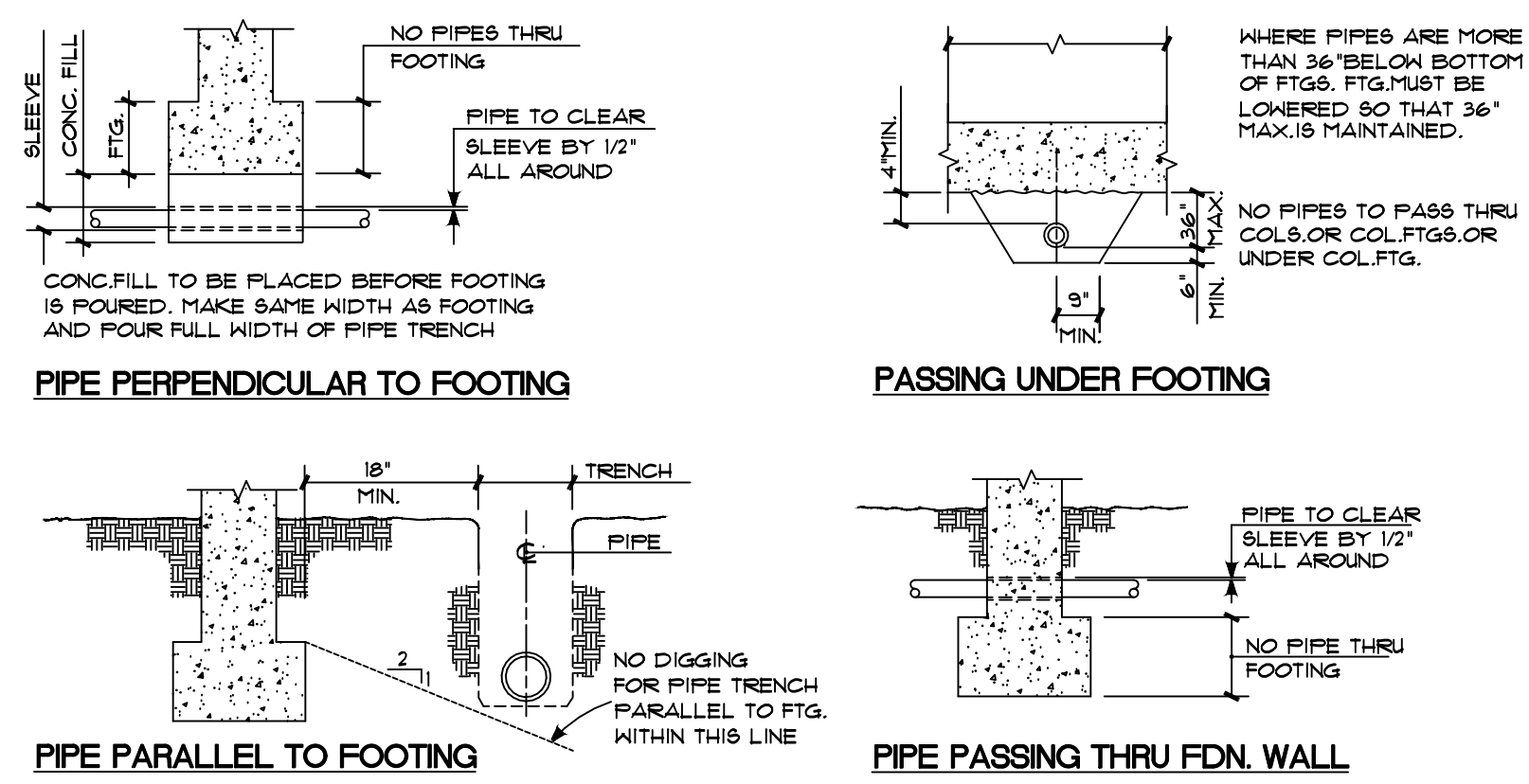


01-12-2022

Paul Barbour and Son, Inc.

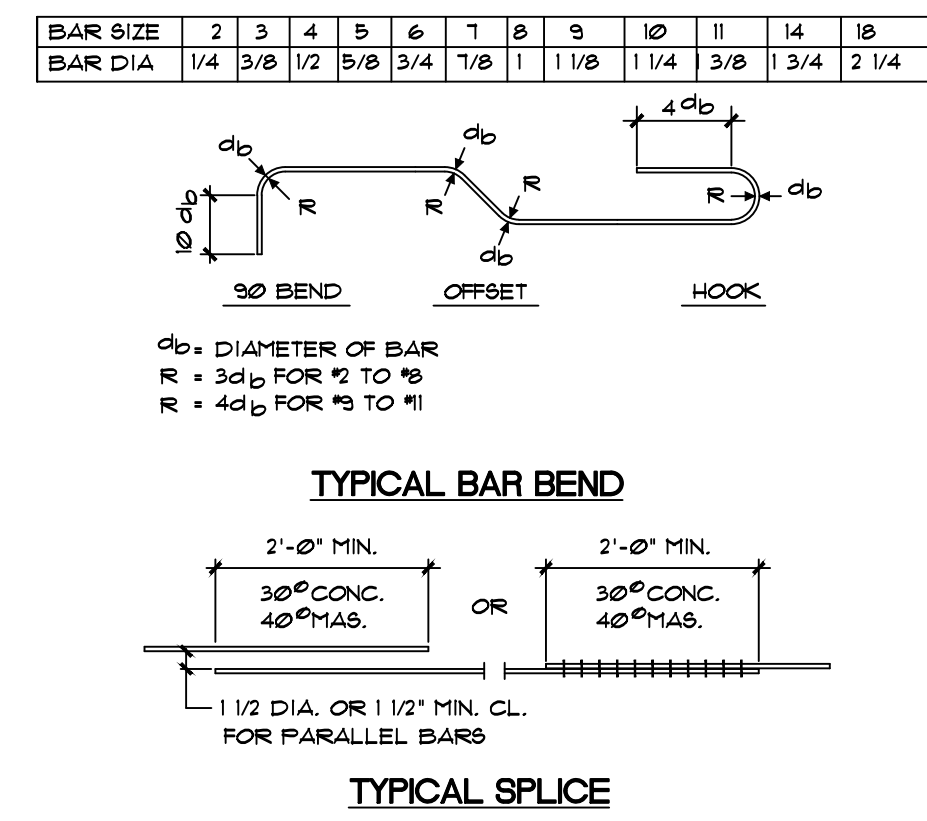
New Office Building
11496 Hwy 410 N.
Fuquay Varina, NC 27526

DATE	1-11-22
SCALE	AS SHOWN
DRAWN	J.H.
JOB	21-1174
SHEET	SP1



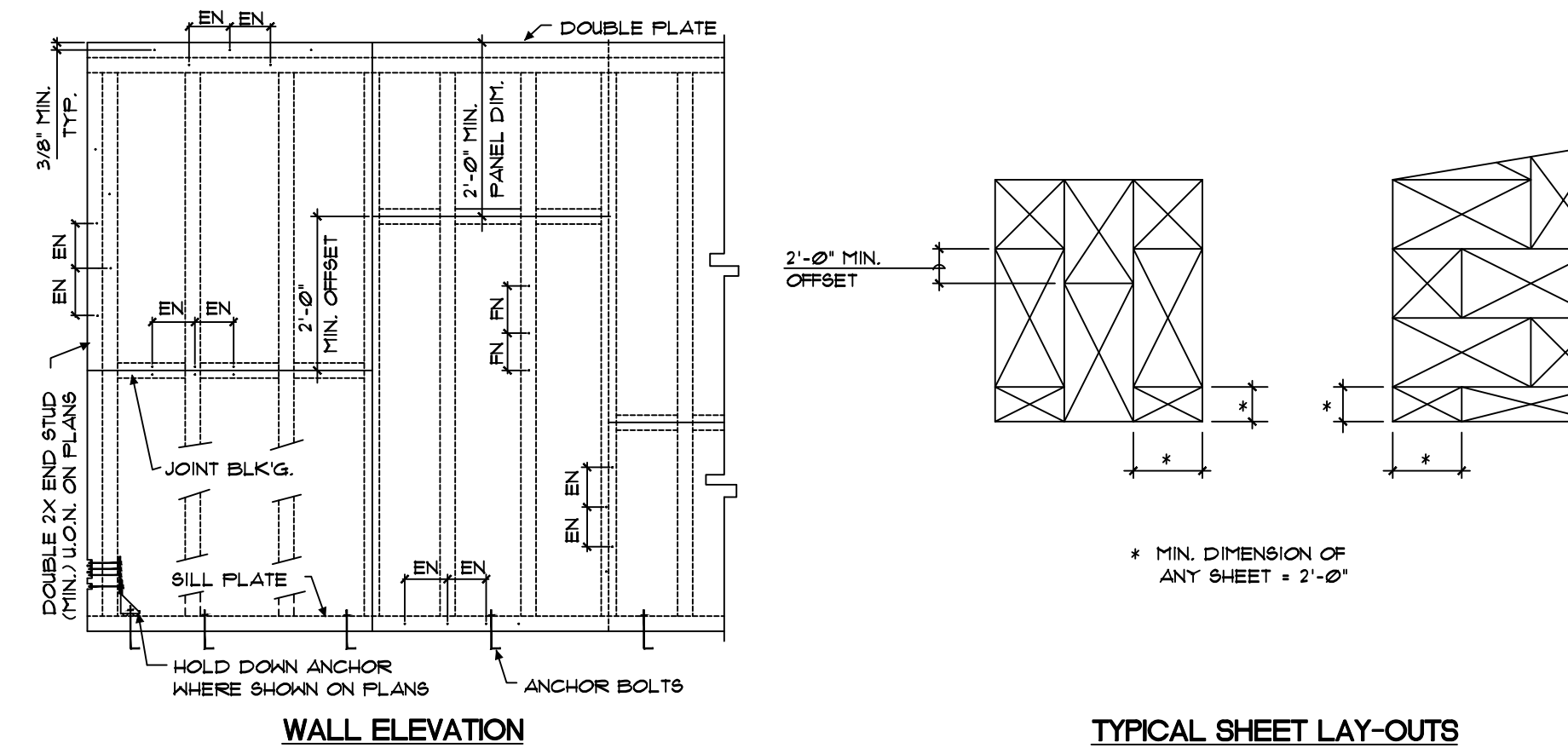
TYPICAL DETAIL OF PIPING AT FOOTINGS AND FOUNDATION WALLS

1



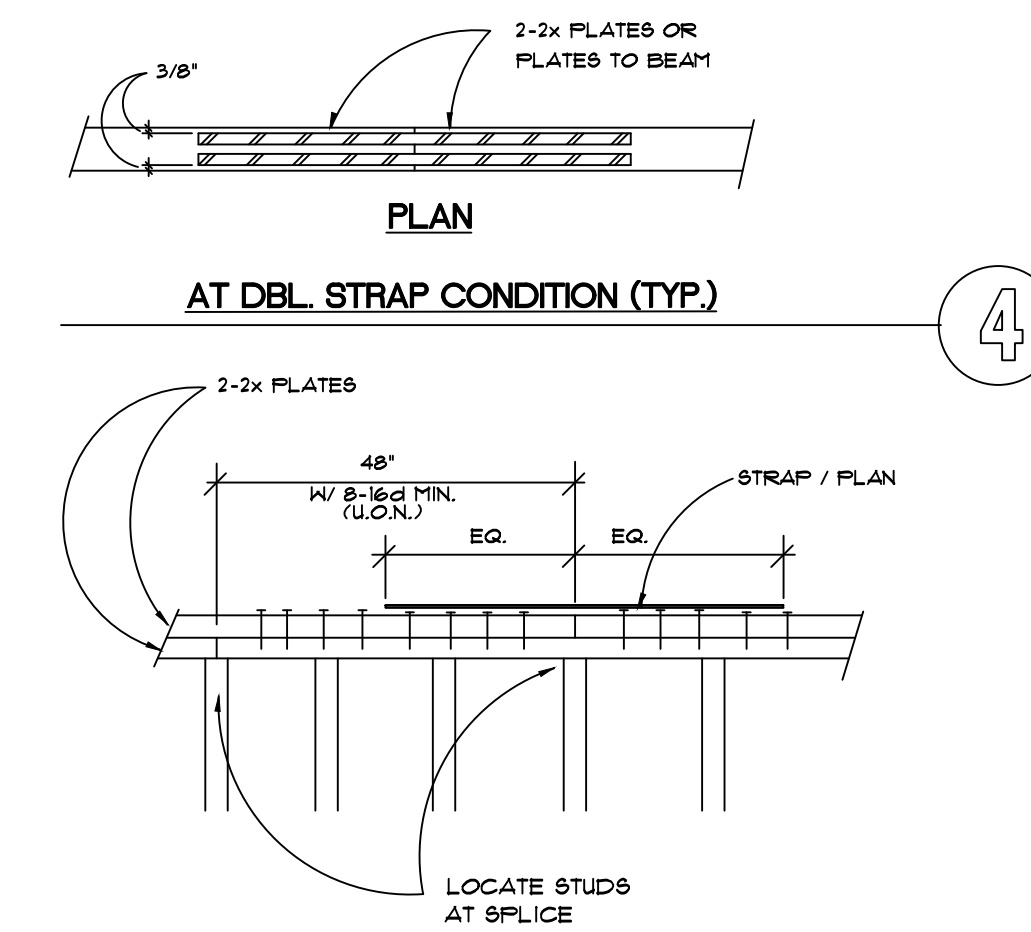
REINF. BAR BEND AND SPLICE DETAIL

2



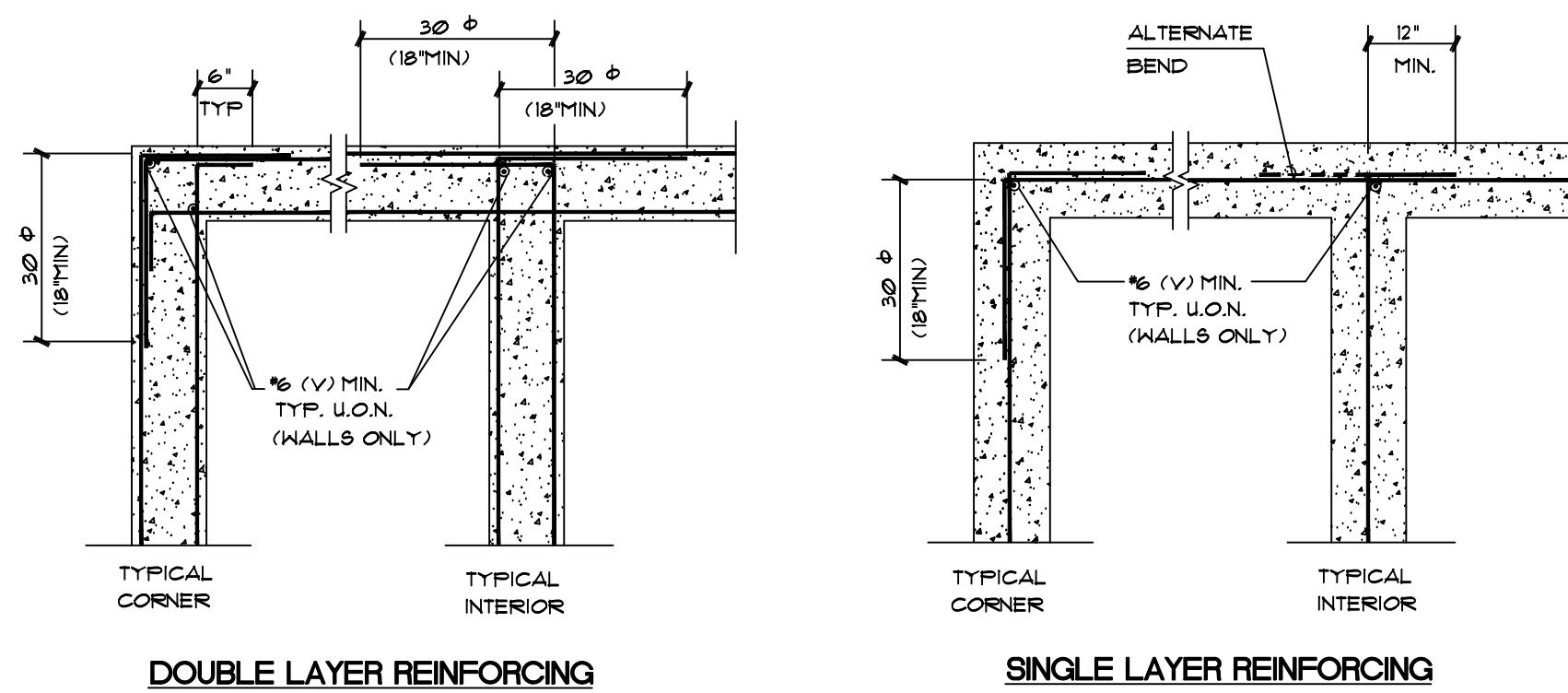
TYPICAL PLYWOOD WALL SHEATHING LAYOUT (VERTICAL)

3



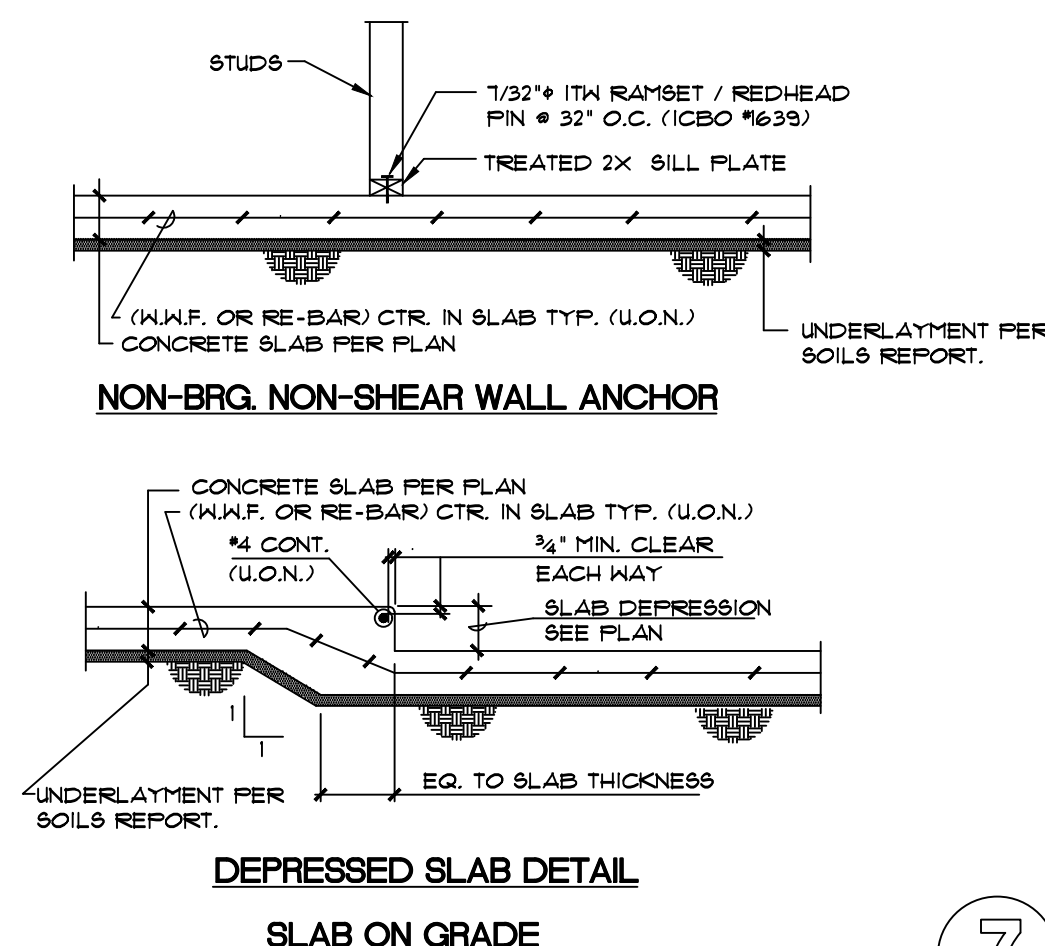
ELEVATION OF PLATE SPLICE (TYP.)

4



TYP. REINF. AT INTERSECTION OF CONCRETE BEAMS, WALLS AND FOOTINGS

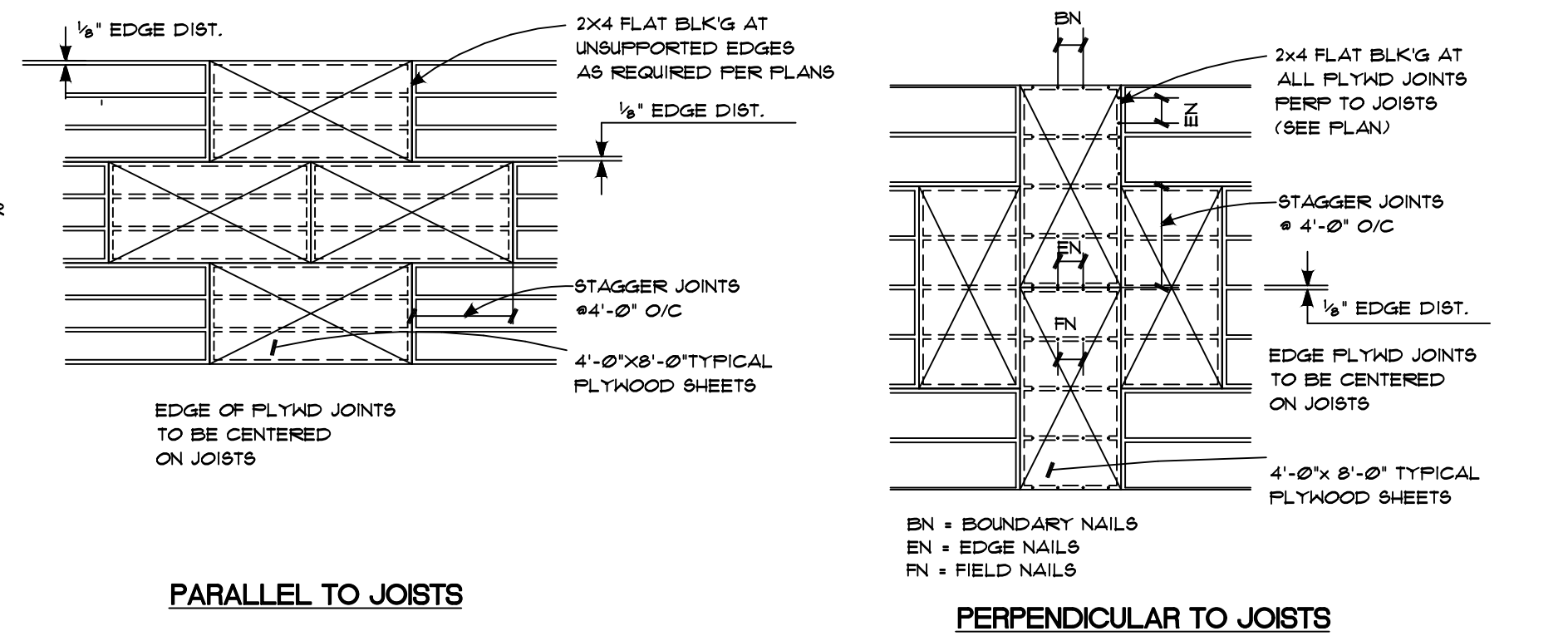
6



DEPRESSED SLAB DETAIL

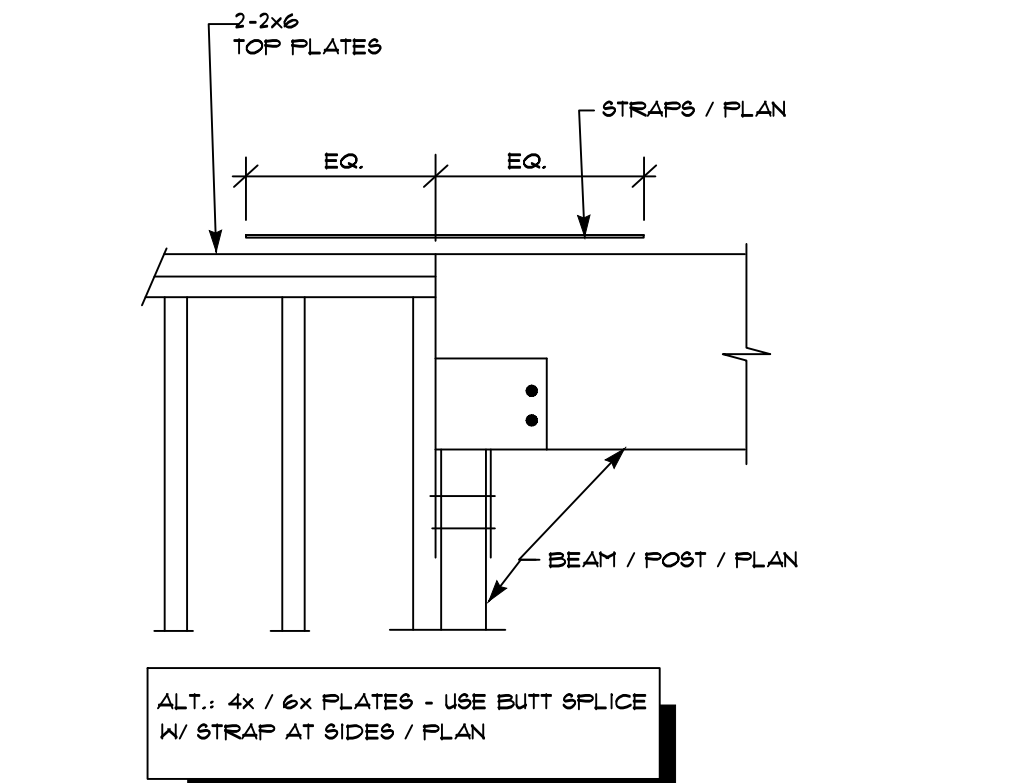
SLAB ON GRADE

7



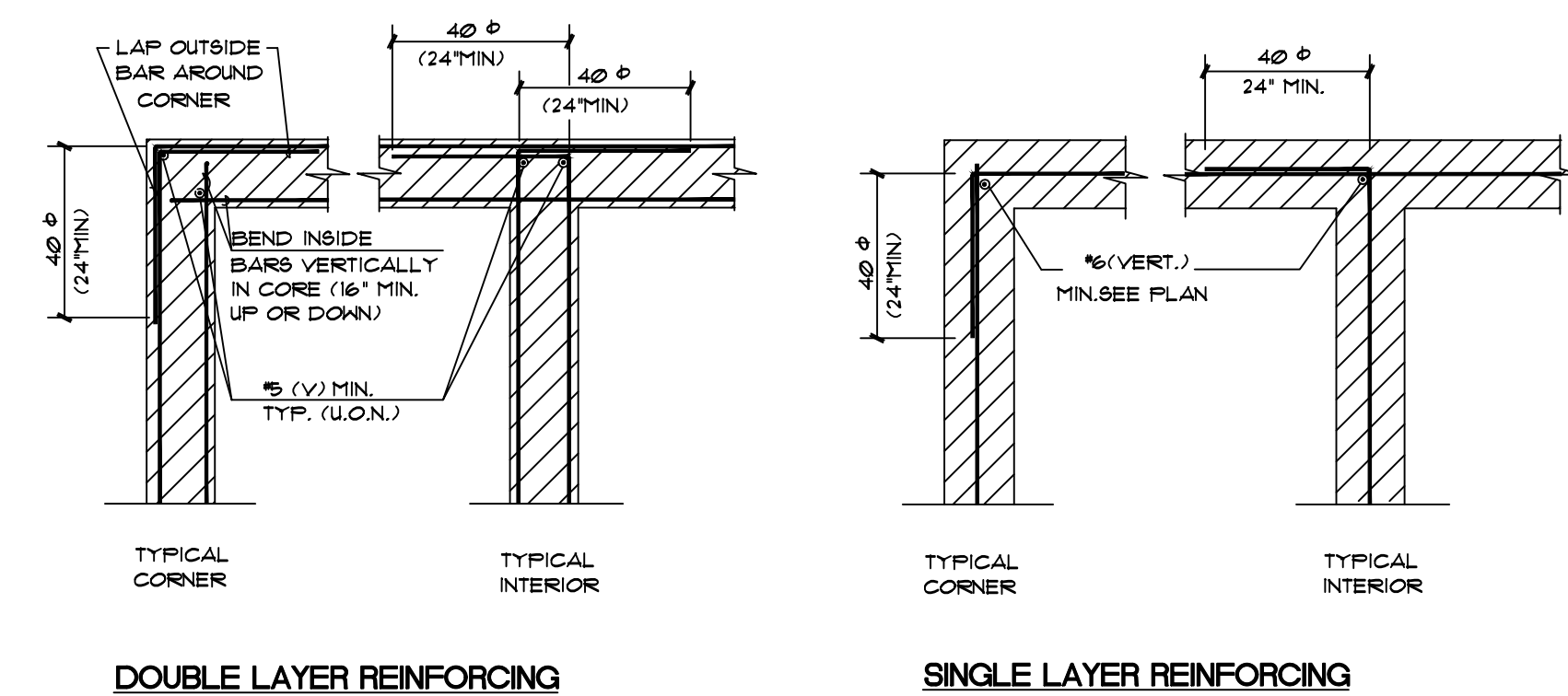
TYPICAL PLYWOOD SHEATHING LAYOUT (HORIZONTAL)

8



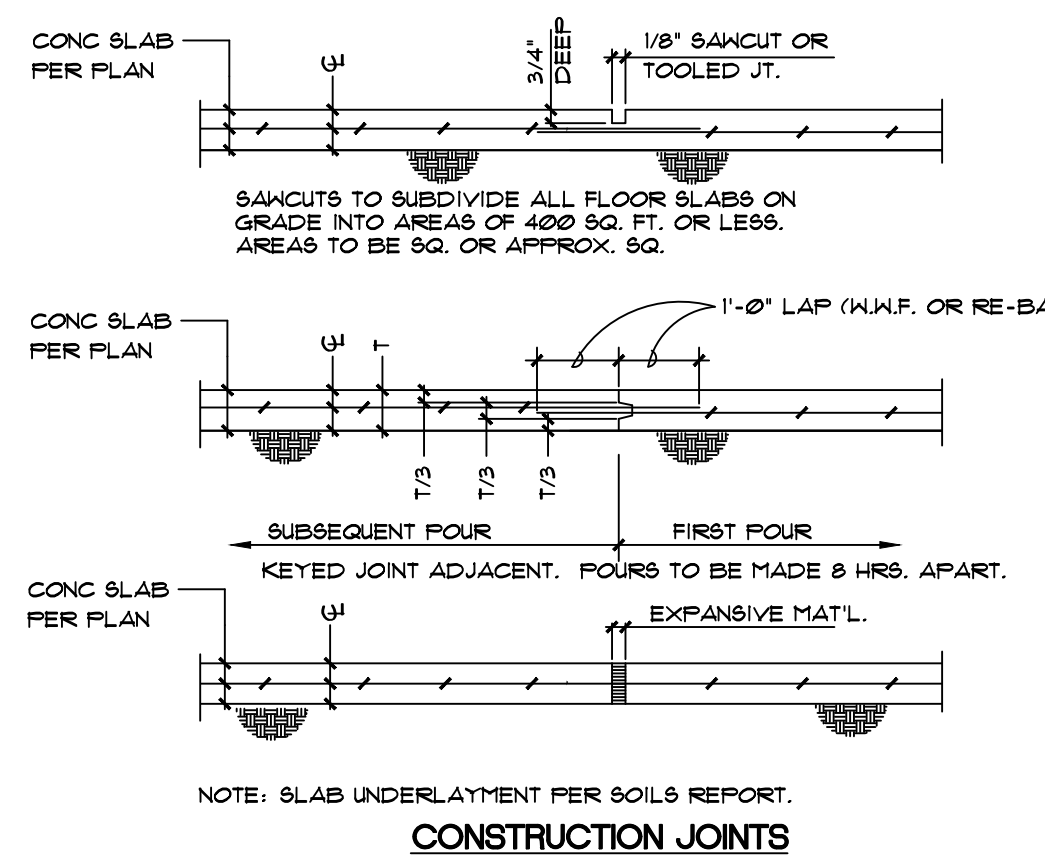
BEAM TO TOP PLATE CONN. (TYP.)

9



TYPICAL REINFORCING AT INTERSECTION OF MASONRY WALLS

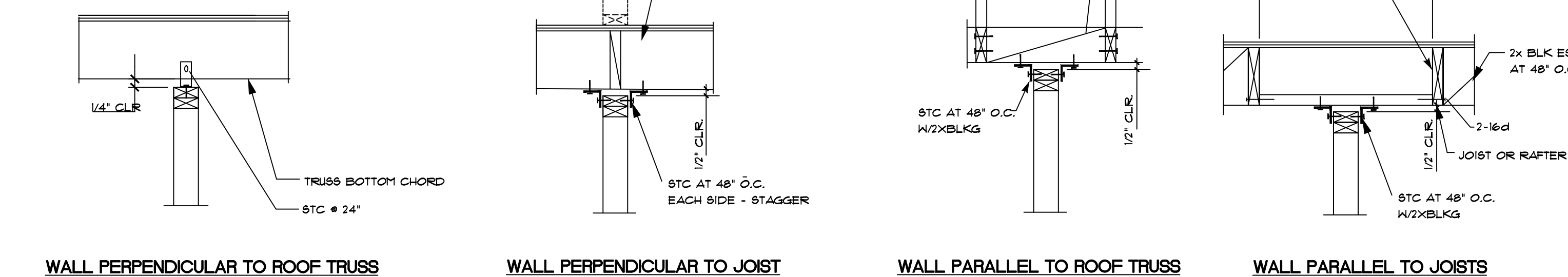
10



CONSTRUCTION JOINTS

SLAB ON GRADE

11



WALL PERPENDICULAR TO ROOF TRUSS

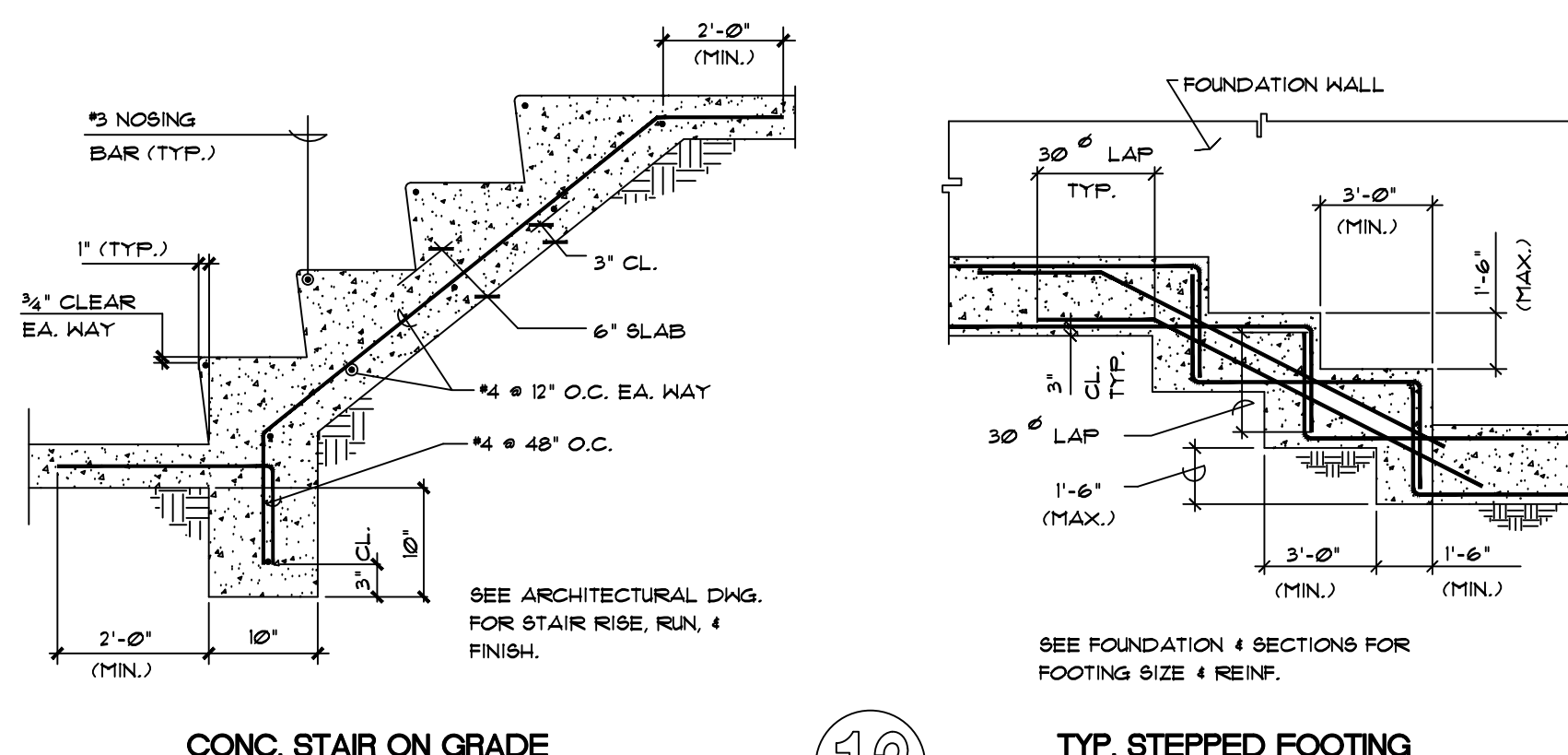
WALL PERPENDICULAR TO JOIST

WALL PARALLEL TO ROOF TRUSS

WALL PARALLEL TO JOISTS

TYPICAL NON-BEARING WALL DETAIL

12

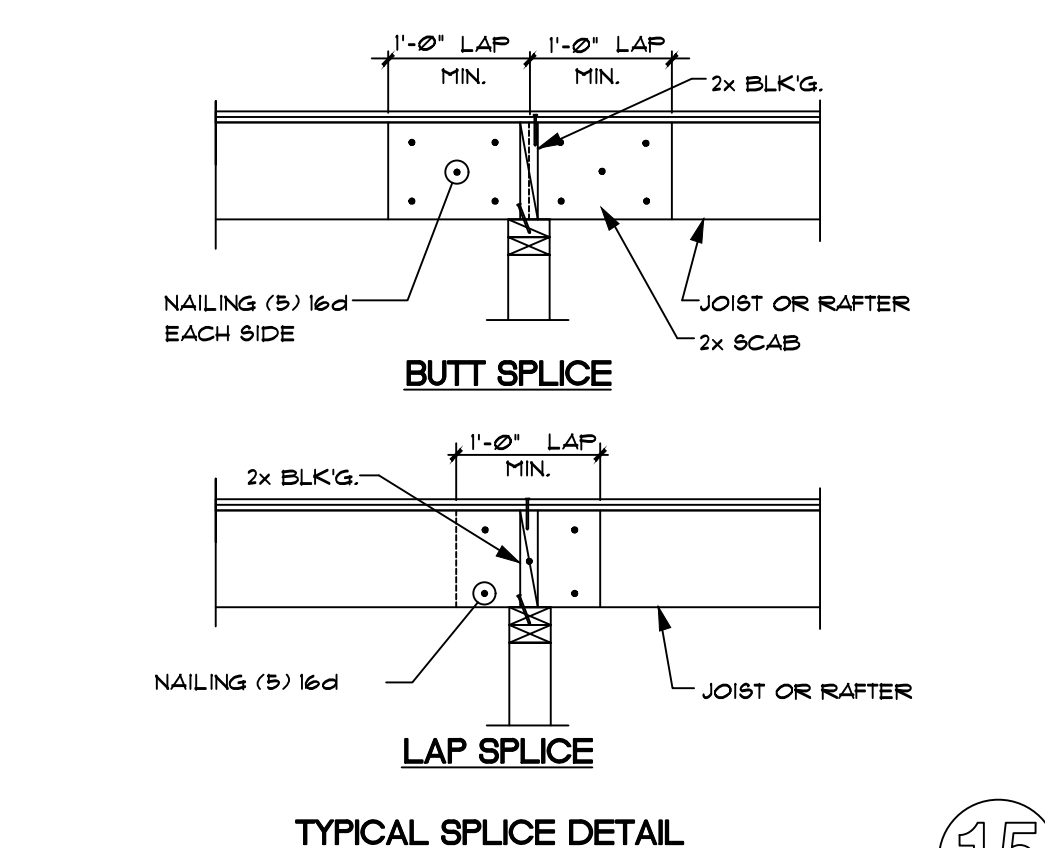


CONC. STAIR ON GRADE

TYP. STEPPED FOOTING

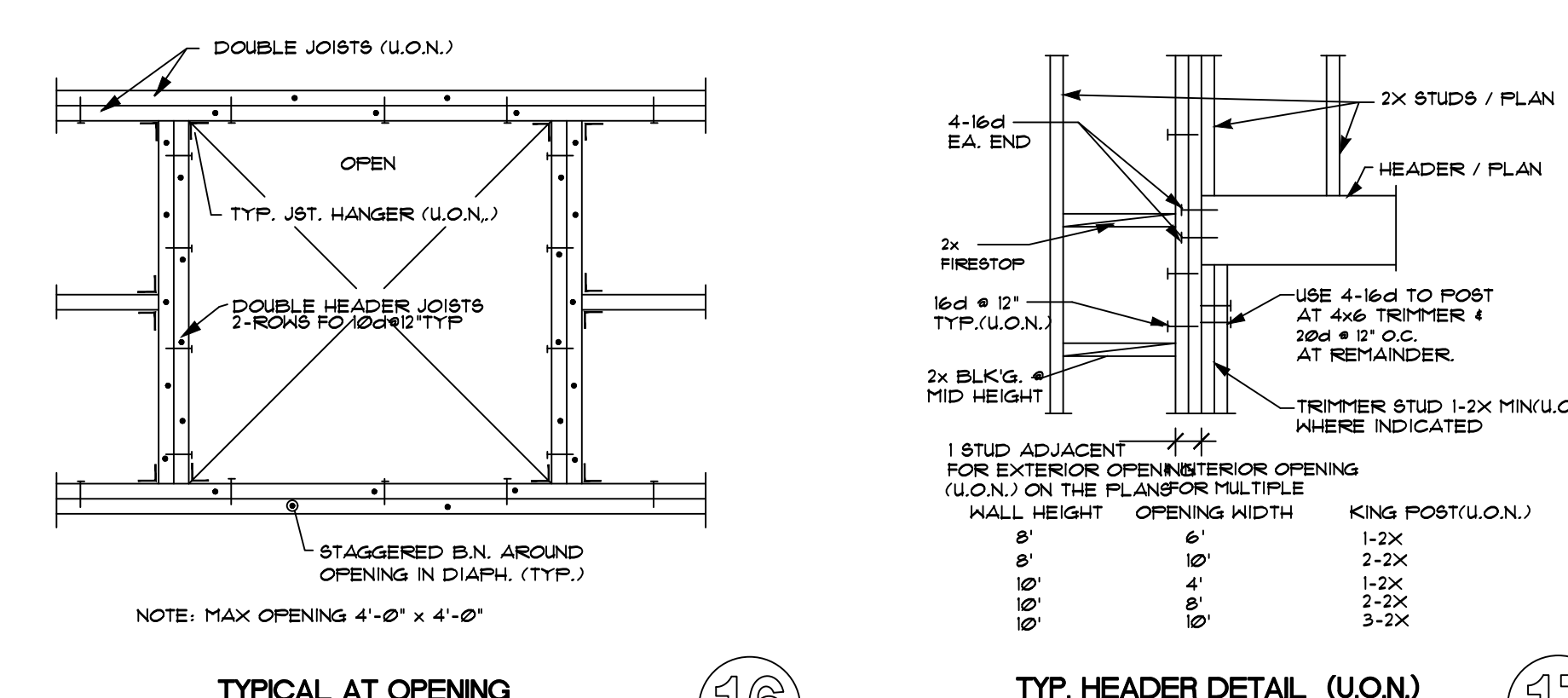
13

14



TYPICAL SPLICE DETAIL

15

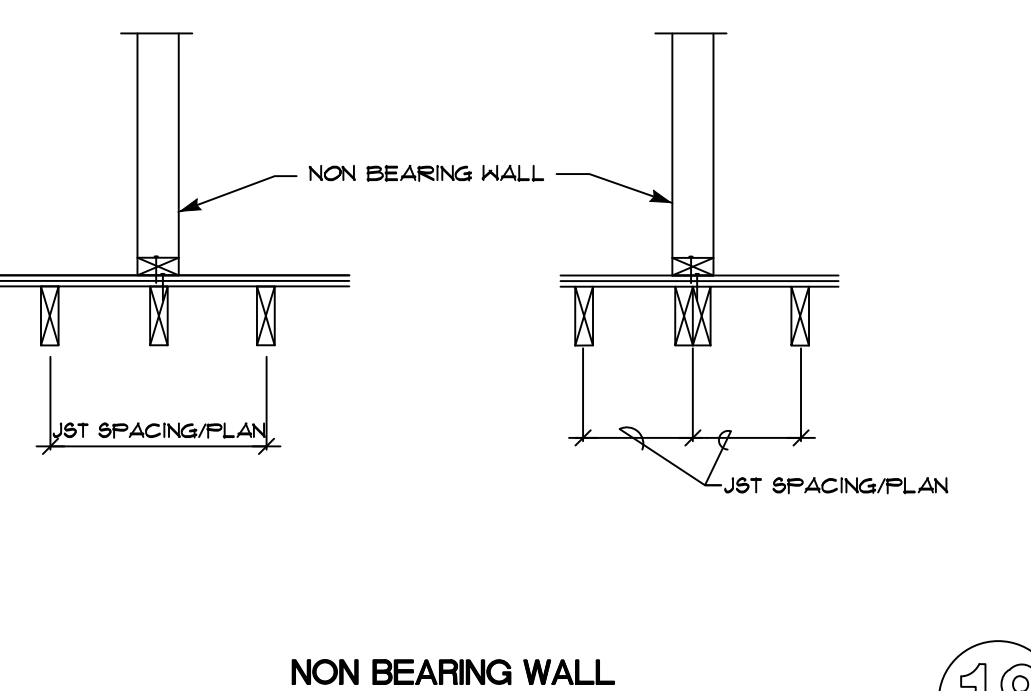


TYPICAL AT OPENING

TYP. HEADER DETAIL (U.O.N.)

16

17



NON BEARING WALL

18

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New Office Building
11496 Hwy 410 N.
Fuquay Varina, NC 27526

DATE 1-11-22
SCALE AS SHOWN
DRAWN J.H.
JOB 21-1174
SHEET

SP2

FASTENER SCHEDULE FOR STRUCTURAL MEMBERS

DESCRIPTION OF BUILDING ELEMENTS	NUMBER & TYPE OF FASTENER ¹	SPACING OF FASTENERS
JOIST TO SILL OR GIRDER, TOE NAIL	(3) 8d (2 1/2"x0.131")	-
1"x 6" SUBFLOOR OR LESS TO EACH JOIST, FACE NAIL	(2) 8d (2 1/2"x0.131") (2) STAPLES - 1/4"	-
2" SUBFLOOR TO JOIST OR GIRDER, BLIND AND FACE NAIL	(2) 16d (3 1/2"x0.135")	-
SOLE PLATE TO JOIST OR BLOCKING, FACE NAIL	16d (3 1/2"x0.135")	16" o.c.
TOP OR SOLE PLATE TO STUD, END NAIL	(2) 16d (3 1/2"x0.135")	-
STUD TO SOLE PLATE, TOE NAIL	(3) 8d (2 1/2"x0.131") OR (2) 8d (3 1/2"x0.135")	-
DOUBLE STUDS, FACE NAIL	16d (3 1/2"x0.135")	24" o.c.
DOUBLE TOP PLATES, FACE NAIL	16d (3 1/2"x0.135")	24" o.c.
SOLE PLATE TO JOIST OR BLOCKING AT BRACED WALL PANELS	(3) 16d (3 1/2"x0.135")	16" o.c.
DOUBLE TOP PLATES, MINIMUM 24-INCH OFFSET OF END JOINTS, FACE NAIL IN LAPPED AREA	(8) 16d (3 1/2"x0.135")	-
BLOCKING BETWEEN JOIST OR RAFTERS TO TOP PLATE, TOE NAIL	(3) 8d (2 1/2"x0.131")	-
RIM JOIST TO TOP PLATE, TOE NAIL	8d (2 1/2"x0.131")	6" o.c.
TOP PLATES, LAPS AT CORNERS AND INTERSECTIONS, FACE NAIL	(2) 16d (3 1/2"x0.135")	-
BUILT-UP HEADER, TWO PIECES WITH 1/2" SPACER	16d (3 1/2"x0.135")	16" o.c. ALONG EACH EDGE
CONTINUED HEADER, TWO PIECES	16d (3 1/2"x0.135")	16" o.c. ALONG EACH EDGE
CEILING JOISTS TO PLATE, TOE NAIL	(3) 8d (2 1/2"x0.131")	-
CONTINUOUS HEADER TO STUD, TOE NAIL	(4) 8d (2 1/2"x0.131")	-
CEILING JOIST, LAPS OVER PARTITIONS, FACE NAIL	(3) 16d (3 1/2"x0.135")	-
CEILING JOIST TO PARALLEL RAFTERS, FACE NAIL	(3) 16d (3 1/2"x0.135")	-
RAFTER TO PLATE, TOE NAIL	(2) 8d (2 1/2"x0.131")	-
1" BRACE TO EACH STUD AND PLATE, FACE NAIL	(2) 8d (2 1/2"x0.131") (2) STAPLES - 1/4"	-
1"x6" SHEATHING TO EACH BEARING, FACE NAIL	(2) 8d (2 1/2"x0.131")	-
1"x8" SHEATHING TO EACH BEARING, FACE NAIL	(3) 8d (2 1/2"x0.131") (2) STAPLES - 1/4"	-
WIDER THAN 1"x8" SHEATHING TO EACH BEARING, FACE NAIL	(3) 8d (2 1/2"x0.131") (4) STAPLES - 1/4"	-
BUILT-UP CORNER STUDS	16d (3 1/2"x0.135")	24" o.c.
BUILT-UP GIRDERS AND BEAMS, 2-INCH LUMBER LAYERS	16d (3 1/2"x0.135")	NAIL EACH LAYER AS FOLLOWS: 32" O.C. AT TOP & BOTTOM & STAGG'D TWO NAILS @ ENDS @ EA SPLICE
2" PLANKS	(2) 16d (3 1/2"x0.135")	AT EACH BEARING
ROOF RAFTERS TO RIDGE, VALLEY OR HIP RAFTERS, FACE NAIL	(4) 16d (3 1/2"x0.135")	-
RAFTER TIES TO RAFTERS, FACE NAIL	(3) 8d (2 1/2"x0.131")	-
COLLAR TIE TO RAFTER, FACE NAIL, OR 1/4"x0.06 GA RIDGE STRAP	(3) 16d (3 1/2"x0.135")	-
LEDGER STRIP	(3) 16d COMMON, (4) 3"x4"x0.131" NAIL, (4) 3"x4" GAGE STAPLE	FACE NAIL AT 4" ON CENTER UNDER EACH JOIST

DESCRIPTION OF BUILDING MATERIALS	DESCRIPTION OF FASTENER ¹	SPACING OF FASTENERS EDGES (INCHES) INTERMEDIATE SUPPORTS (4" INCHES)
WOOD STRUCT. PANELS, SUBFLOOR, ROOF & WALL SHEATHING TO FRAMING, & PARTICLEBOARD WALL SHEATHING TO FRAMING	5/16"-1 1/4" 8d COMMON (2 1/2"x0.131") NAIL (SUBFLR. WALL) 8d COMMON (2 1/2"x0.131") NAIL (ROOF)	6 12
19/32"-1"	8d COMMON (2 1/2"x0.131") NAIL	6
1/8"-1/4"	16d COMMON (3 1/2"x0.135") NAIL OR 8d (2 1/2"x0.131") DEFORMED NAIL	6 12
OTHER WALL SHEATHING		
1/2" STRUCTURAL CELLULOSE FIBERBOARD SHEATHING	1/2" GALVANIZED ROOFING NAIL, 8d COMMON (2 1/2"x0.131") NAIL, STAPLE 1/4" x 1 1/2" LONG	3 6
2 1/2" STRUCT. CELLULOSE FIBERBOARD SHEATHING	1 1/2" GALVANIZED ROOFING NAIL, 8d COMMON (2 1/2"x0.131") NAIL, STAPLE 1/4" x 1 1/2" LONG	3 6
1/2" GYPSUM SHEATHING	1/2" GALVANIZED ROOFING NAIL, 8d COMMON (2 1/2"x0.131") NAIL, STAPLE GALV 1/2" LONG, 1 1/2" SCREENS - TYPE W OR S	4 8
3/8" GYPSUM SHEATHING	3/8" GALVANIZED ROOFING NAIL, 8d COMMON (2 1/2"x0.131") NAIL, STAPLE GALV 3/8" LONG, 1 1/2" SCREENS - TYPE W OR S	4 8
WOOD STRUCTURAL PANELS COMBINATION SUBFLOOR UNDERLAYMENT TO FRAMING		
1/4" AND LESS	8d DEFORMED (2"x0.020") NAIL OR 8d COMMON (2 1/2"x0.131") NAIL	6 12
1/2"-1"	8d COMMON (2 1/2"x0.131") NAIL OR 8d DEFORMED (2 1/2"x0.131") NAIL	6 12
1 1/8"-1 1/4"	16d COMMON (3 1/2"x0.135") NAIL OR 8d DEFORMED (2 1/2"x0.131") NAIL	6 12

- FOR S1: 1 INCH x 25.4 mm, 1 FOOT x 304.8 mm, 1 MILE PER HOUR @ 0.441 m/s, 1 kt = 6.898 MPH.
- a. ALL NAILS ARE SMOOTH-CORNER BOX OR DEFORMED SHANKS EXCEPT OTHERWISE STATED. NAILS USED FOR FRAMING AND SHEATHING CONNECTIONS SHALL HAVE MINIMUM AVERAGE BENDING YIELD STRENGTHS AS SHOWN. 200 KI FOR SHANK DIAMETER OF 1/8" TO 3/4" (200 COMMON NAIL) 280 KI FOR SHANK DIAMETERS LARGER THAN 3/4" BUT NOT LARGER THAN 1 1/8" INCH, AND 180 KI FOR SHANK DIAMETERS OF 3/4" INCH OR LESS.
- b. STAPLES ARE 16 GAGE WIRE AND HAVE A MINIMUM 1/8" INCH ON DIAMETER CROWN WIDTH.
- c. NAILS SHALL BE SPACED AT NOT MORE THAN 6 INCHES ON CENTER AT ALL SUPPORTS WHERE SPANS ARE 48 INCHES OR GREATER.
- d. FOUR-FOOT-BY-8-FOOT OR 4-FOOT-BY-12-FOOT PANELS SHALL BE APPLIED VERTICALLY.
- e. SPACING OF FASTENERS NOT INCLUDED IN THIS TABLE SHALL BE BASED ON TABLE 1602.3 (2)
- f. FOR REGIONS HAVING BASIC WIND SPEED OF 100 MPH OR GREATER, 8d DEFORMED (2 1/2"x0.131") NAILS SHALL BE USED FOR ATTACHING FLYWOOD AND WOOD STRUCTURAL PANEL ROOF SHEATHING TO FRAMING WITHIN MINIMUM 48-INCH DISTANCE FROM GABLE END WALLS. IF MEAN ROOF HEIGHT IS MORE THAN 20 FEET, UP TO 30 FEET MAXIMUM.
- g. FOR REGIONS HAVING BASIC WIND SPEED OF 100 MPH OR LESS, NAILS FOR ATTACHING WOOD STRUCTURAL PANEL ROOF SHEATHING TO GABLE END WALL FRAMING SHALL BE SPACED 6 INCHES ON CENTER WHEN BASIC WIND SPEED IS GREATER THAN 100 MPH NAILS FOR ATTACHING PANEL ROOF SHEATHING TO INTERMEDIATE SUPPORTS SHALL BE SPACED 6 INCHES ON CENTER FOR MINIMUM 48-INCH DISTANCE FROM RIDGES, EAVES AND GABLE END WALLS AND 4 INCHES ON CENTER TO GABLE END WALL FRAMING.
- h. GYPSUM SHEATHING SHALL CONFORM TO ASTM C78 AND SHALL BE INSTALLED IN ACCORDANCE WITH GA 253. FIBERBOARD SHEATHING SHALL CONFORM TO ASTM C208.
- i. SPACING OF FASTENERS ON FLOOR SHEATHING PANEL EDGES APPLIED TO PANEL EDGES SUPPORTED BY FRAMING MEMBERS AND REQUIRED BLOCKING AND AT ALL FLOOR PERIPHERIES ONLY. SPACING OF FASTENERS ON ROOF SHEATHING PANEL EDGES APPLIED TO PANEL EDGES SUPPORTED BY FRAMING MEMBERS AND REQUIRED BLOCKING. BLOCKING OF ROOF OR FLOOR SHEATHING PANEL EDGES PERPENDICULAR TO THE FRAMING MEMBERS NEED NOT BE PROVIDED EXCEPT AS REQUIRED BY OTHER PROVISIONS OF THIS CODE. FLOOR PERIPHERY SHALL BE SUPPORTED BY FRAMING MEMBERS OR SOLID BLOCKING. ROOF OR FLOOR SHEATHING 1/8"-INCH OR GREATER IN THICKNESS DOES NOT REQUIRE PERIMETER BLOCKING.

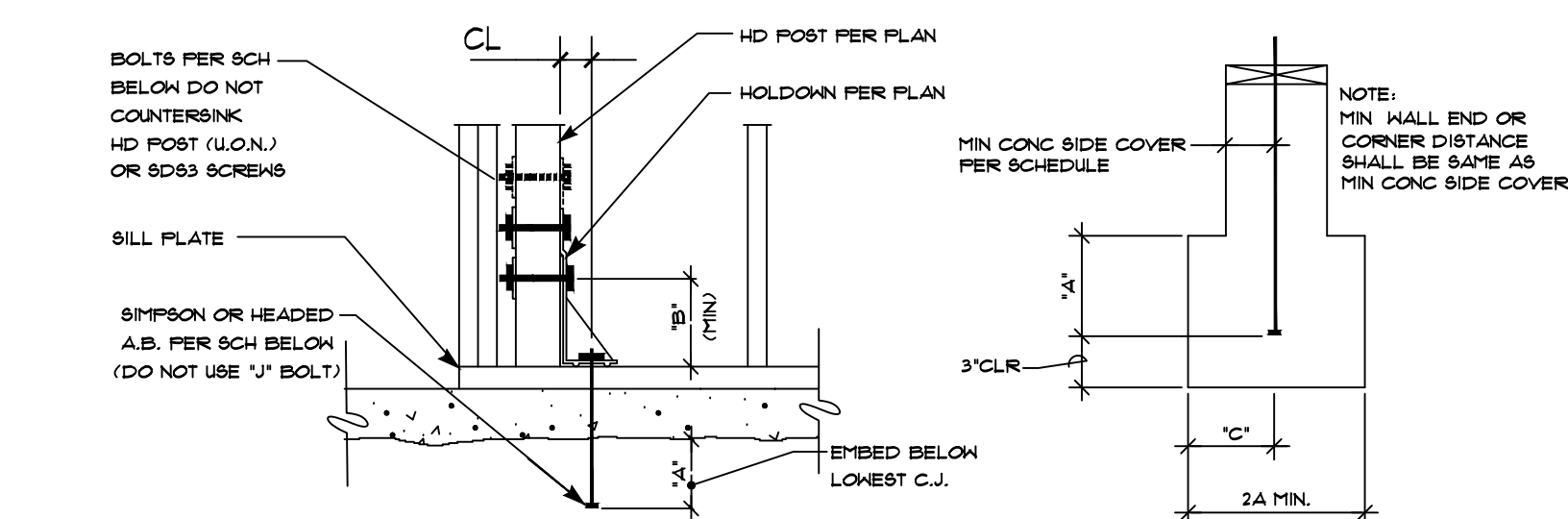
COMMON ABBREVIATIONS:

C.N.	COMMON NAILS	PLY.	PLYWOOD
BLK.	BLOCKED	GYP.	GYPSUM
(BS)	BOTH SIDES	BD.	BOARD
IND.	FOUNDATION	CONN.	CONNECTION
E	PLATE	T.N.	TOE NAIL
A.B.	ANCHOR BOLTS	(U.O.N.)	UNLESS OTHERWISE NOTED
L	LAG BOLTS	SCHE.	SCHEDULE
(ST)	STAGGERED	STR. I	STRUCTURAL I
REF.	REFERENCE	CDX	EXTERIOR GRADE PLYWOOD
GA.	GUAGE	BLKG	BLOCKING
MIN.	MINIMUM	REQD.	REQUIRED
MAX.	MAXIMUM	BLK.	SHEAR WALL
CL	ON CENTER	CL	CENTERLINE

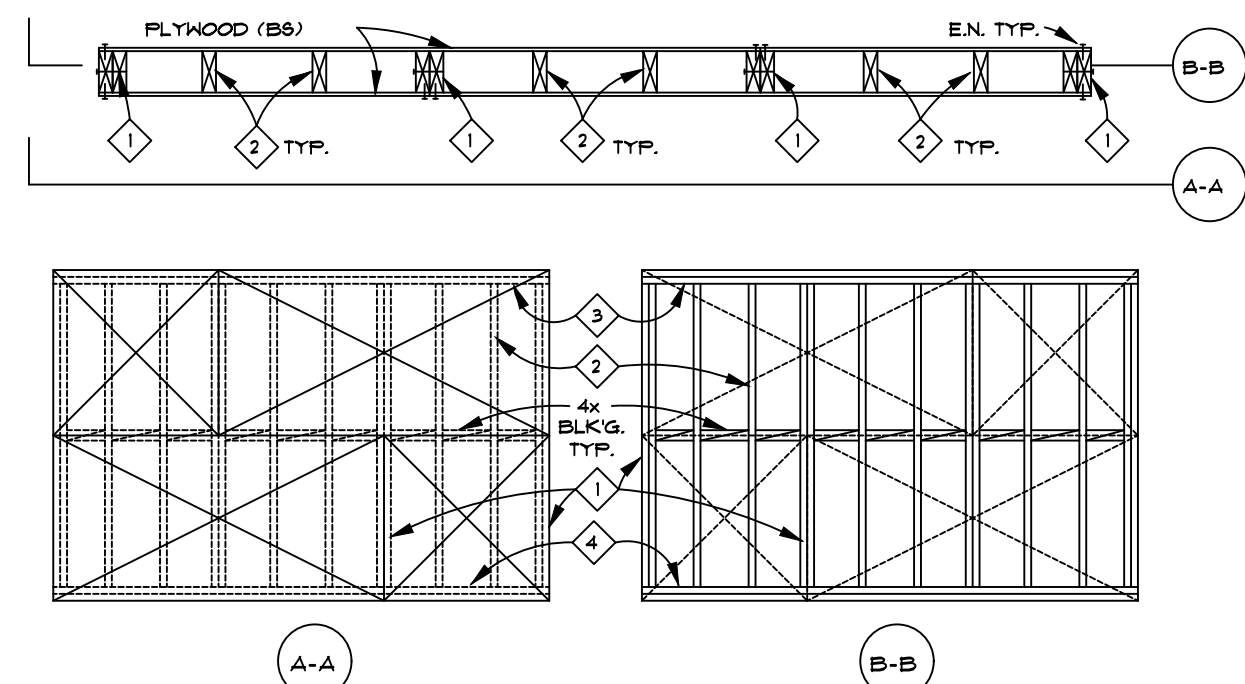
HOLDOWN NOTES

- ALL HOLDOWN ANCHOR BOLTS SHALL BE SECURELY FASTENED IN PLACE PRIOR TO PLACING CONCRETE.
- PROVIDE COVER AT EARTH FORMED SURFACES PER REINFORCING STEEL NOTES AT ALL HOLDOWN ANCHOR BOLTS (U.O.N.).
- PROVIDE 8" MINIMUM FOOTING OR STEM WALL WIDTH.
- SEE DETAIL (A) (EACH WAY AT CORNER)
- USE S8TBL MODELS FOR HOLDOWNS ON (2) 2x AND 4x SILL PLATES.

MARK	MIN HD POST	BOLTS/ NAILS/ SCREENS	ANCHOR BOLT MONO FOUR TWO FOUR	MIN CONC (2) COVER ON ANCHOR BOLT	DIMENSIONS			
					A (MIN)	*B* (MIN)	CL	*C*
LTT19	(2) 2x4	(8) 10d	S8TB16	S8TB20	1 1/4"		1 3/8"	
LTT131	(2) 2x4	(18) 10d	S8TB16	S8TB20	1 1/4"		1 3/8"	
HTT4	(2) 2x4	(18) 10d	S8TB20	S8TB24	1 1/4"		1 5/16"	
HTT5	(2) 2x4	(26) 10d	S8TB24	S8TB28	1 1/4"		1 5/16"	
HU2	(2) 2x4	(6) SDB3	S8TB16 (5)	S8TB20 (5)	1 1/4"		1 5/16"	
HU4	(2) 2x4	(10) SDB3	S8TB16 (5)	S8TB20 (5)	1 1/4"		1 5/16"	
HU5	(2) 2x4	(14) SDB3	S8TB24 (5)	S8TB28 (5)	1 1/4"		1 5/16"	
HU8	(3) 2x4	(20) SDB4 1/2	S8TB28 (5)	S8TB34 (5)	1 1/4"		1 3/8"	
HDQ8	(3) 2x4	(20) SDB4 1/2	S8TB28 (5)	S8TB34 (5)	1 1/4"		1 1/4"	
HU11	(6) 2x4	(30) SDB8	1"φ	1"φ	1 1/4"	16"	1 3/8"	
HHDQ11	(4) 2x4	(24) SDB6	1"φ	1"φ	1 1/4"	16"	1 1/2"	
HHDQ14	(6) 2x4	(30) SDB8	1"φ	1"φ	1 1/4"	20"	1 1/2"	
HU14	(4) 2x4	(36) SDB6	1"φ	1"φ	1 1/4"	20"	1 9/16"	
HD19	6x6	(5) 1"φ	1 1/4"φ	1 1/4"φ	1 1/4"	20"	2 1/8" 20" (4)	

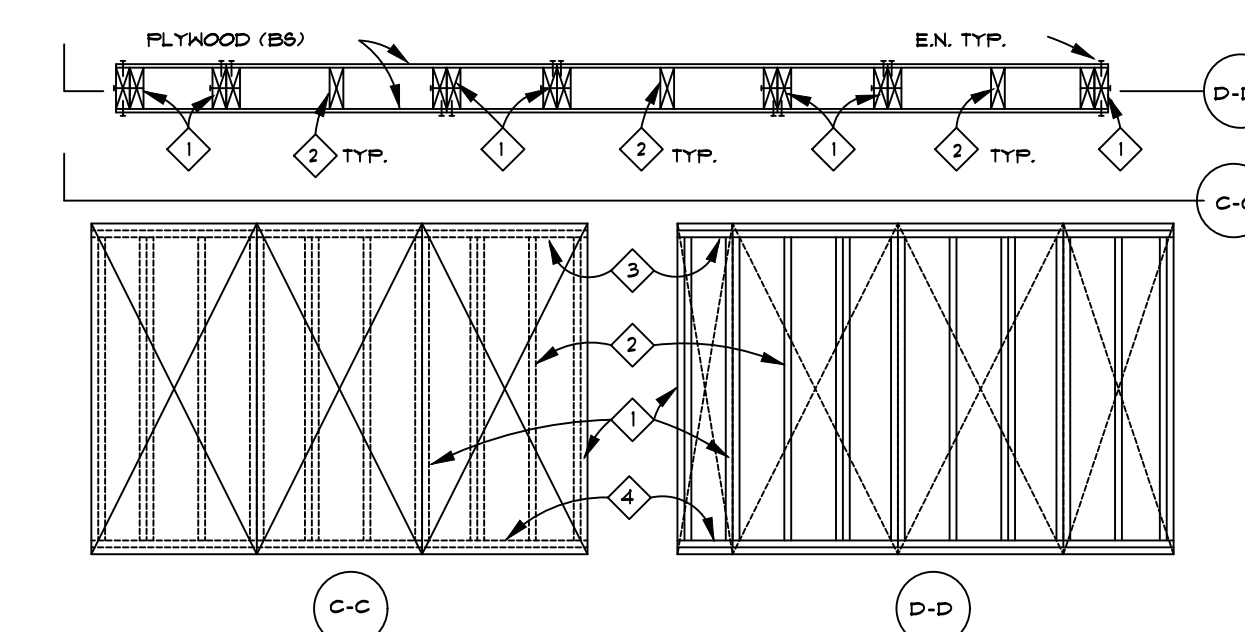


TYPICAL HOLDOWN DETAIL

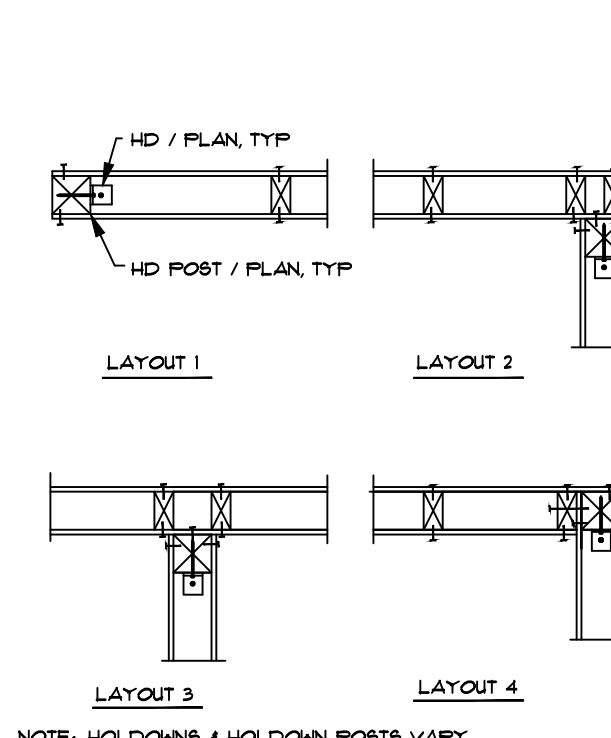


DOUBLE SHEAR WALL SECTIONS (PLYWOOD HORIZ.)

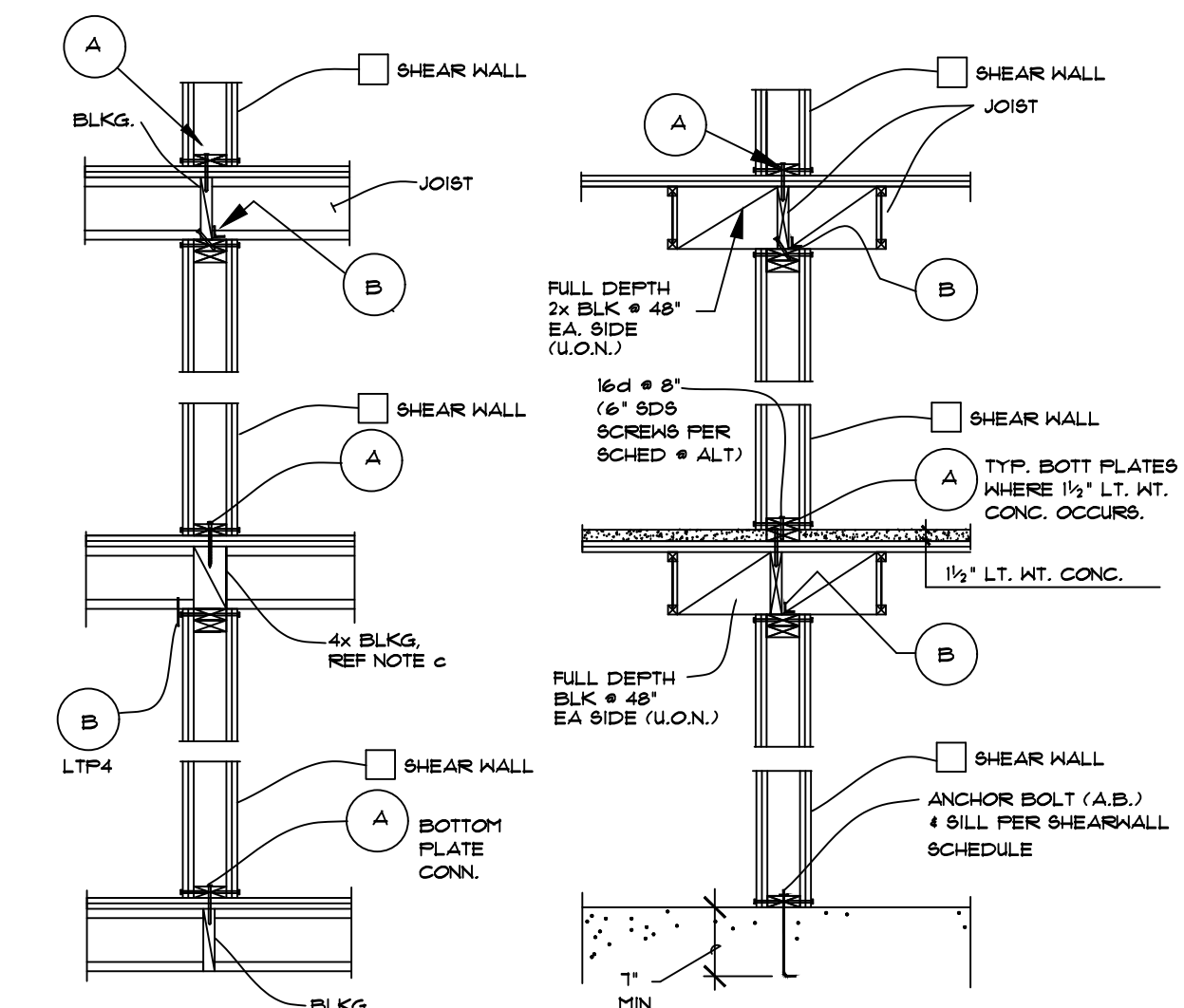
DOUBLE SHEAR WALL POST/STUD SCHEDULE



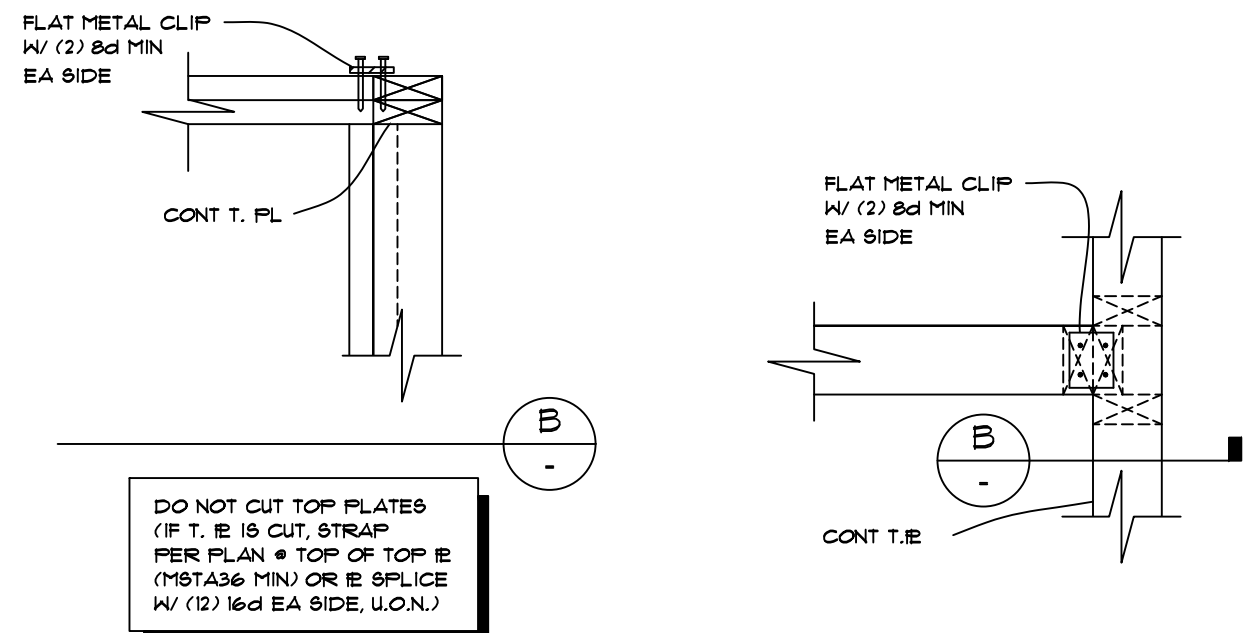
DOUBLE SHEAR WALL SECTIONS (PLYWOOD VERT.)



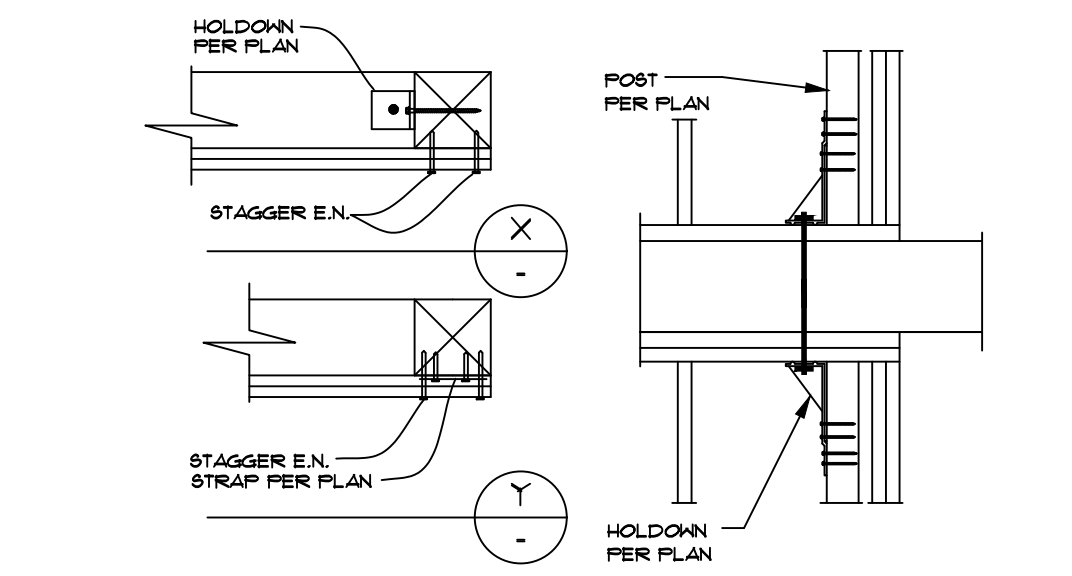
TYPICAL HOLDOWN LOCATION



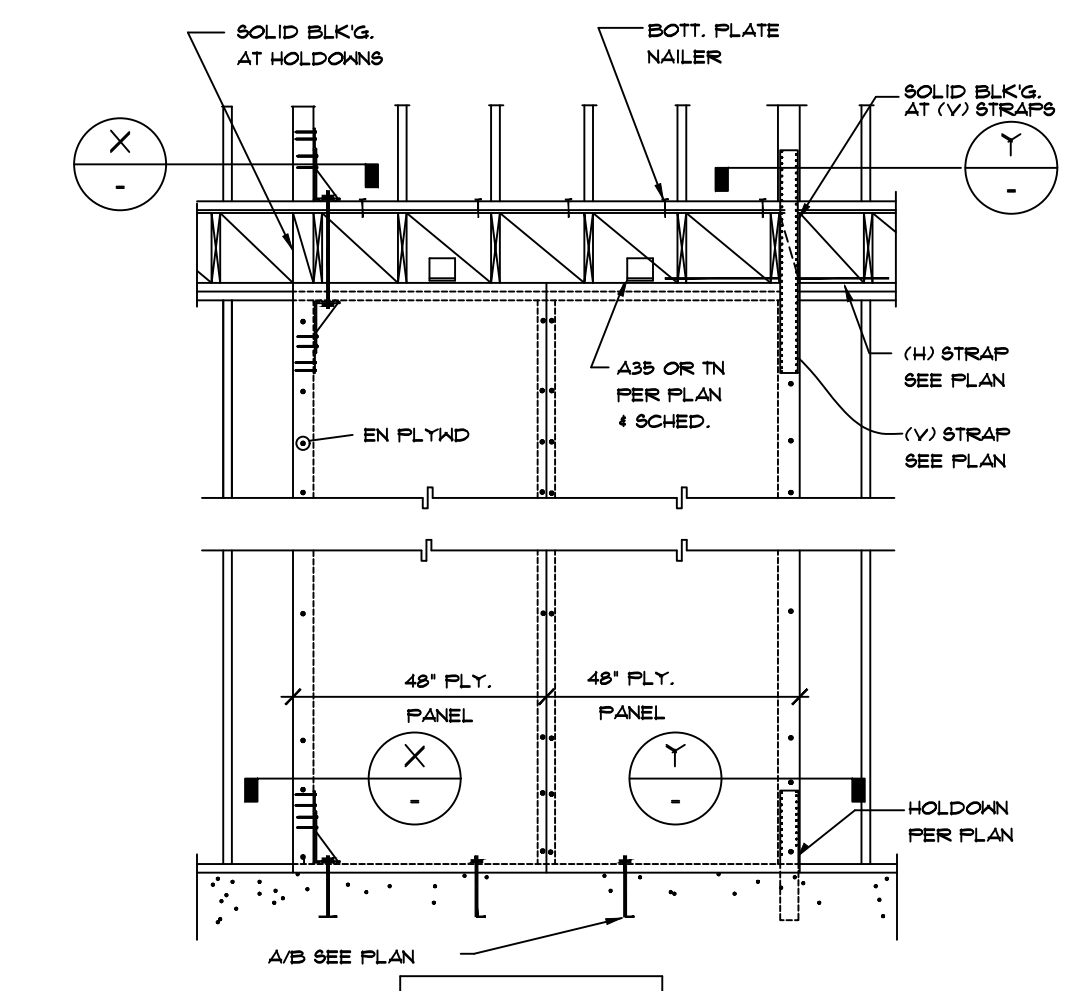
SHEAR WALL SECTIONS



PERPENDICULAR WALLS • EXTERIOR/INTERIOR S.W.'s + DRAG LINES



HOLDOWN BETWEEN FLOORS



HOLDOWN BETWEEN FLOORS

SHEAR WALL SCHEDULE

MARK	SHEAR WALL MATERIAL (1)	EDGE NAILING W/ C.N. (2)	FOUNDATION CONN. 1/2" DIA ANCHOR BOLT SPACING (3) 2x SILL (2) 2x SILL	FLOOR CONNECTION (4) FLOOR CONNECTION BOTTOM PLATE NAIL OR SCREEN SPACING (4) 16d SINKER SDB 1/4x4 1/2 @	SHEAR TRANSFER LTP4 (4) OR A35 SPACING BLK OR JOIST TO TOP (5) (6)	ALLOWABLE SHEAR (LB/FT)	REFERENCE NOTES (7)
1	3/8" PLY CDX	8d @ 6"	48"	6"	32" OR T.N. 16d SINKERS AT 6"	200	d
2	3/8" PLY CDX	8d @ 6"	48"	4"	22" OR T.N. 16d SINKERS AT 4"	282	d
3	3/8" PLY CDX	8d @ 4"	31"	3"	15" OR T.N. 16d SINKERS AT 2"	411	d
4	3/8" PLY CDX	8d @ 3"	24"	-	9"	529	c, d, e
5	3/8" PLY STR I	8d @ 2"	-	22"	-	683	c, d, e
6	3/8" PLY CDX (BS)	8d @ 2"	-	11"	-	1366	a, b, c, e
7	1/2"x2" PLY STR I	10d @ 3"	18"	-	6"	108	c, d, e
8	1/2"x2" PLY STR I	10d @ 2"	-	13"	-	1120	a, c, e
9	1/2"x2" PLY STR I (BS)	10d @ 3"	-	10"	3"	1416	a, b, c, e
10	1/2"x2" PLY STR I (BS)	10d @ 3"	-	8"	2" STAGG'D	1711	a, b, c, e
11	1/2"x2" PLY STR I (BS)	10d @ 2"	-	7"	2" STAGG'D	2150	a, b, c, e

- ALL PLYWOOD PANEL EDGES TO BE BLOCKED AND EDGE NAILED. F.N. 12" O.C. W/ MATCHING FASTENERS TO E.N.
 - ALL FLYWOOD NAILING SHOULD BE WITH COMMON NAILS (MAY SUBSTITUTE GALVANIZED BOX). FIELD NAILING SHALL BE WITH COMMON NAILS @ 12" o.c.
 - ALL ANCHOR BOLTS TO BE 10" LONG @ 2x SILL & 12" LONG @ (2) 2x SILL. ALL ANCHORS SHALL HAVE MIN 1/4"x3"x2" PLATE WASHERS. PLACE ALL ANCHORS A MINIMUM DISTANCE OF 5" (AND A MAXIMUM OF 12") FROM THE ENDS OF SILL PLATES. MINIMUM 2 ANCHOR BOLTS PER WALL OR PIECE OF SILL.
 - SDB 1/4x4 1/2 SCREWS MAY BE SUBSTITUTED WITH 3/8"x6"x6" LAG SCREWS WITH PREDRILLED HOLES. MINIMUM STAGGER DISTANCE IS 1 1/2". MINIMUM EDGE DISTANCE IS 1". VERIFY MINIMUM SDB/LAG SCREW SPACING AND LEAD HOLE REQUIREMENTS WITH SCL MANUF. OR USE 4x MEMBERS PER REF NOTE c.
 - AT ALL ROOF PLATES W/ FRAMING PERPENDICULAR TO WALL USE A35 @ 24" OR HI PER TRUSS @ 24" MAX) TYPICAL, U.O.N.
 - USE FULL DEPTH 2x FOR RIM OR BLOCKING (U.O.N.).
- REFERENCE NOTES:
- (2) 2x SILL, 4 4x BLOCKING REQUIRED @ INTERMEDIATE PANEL JOINTS. STITCH NAIL (2) 2x TOP (BS) AND SILL USING 16d COMMON STAGG'D AT MATCHING SPACING TO PANEL E.N.
 - SHEARWALLS WITH FLYWOOD ON BOTH SIDES: VERTICAL INTERIOR JOINTS ON OPPOSITE SIDES OF THE WALL SHALL BE STAGGERED.
 - CONNECTIONS TO FLOOR FRAMING USE 4x MIN RIM OR BLOCKING BENEATH WALLS WITH 16d SINKERS @ 2" o.c. OR CONNECTIONS WITH SDB/LAG SCREWS AS REQUIRED BY SCL MANUFACTURER.
 - 2x BLKG REQUIRED AT ALL PANEL JOINTS.
 - USE 6" LONG SDB SCREW FLOOR CONNECTION W/ (2) 2x SILL.

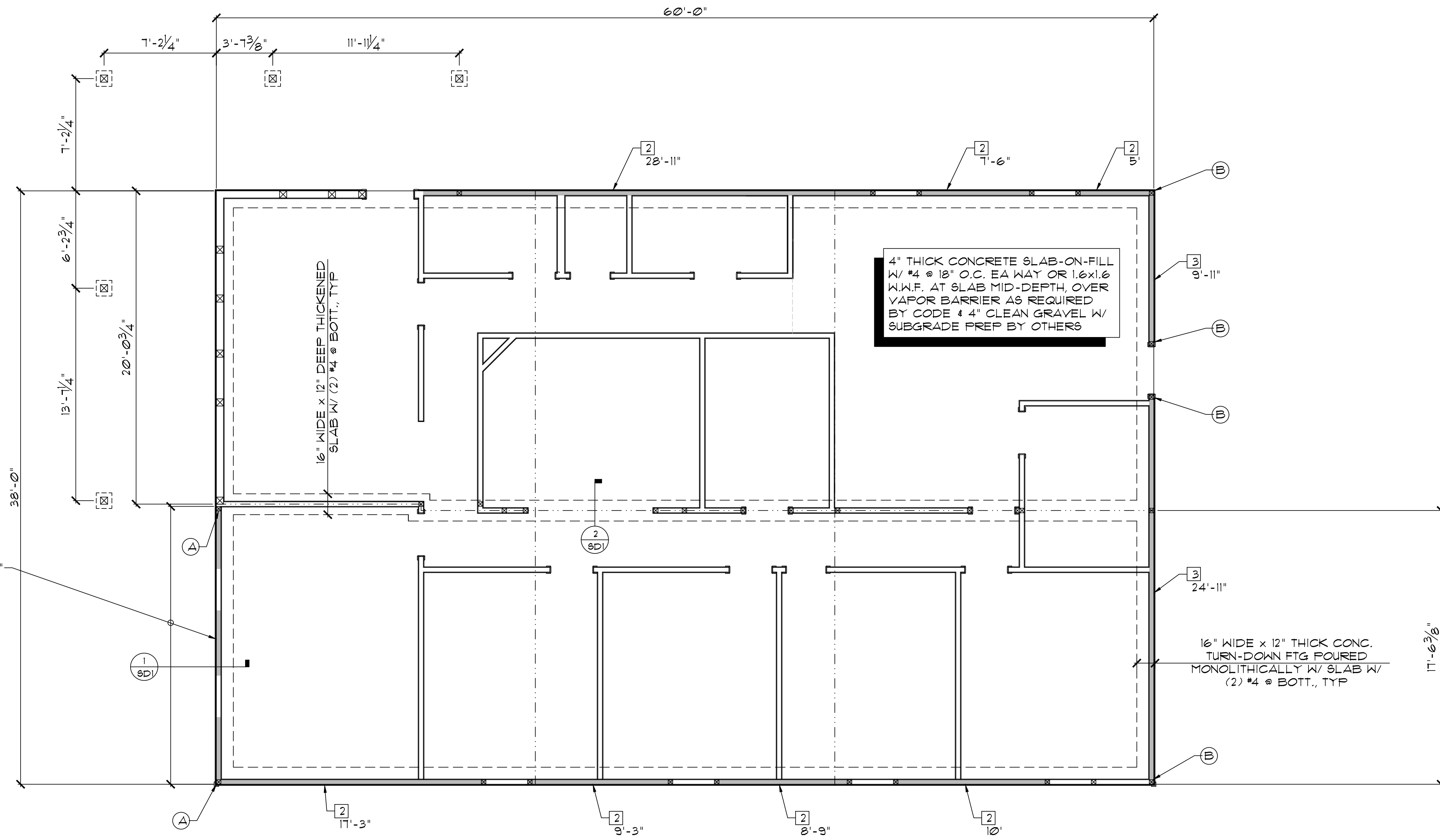
FOUNDATION NOTES:

1. ASSUMED SOIL BEARING CAPACITY IS 2,000 PSF. CONTRACTOR MUST CONTACT A SOILS ENGINEER IF UNSUITABLE SOILS ARE ENCOUNTERED.
2. ADEQUATE DRAINAGE SHALL BE PROVIDED FOR THE SURFACE AREA ADJACENT TO THE STRUCTURE SUCH THAT WATER DRAINS AWAY FROM STRUCTURE.
3. VERIFY ALL DIMENSIONS W/ FLOOR PLAN (BY OTHERS) PRIOR TO WORK.
4. LOCATION AND ORIENTATION OF STRUCTURE OF PROPERTY BY OTHERS.
5. FOR ADDITIONAL NOTES, SEE "SF" SHEETS.

LEGEND:

- INDICATES CRACK CONTROL JOINT PER SHEET SF1
- ☒ INDICATES TREATED 6x6 POST W/ "POST BASE" TO CENTER OF 16"x16"x12" THK POURED CONC. FTG
- ▭ INDICATES MONO FOUR CONCRETE TURNDOWN FTG PER PLAN
- ▭ INDICATES SHEARWALL ABOVE SLAB LEVEL, SEE SHEET SF3 FOR SCHEDULE
- ▭ INDICATES STUD WALLS ABOVE SLAB - 2x6 @ 16" O.C. OR (2) 2x4 @ 12" O.C. SPP STUDS AT ALL EXTERIOR WALLS AND INTERIOR BEARING WALLS (2x4 @ 24" O.C. MAX INTERIOR NON LOAD-BEARING WALLS), TYP (U.O.N.)
- Ⓐ INDICATES (3) 2x W/ "HDU8-9D93"
- Ⓑ INDICATES (2) 2x W/ "HDU5-9D93"
- ☒ INDICATES POST ABOVE SLAB LEVEL, SEE SHEET S2 FOR SIZES (U.O.N.)

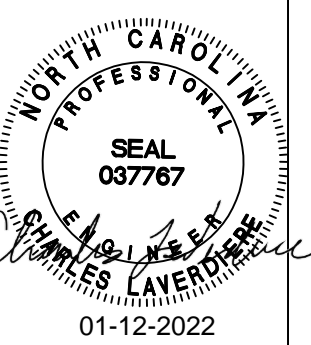
Ⓢ (B.S.) ENTIRE WALL W/ "C614" HORIZ STRAP W/ 10d @ 3" O.C. ABV @ BLW OPENING TO TIGHT FIT 2x BLKG PER



FOUNDATION PLAN
SCALE: 1/4"=1'-0"

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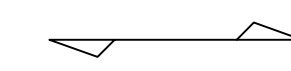
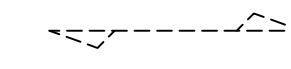

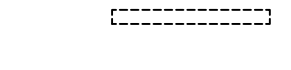



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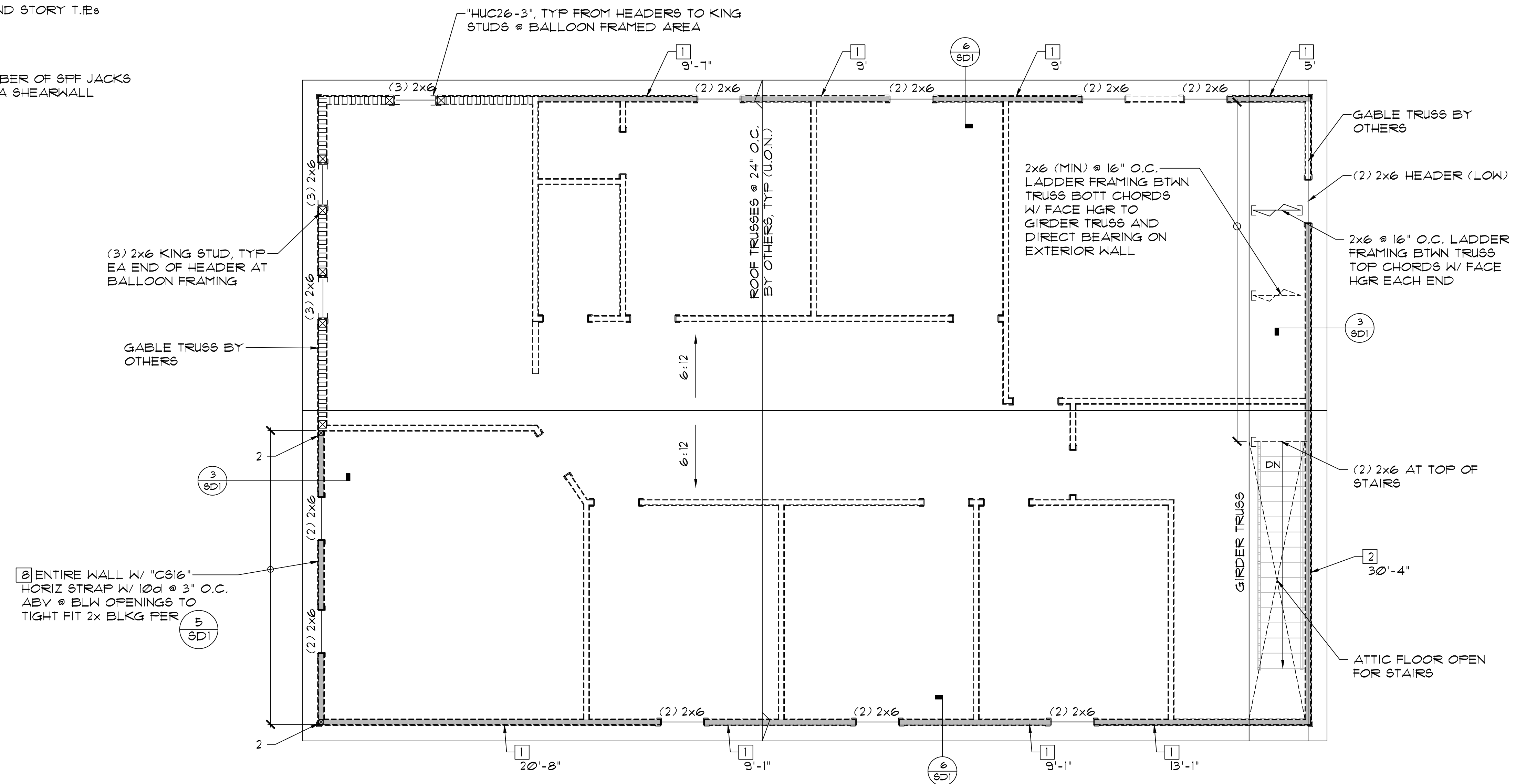
DATE	1-11-22
SCALE	AS SHOWN
DRAWN	J.H.
JOB	21-1174
SHEET	

ROOF FRAMING NOTES:

1. ROOF SHEATHING IS TO BE 7/16" OSB (24/16) B.N. 4 E.N. 8d @ 6", F.N. 8d @ 12" U.O.N. OR CODE-APPROVED ALTERNATIVE.
2. (15) 16d EACH SIDE OF TOP PLATE SPLICES, TYP U.O.N.
3. TRUSSES TO BE TIED TO TOP PLATES USING AN "H2.5A" CLIP AT EACH TRUSS END.
4. ATTIC VENTILATION BY OTHERS.
5. VERIFY ALL DIMENSIONS WITH FLOOR PLAN (BY OTHERS) PRIOR TO WORK.
6. ALL GABLE WALLS ARE TO BE CONTINUOUSLY SHEATHED AS 1 MIN.
7. FOR ADDITIONAL NOTES, SEE "SP" SHEETS.

LEGEND:

-  INDICATES ROOF TRUSSES BY OTHERS
-  INDICATES ATTIC FLOOR FRAMING PER PLAN
-  INDICATES WALLS BENEATH ROOF LEVEL - 2x6 @ 16" O.C. OR (2) 2x4 @ 12" O.C. SFF STUDS AT ALL EXTERIOR WALLS (2x4 @ 24" O.C. MAX INTERIOR NON LOAD-BEARING WALLS), TYP (U.O.N.)
-  INDICATES (2) 2x6 @ 16" O.C. BALLOON FRAMED STUD WALL BENEATH ROOF LEVEL
-  INDICATES LIMITS OF SHEARWALL BENEATH SECOND STORY T.I.E.s (FOR SCHEDULE SEE SHEET SP3)
-  INDICATES BEAM / HEADER PER PLAN
-  INDICATES VERTICAL SUPPORT W/ INDICATED NUMBER OF SFF JACKS REQ'D. E.N. WHERE SUPPORT IS LOCATED WITHIN A SHEARWALL



ROOF FRAMING PLAN
SCALE: 1/4" = 1'-0"

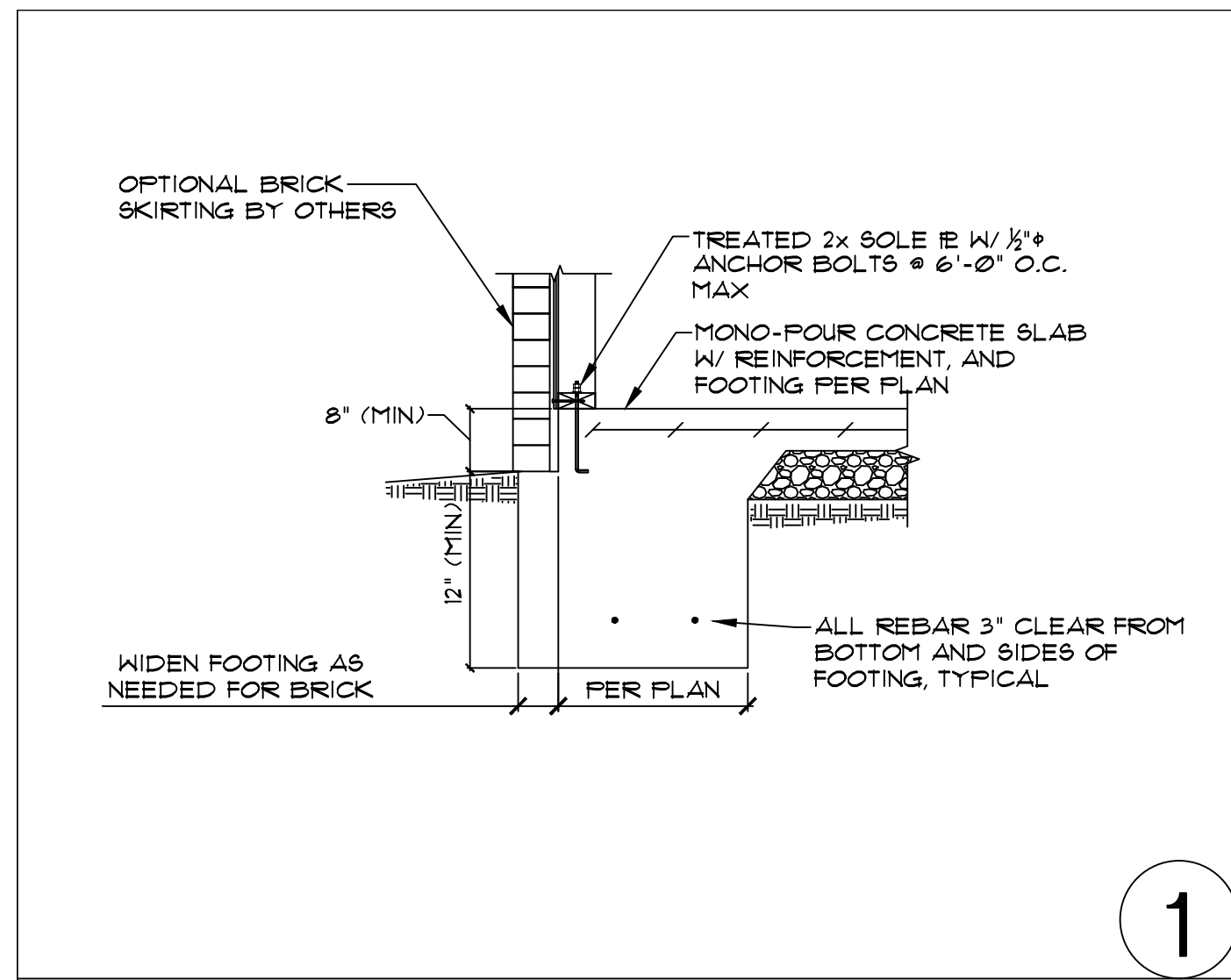
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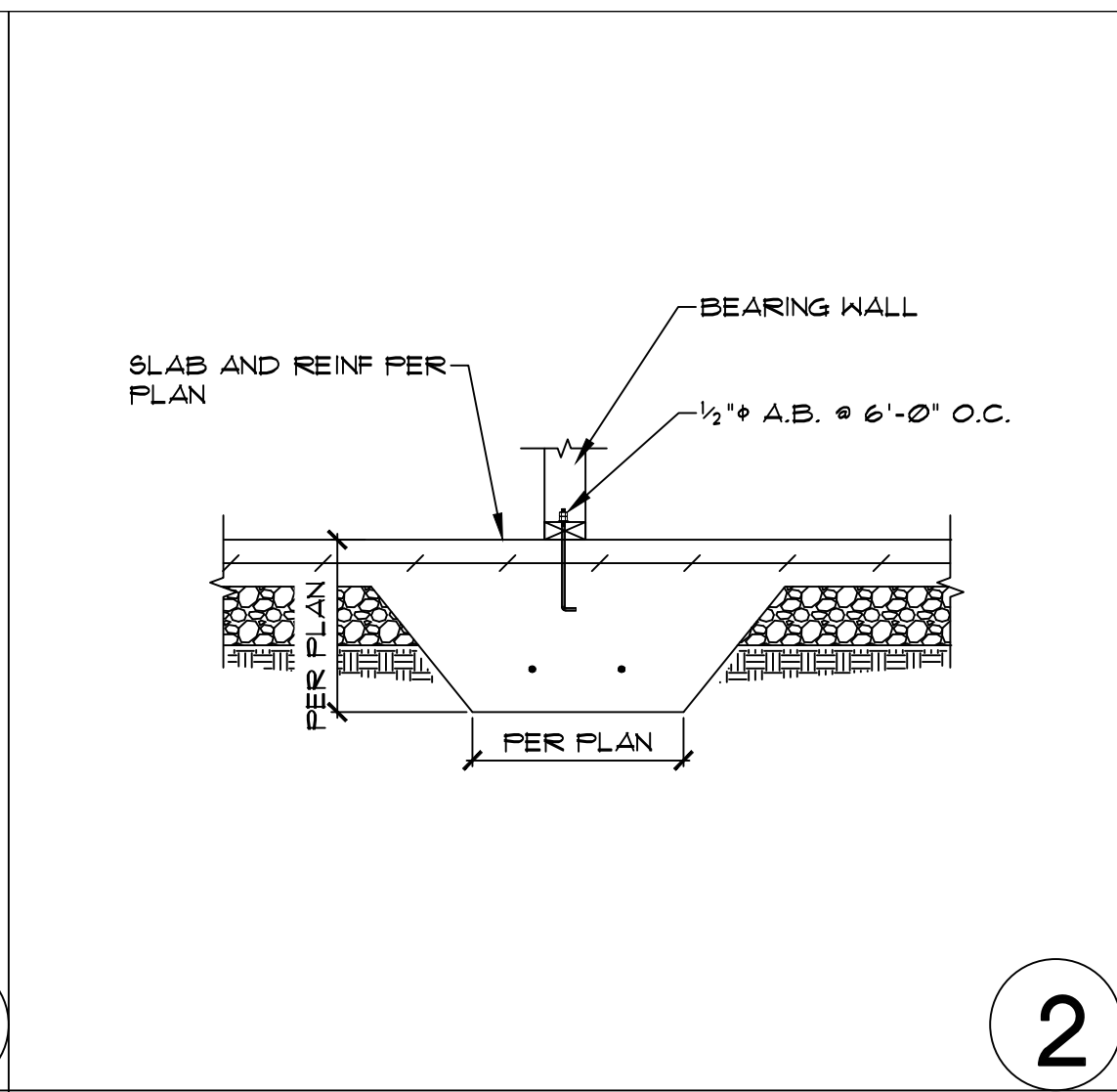
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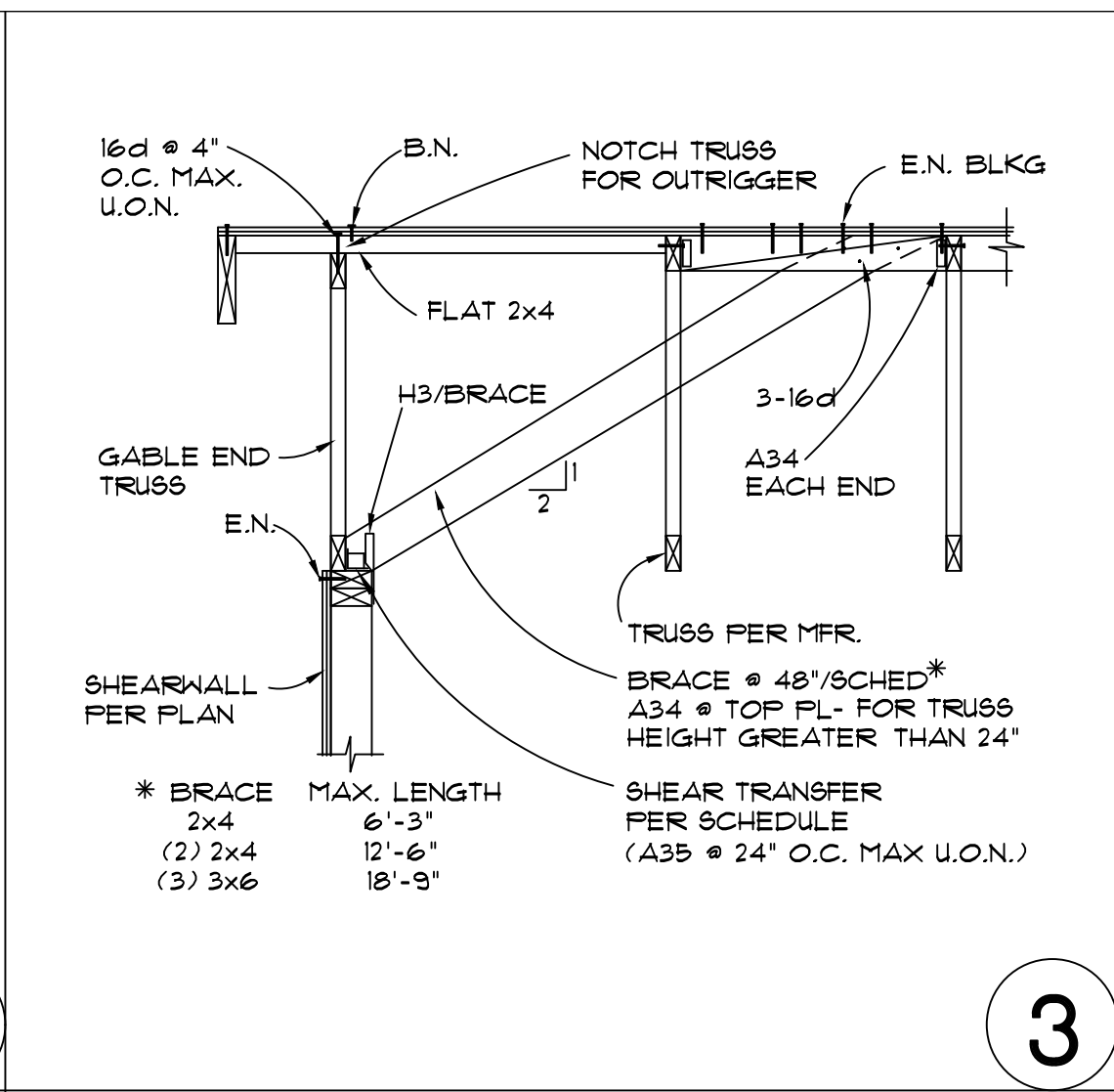
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DRAWN J.H.
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SHEET



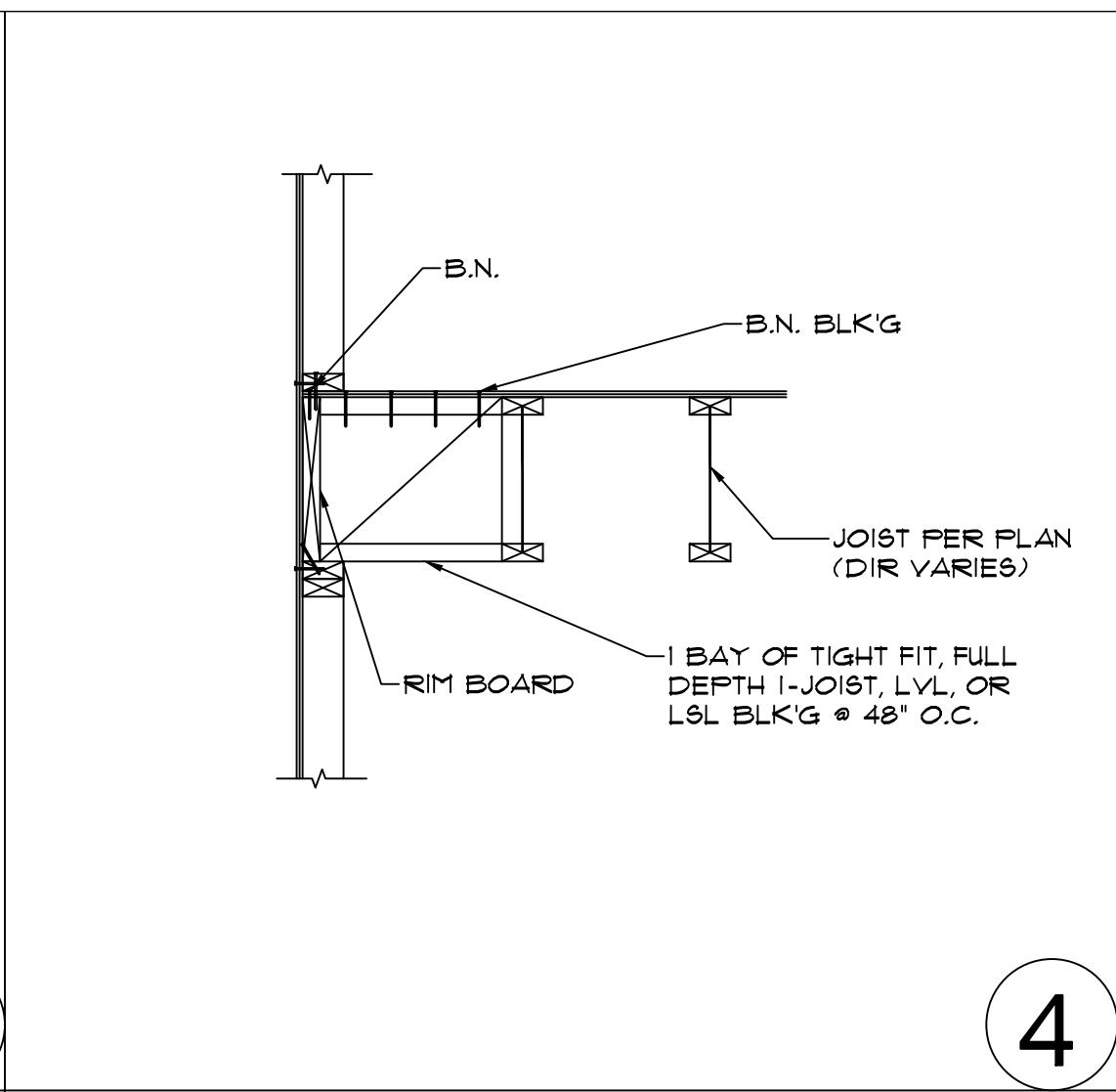
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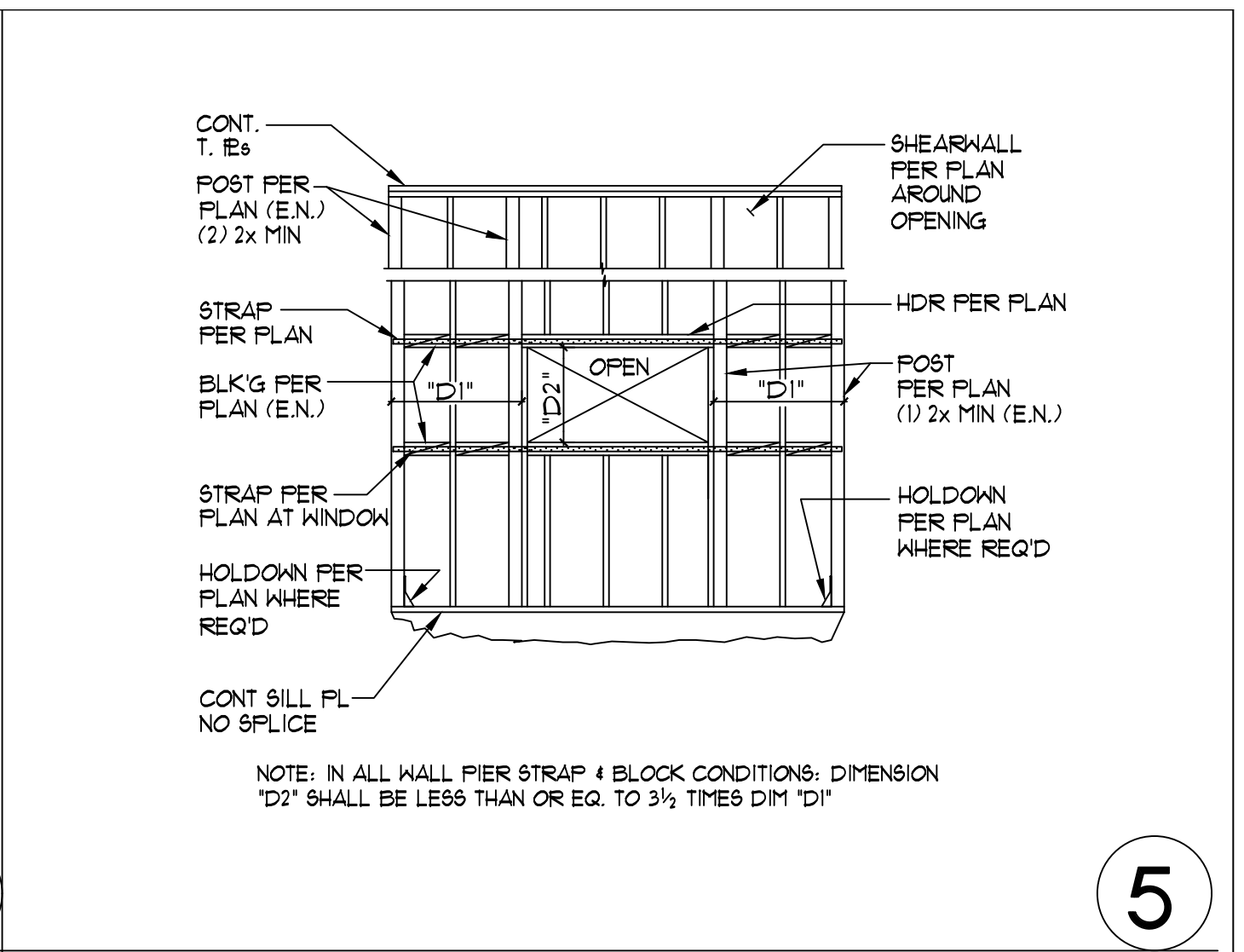
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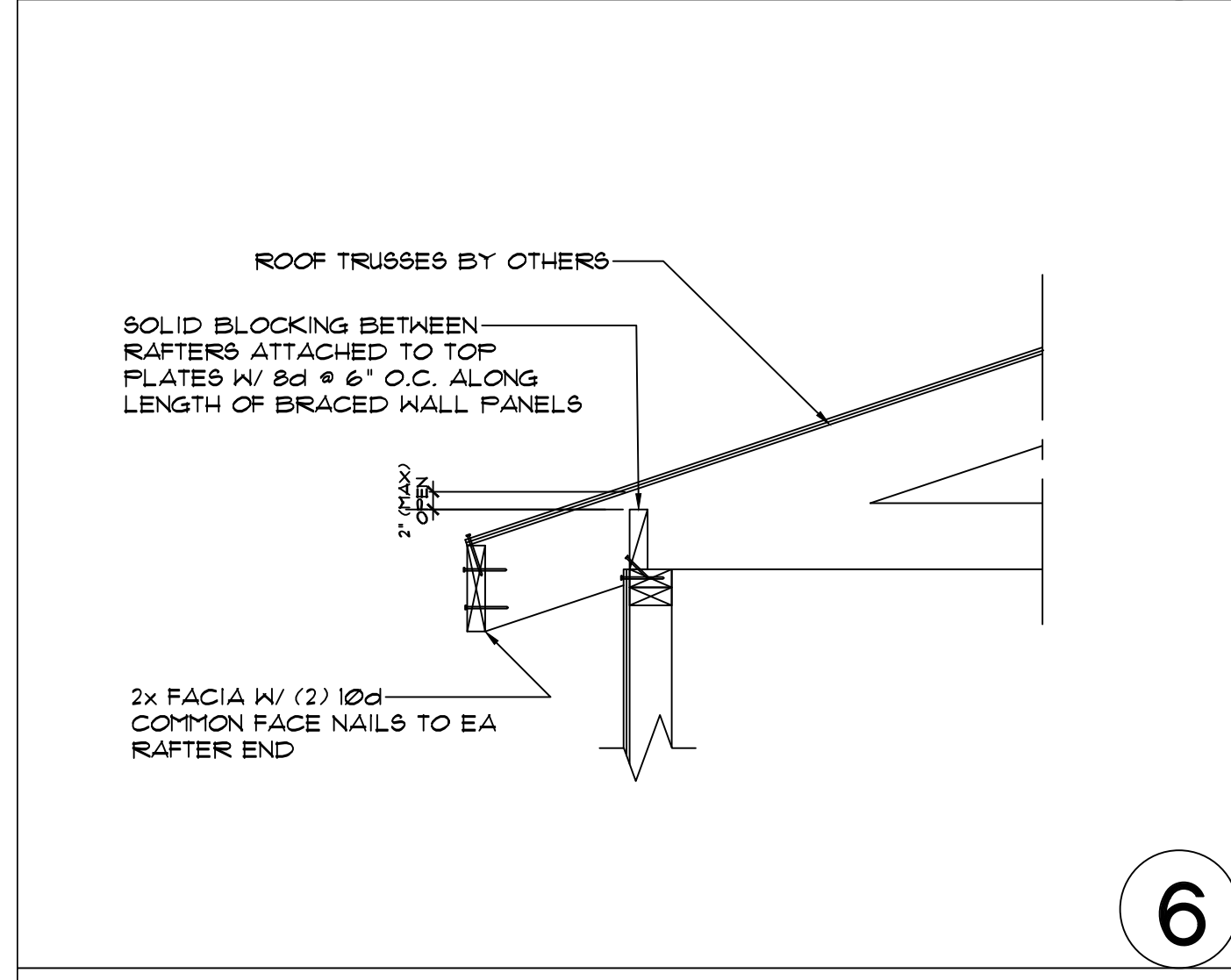
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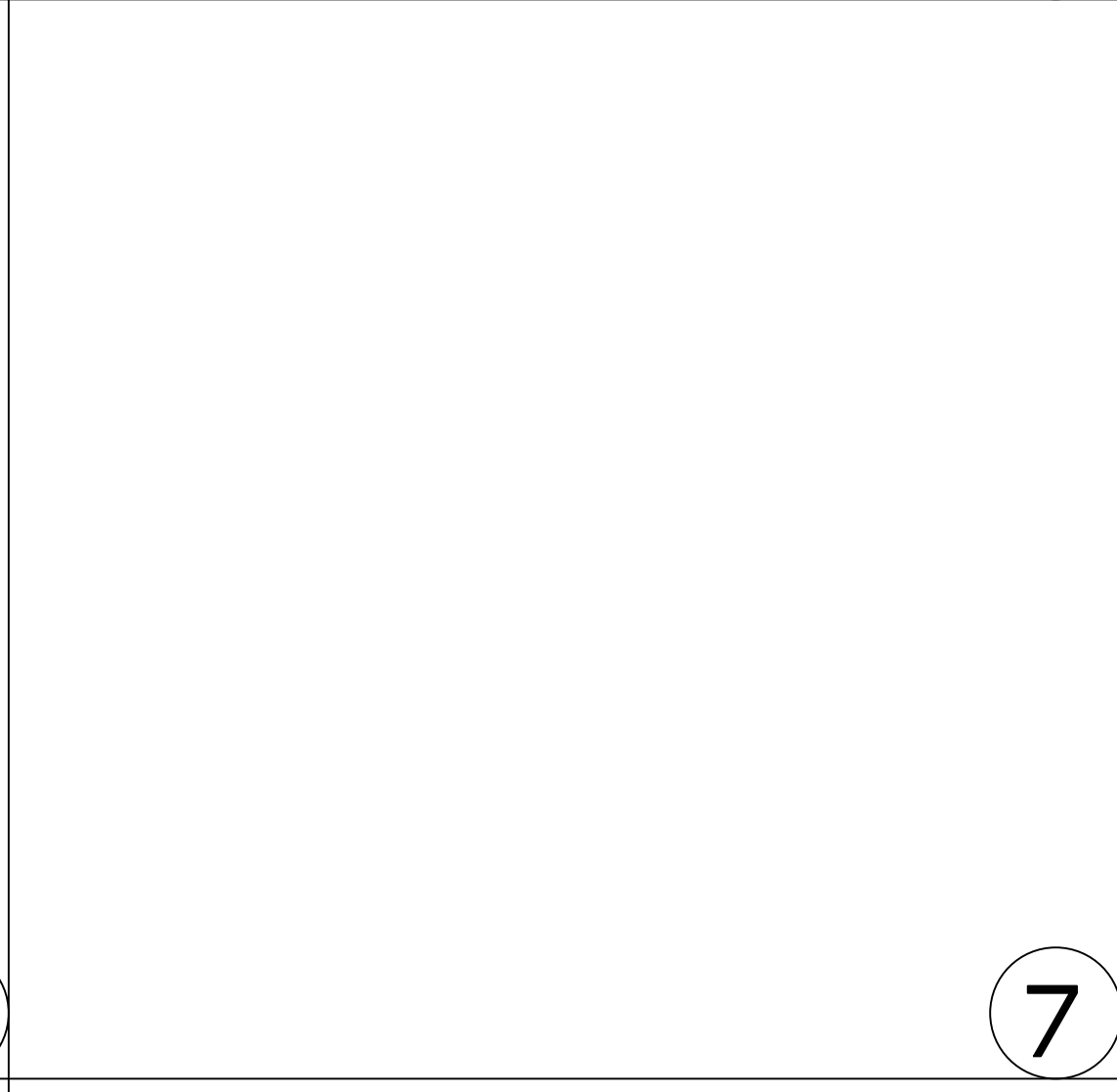
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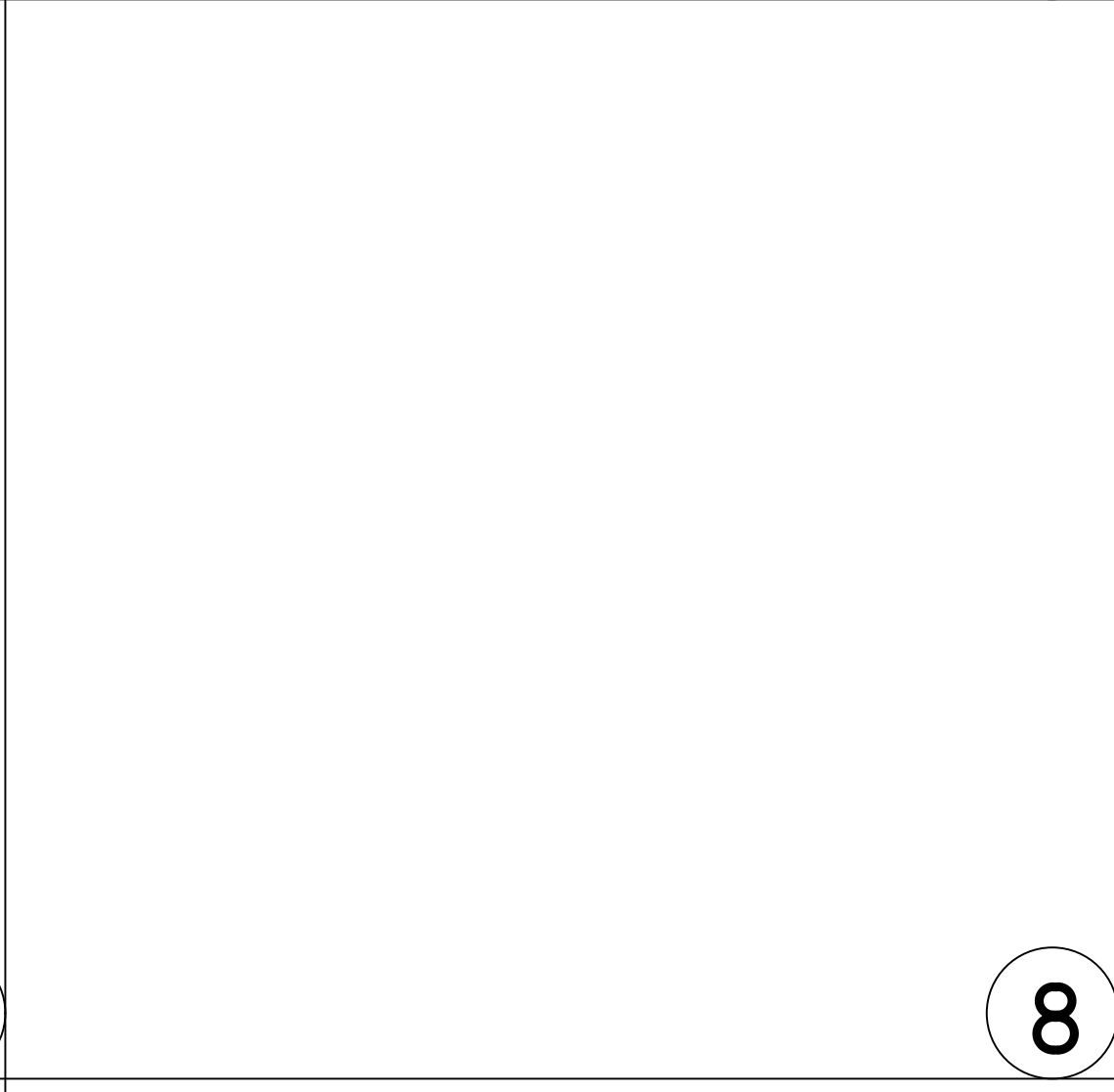
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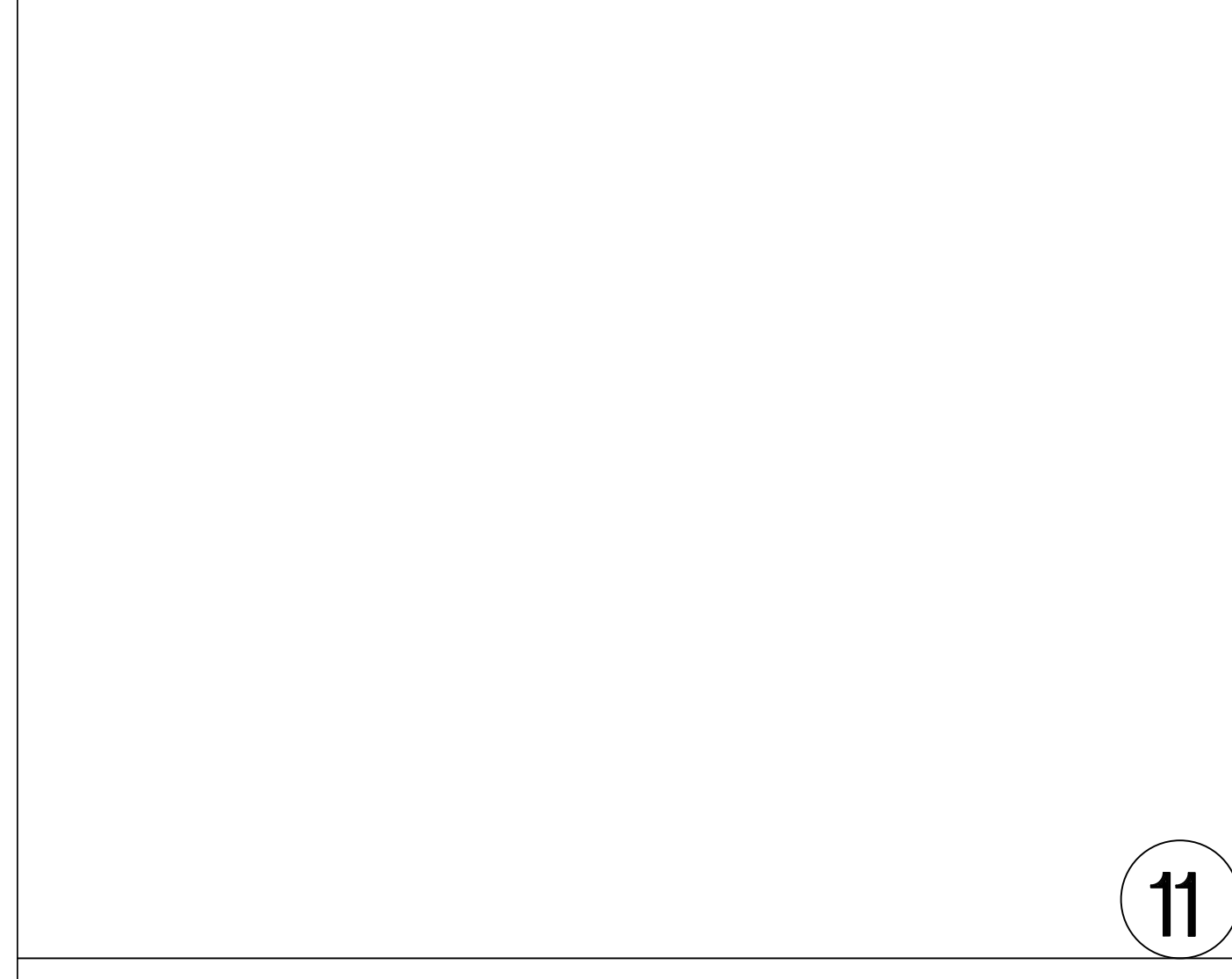
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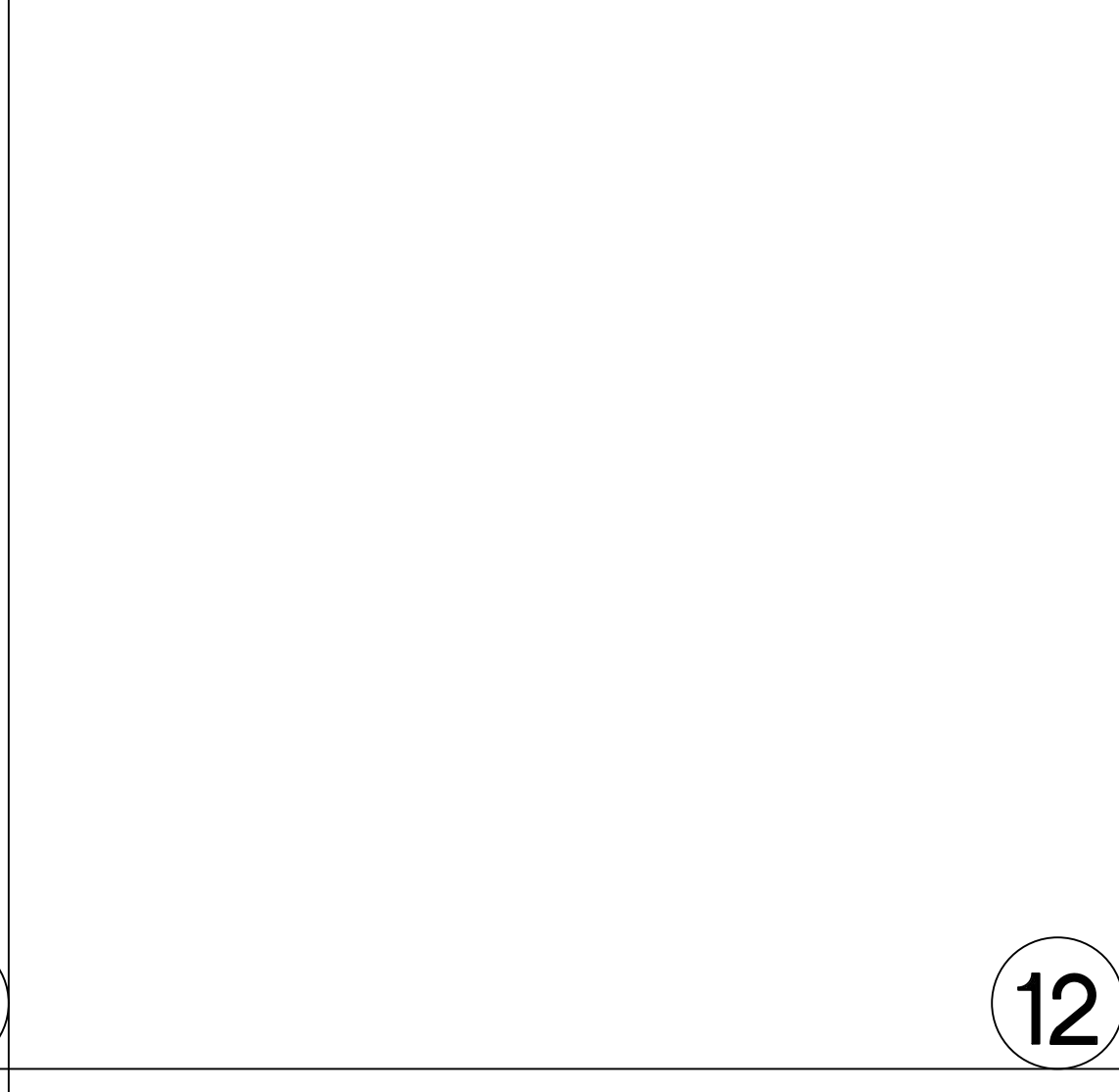
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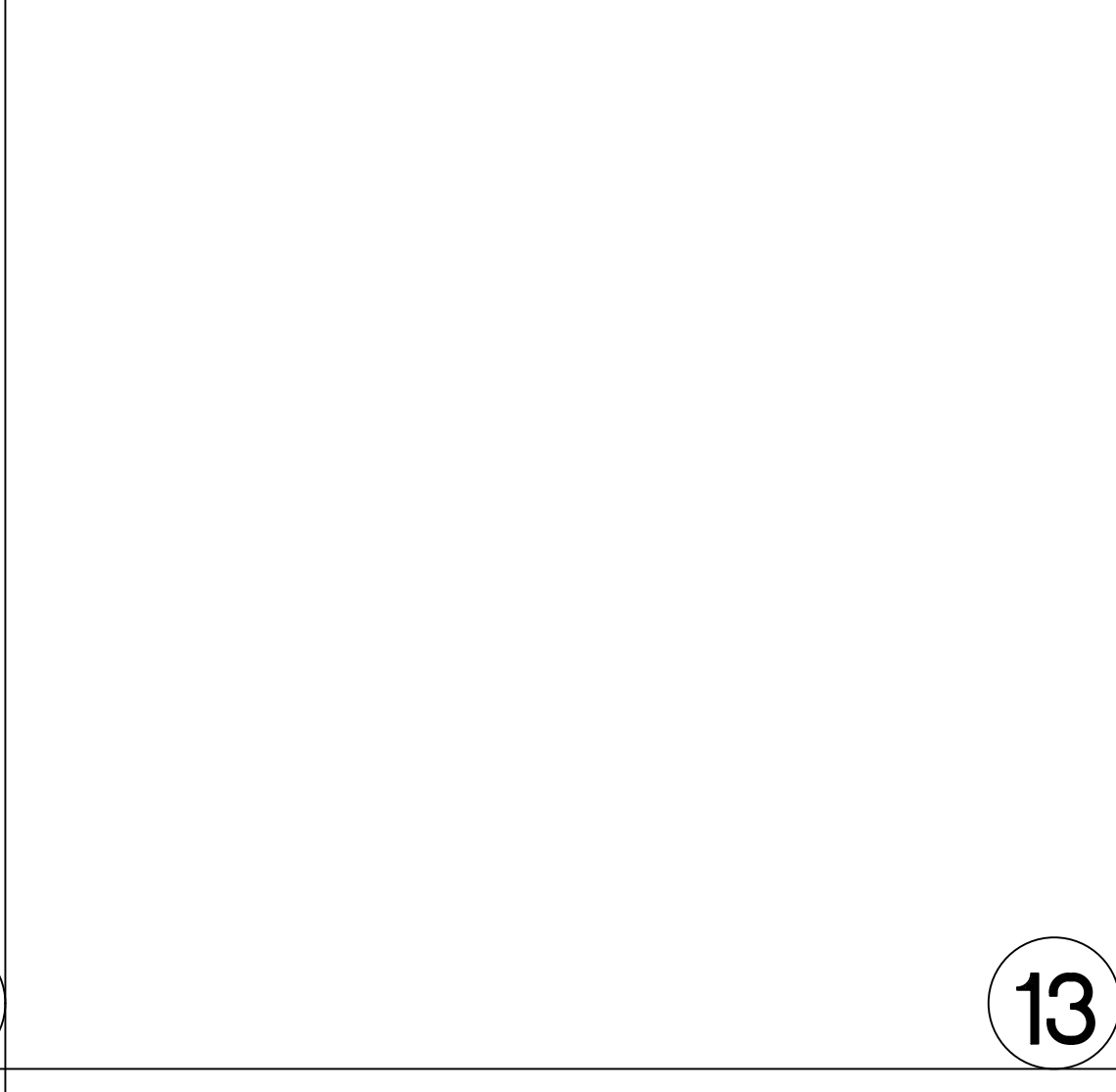
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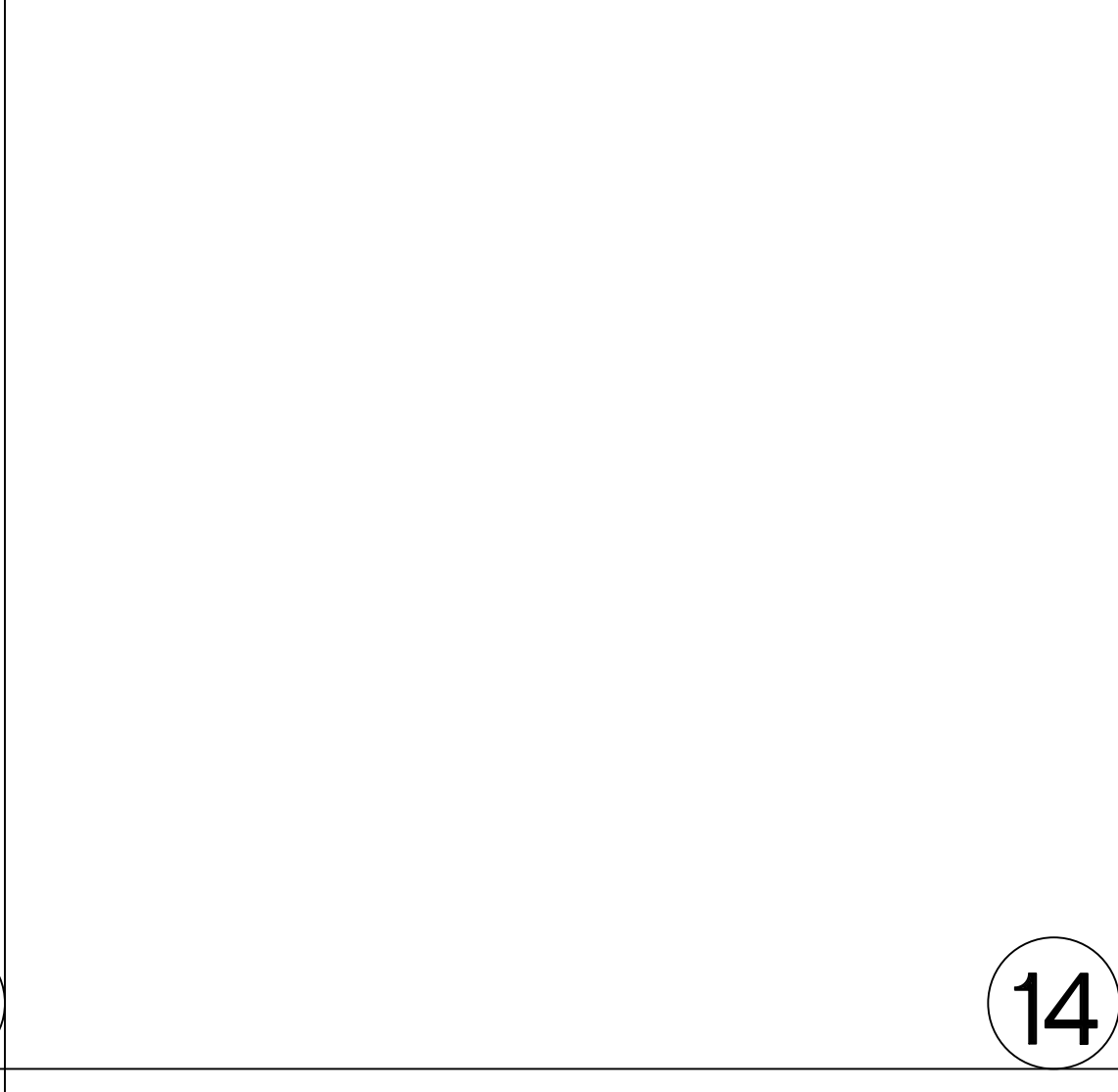
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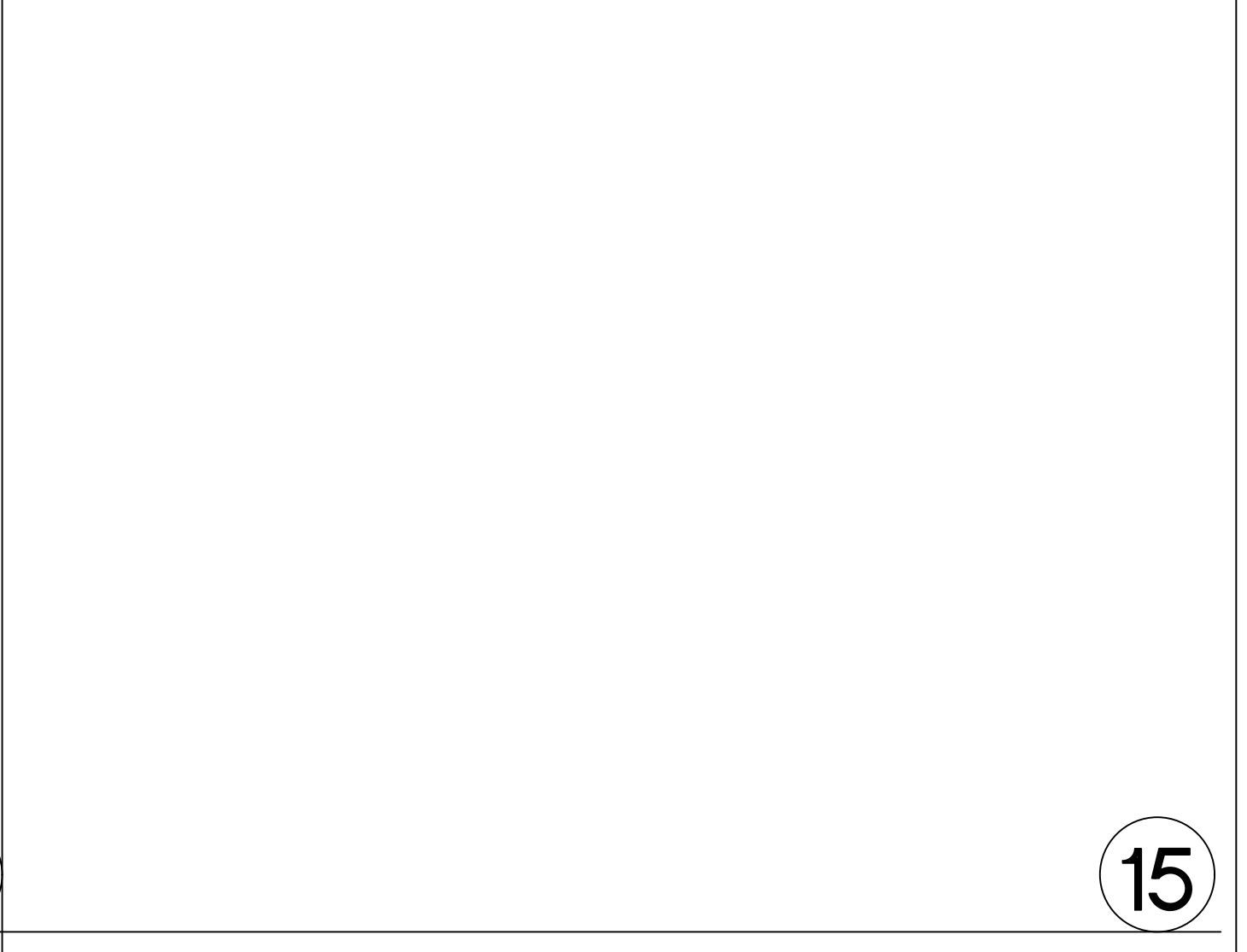
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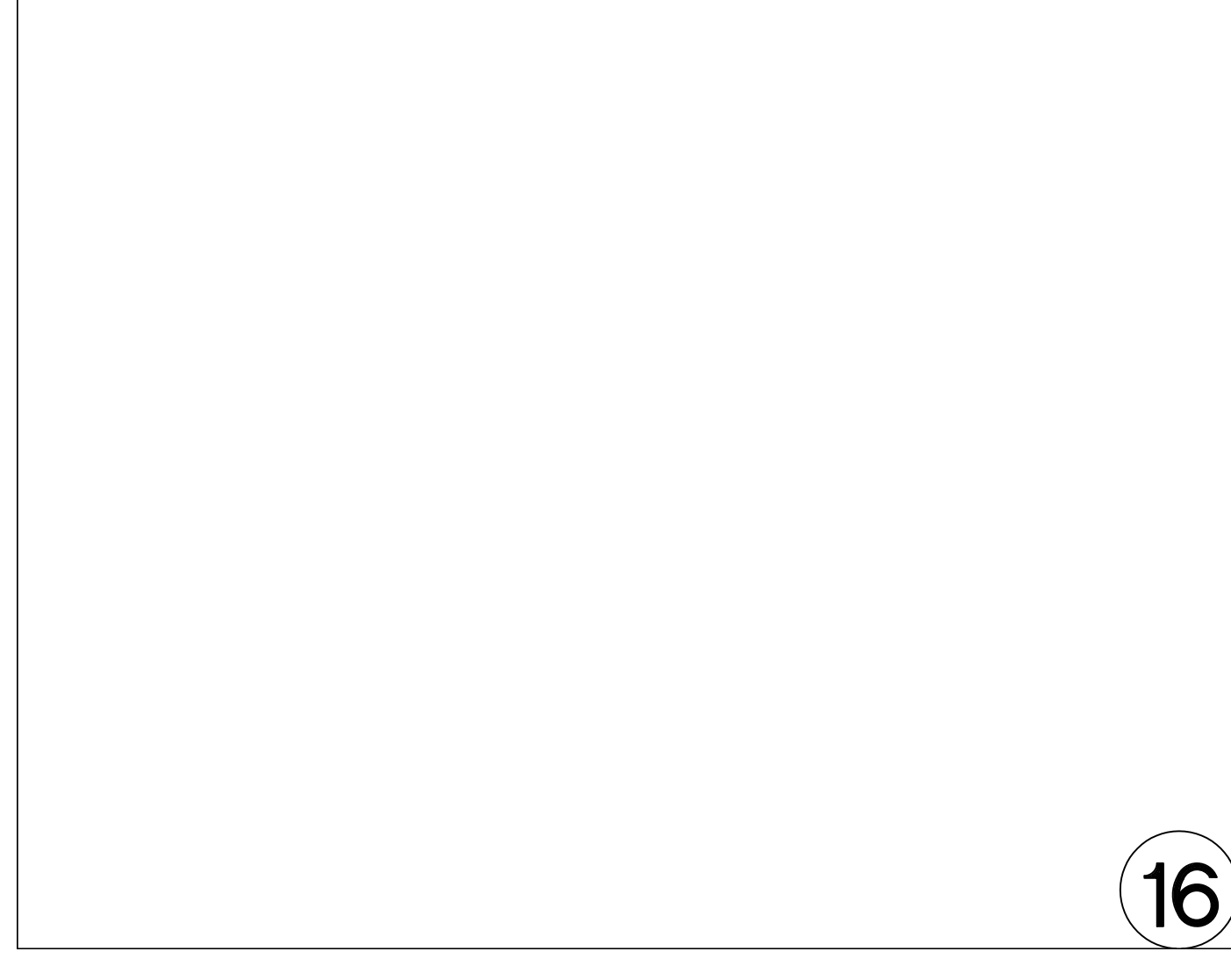
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15



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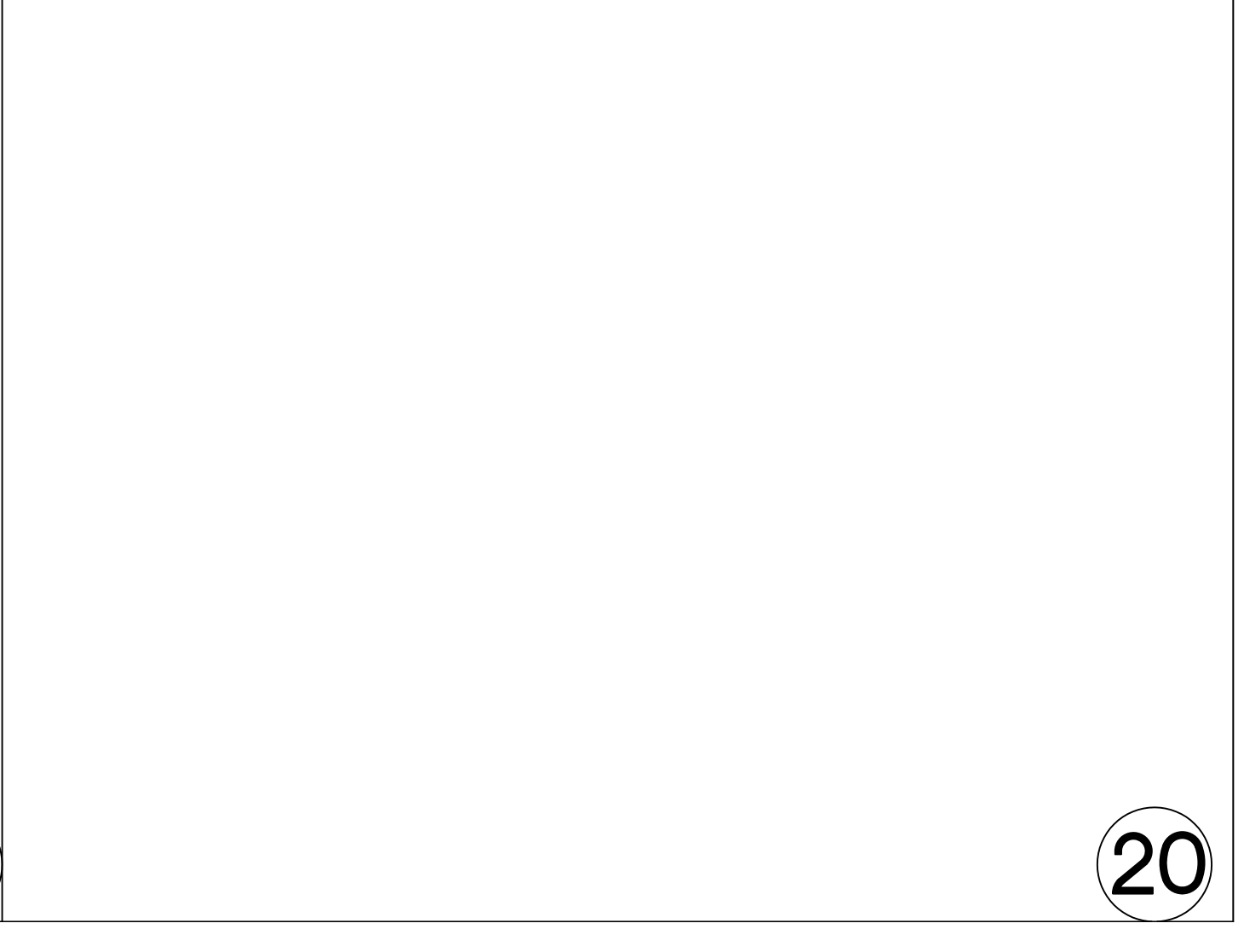
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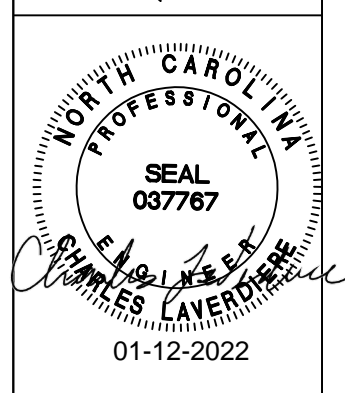


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SHEET	SD1