

Date: July 8, 2021



NC LIC# P-1403

Tower Engineering Professionals  
326 Tryon Road  
Raleigh, NC 27603  
(919) 661-6351

**Subject: Structural Analysis Report**

**Carrier Designation:**

**Sprint PCS Co-Locate**  
**Site Number:**  
**Site Name:**

5RA0884A  
N/A

**Crown Castle Designation:**

**BU Number:**  
**Site Name:**  
**JDE Job Number:**  
**Work Order Number:**  
**Order Number:**

879743  
(P3) Erwin/McLamb (F1)  
631410  
1988834  
540848 Rev. 0

**Engineering Firm Designation:**

**TEP Project Number:** 148377.565498

**Site Data:**

**926 S. 13th Street, Erwin, Harnett County, NC 28339**  
**Latitude 35° 19' 3.65", Longitude -78° 41' 13.67"**  
**250 Foot - Self-Supporting Tower**

Tower Engineering Professionals is pleased to submit this "Structural Analysis Report" to determine the structural integrity of the above-mentioned tower.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC5: Proposed Equipment Configuration

**Sufficient Capacity**

This analysis utilizes an ultimate 3-second gust wind speed of 119 mph as required by the 2018 North Carolina State Building Code. Applicable Standard references and design criteria are listed in Section 2 - Analysis Criteria.

Structural analysis prepared by: Jonathan C. McGinnis, E.I. / BHM

Respectfully submitted by:

Luke Meadows, P.E.



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## 1) INTRODUCTION

This tower is a 250-ft self-supporting tower designed by Rohn. The tower has been modified per reinforcement drawings prepared by GDP Group in January of 2014.

## 2) ANALYSIS CRITERIA

TIA-222 Revision:	TIA-222-H
Risk Category:	II
Wind Speed:	119 mph
Exposure Category:	C
Topographic Factor:	1.0
Ice Thickness:	1.5 in
Wind Speed with Ice:	30 mph
Service Wind Speed:	60 mph

Table 1 - Proposed Equipment Configuration

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
246.0	248.0	3	Ericsson	AIR 32 B2A B66AA_T-MOBILE w/ Mount Pipe	3	1-5/8
		3	Ericsson	AIR6449 B41_T-MOBILE w/ Mount Pipe		
		3	RFS Celwave	APXVAALL24_43-U-NA20_TMO w/ Mount Pipe		
		3	Ericsson	RADIO 4449 B71 B85A_T-MOBILE		
	3	Ericsson	RADIO 4415 B25_TMO			
	246.0	3	Perfect Vision	PV-SFA-12-3-96		

Table 2 - Other Considered Equipment

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
237.0	238.0	6	Commscope	SBNHH-1D65C	3 4 12	3/8 3/4 2-1/4
		3	Cci Antennas	TPA65R-BU8D		
		3	Ericsson	RRUS-11		
		3	Ericsson	RRUS 4478 B14		
		3	Ericsson	RADIO 8843 B2/B66A		
		6	RFS Celwave	ATM192012-0		
	2	Raycap	DC6-48-60-18-8F			
	237.0	1	Tower Mounts	Sector Mount [SM 1303-3]		



### 3) ANALYSIS PROCEDURE

**Table 3 - Documents Provided**

Document	Reference	Source
Geotechnical Report	1575212	CCISites
Tower Foundation Drawings	1486060	CCISites
Tower Manufacturer Drawings	2115835	CCISites
Tower Reinforcement Drawings	4349928	CCISites
Post-Modification Inspection	4967597	CCISites

#### 3.1) Analysis Method

tnxTower (version 8.1.1.0), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A. When applicable, Crown Castle has calculated and provided the effective area for panel antennas using approved methods following the intent of the TIA-222 Standard.

#### 3.2) Assumptions

- 1) The tower and structures were maintained in accordance with the TIA-222 Standard.
- 2) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2, and the referenced drawings.

This analysis may be affected if any assumptions are not valid or have been made in error. Tower Engineering Professionals should be notified to determine the effect on the structural integrity of the tower.

### 4) ANALYSIS RESULTS

**Table 4 - Section Capacity (Summary)**

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (k)	$\phi P_{allow}$ (k)	% Capacity	Pass / Fail
T1	250 - 240	Leg	ROHN 2.5 STD	3	-6.44	60.05	10.7	Pass
T2	240 - 236	Leg	ROHN 2.5 STD	21	-14.31	66.74	21.4	Pass
T3	236 - 232	Leg	ROHN 2.5 STD	30	-22.69	66.74	34.0	Pass
T4	232 - 228	Leg	ROHN 2.5 STD	39	-33.77	66.74	50.6	Pass
T5	228 - 224	Leg	ROHN 2.5 STD	48	-45.19	66.74	67.7	Pass
T6	224 - 220	Leg	ROHN 2.5 STD	57	-56.69	66.74	84.9	Pass
T7	220 - 200	Leg	ROHN 3 EH	66	-92.91	125.02	74.3	Pass
T8	200 - 180	Leg	ROHN 3.5 EH	99	-120.83	148.90	81.1	Pass
T9	180 - 160	Leg	ROHN 4 EH	126	-145.87	167.90	86.9	Pass
T10	160 - 140	Leg	ROHN 5 EH	147	-171.67	251.34	68.3 70.8 (b)	Pass
T11	140 - 120	Leg	ROHN 5 EH	168	-198.64	251.36	79.0	Pass
T12	120 - 100	Leg	ROHN 6 EH	189	-223.13	318.91	70.0	Pass
T13	100 - 80	Leg	ROHN 6 EH	204	-250.86	318.94	78.7	Pass
T14	80 - 60	Leg	ROHN 8 EHS	219	-277.49	405.68	68.4	Pass
T15	60 - 40	Leg	ROHN 8 EHS	234	-305.68	405.73	75.3	Pass
T16	40 - 20	Leg	ROHN 8 EHS	249	-333.60	405.69	82.2	Pass



Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (k)	$\Phi P_{allow}$ (k)	% Capacity	Pass / Fail	
T17	20 - 0	Leg	ROHN 8 EH	264	-341.73	530.68	64.4	Pass	
T1	250 - 240	Diagonal	L1 1/2x1 1/2x3/16	10	-2.42	9.80	24.7 45.8 (b)	Pass	
T2	240 - 236	Diagonal	L1 1/2x1 1/2x3/16	24	-2.71	11.61	23.3 52.4 (b)	Pass	
T3	236 - 232	Diagonal	L1 1/2x1 1/2x3/16	33	-4.68	11.89	39.4 65.1 (b) <sup>3</sup>	Pass	
T4	232 - 228	Diagonal	L1 1/2x1 1/2x3/16	43	-4.95	11.89	41.7 67.6 (b) <sup>3</sup>	Pass	
T5	228 - 224	Diagonal	L1 1/2x1 1/2x3/16	54	-5.25	11.89	44.2 72.8 (b) <sup>3</sup>	Pass	
T6	224 - 220	Diagonal	L1 1/2x1 1/2x3/16	62	-5.80	11.89	48.8 77.4 (b) <sup>3</sup>	Pass	
T7	220 - 200	Diagonal	L1 1/2x1 1/2x3/16	72	-3.67	7.19	51.1 68.7 (b)	Pass	
T8	200 - 180	Diagonal	L1 3/4x1 3/4x3/16	105	-4.43	6.84	64.8 70.2 (b)	Pass	
T9	180 - 160	Diagonal	L2 1/2x2 1/2x3/16	132	-5.61	12.66	44.3 83.7 (b)	Pass	
T10	160 - 140	Diagonal	L2 1/2x2 1/2x3/16	153	-6.05	9.68	62.5 94.6 (b)	Pass	
T11	140 - 120	Diagonal	L2 1/2x2 1/2x1/4	174	-7.05	9.83	71.7 94.5 (b)	Pass	
T12	120 - 100	Diagonal	L3x3x1/4	195	-8.34	11.48	72.7	Pass	
T13	100 - 80	Diagonal	L3 1/2x3 1/2x1/4	210	-9.81	15.97	61.4 78.3 (b)	Pass	
T14	80 - 60	Diagonal	L3 1/2x3 1/2x1/4	225	-9.47	13.53	70.0	Pass	
T15	60 - 40	Diagonal	L4x4x1/4	240	-12.14	17.55	69.2 78.5 (b)	Pass	
T16	40 - 20	Diagonal	L4x4x1/4	255	-11.87	14.95	79.4	Pass	
T17	20 - 0	Diagonal	ROHN 2.5 STD	285	-16.89	16.97	99.5	Pass	
T17	20 - 0	Horizontal	ROHN 2.5 STD	281	-9.59	16.79	57.1	Pass	
T1	250 - 240	Top Girt	L1 1/2x1 1/2x3/16	5	-0.41	5.55	7.5 8.8 (b)	Pass	
T17	20 - 0	Redund Horz 1 Bracing	ROHN 1.5 STD	267	-1.50	14.44	10.4	Pass	
T17	20 - 0	Redund Diag 1 Bracing	ROHN 1.5 STD	284	-0.76	4.40	17.4	Pass	
T17	20 - 0	Redund Hip 1 Bracing	ROHN 1.5 STD	288	-0.05	12.84	0.4	Pass	
T17	20 - 0	Redund Hip Diagonal 1 Bracing	ROHN 2.5 STD	289	-0.07	11.05	0.7	Pass	
T17	20 - 0	Inner Bracing	ROHN 3 STD	294	-0.01	31.26	0.3	Pass	
							Summary		
							Leg (T9)	86.9	Pass
							Diagonal (T17)	99.5	Pass
							Horizontal (T17)	57.1	Pass
							Top Girt (T1)	8.8	Pass
							Redund Horz 1 Bracing (T17)	10.4	Pass

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (k)	$\Phi P_{allow}$ (k)	% Capacity	Pass / Fail
						Redund Diag 1 Bracing (T17)	17.4	Pass
						Redund Hip 1 Bracing (T17)	0.4	Pass
						Redund Hip Diagonal 1 Bracing (T17)	0.7	Pass
						Inner Bracing (T17)	0.3	Pass
						Bolt Checks	90.1	Pass
						<b>RATING =</b>	<b>99.5</b>	<b>Pass</b>

**Table 5 - Tower Component Stresses vs. Capacity - LC5**

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1,2	Anchor Rods	-	51.6	Pass
1,2	Base Foundation Structural	-	24.9	Pass
1,2	Base Foundation Soil Interaction	-	50.1	Pass

<b>Structure Rating (max from all components) =</b>	<b>99.5%</b>
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Notes:

- 1) See additional documentation in "Appendix C - Additional Calculations" for calculations supporting the % capacity listed.
- 2) Rating per TIA-222-H Section 15.5
- 3) Connection capacity evaluated in separate TNX analysis to account for existing modification.

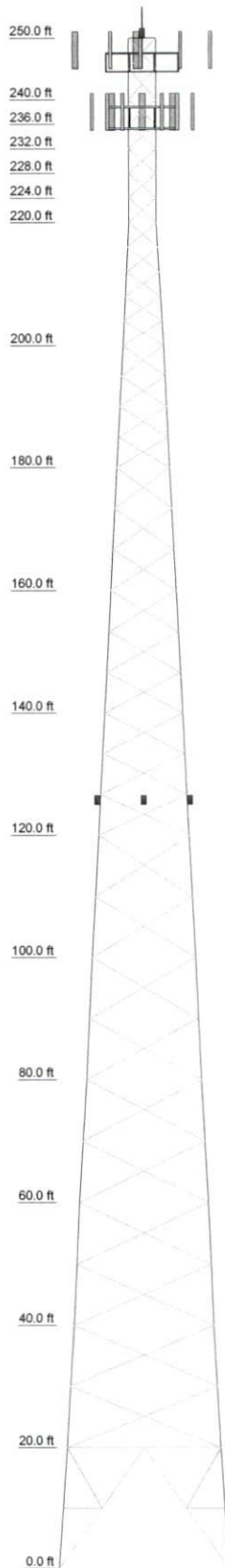
**4.1) Recommendations**

- 1) The tower and its foundation have sufficient capacity to carry the proposed load configuration. No modifications are required at this time.

**APPENDIX A**  
**TNXTOWER OUTPUT**



Section	T17	T16	T14	T13	T12	T11	T10	T9	T8	T7	T6	T5	T4	T3	T2	T1
Legs	ROHN 8 EH	ROHN 8 EHS			ROHN 6 EH	A572-50	ROHN 5 EH	ROHN 4 EH	ROHN 3.5 EH	ROHN 3 EH						
Leg Grade	ROHN 2.5 STD	L4x4x1/4	L3 1/2x3 1/2x1/4	L3x3x1/4	L2 1/2x2 1/2x1/4	L2 1/2x2 1/2x3/16	L2 1/2x2 1/2x3/16	L1 3/4x1 3/4x3/16	L1 1/2x1 1/2x3/16	L1 1/2x1 1/2x3/16						
Diagonals	ROHN 2.5 STD															
Diagonal Grade																
Top Chords																
Horizontals	ROHN 2.5 STD															
Red. Horizontals	ROHN 1.5 STD															
Red. Diagonals	ROHN 1.5 STD															
Red. Hips	ROHN 1.5 STD															
Inner Bracing	ROHN 3 STD															
Face Width (ft)	27.7995	23.04	18.88	16.92	14.83	12.82	10.7	8.63	6.63							
# Panels @ (ft)	1 @ 20		10 @ 10				9 @ 6.66667		4 @ 5	10 @ 4						2 @ 5
Weight (K)	33.0	4.3	3.7	3.3	3.0	2.5	2.1	1.6	1.3	1.0						0.4



**SYMBOL LIST**

MARK	SIZE	MARK	SIZE
A	L1 1/2x1 1/2x3/16		

**MATERIAL STRENGTH**

GRADE	Fy	Fu	GRADE	Fy	Fu
A572-50	50 ksi	65 ksi	A36	36 ksi	58 ksi

**TOWER DESIGN NOTES**

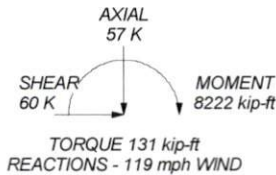
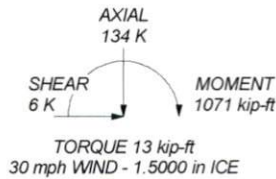
1. Tower is located in Harnett County, North Carolina.
2. Tower designed for Exposure C to the TIA-222-H Standard.
3. Tower designed for a 119 mph basic wind in accordance with the TIA-222-H Standard.
4. Tower is also designed for a 30 mph basic wind with 1.50 in ice. Ice is considered to increase in thickness with height.
5. Deflections are based upon a 60 mph wind.
6. Tower Risk Category II.
7. Topographic Category 1 with Crest Height of 0.00 ft
8. TOWER RATING: 99.5%


ALL REACTIONS ARE FACTORED

MAX. CORNER REACTIONS AT BASE:

DOWN: 361 K  
SHEAR: 38 K

UPLIFT: -308 K  
SHEAR: 33 K



 Tower Engineering Professionals	<b>Tower Engineering Professionals</b>		Job: <b>(P3) Erwin/McLamb (F1) (BU: 879743)</b>		
	326 Tryon Rd. Raleigh, NC 27603		Project: <b>TEP No. 148377.565498</b>		
	Phone: (919) 661-6351		Client: <b>Crown Castle</b>	Drawn by: <b>jcmcginnis</b>	App'd:
	FAX: (919) 661-6350		Code: <b>TIA-222-H</b>	Date: <b>07/08/21</b>	Scale: <b>NTS</b>
			Path:		Dwg No. <b>E-1</b>

<b>tnxTower</b>  <b>Tower Engineering Professionals</b> 326 Tryon Rd. Raleigh, NC 27603 Phone: (919) 661-6351 FAX: (919) 661-6350	<b>Job</b> (P3) Erwin/McLamb (F1) (BU: 879743)	<b>Page</b> 1 of 32
	<b>Project</b> TEP No. 148377.565498	<b>Date</b> 16:58:28 07/08/21
	<b>Client</b> Crown Castle	<b>Designed by</b> jcmcginnis

## Tower Input Data

The main tower is a 3x free standing tower with an overall height of 250.00 ft above the ground line.

The base of the tower is set at an elevation of 0.00 ft above the ground line.

The face width of the tower is 4.58 ft at the top and 27.80 ft at the base.

This tower is designed using the TIA-222-H standard.

The following design criteria apply:

Tower is located in Harnett County, North Carolina.

Tower base elevation above sea level: 150.00 ft.

Basic wind speed of 119 mph.

Risk Category II.

Exposure Category C.

Simplified Topographic Factor Procedure for wind speed-up calculations is used.

Topographic Category: 1.

Crest Height: 0.00 ft.

Nominal ice thickness of 1.5000 in.

Ice thickness is considered to increase with height.

Ice density of 56 pcf.

A wind speed of 30 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 60 mph.

A non-linear (P-delta) analysis was used.

Pressures are calculated at each section.

Stress ratio used in tower member design is 1.

Tower analysis based on target reliabilities in accordance with Annex S.

Load Modification Factors used:  $K_{es}(F_w) = 0.95$ ,  $K_{es}(t_i) = 0.85$ .

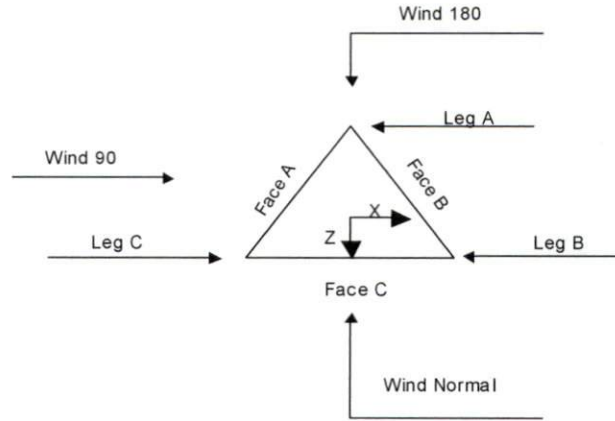
Maximum demand-capacity ratio is: 1.05.

Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

## Options

<ul style="list-style-type: none"> <li>Consider Moments - Legs</li> <li>Consider Moments - Horizontals</li> <li>Consider Moments - Diagonals</li> <li>Use Moment Magnification</li> <li>√ Use Code Stress Ratios</li> <li>√ Use Code Safety Factors - Guys</li> <li>Escalate Ice</li> <li>Always Use Max Kz</li> <li>Use Special Wind Profile</li> <li>√ Include Bolts In Member Capacity</li> <li>Leg Bolts Are At Top Of Section</li> <li>√ Secondary Horizontal Braces Leg</li> <li>Use Diamond Inner Bracing (4 Sided)</li> <li>√ SR Members Have Cut Ends</li> <li>√ SR Members Are Concentric</li> </ul>	<ul style="list-style-type: none"> <li>Distribute Leg Loads As Uniform</li> <li>Assume Legs Pinned</li> <li>√ Assume Rigid Index Plate</li> <li>√ Use Clear Spans For Wind Area</li> <li>√ Use Clear Spans For KL/r</li> <li>Retension Guys To Initial Tension</li> <li>√ Bypass Mast Stability Checks</li> <li>√ Use Azimuth Dish Coefficients</li> <li>√ Project Wind Area of Appurt.</li> <li>Autocalc Torque Arm Areas</li> <li>Add IBC .6D+W Combination</li> <li>√ Sort Capacity Reports By Component</li> <li>Triangulate Diamond Inner Bracing</li> <li>Treat Feed Line Bundles As Cylinder</li> <li>Ignore KL/ry For 60 Deg. Angle Legs</li> </ul>	<ul style="list-style-type: none"> <li>Use ASCE 10 X-Brace Ly Rules</li> <li>Calculate Redundant Bracing Forces</li> <li>Ignore Redundant Members in FEA</li> <li>√ SR Leg Bolts Resist Compression</li> <li>All Leg Panels Have Same Allowable</li> <li>Offset Girt At Foundation</li> <li>√ Consider Feed Line Torque</li> <li>√ Include Angle Block Shear Check</li> <li>Use TIA-222-H Bracing Resist. Exemption</li> <li>Use TIA-222-H Tension Splice Exemption</li> <li style="text-align: center;">Poles</li> <li>Include Shear-Torsion Interaction</li> <li>Always Use Sub-Critical Flow</li> <li>Use Top Mounted Sockets</li> <li>Pole Without Linear Attachments</li> <li>Pole With Shroud Or No Appurtenances</li> <li>Outside and Inside Corner Radii Are Known</li> </ul>
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<b>tnxTower</b>  <b>Tower Engineering Professionals</b> 326 Tryon Rd. Raleigh, NC 27603 Phone: (919) 661-6351 FAX: (919) 661-6350	<b>Job</b> (P3) Erwin/McLamb (F1) (BU: 879743)	<b>Page</b> 2 of 32
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	<b>Client</b> Crown Castle	<b>Designed by</b> jcmcginnis



**Triangular Tower**

**Tower Section Geometry**

<i>Tower Section</i>	<i>Tower Elevation</i>	<i>Assembly Database</i>	<i>Description</i>	<i>Section Width</i>	<i>Number of Sections</i>	<i>Section Length</i>
	<i>ft</i>			<i>ft</i>		<i>ft</i>
T1	250.00-240.00			4.58	1	10.00
T2	240.00-236.00			4.58	1	4.00
T3	236.00-232.00			4.58	1	4.00
T4	232.00-228.00			4.58	1	4.00
T5	228.00-224.00			4.58	1	4.00
T6	224.00-220.00			4.58	1	4.00
T7	220.00-200.00			4.58	1	20.00
T8	200.00-180.00			6.63	1	20.00
T9	180.00-160.00			8.63	1	20.00
T10	160.00-140.00			10.70	1	20.00
T11	140.00-120.00			12.82	1	20.00
T12	120.00-100.00			14.83	1	20.00
T13	100.00-80.00			16.92	1	20.00
T14	80.00-60.00			18.88	1	20.00
T15	60.00-40.00			21.11	1	20.00
T16	40.00-20.00			23.04	1	20.00
T17	20.00-0.00			25.22	1	20.00

**Tower Section Geometry (cont'd)**



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	<b>Client</b> Crown Castle	<b>Designed by</b> jcmcginnis

Tower Section	Tower Elevation	Diagonal Spacing	Bracing Type	Has K Brace End Panels	Has Horizontals	Top Girt Offset	Bottom Girt Offset
	ft	ft				in	in
T1	250.00-240.00	5.00	X Brace	No	No	0.0000	0.0000
T2	240.00-236.00	4.00	X Brace	No	No	0.0000	0.0000
T3	236.00-232.00	4.00	X Brace	No	No	0.0000	0.0000
T4	232.00-228.00	4.00	X Brace	No	No	0.0000	0.0000
T5	228.00-224.00	4.00	X Brace	No	No	0.0000	0.0000
T6	224.00-220.00	4.00	X Brace	No	No	0.0000	0.0000
T7	220.00-200.00	4.00	X Brace	No	No	0.0000	0.0000
T8	200.00-180.00	5.00	X Brace	No	No	0.0000	0.0000
T9	180.00-160.00	6.67	X Brace	No	No	0.0000	0.0000
T10	160.00-140.00	6.67	X Brace	No	No	0.0000	0.0000
T11	140.00-120.00	6.67	X Brace	No	No	0.0000	0.0000
T12	120.00-100.00	10.00	X Brace	No	No	0.0000	0.0000
T13	100.00-80.00	10.00	X Brace	No	No	0.0000	0.0000
T14	80.00-60.00	10.00	X Brace	No	No	0.0000	0.0000
T15	60.00-40.00	10.00	X Brace	No	No	0.0000	0.0000
T16	40.00-20.00	10.00	X Brace	No	No	0.0000	0.0000
T17	20.00-0.00	20.00	K1 Down	No	Yes	0.0000	0.0000

### Tower Section Geometry (cont'd)

Tower Elevation	Leg Type	Leg Size	Leg Grade	Diagonal Type	Diagonal Size	Diagonal Grade
ft						
T1 250.00-240.00	Pipe	ROHN 2.5 STD	A572-50 (50 ksi)	Equal Angle	L1 1/2x1 1/2x3/16	A36 (36 ksi)
T2 240.00-236.00	Pipe	ROHN 2.5 STD	A572-50 (50 ksi)	Equal Angle	L1 1/2x1 1/2x3/16	A36 (36 ksi)
T3 236.00-232.00	Pipe	ROHN 2.5 STD	A572-50 (50 ksi)	Equal Angle	L1 1/2x1 1/2x3/16	A36 (36 ksi)
T4 232.00-228.00	Pipe	ROHN 2.5 STD	A572-50 (50 ksi)	Equal Angle	L1 1/2x1 1/2x3/16	A36 (36 ksi)
T5 228.00-224.00	Pipe	ROHN 2.5 STD	A572-50 (50 ksi)	Equal Angle	L1 1/2x1 1/2x3/16	A36 (36 ksi)
T6 224.00-220.00	Pipe	ROHN 2.5 STD	A572-50 (50 ksi)	Equal Angle	L1 1/2x1 1/2x3/16	A36 (36 ksi)
T7 220.00-200.00	Pipe	ROHN 3 EH	A572-50 (50 ksi)	Equal Angle	L1 1/2x1 1/2x3/16	A36 (36 ksi)
T8 200.00-180.00	Pipe	ROHN 3.5 EH	A572-50 (50 ksi)	Equal Angle	L1 3/4x1 3/4x3/16	A36 (36 ksi)
T9 180.00-160.00	Pipe	ROHN 4 EH	A572-50 (50 ksi)	Equal Angle	L2 1/2x2 1/2x3/16	A36 (36 ksi)
T10 160.00-140.00	Pipe	ROHN 5 EH	A572-50 (50 ksi)	Equal Angle	L2 1/2x2 1/2x3/16	A36 (36 ksi)
T11 140.00-120.00	Pipe	ROHN 5 EH	A572-50 (50 ksi)	Equal Angle	L2 1/2x2 1/2x1/4	A36 (36 ksi)
T12 120.00-100.00	Pipe	ROHN 6 EH	A572-50 (50 ksi)	Equal Angle	L3x3x1/4	A572-50 (50 ksi)
T13 100.00-80.00	Pipe	ROHN 6 EH	A572-50 (50 ksi)	Equal Angle	L3 1/2x3 1/2x1/4	A572-50 (50 ksi)
T14 80.00-60.00	Pipe	ROHN 8 EHS	A572-50 (50 ksi)	Equal Angle	L3 1/2x3 1/2x1/4	A572-50 (50 ksi)
T15 60.00-40.00	Pipe	ROHN 8 EHS	A572-50 (50 ksi)	Equal Angle	L4x4x1/4	A572-50 (50 ksi)
T16 40.00-20.00	Pipe	ROHN 8 EHS	A572-50 (50 ksi)	Equal Angle	L4x4x1/4	A572-50 (50 ksi)
T17 20.00-0.00	Pipe	ROHN 8 EH	A572-50	Pipe	ROHN 2.5 STD	A572-50

<b>tnxTower</b>  <b>Tower Engineering Professionals</b> 326 Tryon Rd. Raleigh, NC 27603 Phone: (919) 661-6351 FAX: (919) 661-6350	<b>Job</b> (P3) Erwin/McLamb (F1) (BU: 879743)	<b>Page</b> 4 of 32
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	<b>Client</b> Crown Castle	<b>Designed by</b> jcmcginnis

Tower Elevation ft	Leg Type	Leg Size	Leg Grade	Diagonal Type	Diagonal Size	Diagonal Grade
			(50 ksi)			(50 ksi)

### Tower Section Geometry (cont'd)

Tower Elevation ft	Top Girt Type	Top Girt Size	Top Girt Grade	Bottom Girt Type	Bottom Girt Size	Bottom Girt Grade
T1 250.00-240.00	Equal Angle	L1 1/2x1 1/2x3/16	A36 (36 ksi)	Solid Round		A36 (36 ksi)

### Tower Section Geometry (cont'd)

Tower Elevation ft	No. of Mid Girts	Mid Girt Type	Mid Girt Size	Mid Girt Grade	Horizontal Type	Horizontal Size	Horizontal Grade
T17 20.00-0.00	None	Flat Bar		A36 (36 ksi)	Pipe	ROHN 2.5 STD	A572-50 (50 ksi)

### Tower Section Geometry (cont'd)

Tower Elevation ft	Secondary Horizontal Type	Secondary Horizontal Size	Secondary Horizontal Grade	Inner Bracing Type	Inner Bracing Size	Inner Bracing Grade
T17 20.00-0.00	Solid Round		A572-50 (50 ksi)	Pipe	ROHN 3 STD	A572-50 (50 ksi)

### Tower Section Geometry (cont'd)

Tower Elevation ft	Redundant Bracing Grade	Redundant Type	Redundant Size	K Factor	
T17 20.00-0.00	A572-50 (50 ksi)	Horizontal (1)	Pipe	ROHN 1.5 STD	1
		Diagonal (1)	Pipe	ROHN 1.5 STD	1
		Hip (1)	Pipe	ROHN 1.5 STD	1
		Hip Diagonal (1)	Pipe	ROHN 2.5 STD	1

### Tower Section Geometry (cont'd)











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Tower Elevation ft	Redundant Horizontal		Redundant Diagonal		Redundant Sub-Diagonal		Redundant Sub-Horizontal		Redundant Vertical		Redundant Hip		Redundant Hip Diagonal	
	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U
T17 20.00-0.00	0.0000	1	0.0000	1	0.0000	1	0.0000	1	0.0000	1	0.0000	1	0.0000	1

### Tower Section Geometry (cont'd)

Tower Elevation ft	Leg Connection Type	Leg		Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
		Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.
T1	Flange	0.6250	4	0.5000	1	0.5000	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0
250.00-240.00		A325N		A325X		A325X		A325X		A325N		A325X		A325N	
T2	Flange	0.7500	0	0.5000	1	0.6250	0	0.0000	0	0.6250	0	0.6250	0	0.6250	0
240.00-236.00		A325N		A325X		A325X		A325X		A325N		A325X		A325N	
T3	Flange	0.7500	0	0.5000	2	0.6250	0	0.0000	0	0.6250	0	0.6250	0	0.6250	0
236.00-232.00		A325N		A325X		A325X		A325X		A325N		A325X		A325N	
T4	Flange	0.7500	0	0.5000	2	0.6250	0	0.0000	0	0.6250	0	0.6250	0	0.6250	0
232.00-228.00		A325N		A325X		A325X		A325X		A325N		A325X		A325N	
T5	Flange	0.7500	0	0.5000	2	0.6250	0	0.0000	0	0.6250	0	0.6250	0	0.6250	0
228.00-224.00		A325N		A325X		A325X		A325X		A325N		A325X		A325N	
T6	Flange	0.7500	4	0.5000	2	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
224.00-220.00		A325N		A325X		A325X		A325X		A325N		A325X		A325N	
T7	Flange	0.8750	4	0.5000	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
220.00-200.00		A325N		A325X		A325X		A325X		A325N		A325X		A325N	
T8	Flange	0.8750	4	0.5000	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
200.00-180.00		A325N		A325X		A325X		A325X		A325N		A325X		A325N	
T9	Flange	1.0000	4	0.5000	1	0.6250	0	0.6250	0	0.6250	0	0.6250	1	0.6250	0
180.00-160.00		A325N		A325X		A325X		A325X		A325N		A325X		A325N	
T10	Flange	1.0000	4	0.5000	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
160.00-140.00		A325N		A325X		A325X		A325X		A325N		A325X		A325N	
T11	Flange	1.0000	6	0.5000	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
140.00-120.00		A325N		A325X		A325X		A325X		A325N		A325X		A325N	
T12	Flange	1.0000	6	0.6250	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
120.00-100.00		A325N		A325X		A325X		A325X		A325N		A325X		A325N	
T13	Flange	1.0000	6	0.6250	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
100.00-80.00		A325N		A325X		A325X		A325X		A325N		A325X		A325N	
T14	Flange	1.0000	8	0.7500	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
80.00-60.00		A325N		A325X		A325X		A325X		A325N		A325X		A325N	
T15	Flange	1.0000	8	0.7500	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
60.00-40.00		A325N		A325X		A325X		A325X		A325N		A325X		A325N	
T16	Flange	1.0000	8	0.7500	1	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
40.00-20.00		A325N		A325X		A325X		A325X		A325N		A325X		A325N	
T17 20.00-0.00	Flange	1.0000	0	0.7500	3	0.6250	0	0.6250	0	0.6250	0	0.7500	2	0.6250	0
		A354-BC		A325X		A325X		A325X		A325N		A325X		A325N	

**Feed Line/Linear Appurtenances - Entered As Round Or Flat**



<b>tnxTower</b>  <b>Tower Engineering Professionals</b> 326 Tryon Rd. Raleigh, NC 27603 Phone: (919) 661-6351 FAX: (919) 661-6350	<b>Job</b> (P3) Erwin/McLamb (F1) (BU: 879743)	<b>Page</b> 9 of 32
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Description	Face or Leg	Allow or Shield	Exclude From Torque Calculation	Component Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	#	# Per Row	Clear Spacing in	Width or Diameter in	Perimeter in	Weight plf
<b>** Safety Line</b>													
Safety Line 3/8	B	No	No	Ar (CaAa)	250.00 - 0.00	0.0000	0.5	1	1	0.3750	0.3750		0.22
FSJ2-50(3/8")	B	No	No	Ar (CaAa)	250.00 - 0.00	-3.0000	0.48	1	1	0.4250	0.4250		0.08
<b>** Step Pegs</b>													
Step Pegs (5/8" SR) 7-in. w/30" step	A	No	No	Ar (CaAa)	120.00 - 0.00	0.0000	0.5	1	1	0.3500	0.3500		0.49
Step Pegs (5/8" SR) 7-in. w/30" step	B	No	No	Ar (CaAa)	250.00 - 0.00	0.0000	0.5	1	1	0.3500	0.3500		0.49
Step Pegs (5/8" SR) 7-in. w/30" step	C	No	No	Ar (CaAa)	120.00 - 0.00	0.0000	0.5	1	1	0.3500	0.3500		0.49
<b>** Feedline Ladder **</b>													
Feedline Ladder (Af)	A	No	No	Af (CaAa)	237.00 - 0.00	0.0000	0.325	1	1	3.0000	3.0000		8.40
Feedline Ladder (Af)	B	No	No	Af (CaAa)	246.00 - 0.00	0.0000	-0.325	1	1	3.0000	3.0000		8.40
<b>**237**</b>													
LDF12-50A(2 -1/4")	A	No	No	Ar (CaAa)	237.00 - 0.00	0.0000	0.325	12	6	0.5000	2.3500		1.22
WR-VG86ST-BRD( 3/4) 860	A	No	No	Ar (CaAa)	237.00 - 0.00	0.0000	0.285	4	2	0.5000	0.7740		0.59
10014(3/8)	A	No	No	Ar (CaAa)	237.00 - 0.00	2.0000	0.35	1	1	0.3750	0.3750		0.08
FB-L98B-034-XXXXXX( 3/8")	A	No	No	Ar (CaAa)	237.00 - 0.00	0.0000	0.265	2	1	0.3937	0.3937		0.05
<b>**246**</b>													
FXL-1873( 1 5/8)	B	No	No	Ar (CaAa)	246.00 - 0.00	0.0000	-0.35	3	3	1.9800	1.9800		0.67
<b>**250**</b>													
FSJ4-50B(1/2" )	B	No	No	Ar (CaAa)	250.00 - 0.00	0.0000	-0.325	1	1	0.5200	0.5200		0.14
<b>*****</b>													

### Feed Line/Linear Appurtenances - Entered As Area

Description	Face or Leg	Allow or Shield	Exclude From Torque Calculation	Component Type	Placement ft	Total Number	C <sub>A,A</sub> ft <sup>2</sup> /ft	Weight plf
<b>*****</b>								

### Feed Line/Linear Appurtenances Section Areas

<b>tnxTower</b>  <b>Tower Engineering Professionals</b> 326 Tryon Rd. Raleigh, NC 27603 Phone: (919) 661-6351 FAX: (919) 661-6350	<b>Job</b> (P3) Erwin/McLamb (F1) (BU: 879743)	<b>Page</b> 10 of 32
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	<b>Client</b> Crown Castle	<b>Designed by</b> jcmcginnis

Tower Section	Tower Elevation ft	Face	$A_R$ ft <sup>2</sup>	$A_F$ ft <sup>2</sup>	$C_A A_A$ In Face ft <sup>2</sup>	$C_A A_A$ Out Face ft <sup>2</sup>	Weight K
T1	250.00-240.00	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	8.234	0.000	0.07
		C	0.000	0.000	0.000	0.000	0.00
T2	240.00-236.00	A	0.000	0.000	3.746	0.000	0.03
		B	0.000	0.000	5.044	0.000	0.05
		C	0.000	0.000	0.000	0.000	0.00
T3	236.00-232.00	A	0.000	0.000	14.983	0.000	0.10
		B	0.000	0.000	5.044	0.000	0.05
		C	0.000	0.000	0.000	0.000	0.00
T4	232.00-228.00	A	0.000	0.000	14.983	0.000	0.10
		B	0.000	0.000	5.044	0.000	0.05
		C	0.000	0.000	0.000	0.000	0.00
T5	228.00-224.00	A	0.000	0.000	14.983	0.000	0.10
		B	0.000	0.000	5.044	0.000	0.05
		C	0.000	0.000	0.000	0.000	0.00
T6	224.00-220.00	A	0.000	0.000	14.983	0.000	0.10
		B	0.000	0.000	5.044	0.000	0.05
		C	0.000	0.000	0.000	0.000	0.00
T7	220.00-200.00	A	0.000	0.000	74.917	0.000	0.51
		B	0.000	0.000	25.220	0.000	0.23
		C	0.000	0.000	0.000	0.000	0.00
T8	200.00-180.00	A	0.000	0.000	74.917	0.000	0.51
		B	0.000	0.000	25.220	0.000	0.23
		C	0.000	0.000	0.000	0.000	0.00
T9	180.00-160.00	A	0.000	0.000	74.917	0.000	0.51
		B	0.000	0.000	25.220	0.000	0.23
		C	0.000	0.000	0.000	0.000	0.00
T10	160.00-140.00	A	0.000	0.000	74.917	0.000	0.51
		B	0.000	0.000	25.220	0.000	0.23
		C	0.000	0.000	0.000	0.000	0.00
T11	140.00-120.00	A	0.000	0.000	74.917	0.000	0.51
		B	0.000	0.000	25.220	0.000	0.23
		C	0.000	0.000	0.000	0.000	0.00
T12	120.00-100.00	A	0.000	0.000	75.617	0.000	0.52
		B	0.000	0.000	25.220	0.000	0.23
		C	0.000	0.000	0.700	0.000	0.01
T13	100.00-80.00	A	0.000	0.000	75.617	0.000	0.52
		B	0.000	0.000	25.220	0.000	0.23
		C	0.000	0.000	0.700	0.000	0.01
T14	80.00-60.00	A	0.000	0.000	75.617	0.000	0.52
		B	0.000	0.000	25.220	0.000	0.23
		C	0.000	0.000	0.700	0.000	0.01
T15	60.00-40.00	A	0.000	0.000	75.617	0.000	0.52
		B	0.000	0.000	25.220	0.000	0.23
		C	0.000	0.000	0.700	0.000	0.01
T16	40.00-20.00	A	0.000	0.000	75.617	0.000	0.52
		B	0.000	0.000	25.220	0.000	0.23
		C	0.000	0.000	0.700	0.000	0.01
T17	20.00-0.00	A	0.000	0.000	75.617	0.000	0.52
		B	0.000	0.000	25.220	0.000	0.23
		C	0.000	0.000	0.700	0.000	0.01

**Feed Line/Linear Appurtenances Section Areas - With Ice**

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	$A_R$ ft <sup>2</sup>	$A_F$ ft <sup>2</sup>	$C_A A_A$ In Face ft <sup>2</sup>	$C_A A_A$ Out Face ft <sup>2</sup>	Weight K
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<b>tnxTower</b>  <b>Tower Engineering Professionals</b> 326 Tryon Rd. Raleigh, NC 27603 Phone: (919) 661-6351 FAX: (919) 661-6350	<b>Job</b> (P3) Erwin/McLamb (F1) (BU: 879743)	<b>Page</b> 11 of 32
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Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	$A_R$ ft <sup>2</sup>	$A_F$ ft <sup>2</sup>	$C_i A_A$ In Face ft <sup>2</sup>	$C_i A_A$ Out Face ft <sup>2</sup>	Weight K
T1	250.00-240.00	A	1.558	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	29.487	0.000	0.41
		C		0.000	0.000	0.000	0.000	0.00
T2	240.00-236.00	A	1.554	0.000	0.000	5.435	0.000	0.10
		B		0.000	0.000	15.865	0.000	0.23
		C		0.000	0.000	0.000	0.000	0.00
T3	236.00-232.00	A	1.551	0.000	0.000	21.727	0.000	0.39
		B		0.000	0.000	15.851	0.000	0.23
		C		0.000	0.000	0.000	0.000	0.00
T4	232.00-228.00	A	1.548	0.000	0.000	21.711	0.000	0.39
		B		0.000	0.000	15.837	0.000	0.23
		C		0.000	0.000	0.000	0.000	0.00
T5	228.00-224.00	A	1.546	0.000	0.000	21.696	0.000	0.39
		B		0.000	0.000	15.823	0.000	0.23
		C		0.000	0.000	0.000	0.000	0.00
T6	224.00-220.00	A	1.543	0.000	0.000	21.680	0.000	0.38
		B		0.000	0.000	15.808	0.000	0.23
		C		0.000	0.000	0.000	0.000	0.00
T7	220.00-200.00	A	1.534	0.000	0.000	108.155	0.000	1.92
		B		0.000	0.000	78.815	0.000	1.13
		C		0.000	0.000	0.000	0.000	0.00
T8	200.00-180.00	A	1.519	0.000	0.000	107.717	0.000	1.90
		B		0.000	0.000	78.410	0.000	1.11
		C		0.000	0.000	0.000	0.000	0.00
T9	180.00-160.00	A	1.502	0.000	0.000	107.236	0.000	1.88
		B		0.000	0.000	77.966	0.000	1.10
		C		0.000	0.000	0.000	0.000	0.00
T10	160.00-140.00	A	1.483	0.000	0.000	106.701	0.000	1.87
		B		0.000	0.000	77.471	0.000	1.09
		C		0.000	0.000	0.000	0.000	0.00
T11	140.00-120.00	A	1.462	0.000	0.000	106.097	0.000	1.85
		B		0.000	0.000	76.914	0.000	1.07
		C		0.000	0.000	0.000	0.000	0.00
T12	120.00-100.00	A	1.438	0.000	0.000	111.856	0.000	1.90
		B		0.000	0.000	76.273	0.000	1.05
		C		0.000	0.000	6.453	0.000	0.07
T13	100.00-80.00	A	1.410	0.000	0.000	110.924	0.000	1.87
		B		0.000	0.000	75.517	0.000	1.03
		C		0.000	0.000	6.338	0.000	0.07
T14	80.00-60.00	A	1.375	0.000	0.000	109.782	0.000	1.83
		B		0.000	0.000	74.592	0.000	1.01
		C		0.000	0.000	6.198	0.000	0.07
T15	60.00-40.00	A	1.329	0.000	0.000	108.299	0.000	1.79
		B		0.000	0.000	73.390	0.000	0.97
		C		0.000	0.000	6.016	0.000	0.06
T16	40.00-20.00	A	1.263	0.000	0.000	106.140	0.000	1.72
		B		0.000	0.000	71.642	0.000	0.93
		C		0.000	0.000	5.752	0.000	0.06
T17	20.00-0.00	A	1.132	0.000	0.000	101.859	0.000	1.60
		B		0.000	0.000	68.175	0.000	0.84
		C		0.000	0.000	5.226	0.000	0.05

### Feed Line Center of Pressure

Section	Elevation	$CP_x$	$CP_z$	$CP_x$	$CP_z$
	ft	in	in	Ice in	Ice in



<b>tnxTower</b>  <b>Tower Engineering Professionals</b> 326 Tryon Rd. Raleigh, NC 27603 Phone: (919) 661-6351 FAX: (919) 661-6350	<b>Job</b> (P3) Erwin/McLamb (F1) (BU: 879743)	<b>Page</b> 12 of 32
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	<b>Client</b> Crown Castle	<b>Designed by</b> jcmcginnis

Section	Elevation	CP <sub>X</sub>	CP <sub>Z</sub>	CP <sub>X</sub>	CP <sub>Z</sub>
		in	in	Ice in	Ice in
T1	250.00-240.00	1.8360	-4.4472	4.5252	-3.6554
T2	240.00-236.00	0.5604	-10.1867	3.2258	-8.4609
T3	236.00-232.00	-2.0984	-15.3934	0.4261	-12.7902
T4	232.00-228.00	-2.0984	-15.3934	0.4247	-12.7957
T5	228.00-224.00	-2.0984	-15.3934	0.4232	-12.8014
T6	224.00-220.00	-2.0984	-15.3934	0.4217	-12.8071
T7	220.00-200.00	-2.2502	-17.6569	0.5963	-15.0172
T8	200.00-180.00	-2.5709	-22.0859	0.9102	-19.1785
T9	180.00-160.00	-2.7429	-25.1082	1.1685	-22.7431
T10	160.00-140.00	-2.9814	-28.4494	1.3837	-25.7564
T11	140.00-120.00	-3.2093	-31.6310	1.5777	-28.7698
T12	120.00-100.00	-3.8326	-35.8071	0.1963	-32.4919
T13	100.00-80.00	-3.8361	-36.7075	0.2190	-34.4395
T14	80.00-60.00	-3.9391	-38.1569	0.2262	-36.0125
T15	60.00-40.00	-3.8930	-38.4179	0.2097	-37.3011
T16	40.00-20.00	-4.0390	-40.3275	0.1625	-39.2878
T17	20.00-0.00	-6.3288	-58.8000	0.0174	-49.5380

### Shielding Factor Ka

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K <sub>a</sub> No Ice	K <sub>a</sub> Ice
T1	2	Safety Line 3/8	240.00 - 250.00	0.6000	0.5737
T1	3	FSJ2-50(3/8")	240.00 - 250.00	0.6000	0.5737
T1	6	Step Pegs (5/8" SR) 7-in. w/30" step	240.00 - 250.00	0.6000	0.5737
T1	10	Feedline Ladder (Af)	240.00 - 246.00	0.6000	0.5737
T1	17	FXL-1873( 1 5/8)	240.00 - 246.00	0.6000	0.5737
T1	19	FSJ4-50B(1/2")	240.00 - 250.00	0.6000	0.5737
T2	2	Safety Line 3/8	236.00 - 240.00	0.6000	0.5837
T2	3	FSJ2-50(3/8")	236.00 - 240.00	0.6000	0.5837
T2	6	Step Pegs (5/8" SR) 7-in. w/30" step	236.00 - 240.00	0.6000	0.5837
T2	9	Feedline Ladder (Af)	236.00 - 237.00	0.6000	0.5837
T2	10	Feedline Ladder (Af)	236.00 - 240.00	0.6000	0.5837
T2	12	LDF12-50A(2-1/4")	236.00 - 237.00	0.6000	0.5837
T2	13	WR-VG86ST-BRD( 3/4)	236.00 - 237.00	0.6000	0.5837
T2	14	860 10014(3/8)	236.00 - 237.00	0.6000	0.5837
T2	15	FB-L98B-034-XXXXXX( 3/8")	236.00 - 237.00	0.6000	0.5837
T2	17	FXL-1873( 1 5/8)	236.00 - 240.00	0.6000	0.5837
T2	19	FSJ4-50B(1/2")	236.00 -	0.6000	0.5837

<b>tnxTower</b>  <b>Tower Engineering Professionals</b> 326 Tryon Rd. Raleigh, NC 27603 Phone: (919) 661-6351 FAX: (919) 661-6350	<b>Job</b> (P3) Erwin/McLamb (F1) (BU: 879743)	<b>Page</b> 13 of 32
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Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K <sub>a</sub> No Ice	K <sub>a</sub> Ice
			240.00		
T3	2	Safety Line 3/8	232.00 -	0.6000	0.5841
			236.00		
T3	3	FSJ2-50(3/8")	232.00 -	0.6000	0.5841
			236.00		
T3	6	Step Pegs (5/8" SR) 7-in. w/30" step	232.00 -	0.6000	0.5841
			236.00		
T3	9	Feedline Ladder (Af)	232.00 -	0.6000	0.5841
			236.00		
T3	10	Feedline Ladder (Af)	232.00 -	0.6000	0.5841
			236.00		
T3	12	LDF12-50A(2-1/4")	232.00 -	0.6000	0.5841
			236.00		
T3	13	WR-VG86ST-BRD( 3/4)	232.00 -	0.6000	0.5841
			236.00		
T3	14	860 10014(3/8)	232.00 -	0.6000	0.5841
			236.00		
T3	15	FB-L98B-034-XXXXXX( 3/8")	232.00 -	0.6000	0.5841
			236.00		
T3	17	FXL-1873( 1 5/8)	232.00 -	0.6000	0.5841
			236.00		
T3	19	FSJ4-50B(1/2")	232.00 -	0.6000	0.5841
			236.00		
T4	2	Safety Line 3/8	228.00 -	0.6000	0.5845
			232.00		
T4	3	FSJ2-50(3/8")	228.00 -	0.6000	0.5845
			232.00		
T4	6	Step Pegs (5/8" SR) 7-in. w/30" step	228.00 -	0.6000	0.5845
			232.00		
T4	9	Feedline Ladder (Af)	228.00 -	0.6000	0.5845
			232.00		
T4	10	Feedline Ladder (Af)	228.00 -	0.6000	0.5845
			232.00		
T4	12	LDF12-50A(2-1/4")	228.00 -	0.6000	0.5845
			232.00		
T4	13	WR-VG86ST-BRD( 3/4)	228.00 -	0.6000	0.5845
			232.00		
T4	14	860 10014(3/8)	228.00 -	0.6000	0.5845
			232.00		
T4	15	FB-L98B-034-XXXXXX( 3/8")	228.00 -	0.6000	0.5845
			232.00		
T4	17	FXL-1873( 1 5/8)	228.00 -	0.6000	0.5845
			232.00		
T4	19	FSJ4-50B(1/2")	228.00 -	0.6000	0.5845
			232.00		
T5	2	Safety Line 3/8	224.00 -	0.6000	0.5849
			228.00		
T5	3	FSJ2-50(3/8")	224.00 -	0.6000	0.5849
			228.00		
T5	6	Step Pegs (5/8" SR) 7-in. w/30" step	224.00 -	0.6000	0.5849
			228.00		
T5	9	Feedline Ladder (Af)	224.00 -	0.6000	0.5849
			228.00		
T5	10	Feedline Ladder (Af)	224.00 -	0.6000	0.5849
			228.00		
T5	12	LDF12-50A(2-1/4")	224.00 -	0.6000	0.5849
			228.00		
T5	13	WR-VG86ST-BRD( 3/4)	224.00 -	0.6000	0.5849
			228.00		
T5	14	860 10014(3/8)	224.00 -	0.6000	0.5849
			228.00		
T5	15	FB-L98B-034-XXXXXX( 3/8")	224.00 -	0.6000	0.5849
			228.00		

<b>tnxTower</b>  <b>Tower Engineering Professionals</b> 326 Tryon Rd. Raleigh, NC 27603 Phone: (919) 661-6351 FAX: (919) 661-6350	<b>Job</b> (P3) Erwin/McLamb (F1) (BU: 879743)	<b>Page</b> 14 of 32
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Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K <sub>a</sub> No Ice	K <sub>a</sub> Ice
		3/8")	228.00		
T5	17	FXL-1873( 1 5/8)	224.00 -	0.6000	0.5849
			228.00		
T5	19	FSJ4-50B(1/2")	224.00 -	0.6000	0.5849
			228.00		
T6	2	Safety Line 3/8	220.00 -	0.6000	0.5853
			224.00		
T6	3	FSJ2-50(3/8")	220.00 -	0.6000	0.5853
			224.00		
T6	6	Step Pegs (5/8" SR) 7-in. w/30" step	220.00 -	0.6000	0.5853
			224.00		
T6	9	Feedline Ladder (Af)	220.00 -	0.6000	0.5853
			224.00		
T6	10	Feedline Ladder (Af)	220.00 -	0.6000	0.5853
			224.00		
T6	12	LDF12-50A(2-1/4")	220.00 -	0.6000	0.5853
			224.00		
T6	13	WR-VG86ST-BRD( 3/4)	220.00 -	0.6000	0.5853
			224.00		
T6	14	860 10014(3/8)	220.00 -	0.6000	0.5853
			224.00		
T6	15	FB-L98B-034-XXXXXX( 3/8")	220.00 -	0.6000	0.5853
			224.00		
T6	17	FXL-1873( 1 5/8)	220.00 -	0.6000	0.5853
			224.00		
T6	19	FSJ4-50B(1/2")	220.00 -	0.6000	0.5853
			224.00		
T7	2	Safety Line 3/8	200.00 -	0.6000	0.6000
			220.00		
T7	3	FSJ2-50(3/8")	200.00 -	0.6000	0.6000
			220.00		
T7	6	Step Pegs (5/8" SR) 7-in. w/30" step	200.00 -	0.6000	0.6000
			220.00		
T7	9	Feedline Ladder (Af)	200.00 -	0.6000	0.6000
			220.00		
T7	10	Feedline Ladder (Af)	200.00 -	0.6000	0.6000
			220.00		
T7	12	LDF12-50A(2-1/4")	200.00 -	0.6000	0.6000
			220.00		
T7	13	WR-VG86ST-BRD( 3/4)	200.00 -	0.6000	0.6000
			220.00		
T7	14	860 10014(3/8)	200.00 -	0.6000	0.6000
			220.00		
T7	15	FB-L98B-034-XXXXXX( 3/8")	200.00 -	0.6000	0.6000
			220.00		
T7	17	FXL-1873( 1 5/8)	200.00 -	0.6000	0.6000
			220.00		
T7	19	FSJ4-50B(1/2")	200.00 -	0.6000	0.6000
			220.00		
T8	2	Safety Line 3/8	180.00 -	0.6000	0.6000
			200.00		
T8	3	FSJ2-50(3/8")	180.00 -	0.6000	0.6000
			200.00		
T8	6	Step Pegs (5/8" SR) 7-in. w/30" step	180.00 -	0.6000	0.6000
			200.00		
T8	9	Feedline Ladder (Af)	180.00 -	0.6000	0.6000
			200.00		
T8	10	Feedline Ladder (Af)	180.00 -	0.6000	0.6000
			200.00		
T8	12	LDF12-50A(2-1/4")	180.00 -	0.6000	0.6000
			200.00		
T8	13	WR-VG86ST-BRD( 3/4)	180.00 -	0.6000	0.6000



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Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K <sub>a</sub> No Ice	K <sub>a</sub> Ice
			200.00		
T8	14	860 10014(3/8)	180.00 -	0.6000	0.6000
			200.00		
T8	15	FB-L98B-034-XXXXXX(3/8")	180.00 -	0.6000	0.6000
			200.00		
T8	17	FXL-1873( 1 5/8)	180.00 -	0.6000	0.6000
			200.00		
T8	19	FSJ4-50B(1/2")	180.00 -	0.6000	0.6000
			200.00		
T9	2	Safety Line 3/8	160.00 -	0.6000	0.6000
			180.00		
T9	3	FSJ2-50(3/8")	160.00 -	0.6000	0.6000
			180.00		
T9	6	Step Pegs (5/8" SR) 7-in. w/30" step	160.00 -	0.6000	0.6000
			180.00		
T9	9	Feedline Ladder (Af)	160.00 -	0.6000	0.6000
			180.00		
T9	10	Feedline Ladder (Af)	160.00 -	0.6000	0.6000
			180.00		
T9	12	LDF12-50A(2-1/4")	160.00 -	0.6000	0.6000
			180.00		
T9	13	WR-VG86ST-BRD( 3/4)	160.00 -	0.6000	0.6000
			180.00		
T9	14	860 10014(3/8)	160.00 -	0.6000	0.6000
			180.00		
T9	15	FB-L98B-034-XXXXXX(3/8")	160.00 -	0.6000	0.6000
			180.00		
T9	17	FXL-1873( 1 5/8)	160.00 -	0.6000	0.6000
			180.00		
T9	19	FSJ4-50B(1/2")	160.00 -	0.6000	0.6000
			180.00		
T10	2	Safety Line 3/8	140.00 -	0.6000	0.6000
			160.00		
T10	3	FSJ2-50(3/8")	140.00 -	0.6000	0.6000
			160.00		
T10	6	Step Pegs (5/8" SR) 7-in. w/30" step	140.00 -	0.6000	0.6000
			160.00		
T10	9	Feedline Ladder (Af)	140.00 -	0.6000	0.6000
			160.00		
T10	10	Feedline Ladder (Af)	140.00 -	0.6000	0.6000
			160.00		
T10	12	LDF12-50A(2-1/4")	140.00 -	0.6000	0.6000
			160.00		
T10	13	WR-VG86ST-BRD( 3/4)	140.00 -	0.6000	0.6000
			160.00		
T10	14	860 10014(3/8)	140.00 -	0.6000	0.6000
			160.00		
T10	15	FB-L98B-034-XXXXXX(3/8")	140.00 -	0.6000	0.6000
			160.00		
T10	17	FXL-1873( 1 5/8)	140.00 -	0.6000	0.6000
			160.00		
T10	19	FSJ4-50B(1/2")	140.00 -	0.6000	0.6000
			160.00		
T11	2	Safety Line 3/8	120.00 -	0.6000	0.6000
			140.00		
T11	3	FSJ2-50(3/8")	120.00 -	0.6000	0.6000
			140.00		
T11	6	Step Pegs (5/8" SR) 7-in. w/30" step	120.00 -	0.6000	0.6000
			140.00		
T11	9	Feedline Ladder (Af)	120.00 -	0.6000	0.6000
			140.00		
T11	10	Feedline Ladder (Af)	120.00 -	0.6000	0.6000

<b>tnxTower</b>  <b>Tower Engineering Professionals</b> 326 Tryon Rd. Raleigh, NC 27603 Phone: (919) 661-6351 FAX: (919) 661-6350	<b>Job</b> (P3) Erwin/McLamb (F1) (BU: 879743)	<b>Page</b> 16 of 32
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Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K <sub>a</sub> No Ice	K <sub>a</sub> Ice
T11	12	LDF12-50A(2-1/4")	140.00	0.6000	0.6000
T11	13	WR-VG86ST-BRD( 3/4)	120.00 - 140.00	0.6000	0.6000
T11	14	860 10014(3/8)	120.00 - 140.00	0.6000	0.6000
T11	15	FB-L98B-034-XXXXXX( 3/8")	120.00 - 140.00	0.6000	0.6000
T11	17	FXL-1873( 1 5/8)	120.00 - 140.00	0.6000	0.6000
T11	19	FSJ4-50B(1/2")	120.00 - 140.00	0.6000	0.6000
T12	2	Safety Line 3/8	100.00 - 120.00	0.6000	0.6000
T12	3	FSJ2-50(3/8")	100.00 - 120.00	0.6000	0.6000
T12	5	Step Pegs (5/8" SR) 7-in. w/30" step	100.00 - 120.00	0.6000	0.6000
T12	6	Step Pegs (5/8" SR) 7-in. w/30" step	100.00 - 120.00	0.6000	0.6000
T12	7	Step Pegs (5/8" SR) 7-in. w/30" step	100.00 - 120.00	0.6000	0.6000
T12	9	Feedline Ladder (Af)	100.00 - 120.00	0.6000	0.6000
T12	10	Feedline Ladder (Af)	100.00 - 120.00	0.6000	0.6000
T12	12	LDF12-50A(2-1/4")	100.00 - 120.00	0.6000	0.6000
T12	13	WR-VG86ST-BRD( 3/4)	100.00 - 120.00	0.6000	0.6000
T12	14	860 10014(3/8)	100.00 - 120.00	0.6000	0.6000
T12	15	FB-L98B-034-XXXXXX( 3/8")	100.00 - 120.00	0.6000	0.6000
T12	17	FXL-1873( 1 5/8)	100.00 - 120.00	0.6000	0.6000
T12	19	FSJ4-50B(1/2")	100.00 - 120.00	0.6000	0.6000
T13	2	Safety Line 3/8	80.00 - 100.00	0.6000	0.6000
T13	3	FSJ2-50(3/8")	80.00 - 100.00	0.6000	0.6000
T13	5	Step Pegs (5/8" SR) 7-in. w/30" step	80.00 - 100.00	0.6000	0.6000
T13	6	Step Pegs (5/8" SR) 7-in. w/30" step	80.00 - 100.00	0.6000	0.6000
T13	7	Step Pegs (5/8" SR) 7-in. w/30" step	80.00 - 100.00	0.6000	0.6000
T13	9	Feedline Ladder (Af)	80.00 - 100.00	0.6000	0.6000
T13	10	Feedline Ladder (Af)	80.00 - 100.00	0.6000	0.6000
T13	12	LDF12-50A(2-1/4")	80.00 - 100.00	0.6000	0.6000
T13	13	WR-VG86ST-BRD( 3/4)	80.00 - 100.00	0.6000	0.6000
T13	14	860 10014(3/8)	80.00 - 100.00	0.6000	0.6000
T13	15	FB-L98B-034-XXXXXX( 3/8")	80.00 - 100.00	0.6000	0.6000
T13	17	FXL-1873( 1 5/8)	80.00 - 100.00	0.6000	0.6000
T13	19	FSJ4-50B(1/2")	80.00 - 100.00	0.6000	0.6000
T14	2	Safety Line 3/8	60.00 - 80.00	0.6000	0.6000
T14	3	FSJ2-50(3/8")	60.00 - 80.00	0.6000	0.6000
T14	5	Step Pegs (5/8" SR) 7-in. w/30" step	60.00 - 80.00	0.6000	0.6000
T14	6	Step Pegs (5/8" SR) 7-in. w/30" step	60.00 - 80.00	0.6000	0.6000

<b>tnxTower</b>  <b>Tower Engineering Professionals</b> 326 Tryon Rd. Raleigh, NC 27603 Phone: (919) 661-6351 FAX: (919) 661-6350	<b>Job</b> (P3) Erwin/McLamb (F1) (BU: 879743)	<b>Page</b> 17 of 32
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Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K <sub>a</sub> No Ice	K <sub>a</sub> Ice
T14	7	Step Pegs (5/8" SR) 7-in. w/30" step	60.00 - 80.00	0.6000	0.6000
T14	9	Feedline Ladder (Af)	60.00 - 80.00	0.6000	0.6000
T14	10	Feedline Ladder (Af)	60.00 - 80.00	0.6000	0.6000
T14	12	LDF12-50A(2-1/4")	60.00 - 80.00	0.6000	0.6000
T14	13	WR-VG86ST-BRD( 3/4)	60.00 - 80.00	0.6000	0.6000
T14	14	860 10014(3/8)	60.00 - 80.00	0.6000	0.6000
T14	15	FB-L98B-034-XXXXXX( 3/8")	60.00 - 80.00	0.6000	0.6000
T14	17	FXL-1873( 1 5/8)	60.00 - 80.00	0.6000	0.6000
T14	19	FSJ4-50B(1/2")	60.00 - 80.00	0.6000	0.6000
T15	2	Safety Line 3/8	40.00 - 60.00	0.6000	0.6000
T15	3	FSJ2-50(3/8")	40.00 - 60.00	0.6000	0.6000
T15	5	Step Pegs (5/8" SR) 7-in. w/30" step	40.00 - 60.00	0.6000	0.6000
T15	6	Step Pegs (5/8" SR) 7-in. w/30" step	40.00 - 60.00	0.6000	0.6000
T15	7	Step Pegs (5/8" SR) 7-in. w/30" step	40.00 - 60.00	0.6000	0.6000
T15	9	Feedline Ladder (Af)	40.00 - 60.00	0.6000	0.6000
T15	10	Feedline Ladder (Af)	40.00 - 60.00	0.6000	0.6000
T15	12	LDF12-50A(2-1/4")	40.00 - 60.00	0.6000	0.6000
T15	13	WR-VG86ST-BRD( 3/4)	40.00 - 60.00	0.6000	0.6000
T15	14	860 10014(3/8)	40.00 - 60.00	0.6000	0.6000
T15	15	FB-L98B-034-XXXXXX( 3/8")	40.00 - 60.00	0.6000	0.6000
T15	17	FXL-1873( 1 5/8)	40.00 - 60.00	0.6000	0.6000
T15	19	FSJ4-50B(1/2")	40.00 - 60.00	0.6000	0.6000
T16	2	Safety Line 3/8	20.00 - 40.00	0.6000	0.6000
T16	3	FSJ2-50(3/8")	20.00 - 40.00	0.6000	0.6000
T16	5	Step Pegs (5/8" SR) 7-in. w/30" step	20.00 - 40.00	0.6000	0.6000
T16	6	Step Pegs (5/8" SR) 7-in. w/30" step	20.00 - 40.00	0.6000	0.6000
T16	7	Step Pegs (5/8" SR) 7-in. w/30" step	20.00 - 40.00	0.6000	0.6000
T16	9	Feedline Ladder (Af)	20.00 - 40.00	0.6000	0.6000
T16	10	Feedline Ladder (Af)	20.00 - 40.00	0.6000	0.6000
T16	12	LDF12-50A(2-1/4")	20.00 - 40.00	0.6000	0.6000
T16	13	WR-VG86ST-BRD( 3/4)	20.00 - 40.00	0.6000	0.6000
T16	14	860 10014(3/8)	20.00 - 40.00	0.6000	0.6000
T16	15	FB-L98B-034-XXXXXX( 3/8")	20.00 - 40.00	0.6000	0.6000
T16	17	FXL-1873( 1 5/8)	20.00 - 40.00	0.6000	0.6000
T16	19	FSJ4-50B(1/2")	20.00 - 40.00	0.6000	0.6000
T17	2	Safety Line 3/8	0.00 - 20.00	0.6000	0.6000
T17	3	FSJ2-50(3/8")	0.00 - 20.00	0.6000	0.6000
T17	5	Step Pegs (5/8" SR) 7-in. w/30" step	0.00 - 20.00	0.6000	0.6000
T17	6	Step Pegs (5/8" SR) 7-in. w/30" step	0.00 - 20.00	0.6000	0.6000
T17	7	Step Pegs (5/8" SR) 7-in. w/30" step	0.00 - 20.00	0.6000	0.6000
T17	9	Feedline Ladder (Af)	0.00 - 20.00	0.6000	0.6000
T17	10	Feedline Ladder (Af)	0.00 - 20.00	0.6000	0.6000
T17	12	LDF12-50A(2-1/4")	0.00 - 20.00	0.6000	0.6000
T17	13	WR-VG86ST-BRD( 3/4)	0.00 - 20.00	0.6000	0.6000
T17	14	860 10014(3/8)	0.00 - 20.00	0.6000	0.6000
T17	15	FB-L98B-034-XXXXXX( 3/8")	0.00 - 20.00	0.6000	0.6000
T17	17	FXL-1873( 1 5/8)	0.00 - 20.00	0.6000	0.6000
T17	19	FSJ4-50B(1/2")	0.00 - 20.00	0.6000	0.6000



<b>tnxTower</b>  <b>Tower Engineering Professionals</b> 326 Tryon Rd. Raleigh, NC 27603 Phone: (919) 661-6351 FAX: (919) 661-6350	<b>Job</b> (P3) Erwin/McLamb (F1) (BU: 879743)	<b>Page</b> 18 of 32
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	<b>Client</b> Crown Castle	<b>Designed by</b> jcmcginnis

### Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>A,A</sub> Front	C <sub>A,A</sub> Side	Weight	
			Horz	Lateral						
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K	
<b>** Lightning Rod **</b>										
5/8"x8-ft LRod	C	None			0.0000	250.00	No Ice	0.50	0.50	0.01
							1/2" Ice	1.31	1.31	0.01
							1" Ice	2.14	2.14	0.02
							2" Ice	3.61	3.61	0.06
12" Dia. x 30" Beacon	C	None			0.0000	250.00	No Ice	1.25	1.25	0.03
							1/2" Ice	1.96	1.96	0.05
							1" Ice	2.18	2.18	0.08
							2" Ice	2.64	2.64	0.14
3" x 6" SideLight	A	From Leg	0.50		0.0000	125.00	No Ice	0.09	0.09	0.00
			0.00				1/2" Ice	0.14	0.14	0.00
			0.00				1" Ice	0.19	0.19	0.00
							2" Ice	0.34	0.34	0.01
3" x 6" SideLight	B	From Leg	0.50		0.0000	125.00	No Ice	0.09	0.09	0.00
			0.00				1/2" Ice	0.14	0.14	0.00
			0.00				1" Ice	0.19	0.19	0.00
							2" Ice	0.34	0.34	0.01
3" x 6" SideLight	C	From Leg	0.50		0.0000	125.00	No Ice	0.09	0.09	0.00
			0.00				1/2" Ice	0.14	0.14	0.00
			0.00				1" Ice	0.19	0.19	0.00
							2" Ice	0.34	0.34	0.01
<b>** 246 **</b>										
AIR 32 B2A	A	From Leg	4.00		30.0000	246.00	No Ice	3.76	3.15	0.19
B66AA_T-MOBILE w/			-6.00				1/2" Ice	4.12	3.49	0.25
Mount Pipe			2.00				1" Ice	4.48	3.84	0.32
							2" Ice	5.24	4.58	0.48
AIR 32 B2A	B	From Leg	4.00		30.0000	246.00	No Ice	3.76	3.15	0.19
B66AA_T-MOBILE w/			-6.00				1/2" Ice	4.12	3.49	0.25
Mount Pipe			2.00				1" Ice	4.48	3.84	0.32
							2" Ice	5.24	4.58	0.48
AIR 32 B2A	C	From Leg	4.00		30.0000	246.00	No Ice	3.76	3.15	0.19
B66AA_T-MOBILE w/			-6.00				1/2" Ice	4.12	3.49	0.25
Mount Pipe			2.00				1" Ice	4.48	3.84	0.32
							2" Ice	5.24	4.58	0.48
AIR6449 B41_T-MOBILE	A	From Leg	4.00		30.0000	246.00	No Ice	5.19	2.71	0.13
w/ Mount Pipe			6.00				1/2" Ice	5.59	3.04	0.17
			2.00				1" Ice	6.02	3.38	0.23
							2" Ice	6.90	4.12	0.35
AIR6449 B41_T-MOBILE	B	From Leg	4.00		30.0000	246.00	No Ice	5.19	2.71	0.13
w/ Mount Pipe			6.00				1/2" Ice	5.59	3.04	0.17
			2.00				1" Ice	6.02	3.38	0.23
							2" Ice	6.90	4.12	0.35
AIR6449 B41_T-MOBILE	C	From Leg	4.00		30.0000	246.00	No Ice	5.19	2.71	0.13
w/ Mount Pipe			6.00				1/2" Ice	5.59	3.04	0.17
			2.00				1" Ice	6.02	3.38	0.23
							2" Ice	6.90	4.12	0.35
APXVAALL24_43-U-NA20	A	From Leg	4.00		30.0000	246.00	No Ice	14.69	6.87	0.18

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	<b>Project</b>		TEP No. 148377.565498		<b>Date</b>		16:58:28 07/08/21	
	<b>Client</b>		Crown Castle		<b>Designed by</b>		jcmcginnis	

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>A,A</sub> Front	C <sub>A,A</sub> Side	Weight
			Horz	Lateral Vert					
_TMO w/ Mount Pipe			-2.00			1/2" Ice	15.46	7.55	0.31
			2.00			1" Ice	16.23	8.25	0.45
						2" Ice	17.82	9.67	0.78
APXVAALL24_43-U-NA20	B	From Leg	4.00		30.0000	No Ice	14.69	6.87	0.18
_TMO w/ Mount Pipe			-2.00			1/2" Ice	15.46	7.55	0.31
			2.00			1" Ice	16.23	8.25	0.45
						2" Ice	17.82	9.67	0.78
APXVAALL24_43-U-NA20	C	From Leg	4.00		30.0000	No Ice	14.69	6.87	0.18
_TMO w/ Mount Pipe			-2.00			1/2" Ice	15.46	7.55	0.31
			2.00			1" Ice	16.23	8.25	0.45
						2" Ice	17.82	9.67	0.78
RADIO 4449 B71	A	From Leg	4.00		30.0000	No Ice	1.97	1.59	0.07
B85A_T-MOBILE			-6.00			1/2" Ice	2.15	1.75	0.09
			2.00			1" Ice	2.33	1.92	0.12
						2" Ice	2.72	2.28	0.17
RADIO 4449 B71	B	From Leg	4.00		30.0000	No Ice	1.97	1.59	0.07
B85A_T-MOBILE			-6.00			1/2" Ice	2.15	1.75	0.09
			2.00			1" Ice	2.33	1.92	0.12
						2" Ice	2.72	2.28	0.17
RADIO 4449 B71	C	From Leg	4.00		30.0000	No Ice	1.97	1.59	0.07
B85A_T-MOBILE			-6.00			1/2" Ice	2.15	1.75	0.09
			2.00			1" Ice	2.33	1.92	0.12
						2" Ice	2.72	2.28	0.17
RADIO 4415 B25_TMO	A	From Leg	4.00		30.0000	No Ice	1.86	0.87	0.05
			6.00			1/2" Ice	2.03	1.00	0.06
			2.00			1" Ice	2.20	1.13	0.08
						2" Ice	2.58	1.43	0.12
RADIO 4415 B25_TMO	B	From Leg	4.00		30.0000	No Ice	1.86	0.87	0.05
			6.00			1/2" Ice	2.03	1.00	0.06
			2.00			1" Ice	2.20	1.13	0.08
						2" Ice	2.58	1.43	0.12
RADIO 4415 B25_TMO	C	From Leg	4.00		30.0000	No Ice	1.86	0.87	0.05
			6.00			1/2" Ice	2.03	1.00	0.06
			2.00			1" Ice	2.20	1.13	0.08
						2" Ice	2.58	1.43	0.12
8' x 2" Mount Pipe	A	From Leg	4.00		0.0000	No Ice	1.90	1.90	0.03
			2.00			1/2" Ice	2.73	2.73	0.04
			0.00			1" Ice	3.40	3.40	0.06
						2" Ice	4.40	4.40	0.12
8' x 2" Mount Pipe	B	From Leg	4.00		0.0000	No Ice	1.90	1.90	0.03
			2.00			1/2" Ice	2.73	2.73	0.04
			0.00			1" Ice	3.40	3.40	0.06
						2" Ice	4.40	4.40	0.12
8' x 2" Mount Pipe	C	From Leg	4.00		0.0000	No Ice	1.90	1.90	0.03
			2.00			1/2" Ice	2.73	2.73	0.04
			0.00			1" Ice	3.40	3.40	0.06
						2" Ice	4.40	4.40	0.12
Sector Mount [SM 1303-3]	C	None			0.0000	No Ice	38.78	38.78	1.10
						1/2" Ice	46.78	46.78	1.76
						1" Ice	54.73	54.73	2.57
						2" Ice	70.62	70.62	4.60
** 245 **									
** 237 **									
(2) SBNHH-1D65C w/	A	From Leg	4.00		-20.0000	No Ice	5.56	4.47	0.08
Mount Pipe			0.00			1/2" Ice	6.07	4.97	0.16
			1.00			1" Ice	6.59	5.48	0.26
						2" Ice	7.66	6.52	0.49

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	<b>Client</b> Crown Castle	<b>Designed by</b> jcmcginnis

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>A</sub> A <sub>1</sub>		Weight	
			Horz	Lateral			Front	Side		
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K	
(2) SBNHH-1D65C w/ Mount Pipe	B	From Leg	4.00		-20.0000	237.00	No Ice	5.56	4.47	0.08
			0.00				1/2" Ice	6.07	4.97	0.16
			1.00				1" Ice	6.59	5.48	0.26
							2" Ice	7.66	6.52	0.49
(2) SBNHH-1D65C w/ Mount Pipe	C	From Leg	4.00		-20.0000	237.00	No Ice	5.56	4.47	0.08
			0.00				1/2" Ice	6.07	4.97	0.16
			1.00				1" Ice	6.59	5.48	0.26
							2" Ice	7.66	6.52	0.49
TPA65R-BU8D w/ Mount Pipe	A	From Leg	4.00		-20.0000	237.00	No Ice	15.94	7.91	0.12
			-2.00				1/2" Ice	16.87	8.76	0.24
			1.00				1" Ice	17.82	9.63	0.36
							2" Ice	19.76	11.40	0.67
TPA65R-BU8D w/ Mount Pipe	B	From Leg	4.00		-20.0000	237.00	No Ice	15.94	7.91	0.12
			-2.00				1/2" Ice	16.87	8.76	0.24
			1.00				1" Ice	17.82	9.63	0.36
							2" Ice	19.76	11.40	0.67
TPA65R-BU8D w/ Mount Pipe	C	From Leg	4.00		-20.0000	237.00	No Ice	15.94	7.91	0.12
			-2.00				1/2" Ice	16.87	8.76	0.24
			1.00				1" Ice	17.82	9.63	0.36
							2" Ice	19.76	11.40	0.67
RRUS-11	A	From Leg	4.00		-20.0000	237.00	No Ice	2.78	1.19	0.05
			-6.00				1/2" Ice	2.99	1.33	0.07
			1.00				1" Ice	3.21	1.49	0.09
							2" Ice	3.66	1.83	0.15
RRUS-11	B	From Leg	4.00		-20.0000	237.00	No Ice	2.78	1.19	0.05
			-6.00				1/2" Ice	2.99	1.33	0.07
			1.00				1" Ice	3.21	1.49	0.09
							2" Ice	3.66	1.83	0.15
RRUS-11	C	From Leg	4.00		-20.0000	237.00	No Ice	2.78	1.19	0.05
			-6.00				1/2" Ice	2.99	1.33	0.07
			1.00				1" Ice	3.21	1.49	0.09
							2" Ice	3.66	1.83	0.15
RRUS 4478 B14	A	From Leg	4.00		-20.0000	237.00	No Ice	1.84	1.06	0.06
			-6.00				1/2" Ice	2.01	1.20	0.08
			1.00				1" Ice	2.19	1.34	0.09
							2" Ice	2.57	1.66	0.14
RRUS 4478 B14	B	From Leg	4.00		-20.0000	237.00	No Ice	1.84	1.06	0.06
			-6.00				1/2" Ice	2.01	1.20	0.08
			1.00				1" Ice	2.19	1.34	0.09
							2" Ice	2.57	1.66	0.14
RRUS 4478 B14	C	From Leg	4.00		-20.0000	237.00	No Ice	1.84	1.06	0.06
			-6.00				1/2" Ice	2.01	1.20	0.08
			1.00				1" Ice	2.19	1.34	0.09
							2" Ice	2.57	1.66	0.14
RADIO 8843 B2/B66A	A	From Leg	4.00		-20.0000	237.00	No Ice	1.64	1.38	0.08
			-2.00				1/2" Ice	1.80	1.53	0.09
			1.00				1" Ice	1.97	1.69	0.11
							2" Ice	2.33	2.02	0.16
RADIO 8843 B2/B66A	B	From Leg	4.00		-20.0000	237.00	No Ice	1.64	1.38	0.08
			-2.00				1/2" Ice	1.80	1.53	0.09
			1.00				1" Ice	1.97	1.69	0.11
							2" Ice	2.33	2.02	0.16
RADIO 8843 B2/B66A	C	From Leg	4.00		-20.0000	237.00	No Ice	1.64	1.38	0.08
			-2.00				1/2" Ice	1.80	1.53	0.09
			1.00				1" Ice	1.97	1.69	0.11
							2" Ice	2.33	2.02	0.16
(2) ATM192012-0	A	From Leg	4.00		-20.0000	237.00	No Ice	0.96	0.50	0.01



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Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment °	Placement ft	C <sub>A</sub> A <sub>A</sub> Front ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Side ft <sup>2</sup>	Weight K
			Horz ft	Lateral Vert ft					
				-2.00		1/2" Ice	1.08	0.59	0.02
				1.00		1" Ice	1.21	0.69	0.03
						2" Ice	1.50	0.91	0.06
(2) ATM192012-0	B	From Leg	4.00	-20.0000	237.00	No Ice	0.96	0.50	0.01
			-2.00			1/2" Ice	1.08	0.59	0.02
			1.00			1" Ice	1.21	0.69	0.03
						2" Ice	1.50	0.91	0.06
(2) ATM192012-0	C	From Leg	4.00	-20.0000	237.00	No Ice	0.96	0.50	0.01
			-2.00			1/2" Ice	1.08	0.59	0.02
			1.00			1" Ice	1.21	0.69	0.03
						2" Ice	1.50	0.91	0.06
DC6-48-60-18-8F	A	From Leg	4.00	-20.0000	237.00	No Ice	1.21	1.21	0.03
			-6.00			1/2" Ice	1.89	1.89	0.05
			1.00			1" Ice	2.11	2.11	0.08
						2" Ice	2.57	2.57	0.14
DC6-48-60-18-8F	B	From Leg	4.00	-20.0000	237.00	No Ice	1.21	1.21	0.03
			-6.00			1/2" Ice	1.89	1.89	0.05
			1.00			1" Ice	2.11	2.11	0.08
						2" Ice	2.57	2.57	0.14
6' x 2" Mount Pipe	A	From Leg	4.00	0.0000	237.00	No Ice	1.43	1.43	0.02
			2.00			1/2" Ice	1.92	1.92	0.03
			0.00			1" Ice	2.29	2.29	0.05
						2" Ice	3.06	3.06	0.09
6' x 2" Mount Pipe	B	From Leg	4.00	0.0000	237.00	No Ice	1.43	1.43	0.02
			2.00			1/2" Ice	1.92	1.92	0.03
			0.00			1" Ice	2.29	2.29	0.05
						2" Ice	3.06	3.06	0.09
6' x 2" Mount Pipe	C	From Leg	4.00	0.0000	237.00	No Ice	1.43	1.43	0.02
			2.00			1/2" Ice	1.92	1.92	0.03
			0.00			1" Ice	2.29	2.29	0.05
						2" Ice	3.06	3.06	0.09
Sector Mount [SM 1303-3]	C	None		0.0000	237.00	No Ice	38.78	38.78	1.10
						1/2" Ice	46.78	46.78	1.76
						1" Ice	54.73	54.73	2.57
						2" Ice	70.62	70.62	4.60

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### Load Combinations

Comb. No.	Description
1	Dead Only
2	1.2 Dead+1.0 Wind 0 deg - No Ice
3	0.9 Dead+1.0 Wind 0 deg - No Ice
4	1.2 Dead+1.0 Wind 30 deg - No Ice
5	0.9 Dead+1.0 Wind 30 deg - No Ice
6	1.2 Dead+1.0 Wind 60 deg - No Ice
7	0.9 Dead+1.0 Wind 60 deg - No Ice
8	1.2 Dead+1.0 Wind 90 deg - No Ice
9	0.9 Dead+1.0 Wind 90 deg - No Ice
10	1.2 Dead+1.0 Wind 120 deg - No Ice

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Comb. No.	Description
11	0.9 Dead+1.0 Wind 120 deg - No Ice
12	1.2 Dead+1.0 Wind 150 deg - No Ice
13	0.9 Dead+1.0 Wind 150 deg - No Ice
14	1.2 Dead+1.0 Wind 180 deg - No Ice
15	0.9 Dead+1.0 Wind 180 deg - No Ice
16	1.2 Dead+1.0 Wind 210 deg - No Ice
17	0.9 Dead+1.0 Wind 210 deg - No Ice
18	1.2 Dead+1.0 Wind 240 deg - No Ice
19	0.9 Dead+1.0 Wind 240 deg - No Ice
20	1.2 Dead+1.0 Wind 270 deg - No Ice
21	0.9 Dead+1.0 Wind 270 deg - No Ice
22	1.2 Dead+1.0 Wind 300 deg - No Ice
23	0.9 Dead+1.0 Wind 300 deg - No Ice
24	1.2 Dead+1.0 Wind 330 deg - No Ice
25	0.9 Dead+1.0 Wind 330 deg - No Ice
26	1.2 Dead+1.0 Ice+1.0 Temp
27	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
28	1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp
29	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp
30	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
31	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp
32	1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp
33	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp
34	1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp
35	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp
36	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp
37	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp
38	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp
39	Dead+Wind 0 deg - Service
40	Dead+Wind 30 deg - Service
41	Dead+Wind 60 deg - Service
42	Dead+Wind 90 deg - Service
43	Dead+Wind 120 deg - Service
44	Dead+Wind 150 deg - Service
45	Dead+Wind 180 deg - Service
46	Dead+Wind 210 deg - Service
47	Dead+Wind 240 deg - Service
48	Dead+Wind 270 deg - Service
49	Dead+Wind 300 deg - Service
50	Dead+Wind 330 deg - Service

### Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T1	250 - 240	10.800	39	0.5154	0.1388
T2	240 - 236	9.715	39	0.5123	0.1385
T3	236 - 232	9.286	39	0.5069	0.1379
T4	232 - 228	8.855	39	0.4973	0.1364
T5	228 - 224	8.438	39	0.4825	0.1340
T6	224 - 220	8.031	39	0.4623	0.1307
T7	220 - 200	7.648	39	0.4366	0.1267
T8	200 - 180	5.963	39	0.3541	0.1034
T9	180 - 160	4.597	39	0.2852	0.0835
T10	160 - 140	3.501	39	0.2283	0.0698
T11	140 - 120	2.603	39	0.1885	0.0561
T12	120 - 100	1.870	39	0.1489	0.0453
T13	100 - 80	1.283	39	0.1204	0.0372

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Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T14	80 - 60	0.815	39	0.0917	0.0302
T15	60 - 40	0.458	39	0.0672	0.0232
T16	40 - 20	0.199	39	0.0427	0.0169
T17	20 - 0	0.046	43	0.0181	0.0106

### Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
250.00	5/8"x8-ft LRod	39	10.800	0.5154	0.1388	62521
246.00	AIR 32 B2A B66AA_T-MOBILE w/ Mount Pipe	39	10.364	0.5152	0.1387	62521
237.00	(2) SBNHH-1D65C w/ Mount Pipe	39	9.394	0.5086	0.1381	92244
125.00	3" x 6" SideLight	39	2.039	0.1581	0.0477	31126

### Maximum Tower Deflections - Design Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T1	250 - 240	39.401	2	1.8869	0.5189
T2	240 - 236	35.429	2	1.8755	0.5174
T3	236 - 232	33.859	2	1.8557	0.5154
T4	232 - 228	32.279	2	1.8205	0.5100
T5	228 - 224	30.754	2	1.7660	0.5011
T6	224 - 220	29.264	2	1.6917	0.4886
T7	220 - 200	27.862	2	1.5973	0.4737
T8	200 - 180	21.699	2	1.2938	0.3867
T9	180 - 160	16.708	2	1.0404	0.3122
T10	160 - 140	12.711	2	0.8312	0.2611
T11	140 - 120	9.440	2	0.6852	0.2099
T12	120 - 100	6.773	2	0.5405	0.1695
T13	100 - 80	4.644	2	0.4362	0.1391
T14	80 - 60	2.949	2	0.3320	0.1130
T15	60 - 40	1.654	2	0.2428	0.0866
T16	40 - 20	0.719	2	0.1541	0.0632
T17	20 - 0	0.160	11	0.0654	0.0397

### Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
250.00	5/8"x8-ft LRod	2	39.401	1.8869	0.5189	16400
246.00	AIR 32 B2A B66AA_T-MOBILE w/ Mount Pipe	2	37.804	1.8862	0.5185	16400
237.00	(2) SBNHH-1D65C w/ Mount Pipe	2	34.253	1.8619	0.5162	24057



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Elevation	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
125.00	3" x 6" SideLight	2	7.386	0.5740	0.1784	8497

### Bolt Design Data

Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt K	Allowable Load per Bolt K	Ratio Load Allowable	Allowable Ratio	Criteria
T1	250	Leg	A325N	0.6250	4	1.01	20.34	0.050	1.05	Bolt Tension
		Diagonal	A325X	0.5000	1	2.39	5.22	0.458	1.05	Member Block Shear
		Top Girt	A325X	0.5000	1	0.41	4.69	0.088	1.05	Member Block Shear
T2	240	Diagonal	A325X	0.5000	1	2.73	5.22	0.524	1.05	Member Block Shear
T3	236	Diagonal	A325X	0.5000	2	2.34	4.75	0.493	1.05	Member Block Shear
T4	232	Diagonal	A325X	0.5000	2	2.44	4.75	0.514	1.05	Member Block Shear
T5	228	Diagonal	A325X	0.5000	2	2.63	4.75	0.554	1.05	Member Block Shear
T6	224	Leg	A325N	0.7500	4	12.63	30.10	0.419	1.05	Bolt Tension
		Diagonal	A325X	0.5000	2	2.78	4.75	0.585	1.05	Member Block Shear
T7	220	Leg	A325N	0.8750	4	21.06	41.56	0.507	1.05	Bolt Tension
		Diagonal	A325X	0.5000	1	3.59	5.22	0.687	1.05	Member Block Shear
T8	200	Leg	A325N	0.8750	4	27.42	41.56	0.660	1.05	Bolt Tension
		Diagonal	A325X	0.5000	1	4.35	6.20	0.702	1.05	Member Bearing
T9	180	Leg	A325N	1.0000	4	33.01	54.52	0.606	1.05	Bolt Tension
		Diagonal	A325X	0.5000	1	5.19	6.20	0.837	1.05	Member Bearing
T10	160	Leg	A325N	1.0000	4	38.61	54.52	0.708	1.05	Bolt Tension
		Diagonal	A325X	0.5000	1	5.86	6.20	0.946	1.05	Member Bearing
T11	140	Leg	A325N	1.0000	6	29.60	54.52	0.543	1.05	Bolt Tension
		Diagonal	A325X	0.5000	1	6.92	7.33	0.945	1.05	Gusset Bearing
T12	120	Leg	A325N	1.0000	6	33.04	54.52	0.606	1.05	Bolt Tension
		Diagonal	A325X	0.6250	1	8.14	11.70	0.696	1.05	Member Bearing
T13	100	Leg	A325N	1.0000	6	36.89	54.52	0.677	1.05	Bolt Tension
		Diagonal	A325X	0.6250	1	9.16	11.70	0.783	1.05	Member Bearing
T14	80	Leg	A325N	1.0000	8	30.37	54.52	0.557	1.05	Bolt Tension
		Diagonal	A325X	0.7500	1	9.29	14.14	0.657	1.05	Member Bearing
T15	60	Leg	A325N	1.0000	8	33.23	54.52	0.610	1.05	Bolt Tension
		Diagonal	A325X	0.7500	1	11.10	14.14	0.785	1.05	Member Bearing
T16	40	Leg	A325N	1.0000	8	36.00	54.52	0.660	1.05	Bolt Tension
		Diagonal	A325X	0.7500	1	10.78	14.14	0.763	1.05	Member Bearing
T17	20	Diagonal	A325X	0.7500	3	5.63	24.85	0.227	1.05	Bolt Shear
		Horizontal	A325X	0.7500	2	4.80	24.85	0.193	1.05	Bolt Shear

### Compression Checks

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### Leg Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio P <sub>u</sub> / φP <sub>n</sub>
T1	250 - 240	ROHN 2.5 STD	10.00	5.00	63.3 K=1.00	1.7040	-6.44	57.19	0.113 <sup>1</sup>
T2	240 - 236	ROHN 2.5 STD	4.00	4.00	50.7 K=1.00	1.7040	-14.31	63.56	0.225 <sup>1</sup>
T3	236 - 232	ROHN 2.5 STD	4.00	4.00	50.7 K=1.00	1.7040	-22.69	63.56	0.357 <sup>1</sup>
T4	232 - 228	ROHN 2.5 STD	4.00	4.00	50.7 K=1.00	1.7040	-33.77	63.56	0.531 <sup>1</sup>
T5	228 - 224	ROHN 2.5 STD	4.00	4.00	50.7 K=1.00	1.7040	-45.19	63.56	0.711 <sup>1</sup>
T6	224 - 220	ROHN 2.5 STD	4.00	4.00	50.7 K=1.00	1.7040	-56.69	63.56	0.892 <sup>1</sup>
T7	220 - 200	ROHN 3 EH	20.03	4.01	42.3 K=1.00	3.0159	-92.91	119.06	0.780 <sup>1</sup>
T8	200 - 180	ROHN 3.5 EH	20.03	5.01	46.0 K=1.00	3.6784	-120.83	141.81	0.852 <sup>1</sup>
T9	180 - 160	ROHN 4 EH	20.04	6.68	54.3 K=1.00	4.4074	-145.87	159.91	0.912 <sup>1</sup>
T10	160 - 140	ROHN 5 EH	20.04	6.68	43.6 K=1.00	6.1120	-171.67	239.37	0.717 <sup>1</sup>
T11	140 - 120	ROHN 5 EH	20.03	6.68	43.6 K=1.00	6.1120	-198.64	239.39	0.830 <sup>1</sup>
T12	120 - 100	ROHN 6 EH	20.04	10.02	54.8 K=1.00	8.4049	-223.13	303.73	0.735 <sup>1</sup>
T13	100 - 80	ROHN 6 EH	20.03	10.02	54.8 K=1.00	8.4049	-250.86	303.76	0.826 <sup>1</sup>
T14	80 - 60	ROHN 8 EHS	20.04	10.02	41.2 K=1.00	9.7193	-277.49	386.36	0.718 <sup>1</sup>
T15	60 - 40	ROHN 8 EHS	20.03	10.02	41.2 K=1.00	9.7193	-305.68	386.41	0.791 <sup>1</sup>
T16	40 - 20	ROHN 8 EHS	20.04	10.02	41.2 K=1.00	9.7193	-333.60	386.37	0.863 <sup>1</sup>
T17	20 - 0	ROHN 8 EH	20.06	10.03	41.8 K=1.00	12.7627	-341.73	505.41	0.676 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Diagonal Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio P <sub>u</sub> / φP <sub>n</sub>
T1	250 - 240	L1 1/2x1 1/2x3/16	6.78	3.11	127.2 K=1.00	0.5273	-2.42	9.33	0.259 <sup>1</sup>
T2	240 - 236	L1 1/2x1 1/2x3/16	6.08	2.78	115.2 K=1.01	0.5273	-2.71	11.06	0.245 <sup>1</sup>
T3	236 - 232	L1 1/2x1 1/2x3/16	6.08	2.71	113.3 K=1.02	0.5273	-4.68	11.32	0.414 <sup>1</sup>
T4	232 - 228	L1 1/2x1 1/2x3/16	6.08	2.71	113.3 K=1.02	0.5273	-4.95	11.32	0.437 <sup>1</sup>
T5	228 - 224	L1 1/2x1 1/2x3/16	6.08	2.71	113.3 K=1.02	0.5273	-5.25	11.32	0.464 <sup>1</sup>
T6	224 - 220	L1 1/2x1 1/2x3/16	6.08	2.71	113.3	0.5273	-5.80	11.32	0.512 <sup>1</sup>

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Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T7	220 - 200	L1 1/2x1 1/2x3/16	7.57	3.63	K=1.02 148.5	0.5273	-3.67	6.85	0.537 <sup>1</sup>
T8	200 - 180	L1 3/4x1 3/4x3/16	9.76	4.73	K=1.00 165.2	0.6211	-4.43	6.52	0.680 <sup>1</sup>
T9	180 - 160	L2 1/2x2 1/2x3/16	12.32	6.04	K=1.00 146.4	0.9023	-5.61	12.05	0.465 <sup>1</sup>
T10	160 - 140	L2 1/2x2 1/2x3/16	14.14	6.90	K=1.00 167.4	0.9023	-6.05	9.22	0.656 <sup>1</sup>
T11	140 - 120	L2 1/2x2 1/2x1/4	15.96	7.80	K=1.00 190.7	1.1900	-7.05	9.37	0.753 <sup>1</sup>
T12	120 - 100	L3x3x1/4	19.21	9.47	K=1.00 194.2	1.4400	-8.34	10.93	0.763 <sup>1</sup>
T13	100 - 80	L3 1/2x3 1/2x1/4	20.93	10.31	K=1.00 178.3	1.6900	-9.81	15.21	0.645 <sup>1</sup>
T14	80 - 60	L3 1/2x3 1/2x1/4	22.86	11.20	K=1.00 193.7	1.6900	-9.47	12.89	0.735 <sup>1</sup>
T15	60 - 40	L4x4x1/4	24.68	12.07	K=1.00 182.2	1.9400	-12.14	16.72	0.726 <sup>1</sup>
T16	40 - 20	L4x4x1/4	26.63	13.08	K=1.00 197.5	1.9400	-11.87	14.24	0.834 <sup>1</sup>
T17	20 - 0	ROHN 2.5 STD	24.37	12.18	K=1.00 154.3	1.7040	-16.89	16.17	1.045 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Horizontal Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T17	20 - 0	ROHN 2.5 STD	25.22	12.25	155.2 K=1.00	1.7040	-9.59	15.99	0.600 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Top Girt Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T1	250 - 240	L1 1/2x1 1/2x3/16	4.58	4.13	169.0 K=1.00	0.5273	-0.41	5.28	0.078 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Redundant Horizontal (1) Design Data (Compression)



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Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	KI/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T17	20 - 0	ROHN 1.5 STD	6.31	5.95	114.6 K=1.00	0.7995	-1.50	13.75	0.109 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Redundant Diagonal (1) Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	KI/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T17	20 - 0	ROHN 1.5 STD	11.50	10.77	207.6 K=1.00	0.7995	-0.76	4.19	0.182 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Redundant Hip (1) Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	KI/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T17	20 - 0	ROHN 1.5 STD	6.31	6.31	121.5 K=1.00	0.7995	-0.05	12.23	0.004 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Redundant Hip Diagonal (1) Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	KI/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T17	20 - 0	ROHN 2.5 STD	15.10	15.10	191.2 K=1.00	1.7040	-0.07	10.53	0.007 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Inner Bracing Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	KI/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T17	20 - 0	ROHN 3 STD	12.61	12.61	130.0	2.2285	-0.02	29.77	0.001 <sup>1</sup>

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Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	KI/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
K=1.00									

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Tension Checks

### Leg Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	KI/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T1	250 - 240	ROHN 2.5 STD	10.00	5.00	63.3	1.7040	4.03	76.68	0.053 <sup>1</sup>
T2	240 - 236	ROHN 2.5 STD	4.00	4.00	50.7	1.7040	9.79	76.68	0.128 <sup>1</sup>
T3	236 - 232	ROHN 2.5 STD	4.00	4.00	50.7	1.7040	17.72	76.68	0.231 <sup>1</sup>
T4	232 - 228	ROHN 2.5 STD	4.00	4.00	50.7	1.7040	28.44	76.68	0.371 <sup>1</sup>
T5	228 - 224	ROHN 2.5 STD	4.00	4.00	50.7	1.7040	39.26	76.68	0.512 <sup>1</sup>
T6	224 - 220	ROHN 2.5 STD	4.00	4.00	50.7	1.7040	50.50	76.68	0.659 <sup>1</sup>
T7	220 - 200	ROHN 3 EH	20.03	4.01	42.3	3.0159	84.22	135.72	0.621 <sup>1</sup>
T8	200 - 180	ROHN 3.5 EH	20.03	5.01	46.0	3.6784	109.69	165.53	0.663 <sup>1</sup>
T9	180 - 160	ROHN 4 EH	20.04	6.68	54.3	4.4074	132.06	198.34	0.666 <sup>1</sup>
T10	160 - 140	ROHN 5 EH	20.04	6.68	43.6	6.1120	154.46	275.04	0.562 <sup>1</sup>
T11	140 - 120	ROHN 5 EH	20.03	6.68	43.6	6.1120	177.62	275.04	0.646 <sup>1</sup>
T12	120 - 100	ROHN 6 EH	20.04	10.02	54.8	8.4049	198.22	378.22	0.524 <sup>1</sup>
T13	100 - 80	ROHN 6 EH	20.03	10.02	54.8	8.4049	221.37	378.22	0.585 <sup>1</sup>
T14	80 - 60	ROHN 8 EHS	20.04	10.02	41.2	9.7193	242.94	437.37	0.555 <sup>1</sup>
T15	60 - 40	ROHN 8 EHS	20.03	10.02	41.2	9.7193	265.85	437.37	0.608 <sup>1</sup>
T16	40 - 20	ROHN 8 EHS	20.04	10.02	41.2	9.7193	288.03	437.37	0.659 <sup>1</sup>
T17	20 - 0	ROHN 8 EH	20.06	10.03	41.8	12.7627	291.46	574.32	0.507 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Diagonal Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	KI/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T1	250 - 240	L1 1/2x1 1/2x3/16	6.78	3.11	84.4	0.3076	2.39	13.38	0.179 <sup>1</sup>
T2	240 - 236	L1 1/2x1 1/2x3/16	6.08	2.78	75.7	0.3076	2.73	13.38	0.204 <sup>1</sup>
T3	236 - 232	L1 1/2x1 1/2x3/16	6.08	2.71	75.7	0.3076	4.69	13.38	0.350 <sup>1</sup>
T4	232 - 228	L1 1/2x1 1/2x3/16	6.08	2.71	75.7	0.3076	4.88	13.38	0.365 <sup>1</sup>
T5	228 - 224	L1 1/2x1 1/2x3/16	6.08	2.71	75.7	0.3076	5.27	13.38	0.394 <sup>1</sup>
T6	224 - 220	L1 1/2x1 1/2x3/16	6.08	2.71	75.7	0.3076	5.56	13.38	0.416 <sup>1</sup>
T7	220 - 200	L1 1/2x1 1/2x3/16	6.24	2.98	81.0	0.3076	3.59	13.38	0.268 <sup>1</sup>
T8	200 - 180	L1 3/4x1 3/4x3/16	9.76	4.73	108.0	0.3779	4.35	16.44	0.265 <sup>1</sup>
T9	180 - 160	L2 1/2x2 1/2x3/16	12.32	6.04	94.7	0.5889	5.19	25.62	0.202 <sup>1</sup>
T10	160 - 140	L2 1/2x2 1/2x3/16	14.14	6.90	108.0	0.5889	5.86	25.62	0.229 <sup>1</sup>
T11	140 - 120	L2 1/2x2 1/2x1/4	15.96	7.80	123.4	0.7753	6.92	33.73	0.205 <sup>1</sup>
T12	120 - 100	L3x3x1/4	19.21	9.47	124.2	0.9394	8.14	45.79	0.178 <sup>1</sup>
T13	100 - 80	L3 1/2x3 1/2x1/4	20.93	10.31	114.8	1.1269	9.16	54.94	0.167 <sup>1</sup>

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Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	KI/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T14	80 - 60	L3 1/2x3 1/2x1/4	22.86	11.20	124.8	1.1034	9.29	53.79	0.173 <sup>1</sup>
T15	60 - 40	L4x4x1/4	24.68	12.07	117.2	1.2909	11.10	62.93	0.176 <sup>1</sup>
T16	40 - 20	L4x4x1/4	26.63	13.08	126.9	1.2909	10.78	62.93	0.171 <sup>1</sup>
T17	20 - 0	ROHN 2.5 STD	24.37	12.18	154.3	1.7040	15.71	76.68	0.205 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Horizontal Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	KI/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T17	20 - 0	ROHN 2.5 STD	25.22	12.25	155.2	1.7040	8.55	76.68	0.112 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Top Girt Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	KI/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T1	250 - 240	L1 1/2x1 1/2x3/16	4.58	4.13	114.1	0.3076	0.41	13.38	0.031 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Redundant Horizontal (1) Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	KI/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T17	20 - 0	ROHN 1.5 STD	6.31	5.95	114.6	0.7995	0.90	35.98	0.025 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Redundant Diagonal (1) Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	KI/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T17	20 - 0	ROHN 1.5 STD	11.50	10.77	207.6	0.7995	1.50	35.98	0.042 <sup>1</sup>



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<sup>1</sup>  $P_u / \phi P_n$  controls

### Redundant Hip Diagonal (1) Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	KL/r	A in <sup>2</sup>	P <sub>u</sub> K	$\phi P_n$ K	Ratio $\frac{P_u}{\phi P_n}$
T17	20 - 0	ROHN 2.5 STD	15.10	15.10	191.2	1.7040	0.09	76.68	0.001 <sup>1</sup>

<sup>1</sup>  $P_u / \phi P_n$  controls

### Inner Bracing Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	KL/r	A in <sup>2</sup>	P <sub>u</sub> K	$\phi P_n$ K	Ratio $\frac{P_u}{\phi P_n}$
T17	20 - 0	ROHN 3 STD	12.61	12.61	130.0	2.2285	0.00	100.28	0.000 <sup>1</sup>

<sup>1</sup>  $P_u / \phi P_n$  controls

### Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	$\phi P_{allow}$ K	% Capacity	Pass Fail
T1	250 - 240	Leg	ROHN 2.5 STD	3	-6.44	60.05	10.7	Pass
T2	240 - 236	Leg	ROHN 2.5 STD	21	-14.31	66.74	21.4	Pass
T3	236 - 232	Leg	ROHN 2.5 STD	30	-22.69	66.74	34.0	Pass
T4	232 - 228	Leg	ROHN 2.5 STD	39	-33.77	66.74	50.6	Pass
T5	228 - 224	Leg	ROHN 2.5 STD	48	-45.19	66.74	67.7	Pass
T6	224 - 220	Leg	ROHN 2.5 STD	57	-56.69	66.74	84.9	Pass
T7	220 - 200	Leg	ROHN 3 EH	66	-92.91	125.02	74.3	Pass
T8	200 - 180	Leg	ROHN 3.5 EH	99	-120.83	148.90	81.1	Pass
T9	180 - 160	Leg	ROHN 4 EH	126	-145.87	167.90	86.9	Pass
T10	160 - 140	Leg	ROHN 5 EH	147	-171.67	251.34	68.3	Pass
T11	140 - 120	Leg	ROHN 5 EH	168	-198.64	251.36	79.0	Pass
T12	120 - 100	Leg	ROHN 6 EH	189	-223.13	318.91	70.0	Pass
T13	100 - 80	Leg	ROHN 6 EH	204	-250.86	318.94	78.7	Pass
T14	80 - 60	Leg	ROHN 8 EHS	219	-277.49	405.68	68.4	Pass
T15	60 - 40	Leg	ROHN 8 EHS	234	-305.68	405.73	75.3	Pass
T16	40 - 20	Leg	ROHN 8 EHS	249	-333.60	405.69	82.2	Pass
T17	20 - 0	Leg	ROHN 8 EH	264	-341.73	530.68	64.4	Pass
T1	250 - 240	Diagonal	L1 1/2x1 1/2x3/16	10	-2.42	9.80	24.7	Pass
							70.8 (b)	
							45.8 (b)	
T2	240 - 236	Diagonal	L1 1/2x1 1/2x3/16	24	-2.71	11.61	23.3	Pass
							52.4 (b)	
T3	236 - 232	Diagonal	L1 1/2x1 1/2x3/16	33	-4.68	11.89	39.4	Pass
							65.1 (b)	
T4	232 - 228	Diagonal	L1 1/2x1 1/2x3/16	43	-4.95	11.89	41.7	Pass
							67.6 (b)	

# tnxTower

**Tower Engineering Professionals**  
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Section No.	Elevation ft	Component Type	Size	Critical Element	P K	$\phi P_{allow}$ K	% Capacity	Pass Fail	
T5	228 - 224	Diagonal	L1 1/2x1 1/2x3/16	54	-5.25	11.89	44.2	Pass	
T6	224 - 220	Diagonal	L1 1/2x1 1/2x3/16	62	-5.80	11.89	72.8 (b) 48.8	Pass	
T7	220 - 200	Diagonal	L1 1/2x1 1/2x3/16	72	-3.67	7.19	77.4 (b) 51.1	Pass	
T8	200 - 180	Diagonal	L1 3/4x1 3/4x3/16	105	-4.43	6.84	68.7 (b) 64.8	Pass	
T9	180 - 160	Diagonal	L2 1/2x2 1/2x3/16	132	-5.61	12.66	70.2 (b) 44.3	Pass	
T10	160 - 140	Diagonal	L2 1/2x2 1/2x3/16	153	-6.05	9.68	83.7 (b) 62.5	Pass	
T11	140 - 120	Diagonal	L2 1/2x2 1/2x1/4	174	-7.05	9.83	94.6 (b) 71.7	Pass	
T12	120 - 100	Diagonal	L3x3x1/4	195	-8.34	11.48	94.5 (b) 72.7	Pass	
T13	100 - 80	Diagonal	L3 1/2x3 1/2x1/4	210	-9.81	15.97	78.3 (b) 61.4	Pass	
T14	80 - 60	Diagonal	L3 1/2x3 1/2x1/4	225	-9.47	13.53	70.0	Pass	
T15	60 - 40	Diagonal	L4x4x1/4	240	-12.14	17.55	69.2	Pass	
T16	40 - 20	Diagonal	L4x4x1/4	255	-11.87	14.95	78.5 (b) 79.4	Pass	
T17	20 - 0	Diagonal	ROHN 2.5 STD	285	-16.89	16.97	99.5	Pass	
T17	20 - 0	Horizontal	ROHN 2.5 STD	281	-9.59	16.79	57.1	Pass	
T1	250 - 240	Top Girt	L1 1/2x1 1/2x3/16	5	-0.41	5.55	7.5	Pass	
T17	20 - 0	Redund Horz 1 Bracing	ROHN 1.5 STD	267	-1.50	14.44	8.8 (b) 10.4	Pass	
T17	20 - 0	Redund Diag 1 Bracing	ROHN 1.5 STD	284	-0.76	4.40	17.4	Pass	
T17	20 - 0	Redund Hip 1 Bracing	ROHN 1.5 STD	288	-0.05	12.84	0.4	Pass	
T17	20 - 0	Redund Hip Diagonal 1 Bracing	ROHN 2.5 STD	289	-0.07	11.05	0.7	Pass	
T17	20 - 0	Inner Bracing	ROHN 3 STD	294	-0.01	31.26	0.3	Pass	
							<b>Summary</b>		
							Leg (T9)	86.9	Pass
							Diagonal (T17)	99.5	Pass
							Horizontal (T17)	57.1	Pass
							Top Girt (T1)	8.8	Pass
							Redund Horz 1 Bracing (T17)	10.4	Pass
							Redund Diag 1 Bracing (T17)	17.4	Pass
							Redund Hip 1 Bracing (T17)	0.4	Pass
							Redund Hip Diagonal 1 Bracing (T17)	0.7	Pass
							Inner Bracing (T17)	0.3	Pass
							Bolt Checks	90.1	Pass

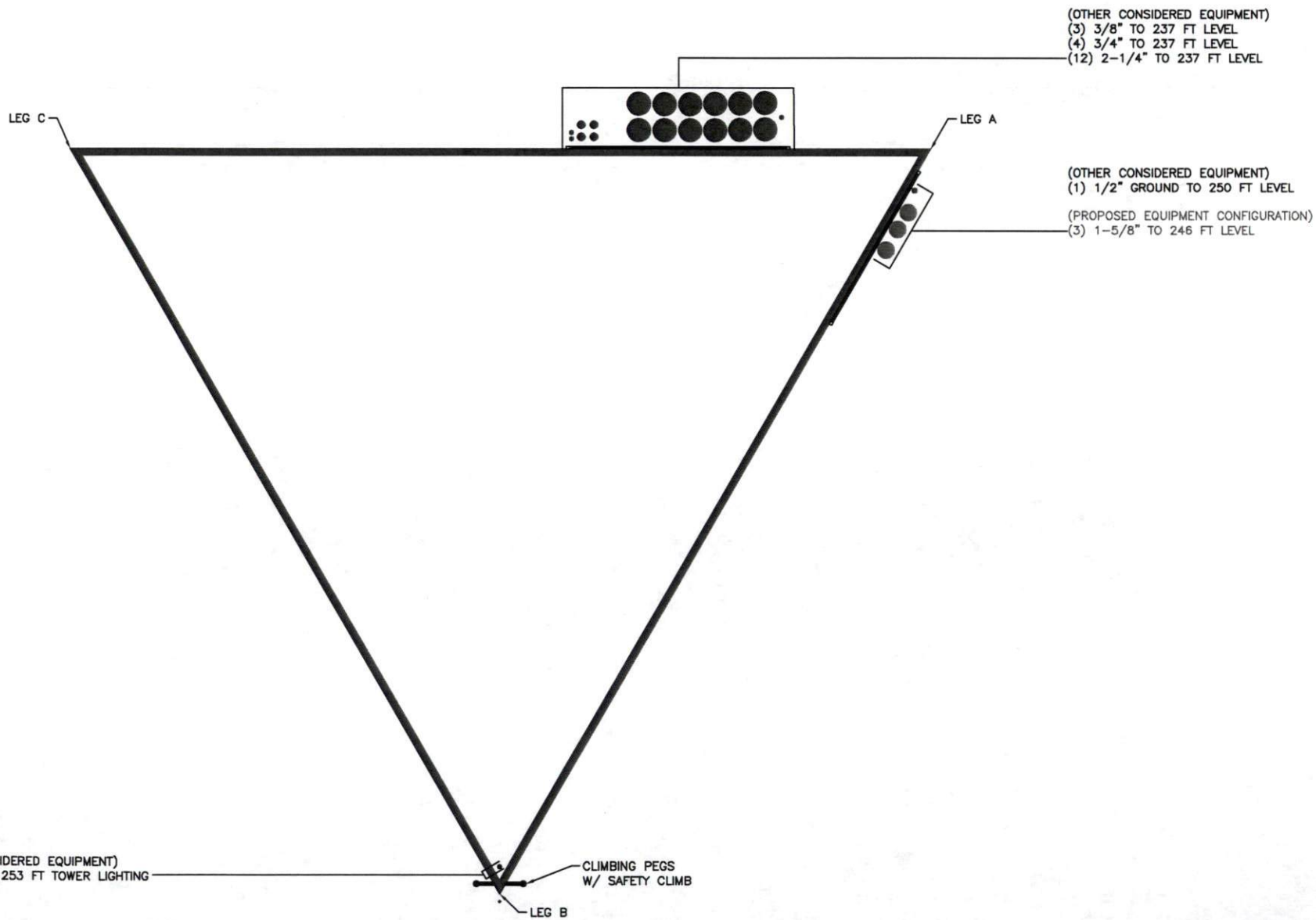
<b>tnxTower</b>  <b>Tower Engineering Professionals</b> 326 Tryon Rd. Raleigh, NC 27603 Phone: (919) 661-6351 FAX: (919) 661-6350	<b>Job</b> (P3) Erwin/McLamb (F1) (BU: 879743)	<b>Page</b> 32 of 32
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Section No.	Elevation ft	Component Type	Size	Critical Element	P K	$\phi P_{allow}$ K	% Capacity	Pass Fail
<b>RATING =</b>							<b>99.5</b>	<b>Pass</b>

Program Version 8.1.1.0 - 6/3/2021 File:C:/Users/jcmcginnis.TOWER/Documents/Structural Analysis/In Progress - 879743/P-280932\_L-565498\_879743\_(P3) ERWINMCLAMB (F1)\_Structural Analysis/tnxTower/(1st) Single Angle, Double Bolt/879743\_1988834\_LC5.eri



**APPENDIX B**  
**BASE LEVEL DRAWING**



**APPENDIX C**  
**ADDITIONAL CALCULATIONS**



# Self Support Anchor Rod Capacity



Site Info	
BU #	879743
Site Name	P3) Erwin/McLamb (F1
Order #	540848 Rev. 0

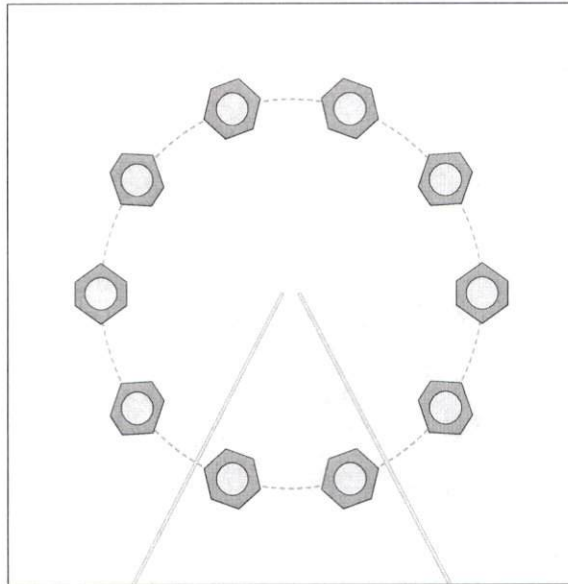
Analysis Considerations	
TIA-222 Revision	H
Grout Considered:	Yes
$l_{ar}$ (in)	0.5

Applied Loads		
	Comp.	Uplift
Axial Force (kips)	0.00	308.00
Shear Force (kips)	0.00	33.00

\*TIA-222-H Section 15.5 Applied

Considered Eccentricity	
Leg Mod Eccentricity (in)	0.000
Anchor Rod N.A Shift (in)	0.000
Total Eccentricity (in)	0.000

\*Anchor Rod Eccentricity Applied



Connection Properties	Analysis Results
-----------------------	------------------

Anchor Rod Data	
(10) 1" $\phi$ bolts (A354-BC N; $F_y=109$ ksi, $F_u=125$ ksi)	
$l_{ar}$ (in):	0.5

Anchor Rod Summary		(units of kips, kip-in)
$P_u_t = 30.8$	$\phi P_n_t = 56.81$	<b>Stress Rating</b>
$V_u = 3.3$	$\phi V_n = 36.82$	<b>51.6%</b>
$M_u = n/a$	$\phi M_n = n/a$	Pass

## Drilled Pier Foundation

BU #:	879743
Site Name:	(P3) ERWIN/MCLAMB(F1)
Order Number:	540848 Rev 0
TIA-222 Revision:	H
Tower Type:	Self Support



Applied Loads		
	Comp.	Uplift
Moment (kip-ft)	0	0
Axial Force (kips)	361	308
Shear Force (kips)	38	33

Material Properties	
Concrete Strength, f <sub>c</sub> :	3 ksi
Rebar Strength, F <sub>y</sub> :	60 ksi
Tie Yield Strength, F <sub>yt</sub> :	60 ksi

Pier Design Data	
Depth	31 ft
Ext. Above Grade	0.5 ft
Pier Section 1	
<i>From 0.5' above grade to 29' below grade</i>	
Pier Diameter	4 ft
Rebar Quantity	18
Rebar Size	9
Rebar Cage Diameter	39 in
Tie Size	5
Tie Spacing	12 in

Rebar & Pier Options

Embedded Pole Inputs

Belled Pier Inputs

Analysis Results		
Soil Lateral Check	Compression	Uplift
D <sub>v=0</sub> (ft from TOC)	15.70	15.70
Soil Safety Factor	26.92	31.00
Max Moment (kip-ft)	360.33	312.91
Rating*	4.7%	4.1%
Soil Vertical Check	Compression	Uplift
Skin Friction (kips)	544.94	544.94
End Bearing (kips)	565.49	-
Weight of Concrete (kips)	53.93	40.44
Total Capacity (kips)	1110.43	585.39
Axial (kips)	414.93	308.00
Rating*	35.6%	50.1%
Reinforced Concrete Flexure	Compression	Uplift
Critical Depth (ft from TOC)	15.97	14.71
Critical Moment (kip-ft)	360.20	311.46
Critical Moment Capacity	1774.43	1192.25
Rating*	19.3%	24.9%
Reinforced Concrete Shear	Compression	Uplift
Critical Depth (ft from TOC)	23.86	23.86
Critical Shear (kip)	48.07	41.75
Critical Shear Capacity	342.93	203.70
Rating*	13.4%	19.5%

Structural Foundation Rating*	24.9%
Soil Interaction Rating*	50.1%

\*Rating per TIA-222-H Section 15.5

Check Limitation	
Apply TIA-222-H Section 15.5:	<input checked="" type="checkbox"/>
N/A	<input type="checkbox"/>
Additional Longitudinal Rebar	
Input Effective Depths (else Actual):	<input type="checkbox"/>
Shear Design Options	
Check Shear along Depth of Pier:	<input checked="" type="checkbox"/>
Utilize Shear-Friction Methodology:	<input type="checkbox"/>
Override Critical Depth:	<input type="checkbox"/>

Go to Soil Calculations

Soil Profile														
Groundwater Depth		12.6		# of Layers		7								
Layer	Top (ft)	Bottom (ft)	Thickness (ft)	Y <sub>soil</sub> (pcf)	Y <sub>concrete</sub> (pcf)	Cohesion (ksf)	Angle of Friction (degrees)	Calculated Ultimate Skin Friction Comp (ksf)	Calculated Ultimate Skin Friction Uplift (ksf)	Ultimate Skin Friction Comp Override (ksf)	Ultimate Skin Friction Uplift Override (ksf)	Ult. Gross Bearing Capacity (ksf)	SPT Blow Count	Soil Type
1	0	2	2	105	150	0	0	0.000	0.000	0.00	0.00			Cohesionless
2	2	5	3	110	150	2	0	1.100	1.100	0.00	0.00			Cohesive
3	5	12.6	7.6	115	150	2.5	0	1.375	1.375	1.38	1.38			Cohesive
4	12.6	14	1.4	52.6	87.6	2.5	0	1.375	1.375	1.38	1.38			Cohesive
5	14	17	3	52.6	87.6	0	42	0.000	0.000	1.70	1.70			Cohesionless
6	17	27	10	57.6	87.6	5.5	0	2.475	2.475	3.03	3.03			Cohesive
7	27	31	4	57.6	87.6	0	45	0.00	0.00	2.50	2.50	60		Cohesionless