AMERICAN BUILDINGS
A NUCOR COMPANY

7200 N Lake Dr, Ste 200 Ph: (706) 562-8020
Columbus, GA 31909 Fax: (706) 562-8017

Version 2020.01.20 (1/20/20)

March 17, 2020

Alexander Design Build. LLC
205 West Main Street
Clayton, NC 27520
Attn.: Kent Alexander

Project Name: Niell's Creek Church Gym
Buildings: A->55'-4"x110'-10"x20'-0"(RCG,4.0:12)

Project Location: Angier, NC 27501
NBG Project #: A20B0267A

This Letter of Design Certification ensures that the materials furnished by the metal building supplier are designed in accordance with the information specified to the metal building supplier on the order documents and summarized by the loading information listed below. The Project Engineer of Record (not the metal building supplier) is responsible for verifying that the building code and design loads meet any and all applicable local requirements.

The Professional Engineer whose seal appears on this Letter of Certification is employed by the metal building manufacturer. and does not serve as or represent the Engineer of Record for this project and shall not be construed as such.

DESIGN LOAD CRITERIA:

Structural Loads Applied in General Accordance with: North Carolina (NCBC 2018)
Risk Category: II - Standard Buildings

PROJECT-WIDE LOADING INFORMATION:

Ground Snow Load: 15.0 psf Snow Exposure Factor,Ce: 0.90 Snow Imp. Factor, Is: 1.00
Roof Live Load: 20.0 psf Reducible As Per Code.

Ultimate Design Wind Velocity: 115 mph Nominal Design Wind Velocity: 89 mph
***Components & Cladding Pressures: 24 psf/ -32 psf
Is Roof to meet UL 90 Requirements?: No Wind Exposure: B

Seismic Criteria: Ss: 0.229 S1: 0.086 • No ground snow included in seismic calculations.
Design Sds / Sd1: 0.244/0.138 Analysis Procedure: Equiv. Lat. Force Procedure
Seis. Imp. Factor, Ie: 1.00 Basic SFRS: Not Detailed for Seismic
Seis. Design Category: C Site Class: D

BUILDING-SPECIFIC LOADING INFORMATION:

	Roof Dead	Collateral Dead		Snow Coefficient		Snow Load (psf)		Wind		Seismic		
Bldg	(psf)*	Pri (psf)	Sec (psf)	Ct	Cs	Ps (psf)	**Pm (psf)	Enclosure	GCpi	R	Cs	V (kips)
A	3.5	3.0	3.0	1.0	1.00	9.45	---	Enclosed	± 0.18	3.00	0.081	8.3

**Primary Structural Not Included*

***P_m is based on the minimum roof snow load calculated per building code or the contract-specified roof snow load, whichever is greater. This value, P_m, is only applied in combination with Dead and Collateral Loads. Roof Snow in other loading conditions is determined per the specified Building Code.*

****Ultimate Design wind pressures to be used for wall exterior component and cladding materials not provided by Metal Building Supplier*

Mezzanine Information:

Floor Dead Load: N/A Floor Collateral Load: N/A Floor Live Load: N/A

Crane Information:

No cranes on building.

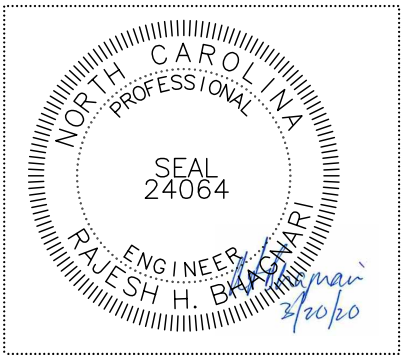
Roof-Top Unit Information

No roof-top units on building.

The design of structural members supporting roof gravity loads is controlled by the more critical effect of roof live load or roof snow applied in accordance with the governing building code.

DESIGN STANDARDS REFERENCED:

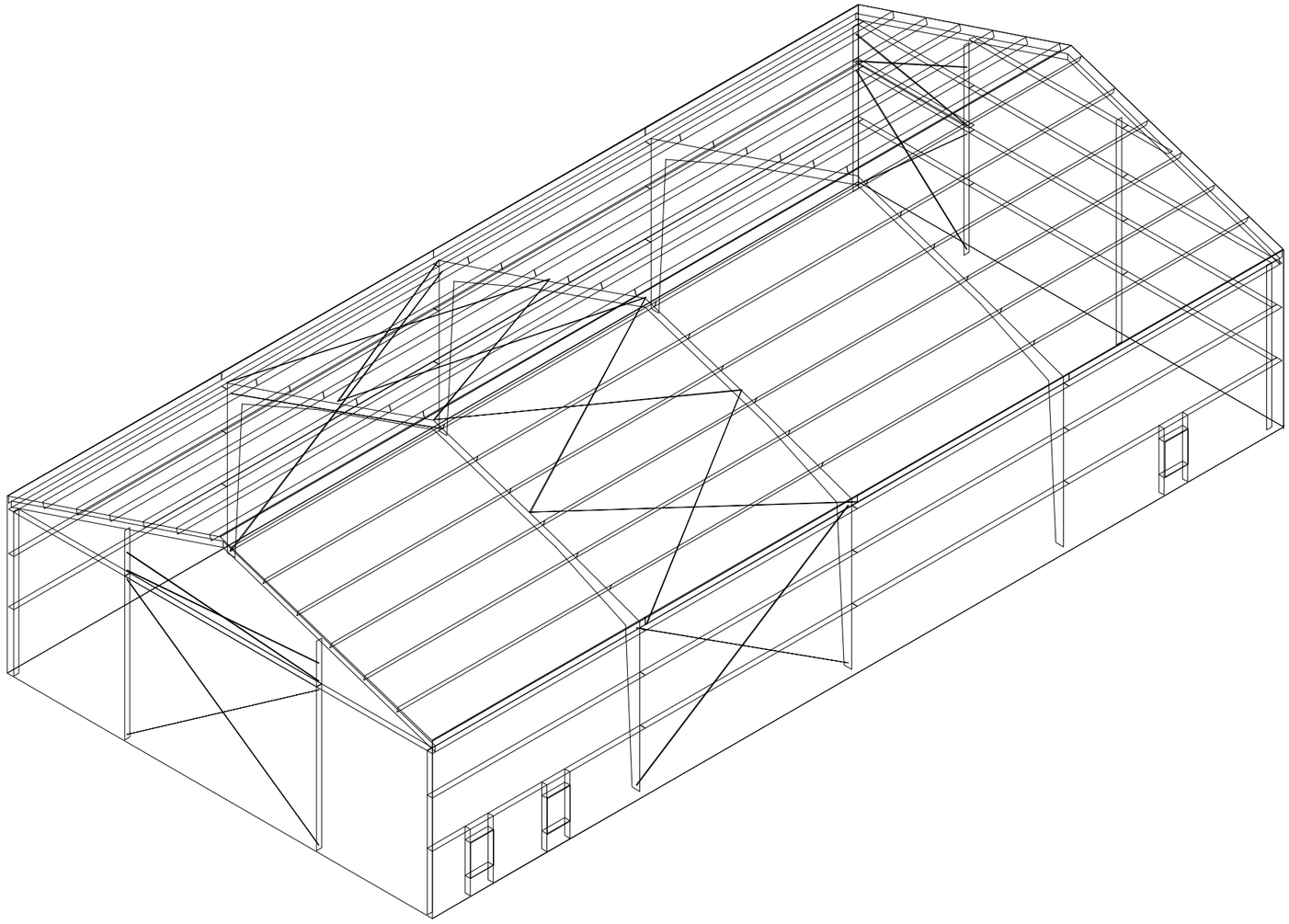
- AISC Specification for Structural Steel Buildings - Steel Construction Manual, 14th Edition, © 2010.
- AISI North-American Specification for the Design of Cold-Formed Steel Structures, © 2012 Edition.
- IBC codes are designed in accordance with ASCE7-10 Edition.
- MBMA Low Rise Building Systems Manual, Latest Edition.
- AWS Latest Edition of Structural Welding Code.
- No buyout structural components provided on this project.



Professional Seal

MBMA METAL BUILDING MANUFACTURERS ASSOCIATION

IAS ACCREDITED Metal Building Systems



BUILDER :ALEXANDER DESIGN BUILD., LLC

CUSTOMER :NIELL'S CREEK CHURCH GYM

LOCATION :ANGIER NC

AN BUILDINGS

A20B0267A

AMERICAN BUILDINGS

PROJECT NUMBER: A20B0267A

PROJECT NAME: Niell's Creek Church Gym

PROJECT LOCATION: Angier, NC 27501

CUSTOMER: Alexander Design Build. LLC

MBMA MEMBER

IAS ACCREDITED Metal Building Systems AC 472

Notes and Specifications

Building Erection Notes

1) The general contractor and/or erector is responsible to safely and properly erect the metal building system in conformance with these drawings, OSHA requirements and metal building system in conformance with these drawings, OSHA requirements and either MBMA or CSA S16 standards pertaining to proper erection. This includes, but is not limited to, the correct use of temporary guys and bracing where needed for squaring, plumbing, and securing the structural and secondary framing. Secondary wall framing members (girts or bar joists) are not designed to function as a work platform or provide safety tie-off attachment in accordance with OSHA requirements. Secondary roof framing members (purlins or bar joists) are not designed to provide safety tie-off attachment in accordance with OSHA requirements.

2) A325 & A490 Bolt tightening requirements:
It is the responsibility of the erector to ensure proper bolt tightness in accordance with applicable regulations. See the RCSC Specification for Structural Joints Using A325 or A490 Bolts for more information.

The following criteria may be used to determine the bolt tightness (i.e., "snug-tight" or "fully-pretensioned"), unless required otherwise by local jurisdiction or contract requirements:
A) All A490 bolts shall be "fully-pretensioned".
B) All A325 bolts in primary framing (rigid frames and bracing) may be "snug-tight", except as follows:
"Fully-pretension" A325 bolts if:
a) Building supports a crane system with a capacity greater than 5 tons.
b) Building supports machinery that creates vibration, impact or stress-reversals on the connections. The Engineer-of-Record for the project should be consulted to evaluate for this condition.
c) The project site is located in a high seismic area. For IBC-based codes, "High Seismic Area" is defined as "Seismic Design Category" of "D", "E", or "F". See the "Building Loads" section of this page for the defined seismic design category for this project.
d) Any connection designated in these drawings as "A325-SC". "Slip-Critical (SC)" connections must be free of paint, oil, or other materials that reduce friction at contact surfaces. Galvanized or lightly rusted surfaces are acceptable.
C) In Canada, all A325 and A490 bolts shall be "fully pre-tensioned", except for secondary members (purlins, girts, opening framing, etc.) and flange braces.
D) Secondary members (purlins, girts, opening framing, etc.) and flange brace connections may always be "snug-tight", unless indicated otherwise in these drawings.

3) The metal building supplier shall be notified prior to any field modifications. Modifications shall be approved by the metal building supplier before work is undertaken.

4) Common Abbreviations:
a) TYP UNO – Typical Unless Noted Otherwise
b) SLV – Short Leg Vertical
c) LLV – Long Leg Vertical
d) NS & FS – Near Side and Far Side
e) O.A.L. – Overall Length
f) SIM – Similar
g) NIC – Not In Contract
h) SL – Steel Line
i) N/A – Not Applicable
j) MBS – Metal Building Supplier

5) Construction loads shall not be placed on any structural steel framework unless such framework is safely bolted, welded, or otherwise adequately secured.

6) Purlins and girts shall not be used as an anchorage point for a fall arrest system unless written approval is obtained from the metal building supplier.

7) Purlins may only be used as a walking/working surface when installing safety systems, after all permanent bridging has been installed and fall protection is provided.

8) Construction loads may be placed only within a zone that is within 8 feet of the center line of the primary support member. CFR bundles should be placed directly over the rigid frames.

9) All lifting devices must meet OSHA or MSHA standards and in no case is it acceptable to use structural members supplied by the MBS as a spreader bar or lifting device.

General Design Notes

1) All structural steel sections and welded plate members are designed in accordance with ANSI/AISC 360 "Specifications for Structural Steel Buildings" or the CAN/CSA S16 "Limit States Design of Steel Structures", as required by the specified building code.
2) All welding of structural steel is based on either AWS D1.1 "Structural Welding Code – Steel" or CAN/CSA W59 "Welded Steel Construction (Metal Arc Welding)", as required by the specified building code.
3) All cold formed members are designed in accordance with ANSI/AISI S100 or CAN/CSA S136 "Specifications for the Design of Cold Formed Steel Structural Members", as required by the specified building code.
4) All welding of cold formed steel is based on AWS D1.3 "Structural Welding Code – Sheet Steel" or CAN/CSA W59 "Welded Steel Construction (Metal Arc Welding)", as required by the specified building code.
5) This Metal Building Supplier facility is IAS AC-472 Accredited and CAN/CSA A660 and W47.1 Certified (if applicable) for the design and manufacturing of Metal Building Systems.
6) If joists are included with this project, they are supplied as a part of the systems engineered metal building and are fabricated in accordance with the requirements of Section 1926.758 of the OSHA safety standards for steel erection, dated January 18, 2001.

Material Specifications

Plate and Flange Material: 5" – 12" Wide, to 1 1/4" Th.	A529 Grade 55
Others	A572 Grade 50
Built-Up Structural Web	A1011 SS (or HSLAS Class 1) Grade 55
Hot-Rolled Structural	A36 or A572 Grade 50 or A992 Grade 50
Structural Tube	A500 Grade B (46 KSI)
Structural Pipe	A500 Grade B (42 KSI)
Cold-Formed Structural	A1011 or A1039 SS (or HSLAS Class 1) or A653 Grade 55
Thru-Fastened Roof Panel	A792 Grade 80
Standing Seam Roof Panel	A792 Grade 50, Class 1
All Wall Panel Profiles	A653 Grade 80, Class 1 or A792 Grade 80, Class 1
Rod Bracing	A529 Grade 50
Welds	AWS D1.1/D1.3 or CSA W59 per Building Code
High-Strength Bolts	A325 Type 1 or A490 Type 1 Heavy Hex
Machine Bolts	A307 Grade A Hex

PRIMARY AND SECONDARY STEEL PRIMER COLOR: RED

ROOF SHEETING, TYPE: S3P 24 GAUGE, FINISH: DARK BRONZE-PVDF

ROOF PANEL CLIP TYPE: TALL

THERMAL BLOCKS: YES EPS FOAM SPACER: ____

COMPOSITE S3P DECK, TYPE: N/A ___GAUGE, FINISH: ____

ROOF LINE TRIM, PAINTED: FOX GRAY-SP

EXTERIOR WALL SHEETING, TYPE: A3P 26 GAUGE, FINISH: GALVALUME PLUS

EXTERIOR WALL CORNER TRIM FINISH: GALVALUME PLUS

EXTERIOR BASE TRIM, PAINTED: BURNISHED SLATE-SP

FRAMED OPENING TRIM, PAINTED: GALVALUME PLUS

WALL FRAMED OPENING, SIZES: FSW (3) 3 W x 4 , window sill at 2

BSW none

LEW none

REW none

INTERIOR WALL SHEETING, TYPE: ____ ___GAUGE, FINISH: ____

INTERIOR CEILING LINER, TYPE: ____ ___GAUGE, FINISH: ____

INTERIOR WALL TRIM, PAINTED: ____

YES	NO		
☒	☐	DOWNSPOUTS PAINTED: FOX GRAY-SP	GUTTERS PAINTED: FOX GRAY-SP
☒	☐	WALKDOORS, QUANTITY: 2	PAINTED: WHITE
☐	☒	WINDOWS: ____	PAINTED: ____
☒	☐	INSULATION (NOT BY MBS),	ROOF: 4 INCH WALLS: 6 INCH
☐	☒	CRANES (SEE CRANE PLAN FOR ADDITIONAL CRANE INFORMATION)	
☐	☒	MEZZANINE (SEE MEZZANINE PLAN FOR ADDITIONAL MEZZANINE INFO)	
☐	☒	WALL TRANSLUCENT PANELS: ____	
☐	☒	ROOF TRANSLUCENT PANELS: ____	
		INSULATED PANELS YES ☐ NO ☐	
☐	☒	PIPE JACKS, SIZE: ____	QUANTITY: ____
☐	☒	ROOF FRAMED OPENINGS, SEE ROOF FRAMING PLAN FOR SIZES	
☐	☒	RIDGE VENTS, 10'-0" LONG X 9" THROAT.	QUANTITY: ____

YES	NO	
☐	☒	FASCIA, PROJECTION: ____ TOP OF FASCIA HEIGHT: ____
		FACE PANEL, TYPE: ____ ___GAUGE, FINISH: ____
		BACK PANEL, TYPE: ____ ___GAUGE, FINISH: ____
		CAP TRIM PAINTED: ____ BASE TRIM PAINTED: ____
☐		CLOSED SYSTEM, CLEAR UNDER SOFFIT TRIM: ____
		SOFFIT PANEL, TYPE: ____ ___GAUGE, FINISH: ____
		SOFFIT TRIM AT BUILDING LINE PAINTED: ____
☐		OPEN SYSTEM, (NO SOFFIT PANEL PROVIDED)
		CLEAR UNDER FASCIA: ____
☐	☒	PARAPET SYSTEM
☐		STRUCTURAL PARAPET ☐ NON-STRUCTURAL PARAPET
		TOP OF PARAPET HEIGHT: ____
		BACKER PANEL, TYPE: ____ ___GAUGE, FINISH: ____
☐	☒	CANOPY, PROJECTION: ____
		AT EAVE LINE ☐ BELOW EAVE ☐
		ROOF PANEL, TYPE: ____ ___GAUGE, FINISH: ____
		SOFFIT PANEL, TYPE: ____ ___GAUGE, FINISH: ____
		SOFFIT TRIM AT BUILDING LINE PAINTED: ____
		CLEAR UNDER CANOPY BEAM: ____
☐	☒	EAVE EXTENSION, PROJECTION: ____
		SOFFIT PANEL, TYPE: ____ ___GAUGE, FINISH: ____
		SOFFIT TRIM AT BUILDING LINE PAINTED: ____
☐	☒	RAKE EXTENSION, PROJECTION: ____
		SOFFIT PANEL, TYPE: ____ ___GAUGE, FINISH: ____
		SOFFIT TRIM AT BUILDING LINE PAINTED: ____
☐	☒	PARTITION WALL SHEETING
		PANEL TYPE: ____ ___GAUGE, FINISH: ____
		PARTITION WALL TRIM COLOR: ____
☐	☒	WAINSCOT
		WALL PANEL, TYPE: ____ ___GAUGE, FINISH: ____
		BASE TRIM PAINTED: ____ JAMB TRIM PAINTED: ____
		TRANSITION TRIM PAINTED: ____

FOR OCCUPANCY (RISK) CATEGORY I OR II, IBC PROVISIONS INDICATE THAT SINGLE-STORY BUILDINGS SHALL HAVE "NO DRIFT LIMIT" PROVIDED THAT INTERIOR WALLS, PARTITIONS, CEILINGS, AND EXTERIOR WALL SYSTEMS HAVE BEEN DESIGNED TO ACCOMMODATE THE SEISMIC STORY DRIFTS. INTERIOR WALLS, PARTITIONS, CEILINGS, OR EXTERIOR WALL SYSTEMS NOT PROVIDED BY THE METAL BUILDING MANUFACTURER SHALL BE DESIGNED AND DETAILED BY OTHERS TO ACCOMMODATE THE SEISMIC STORY DRIFTS. SEISMIC DRIFT VALUES MAY BE OBTAINED FROM THE METAL BUILDING MANUFACTURER.

THIS BUILDING SYSTEM IS BASED ON UNIFORMLY APPLYING THE CONTRACT-SPECIFIED LIVE LOAD AND ROOF SNOW LOAD. IN ADDITION, THE DESIGN IS BASED ON APPLYING A CODE-DEFINED LIVE LOAD (INCLUDING APPLICABLE REDUCTIONS) AND A CODE-DEFINED SNOW LOAD (BASED ON CONTRACT-SPECIFIED GROUND SNOW) FOR ALL PARTIAL LOADING AND UNBALANCED SNOW LOAD CONDITIONS.

THE WALL SYSTEM BY OTHERS MUST WEIGH NO MORE THAN _55_ PSF.

THE SPANDREL BEAMS AND/OR SPANDREL CHANNELS SUPPORTING THE TOP OF THE MASONRY WALLS MUST BE ATTACHED TO THE WALLS WITH A SPACING NOT TO EXCEED 4'-0" O.C. (MAX.). THE SPANDRELS MUST ALSO BE RIGIDLY ATTACHED TO THE WALL NO MORE THAN 6" AWAY FROM EACH PAIR OF INTERMEDIATE STIFFENERS. THIS ATTACHMENT IS DESIGNED AND PROVIDED BY OTHERS (NOT BY THE METAL BUILDING MANUFACTURER). FIELD DRILLING OF THE SPANDRELS FOR A BOLTED CONNECTION WILL BE REQUIRED.

IF SNOW GUARDS OR OTHER DEVICES INTENDED TO HOLD SNOW AND/OR ICE ACCUMULATION ON THE ROOF SYSTEM ARE TO BE USED ON THIS PROJECT, THEY MUST BE INSTALLED UNDER THE GUIDANCE OF THE PROJECT "ENGINEER OF RECORD" (EOR), NOT THE METAL BUILDING MANUFACTURER, SO AS NOT TO EXCEED THE DESIGN ROOF SNOW LOAD ON THIS PROJECT.

ACCESSORIES (DOORS, WINDOWS, ETC.) NOT PROVIDED BY THE METAL BUILDING MANUFACTURER MUST BE DESIGNED AS "COMPONENTS AND CLADDING" IN ACCORDANCE WITH THE SPECIFIC WIND PROVISIONS OF THE REFERENCED BUILDING CODE DISPLAYED ON THE COVER PAGE OF THIS DRAWING PACKET.

FRAMED OPENINGS HAVE BEEN DESIGNED TO SUPPORT WIND LOAD NORMAL TO THE WALL BASED ON THE STANDARD BUILDING CODE CRITERIA. FRAMED OPENINGS HAVE NOT BEEN DESIGNED FOR ANY ADDITIONAL MOMENT OR CATENARY FORCES FROM THE DOOR. ANY CHANGE TO THE INFORMATION SHOWN HERE WILL REQUIRE AN ENGINEERING INVESTIGATION AND POSSIBLE BUILDING REINFORCEMENT.

ABC Design Approved
Joseph Ferrell
03/20/2020 10:33:08 AM

NORTH CAROLINA PROFESSIONAL SEAL 24064 ENGINEER RAJESH H. BHAGNARI

3/20/20

BUILDING LOADS

DESIGN CODE: NCBC 18

ROOF LIVE LOAD: 20.00 PSF MBMA OCC. CLASS: II

LIVE LOAD REDUCIBLE Yes_____

GROUND SNOW LOAD: 15.0 PSF SNOW EXP. FACTOR, Ce: 0.9000

SNOW IMPORTANCE FACTOR, Is: 1.00

WIND: 115 / 89 MPH
(Vult) / (Vasd)

C & C PRESSURES (PSF): 24 / -32

EXPOSURE: B

UL 90 NO

L3P Roof-Const. No.161 ; L3P Roof w/ Translucent Panel-Const. No.167
S3P Roof-Const. No.552 ; S3P Roof w/ Translucent Panel-Const. No.590 ;
Composite CFR Roof-Const. No.552A ; N/A Roof-Const. No. .

SEISMIC INFORMATION Ss: 0.229 S1: 0.086

Design Sds/Sd1: 0.244 / 0.138 Site Class: D

Seismic Imp. Factor: 1.00 Seismic Design Category: C

Analysis Procedure: Equivalent Lateral Force Method

Basic SFRS: Not Detailed for Seismic

NOTES:
1) COLLATERAL DEAD LOADS, UNLESS OTHERWISE NOTED, ARE ASSUMED TO BE UNIFORMLY DISTRIBUTED. WHEN SUSPENDED SPRINKLER SYSTEMS, LIGHTING, HVAC EQUIPMENT, CEILINGS, ETC., ARE SUSPENDED FROM ROOF MEMBERS, CONSULT THE M.B.S. IF THESE CONCENTRATED LOADS EXCEED 500 POUNDS (USING THE WEB MOUNT DETAIL) OR 200 POUNDS (USING THE FLANGE MOUNT DETAIL), OR IF INDIVIDUAL MEMBERS ARE LOADED SIGNIFICANTLY MORE THAN OTHERS.

2) THE DESIGN OF STRUCTURAL MEMBERS SUPPORTING GRAVITY LOADS IS CONTROLLED BY THE MORE CRITICAL EFFECT OF ROOF LIVE LOAD OR ROOF SNOW LOAD, AS DETERMINED BY THE APPLICABLE CODE.

3) Pm IS BASED ON THE MINIMUM ROOF SNOW LOAD CALCULATED PER BUILDING CODE OR THE CONTRACT SPECIFIED SNOW LOAD, WHICHEVER IS GREATER. THIS VALUE, Pm, IS ONLY APPLIED IN COMBINATION WITH THE DEAD AND COLLATERAL LOADS. ROOF SNOW IN OTHER LOADING CONDITIONS IS DETERMINED PER THE SPECIFIED BUILDING CODE.

	BUILDING
ROOF DEAD (PSF):	3.500
PRI. COL. (PSF):	3
SEC. COL. (PSF):	3
SNOW Ct:	1.00
SNOW Cs:	1.00
ROOF SNOW Ps (PSF):	9.45
ROOF SNOW Pm (PSF):	0.00
WIND ENCLOSURE:	Closed
GCpi:	+/-0.18
SEISMIC R:	3
SEISMIC Cs:	0.082
BASE SHEAR (KIPS):	8.25

DRAWING INDEX

ISSUE	DWN	CHK	ENG	PE	DATE
RELEASE FOR CONST. (ABP)	BR	TR	JRF	RHB	3/17/20
RELEASE FOR PERMIT	BR	TR	JRF	RHB	3/17/20

AMERICAN BUILDINGS COMPANY
7200 N. LAKE DRIVE STE. 200
COLUMBUS, GA 31909
PHONE: (706) 562-8020
FAX: (706) 562-8017

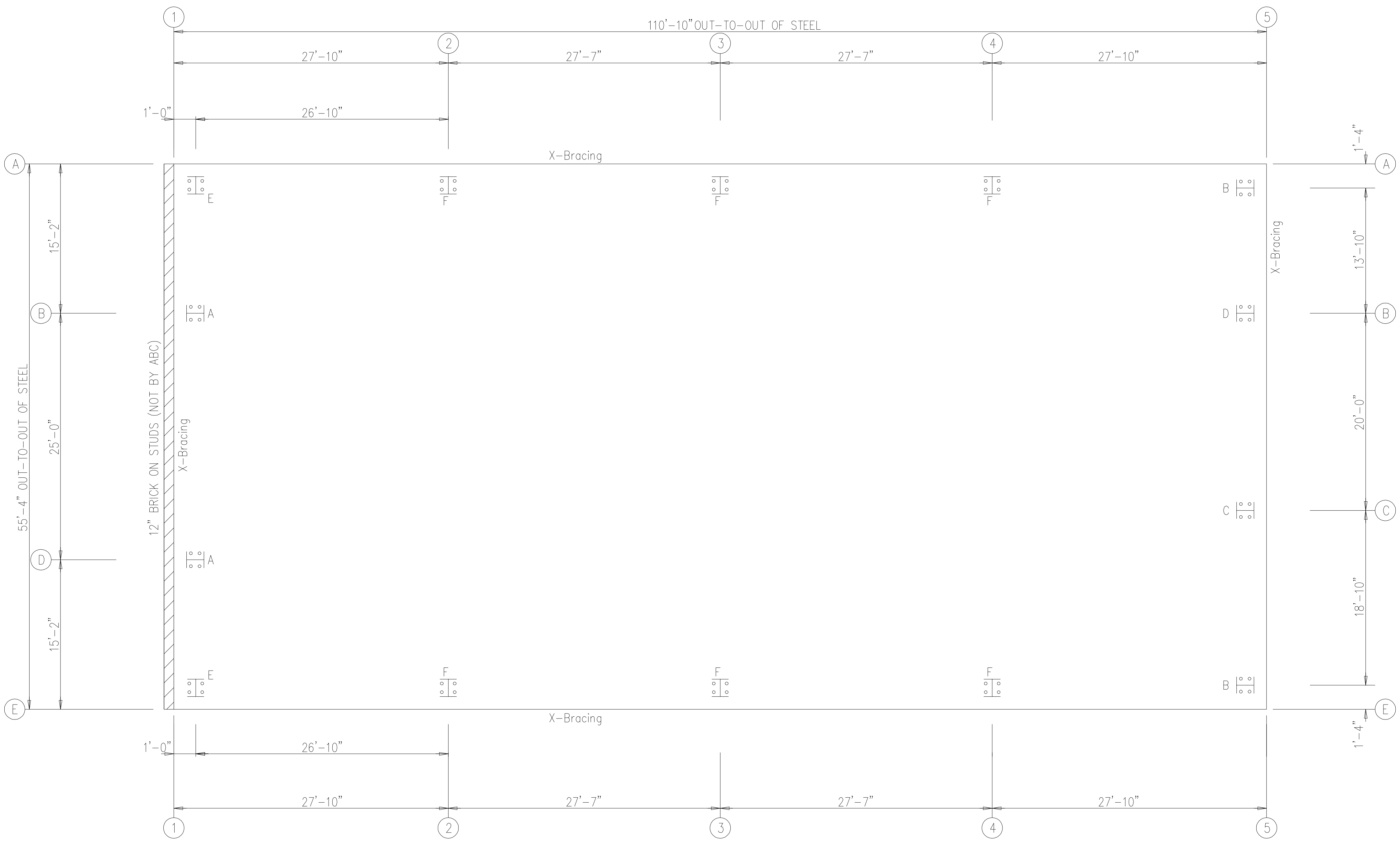
PROJECT NAME: NIELL'S CREEK CHURCH GYM
ANGIER, NC 27501

CUSTOMER NAME: ALEXANDER DESIGN BUILD. LLC
CLAYTON, NC 27520

JOB NUMBER: A20B0267A

SHEET TITLE: COVER

SHEET: C-1



ANCHOR BOLT PLAN
NOTE: All Base Plates @ 100'-0" (U.N.)

o Dia= 3/4"

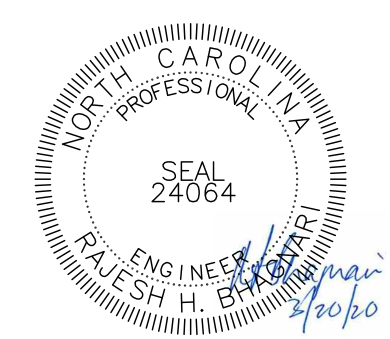
ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type
o 32	Endwall	3/4"	F1554
o 24	Frame	3/4"	F1554

ANCHOR BOLT PLAN

GENERAL NOTES

1. THE SPECIFIED ANCHOR ROD DIAMETER ASSUMES F1554 GRADE 36 UNLESS NOTED OTHERWISE. ANCHOR ROD MATERIAL OF EQUAL DIAMETER MEETING OR EXCEEDING THE STRENGTH REQUIREMENTS SET FORTH ON THESE DRAWINGS MAY BE UTILIZED AT THE DISCRETION OF THE FOUNDATION DESIGN ENGINEER. ANCHOR ROD EMBEDMENT LENGTH SHALL BE DETERMINED BY THE FOUNDATION DESIGN ENGINEER.
2. METAL BUILDING MANUFACTURER IS NOT RESPONSIBLE FOR PROJECT FOUNDATION DESIGN. THE FOUNDATION DESIGN IS THE RESPONSIBILITY OF A REGISTERED PROFESSIONAL ENGINEER, FAMILIAR WITH LOCAL SITE CONDITIONS.
3. ALL ANCHOR RODS, FLAT WASHERS FOR ANCHOR RODS, EXPANSION BOLTS, AS WELL AS ALL CONCRETE/MASONRY EMBEDMENT PLATES ARE NOT BY METAL BUILDING MANUFACTURER.
4. THIS DRAWING IS NOT TO SCALE.
5. FINISHED FLOOR ELEVATION = 100'-0" UNLESS NOTED OTHERWISE.
6. "SINGLE" CEE COLUMNS SHALL BE ORIENTED WITH THE "TOES" TOWARD THE LOW EAVE UNLESS NOTED OTHERWISE.
7. ANCHOR RODS ARE REQUIRED ONLY IN THE QUANTITIES SPECIFIED. BASEPLATES MAY BE FABRICATED WITH MORE HOLES THAN NEEDED FOR THIS PROJECT.
8. THE ANCHOR BOLT LOCATIONS PROVIDED BY METAL BUILDING MANUFACTURER SATISFY PERTINENT REQUIREMENTS FOR THE DESIGN OF THE MATERIALS SUPPLIED BY THE METAL BUILDING MANUFACTURER. PLEASE NOTE THAT THESE REQUIREMENTS MAY NOT SATISFY ALL ANCHOR BOLT CONCRETE EDGE DISTANCE REQUIREMENTS DEPENDING ON THE DETAILS OF THE FOUNDATION DESIGN. BECAUSE FOUNDATION DESIGN IS NOT WITHIN THE METAL BUILDING MANUFACTURER'S SCOPE OF WORK, IT IS THE RESPONSIBILITY OF THE QUALIFIED PROFESSIONAL DESIGNING THE FOUNDATION TO MAKE CERTAIN THAT SUFFICIENT CONCRETE EDGE DISTANCE IS PROVIDED FOR THE ANCHOR BOLTS IN THE DETAILS OF THE FOUNDATION DESIGN.



ISSUE	DWN	CHK	ENG	PE	DATE
RELEASE FOR CONST. (ABP)	BR	TR	JRF	RHB	3/17/20

AMERICAN BUILDINGS COMPANY
7200 N. LAKE DRIVE STE. 200
COLUMBUS, GA 31909
PHONE: (706) 562-8020
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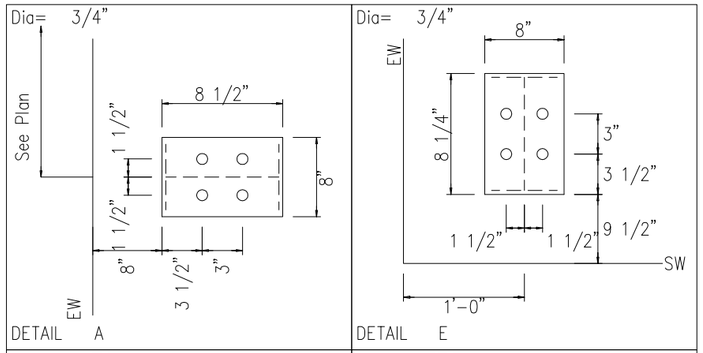
PROJECT NAME
NIELL'S CREEK CHURCH GYM
ANGIER, NC 27501

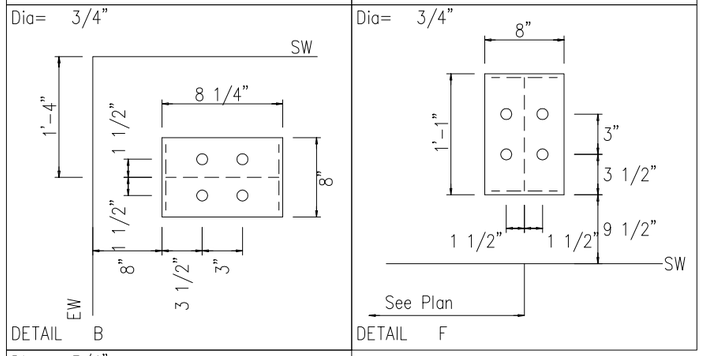
CUSTOMER NAME
ALEXANDER DESIGN BUILD. LLC
CLAYTON, NC 27520

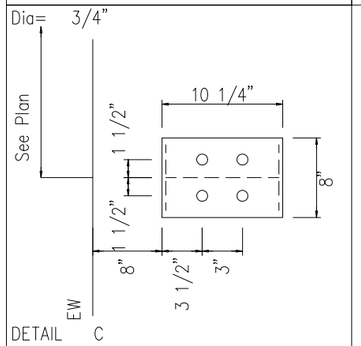
JOB NUMBER
A20B0267A

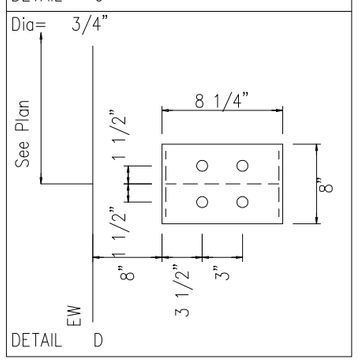
SHEET TITLE
ANDWG-1

SHEET
AB-1





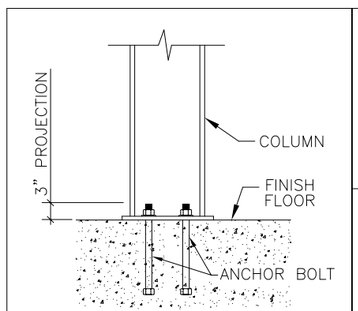




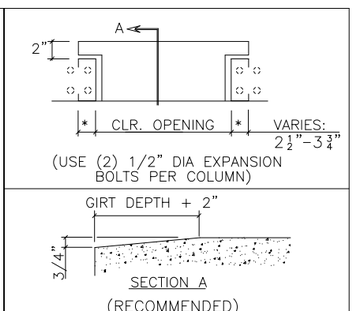
FOUNDATION DESIGN NOTES:

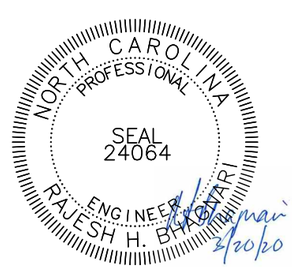
1. THE ORIENTATION OF THE ANCHOR BOLT DETAILS SHOWN ON THIS PAGE MAY NOT COINCIDE WITH THE ACTUAL COLUMN ORIENTATION SHOWN ON THE ANCHOR BOLT DRAWING. PLEASE REFERENCE THE SIDEWALL (SW) AND ENDWALL (EW) STEEL LINES SHOWN ON THE ANCHOR BOLT DETAILS WITH THE ANCHOR BOLT PLAN DURING LAYOUT OF COLUMN AND ANCHOR BOLT LOCATIONS.

2. COLUMN BASE PLATES MAY HAVE MORE HOLES THAN ARE REQUIRED DUE TO PRODUCTION LIMITATIONS. PLEASE FOLLOW ANCHOR BOLT DETAILS FOR QUANTITY OF ANCHOR BOLTS REQUIRED. EXTRA BASE PLATE HOLES DO NOT NEED INFILLED PER THE MBS DESIGN SPECIFICATIONS.



TYPICAL COLUMN BASE PLATE DETAIL





ISSUE	DWN	CHK	ENG	PE	DATE
RELEASE FOR CONST. (ABP)	BR	TR	JRF	RHB	3/17/20

AMERICAN BUILDINGS COMPANY
7200 N. LAKE DRIVE STE. 200
COLUMBUS, GA 31909
PHONE: (706) 562-8020
FAX: (706) 562-8017

PROJECT NAME
NIELL'S CREEK CHURCH GYM
ANGIER, NC 27501

CUSTOMER NAME
ALEXANDER DESIGN BUILD. LLC
CLAYTON, NC 27520

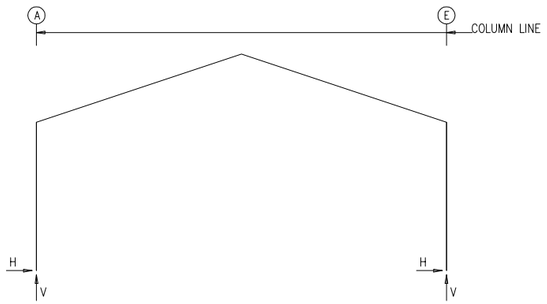
JOB NUMBER
A20B0267A

SHEET TITLE

SHEET

AB-2

FRAME LINES: 2 3 4



RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc._Bolt Qty	Dia	Base_Plate (in) Width	Length	Thick	Grout (in)
2*	A	4	0.750	8.000	13.00	0.375	0.0
2*	E	4	0.750	8.000	13.00	0.375	0.0

2* Frame lines: 2 3 4

ENDWALL COLUMN: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc._Bolt Qty	Dia	Base_Plate (in) Width	Length	Thick	Grout (in)
1	A	4	0.750	8.000	8.250	0.375	0.0
1	B	4	0.750	8.000	8.500	0.375	0.0
1	D	4	0.750	8.000	8.500	0.375	0.0
1	E	4	0.750	8.000	8.250	0.375	0.0
5	E	4	0.750	8.000	8.250	0.375	0.0
5	C	4	0.750	8.000	10.25	0.375	0.0
5	B	4	0.750	8.000	8.250	0.375	0.0
5	A	4	0.750	8.000	8.250	0.375	0.0

GENERAL NOTES

1. ALL LOADING CONDITIONS ARE EXAMINED. THE MAXIMUM AND MINIMUM HORIZONTAL (H) AND VERTICAL (V) REACTIONS AND THE CORRESPONDING VERTICAL (V) OR HORIZONTAL (H) REACTIONS ARE REPORTED.
2. REACTIONS ARE PROVIDED BY LOAD CASE IN ORDER TO AID THE FOUNDATION ENGINEER IN DETERMINING THE APPROPRIATE LOAD FACTORS AND COMBINATION TO BE USED WITH EITHER WORKING STRESS OR ULTIMATE STRENGTH DESIGN METHODS. WIND LOAD CASES ARE GIVEN FOR EACH PRIMARY WIND DIRECTION.
3. FOR ASCE7-10 AND LATER BASED BULDING CODES THE UNFACTORED LOAD CASE REACTIONS DUE TO WIND ARE GENERATED USING ULTIMATE DESIGN WIND SPEEDS (Vult).
4. POSITIVE (+) REACTIONS ARE AS SHOWN ABOVE. FOUNDATION LOADS ARE IN OPPOSITE DIRECTIONS.
5. BRACING REACTIONS ARE IN THE PLANE OF THE BRACE WITH THE HORIZONTAL REACTION (H) ACTING AWAY FROM THE BRACED BAY AND THE VERTICAL REACTION (V) ACTING DOWNWARD.

****** RIGID FRAME LOAD CASE ABBREVIATIONS: ******
Wind_L1/Wind_R1: LATERAL WIND FROM THE LEFT/RIGHT, CASE 1
Wind_L2/Wind_R2: LATERAL WIND FROM THE LEFT/RIGHT, CASE 2
Wind_Ln1/Wind_Ln2: LONGITUDINAL WIND, CASE 1/2
Seismic_L/Seismic_R: LATERAL SEISMIC LOAD FROM LEFT/RIGHT
LWIND#_L#E/LWIND#_R#E: LONGITUDINAL WIND EDGE ZONES
F#UNB_SL_L/F#UNB_SL_R: UNBALANCED ROOF SNOW WITH WIND FROM LEFT/RIGHT
F#PAT_LL #/F#PAT_SL #: PARTIAL LIVE/SNOW LOADING FOR CONTINUOUS BEAM SYSTEMS

****** ENDWALL COLUMN LOAD CASE ABBREVIATIONS: ******
Collat: COLLATERAL LOAD
Rafter Wind_L/Rafter Wind_R: LATERAL WIND FROM THE LEFT/RIGHT
Brace Wind_L/Brace Wind_R: LATERAL WIND FROM THE LEFT/RIGHT
Wind_P/Wind_S: LONGITUDINAL WIND PRESSURE/SUCTION ON COLUMNS
Wind_Ln: LONGITUDINAL WIND SUCTION ON ROOF
Seis_L/Seis_R: LATERAL SEISMIC LOAD FROM LEFT/RIGHT
E#UNB_SL_L/E#UNB_SL_R: UNBALANCED ROOF SNOW WITH WIND FROM LEFT/RIGHT
E#PAT_LL #/E#PAT_SL #: PARTIAL LIVE/SNOW LOADING FOR CONTINUOUS BEAM SYSTEMS

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Snow Horiz	Snow Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert
2*	A	1.4	3.9	1.0	2.5	3.8	9.7	3.0	7.6	−9.2	−15.0	2.3	−9.4
2*	E	−1.4	3.9	−1.0	2.5	−3.8	9.7	−3.0	7.6	−2.3	−9.4	9.2	−15.0

Frame Line	Column Line	Wind_Left2 Horiz	Wind_Left2 Vert	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	Seismic_Left Horiz	Seismic_Left Vert	Seismic_Right Horiz	Seismic_Right Vert
2*	A	−9.9	−9.2	1.6	−3.6	0.9	−12.1	−0.9	−11.0	−0.7	−0.5	0.7	0.5
2*	E	−1.6	−3.6	9.9	−9.2	0.9	−11.0	−0.9	−12.1	−0.7	0.5	0.7	−0.5

Frame Line	Column Line	F1UNB_SL_L Horiz	F1UNB_SL_L Vert	F1UNB_SL_R Horiz	F1UNB_SL_R Vert
2*	A	2.7	7.6	2.7	4.6
2*	E	−2.7	4.6	−2.7	7.6

2* Frame lines: 2 3 4

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Snow Vert	Wind Left1 Vert	Wind Right1 Vert	Wind Left2 Vert	Wind Right2 Vert	Wind Press Horz	Wind Suct Horz	Wind Long1 Vert	Wind Long2 Vert
1	A	0.5	0.2	1.4	0.7	−1.3	−2.1	−0.1	−0.9	−1.5	1.8	−2.0	−1.1
1	B	2.1	1.1	6.9	3.3	−7.7	−4.6	−5.9	−2.8	−4.6	5.1	−7.0	−4.8
1	D	2.1	1.1	6.9	3.3	−4.6	−7.7	−2.8	−5.9	−4.6	5.1	−4.8	−7.0
1	E	0.5	0.2	1.4	0.7	−2.1	−1.3	−0.9	−0.1	−1.5	1.8	−1.1	−2.0

Frm Line	Col Line	Seis Left Vert	Seis Right Vert	E1UNB_SL_L Horz	E1UNB_SL_L Vert	E1UNB_SL_R Horz	E1UNB_SL_R Vert	E1PAT_LL_1 Horz	E1PAT_LL_1 Vert	E1PAT_LL_2 Horz	E1PAT_LL_2 Vert	E1PAT_LL_3 Horz	E1PAT_LL_3 Vert
1	A	0.3	0.1	0.0	0.5	0.0	0.0	0.0	2.2	0.0	−0.8	0.0	1.3
1	B	−0.5	0.0	0.0	4.0	0.0	1.9	0.0	2.4	0.0	4.6	0.0	7.2
1	D	0.0	−0.5	0.0	1.9	0.0	4.0	0.0	2.4	0.0	4.6	0.0	4.3
1	E	0.1	0.3	0.0	0.0	0.0	0.5	0.0	2.2	0.0	−0.8	0.0	−0.7

Frm Line	Col Line	E1PAT_LL_4 Horz	E1PAT_LL_4 Vert
1	A	0.0	−0.7
1	B	0.0	4.3
1	D	0.0	7.2
1	E	0.0	1.3

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Snow Vert	Wind Left1 Vert	Wind Right1 Vert	Wind Left2 Vert	Wind Right2 Vert	Wind Press Horz	Wind Suct Horz	Wind Long1 Vert	Wind Long2 Vert
5	E	0.8	0.4	2.2	1.1	−2.5	−3.2	−1.0	−1.6	−2.0	2.3	−3.2	−1.8
5	C	2.0	1.1	6.7	3.2	−7.3	−3.9	−5.7	−2.3	−5.0	5.5	−6.4	−4.6
5	B	1.5	0.9	5.5	2.6	−4.3	−6.5	−2.8	−5.0	−4.1	4.5	−3.4	−6.3
5	A	0.6	0.3	1.7	0.8	−1.3	−1.8	0.0	−0.6	−1.5	1.8	−1.5	−1.8

Frm Line	Col Line	Seis Left Vert	Seis Right Vert	E2UNB_SL_L Horz	E2UNB_SL_L Vert	E2UNB_SL_R Horz	E2UNB_SL_R Vert	E2PAT_LL_1 Horz	E2PAT_LL_1 Vert	E2PAT_LL_2 Horz	E2PAT_LL_2 Vert	E2PAT_LL_3 Horz	E2PAT_LL_3 Vert
5	E	0.1	−0.1	0.0	1.1	0.0	0.2	0.0	2.7	0.0	−0.3	0.0	2.3
5	C	−0.1	0.1	0.0	3.9	0.0	1.9	0.0	3.3	0.0	3.2	0.0	6.9
5	B	−0.2	−0.1	0.0	1.1	0.0	3.4	0.0	2.0	0.0	3.5	0.0	2.8
5	A	0.3	0.0	0.0	0.2	0.0	0.7	0.0	2.2	0.0	−0.5	0.0	−0.3

Frm Line	Col Line	E2PAT_LL_4 Horz	E2PAT_LL_4 Vert
5	E	0.0	−0.3
5	C	0.0	2.9
5	B	0.0	6.1
5	A	0.0	1.5

BUILDING BRACING REACTIONS

Wall Loc	Wall Line	Col Line	± Reactions(k) Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Panel_Shear (lb/ft) Wind	Panel_Shear (lb/ft) Seis
L_EW	1	B,D	3.6	3.4	3.9	3.6		
F_SW	E	2,3	5.4	3.5	4.3	2.8		
R_EW	5	B,A	3.5	5.7	0.9	1.5		
B_SW	A	3,2	5.4	3.5	4.3	2.8		

ISSUE	DWN	CHK	ENG	PE	DATE
RELEASE FOR CONST. (ABP)	BR	TR	JRF	RHB	3/17/20

AMERICAN BUILDINGS COMPANY
7200 N. LAKE DRIVE STE. 200
COLUMBUS, GA 31909
PHONE: (706) 562-8020
FAX: (706) 562-8017

PROJECT NAME
NIELL'S CREEK CHURCH GYM
ANGIER, NC 27501

CUSTOMER NAME
ALEXANDER DESIGN BUILD. LLC
CLAYTON, NC 27520

JOB NUMBER
A20B0267A

SHEET TITLE
ANDWG-3

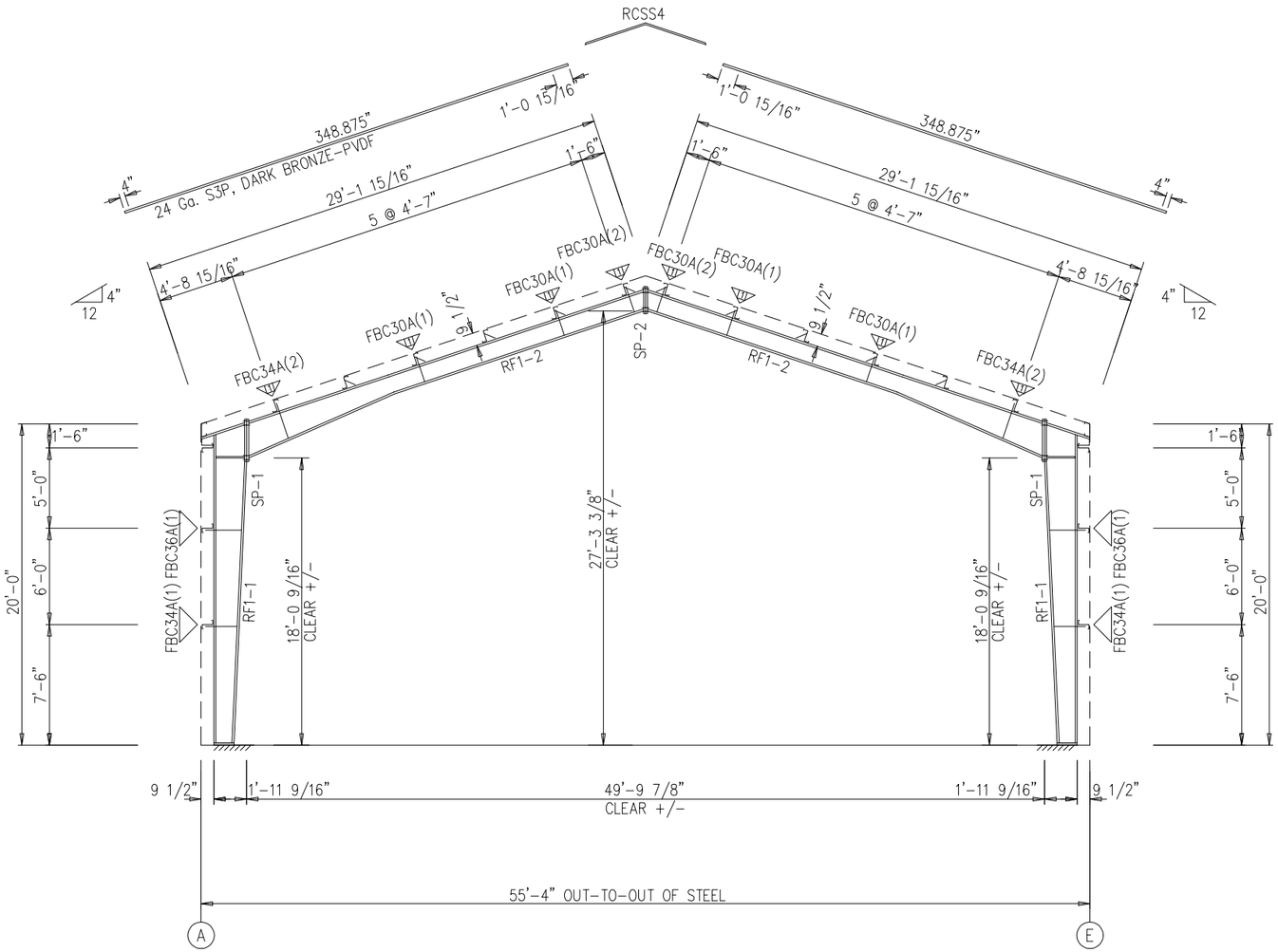
SHEET
AB-3

NORTH CAROLINA PROFESSIONAL SEAL 24064 ENGINEER RAJESH H. BHAGTANI
3/20/20

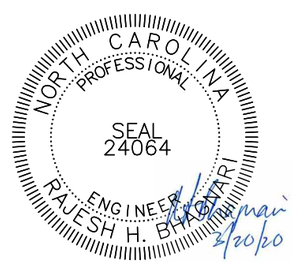
SPLICE PLATE & BOLT TABLE									
Mark	Qty Top	Bot	Int	Type	Dia	Length	Width	Thick	Length
SP-1	4	4	0	A325	0.625	2.25	6"	5/8"	2'-6 7/8"
SP-2	4	4	0	A325	0.625	2.25	6"	3/8"	1'-7 1/4"

▽FLANGE BRACES: (1) One Side; (2) Two Sides
A – L2525105

CONNECTION PLATES		
☐ ID	Qty	Mark/Part
1	8	FBL&N01



RIGID FRAME ELEVATION: FRAME LINE 2 3 4



ISSUE	DWN	CHK	ENG	PE	DATE
RELEASE FOR PERMIT	BR	TR	JRF	RHB	3/17/20

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COLUMBUS, GA 31909
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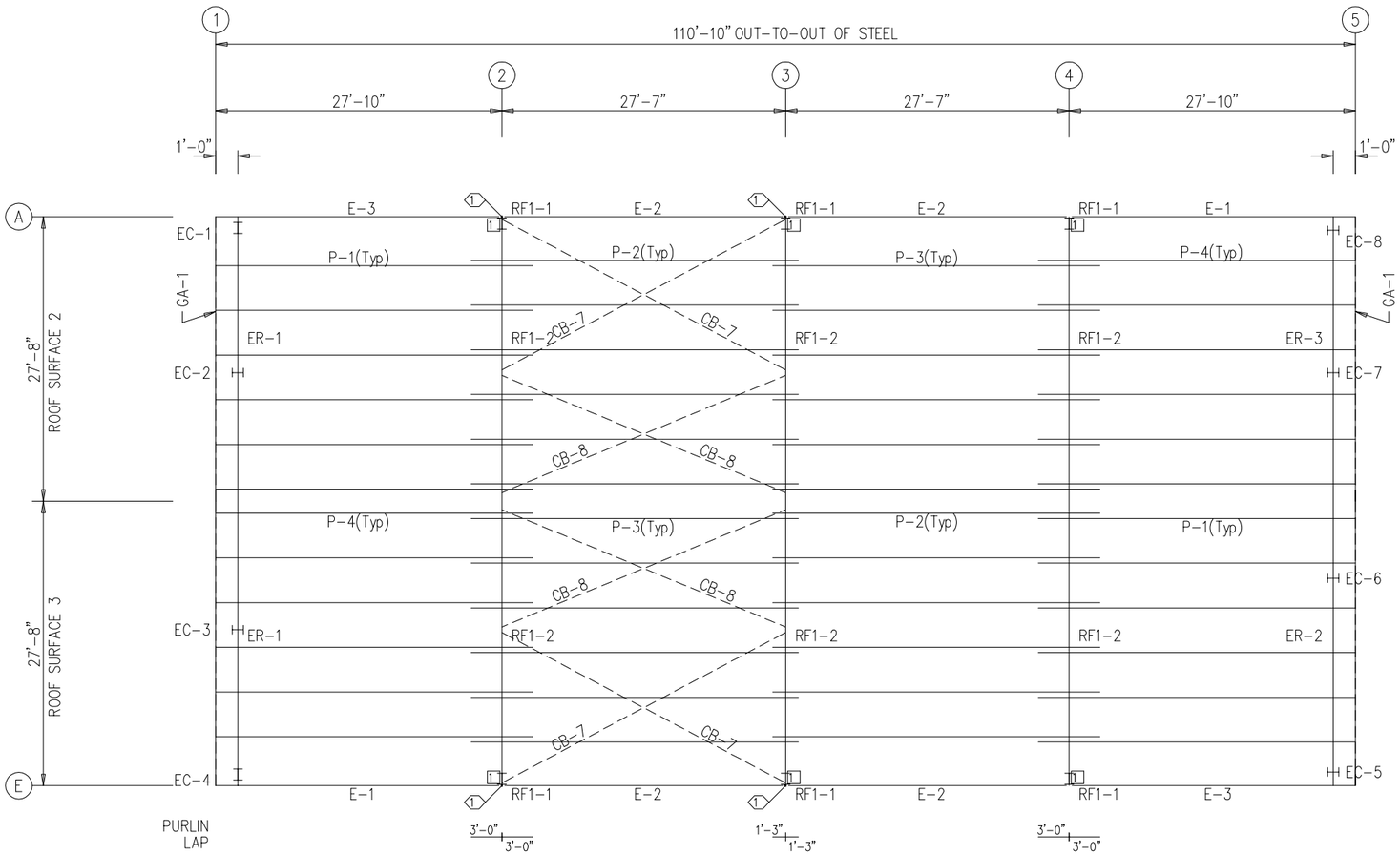
ANGIER, NC 27501

CUSTOMER NAME
ALEXANDER DESIGN BUILD. LLC
CLAYTON, NC 27520

JOB NUMBER
A20B0267A

SHEET TITLE
RFDWG-1

SHEET
E-01



ROOF FRAMING PLAN

TRIM TABLE ROOF PLAN			
◇ID	QUAN	PART	LENGTH
1	8	RCSS4	182.000

SPECIAL BOLTS ROOF PLAN					
◇ ID	QUAN	TYPE	DIA	LENGTH	WASH
1	4	A325	1/2"	2"	1

MEMBER TABLE ROOF PLAN		
MARK	PART	LENGTH
P-1	95Z075	369.750
P-2	95Z067	382.000
P-3	95Z067	382.000
P-4	95Z075	369.750
E-1	95E3060	333.625
E-2	95E3060	330.750
E-3	95E3060	333.625
CB-7	RD05-	378.000
CB-8	RD05-	369.000

CONNECTION PLATES ROOF PLAN		
□ID	QUAN	MARK/PART
1	6	ESC02

ISSUE	DWN	CHK	ENG	PE	DATE
RELEASE FOR PERMIT	BR	TR	JRF	RHB	3/17/20

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ANGIER, NC 27501

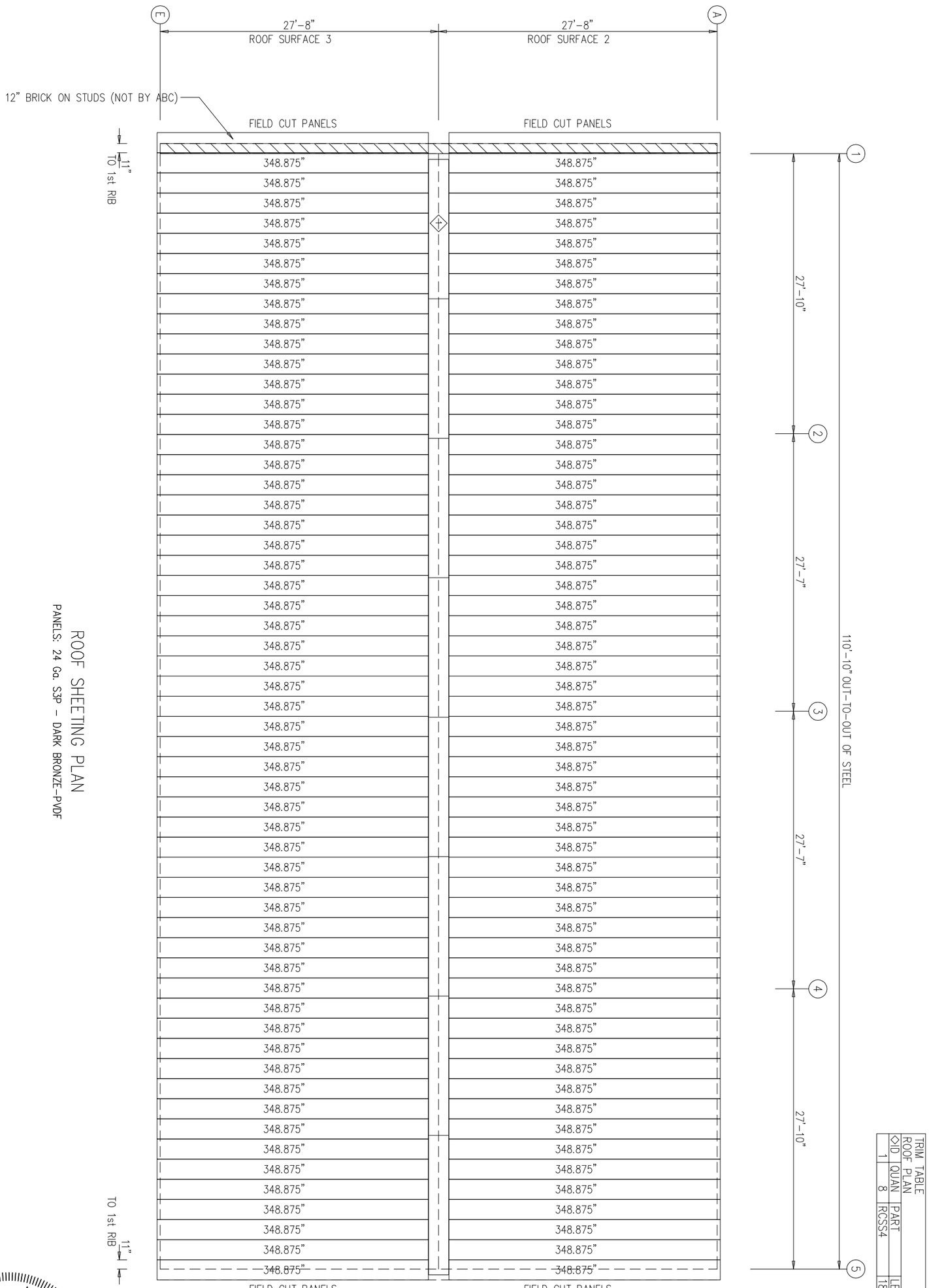
CUSTOMER NAME
ALEXANDER DESIGN BUILD. LLC
CLAYTON, NC 27520

JOB NUMBER
A20B0267A

SHEET TITLE
ROOFDWG

SHEET
E-02

NORTH CAROLINA PROFESSIONAL SEAL 24064 ENGINEER RAJESH H. BHAGTANI
3/20/20



ROOF SHEETING PLAN

PANELS: 24 Ga. S3P – DARK BRONZE-PVDF

TRIM TABLE
ROOF PLAN

◇ID	QUAN	PART	LENGTH
1	8	RCSS4	182.000

NORTH CAROLINA PROFESSIONAL ENGINEER RAJESH H. BHAGNARI SEAL 24064

3/17/20

This seal pertains only to the materials designed and supplied by the Metal Building Manufacturer. The drawings and the metal buildings which they represent are the product of the Metal Building Manufacturer. The registered professional engineer whose seal appears on these drawings is employed by the Metal Building Manufacturer and does not serve as or represent the project engineer of record and shall not be construed as such.

SHEET
E-03

PROJECT NAME
NIELL'S CREEK CHURCH GYM
ANGIER, NC 27501

CUSTOMER NAME
ALEXANDER DESIGN BUILD. LLC
CLAYTON, NC 27520

JOB NUMBER
A20B0267A

SHEET TITLE
ROOFDWG2

AMERICAN BUILDINGS COMPANY
7200 N. LAKE DRIVE STE. 200
COLUMBUS, GA 31909
PHONE: (706) 562-8020
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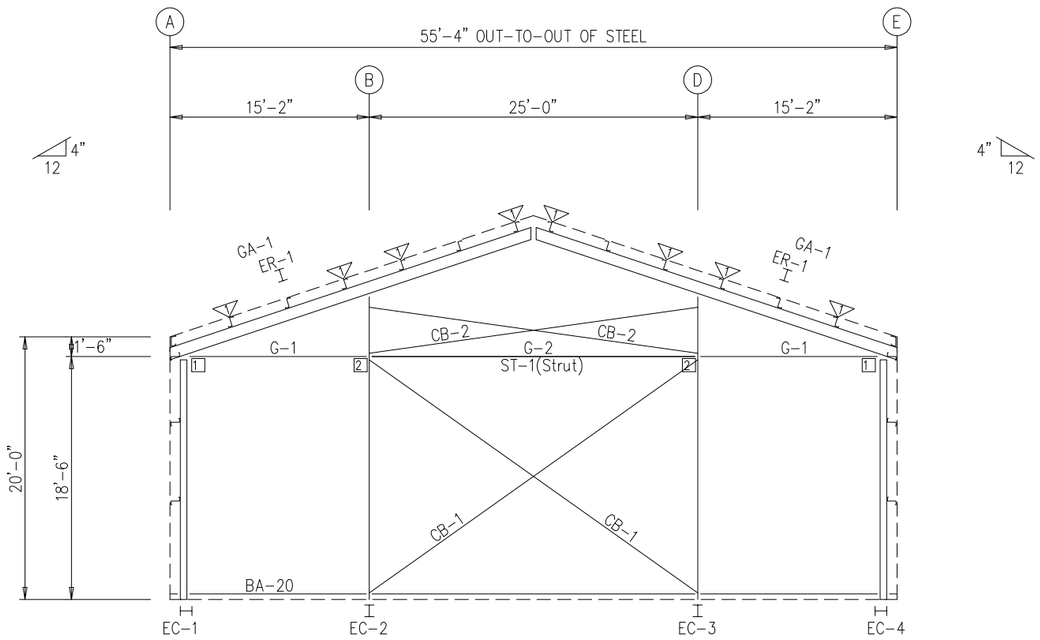
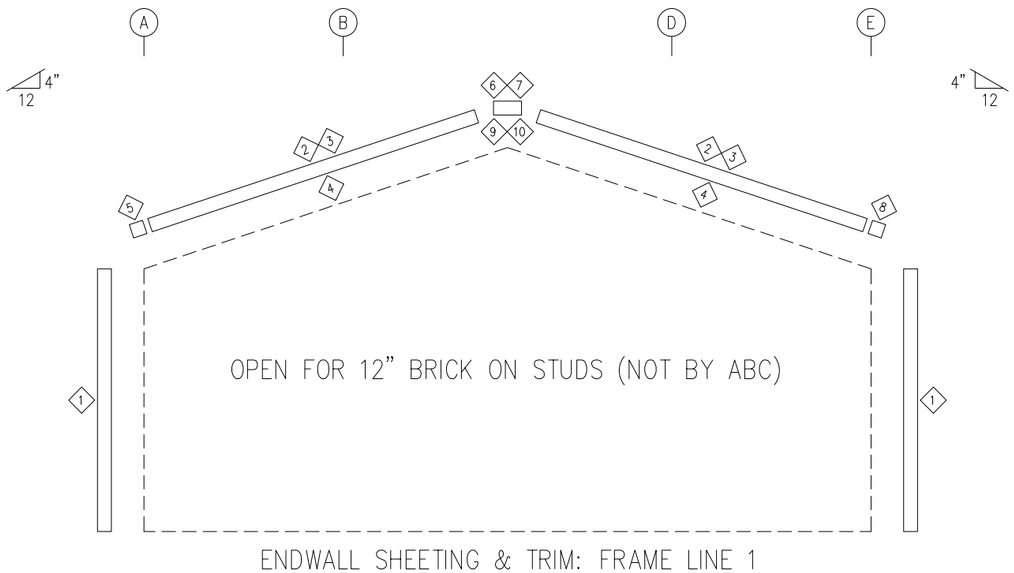
ISSUE	DWN	CHK	ENG	PE	DATE
RELEASE FOR PERMIT	BR	TR	JRF	RHB	3/17/20



ENDWALL FRAMING: FRAME LINE 1



ENDWALL SHEETING & TRIM: FRAME LINE 1

BOLT TABLE FRAME LINE 1				
LOCATION	QUAN	TYPE	DIA	LENGTH
ER-1/ER-1	8	A325	1/2"	2"
Columns/Raf	4	A325	1/2"	2"
Strut	2	A325	1"	3 1/4"

TRIM TABLE FRAME LINE 1		
◇ID	PART	LENGTH
1	FCRA2	182.000
2	RSF1	182.000
3	TRU1	182.000
4	MEC3	182.000
5	TRUECL	8.130
6	TRCU4	27.250
7	TRPBB4	7.500
8	TRUECR	8.130
9	ERECSSR	13.125
10	ERECSSL	13.125

MEMBER TABLE FRAME LINE 1		
MARK	PART	LENGTH
EC-1	W8x10	226.313
EC-2	W8x18	282.063
EC-3	W8x18	282.063
EC-4	W8x10	226.313
ER-1	W0915525	349.688
G-1	SW12x26	157.625
G-2	SW12x26	287.000
ST-1	W08SB075	294.750
CB-1	RD06-	374.000
CB-2	RD05-	313.000

FLANGE BRACE TABLE FRAME LINE 1			
▽ ID	# SIDES	MARK	CLIP
1	1	FBC30	FBL&N01

CONNECTION PLATES FRAME LINE 1	
□ID	MARK/PART
1	t1
2	t2

NORTH CAROLINA PROFESSIONAL SEAL 24064 ENGINEER RAJESH H. BHAGTANI 3/20/20

ISSUE	DWN	CHK	ENG	PE	DATE
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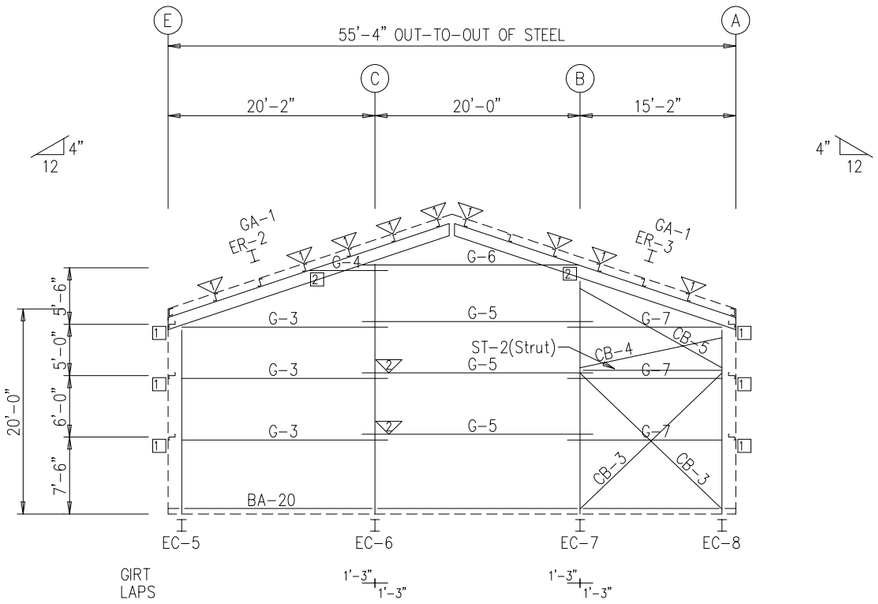
CUSTOMER NAME
ALEXANDER DESIGN BUILD. LLC
CLAYTON, NC 27520

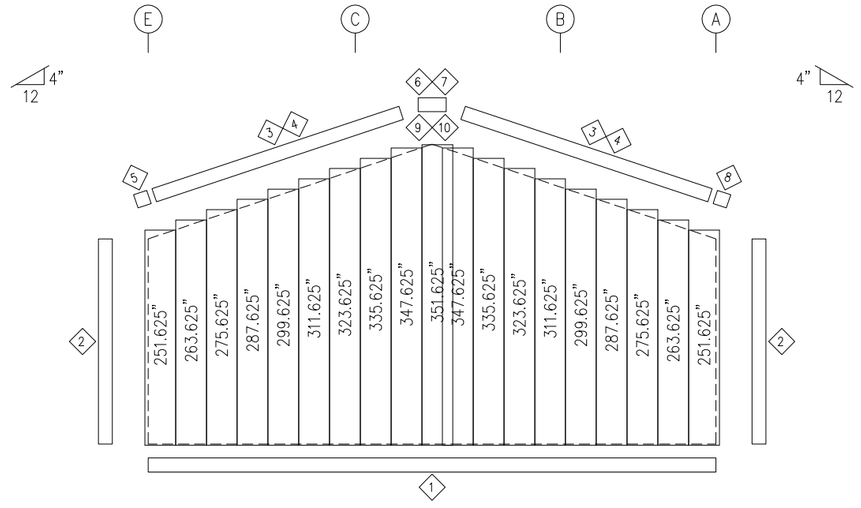
JOB NUMBER
A20B0267A

SHEET TITLE
EWDWG-L

SHEET
E-04



ENDWALL FRAMING: FRAME LINE 5



ENDWALL SHEETING & TRIM: FRAME LINE 5

PANELS: 26 Ga. A3P – GALVALUME PLUS

BOLT TABLE FRAME LINE 5				
LOCATION	QUAN	TYPE	DIA	LENGTH
ER-2/ER-3	8	A325	1/2"	2"
Columns/Raf	4	A325	1/2"	2"
Strut	2	A325	1"	3 1/4"

TRIM TABLE FRAME LINE 5		
◇ID	PART	LENGTH
1	BA-20	240.000
2	FCRA2	182.000
3	TRU1	182.000
4	RSF1	182.000
5	TRUECL	8.130
6	TRCU4	27.250
7	TRPBB4	7.500
8	TRUECR	8.130
9	ERECSSR	13.125
10	ERECSSL	13.125

MEMBER TABLE FRAME LINE 5		
MARK	PART	LENGTH
EC-5	W8x10	226.500
EC-6	W10x15	301.813
EC-7	W8x10	281.813
EC-8	W8x10	226.500
ER-2	W0915525	349.688
ER-3	W0915525	349.688
G-3	08Z054	247.000
G-4	08Z054	82.938
G-5	08Z054	270.000
G-6	08Z054	262.938
G-7	08Z054	187.000
ST-2	W08SB075	160.750
CB-3	RD05-	235.000
CB-4	RD05-	180.000
CB-5	RD05-	201.000

FLANGE BRACE TABLE FRAME LINE 5			
▽ID	# SIDES	MARK	CLIP
1	1	FBC30	FBL&N01
2	1	FBC30	

CONNECTION PLATES FRAME LINE 5	
□ID	MARK/PART
1	GCC03
2	GCC12

ISSUE	DWN	CHK	ENG	PE	DATE
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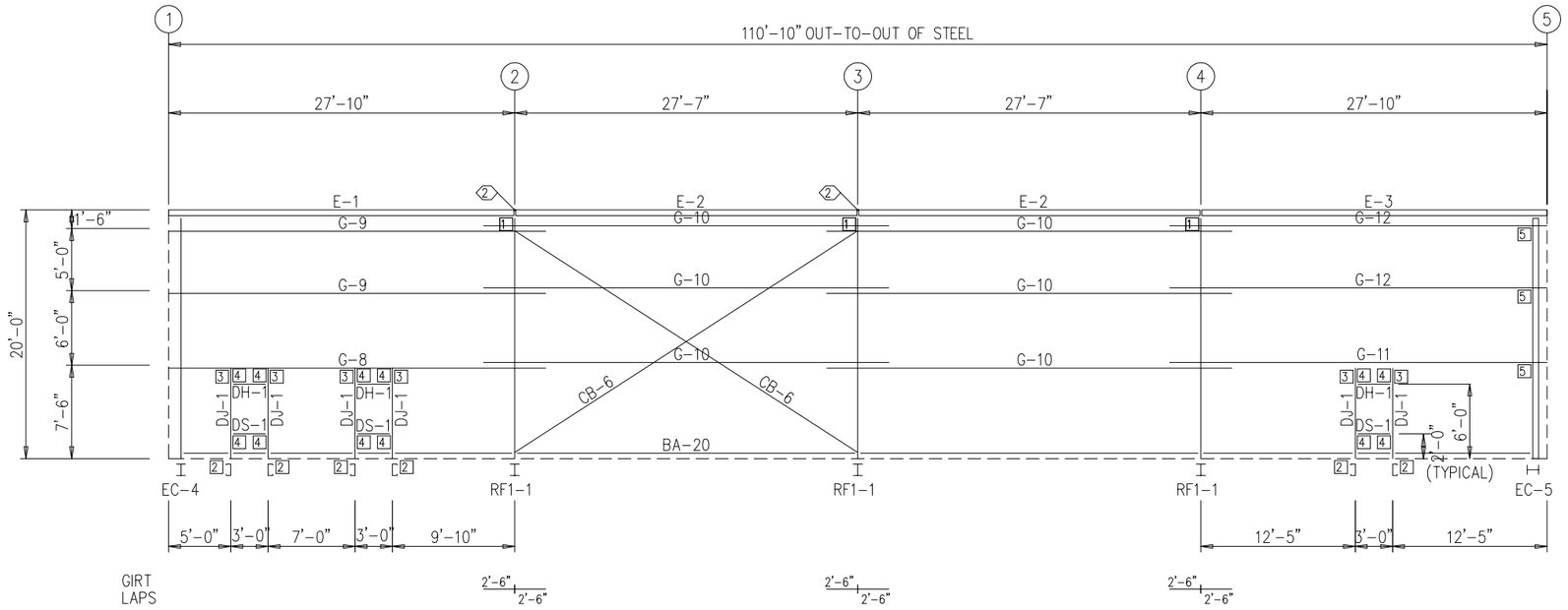
CLAYTON, NC 27520

JOB NUMBER
A20B0267A

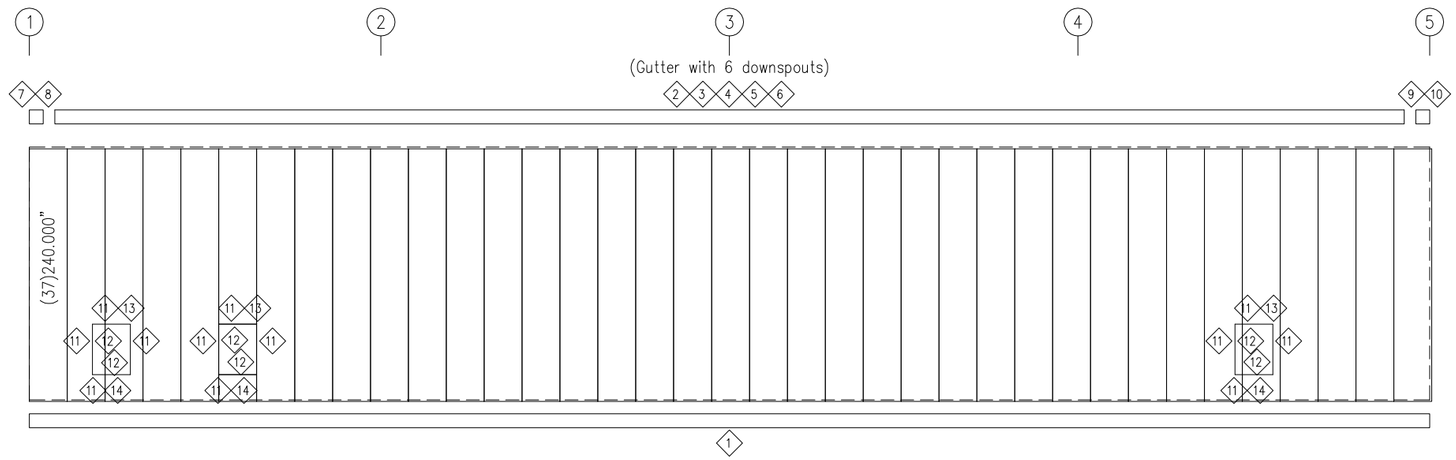
SHEET TITLE
EWDWG-R

SHEET
E-05

NORTH CAROLINA PROFESSIONAL SEAL 24064 ENGINEER RAJESH H. BHAGTARI 3/20/20



SIDEWALL FRAMING: FRAME LINE E



SIDEWALL SHEETING & TRIM: FRAME LINE E

PANELS: 26 Ga. A3P – GALVALUME PLUS

TRIM TABLE FRAME LINE E		
◇ID	PART	LENGTH
1	BA-20	240.000
2	TGT1	182.000
3	TFEC4	182.000
4	CGB4	7.310
5	GC-A	9.940
6	TFSET	122.000
7	GE1R	9.250
8	TCB4R	15.940
9	GE1L	9.250
10	TCB4L	15.940
11	FOCF95	182.000
12	JTD087	87.000
13	HTA044	44.000
14	FSJ1	182.000

SPECIAL BOLTS					
◇ ID	QUAN	TYPE	DIA	LENGTH	WASH
2	4	A325	1/2"	2"	1

MEMBER TABLE FRAME LINE E		
MARK	PART	LENGTH
DJ-1	F95C060	85.750
DH-1	F95C060	36.000
DS-1	F95C060	36.000
E-1	95E3060	333.625
E-2	95E3060	330.750
E-3	95E3060	333.625
G-8	95Z067	363.750
G-9	95Z060	363.750
G-10	95Z060	391.000
G-11	95Z067	363.750
G-12	95Z060	363.750
CB-6	RD05-	408.000

CONNECTION PLATES FRAME LINE E	
□ID	MARK/PART
1	ESC02
2	HCJ01&bh
3	JCT01
4	HCJ01
5	GCC03

NORTH CAROLINA PROFESSIONAL SEAL 24064 ENGINEER RAJESH H. BHAGTARI 3/20/20

This seal pertains only to the materials, details and systems which the Metal Building Manufacturer. The drawings and the metal buildings which they represent are the product of the Metal Building Manufacturer. The registered professional engineer whose seal appears on these drawings is employed by the Metal Building Manufacturer and does not have direct supervision of the work. The engineer of record are shall not be construed as such.

ISSUE	DWN	CHK	ENG	PE	DATE
RELEASE FOR PERMIT	BR	TR	JRF	RHB	3/17/20

AMERICAN BUILDINGS COMPANY
7200 N. LAKE DRIVE STE. 200
COLUMBUS, GA 31909
PHONE: (706) 562-8020
FAX: (706) 562-8017

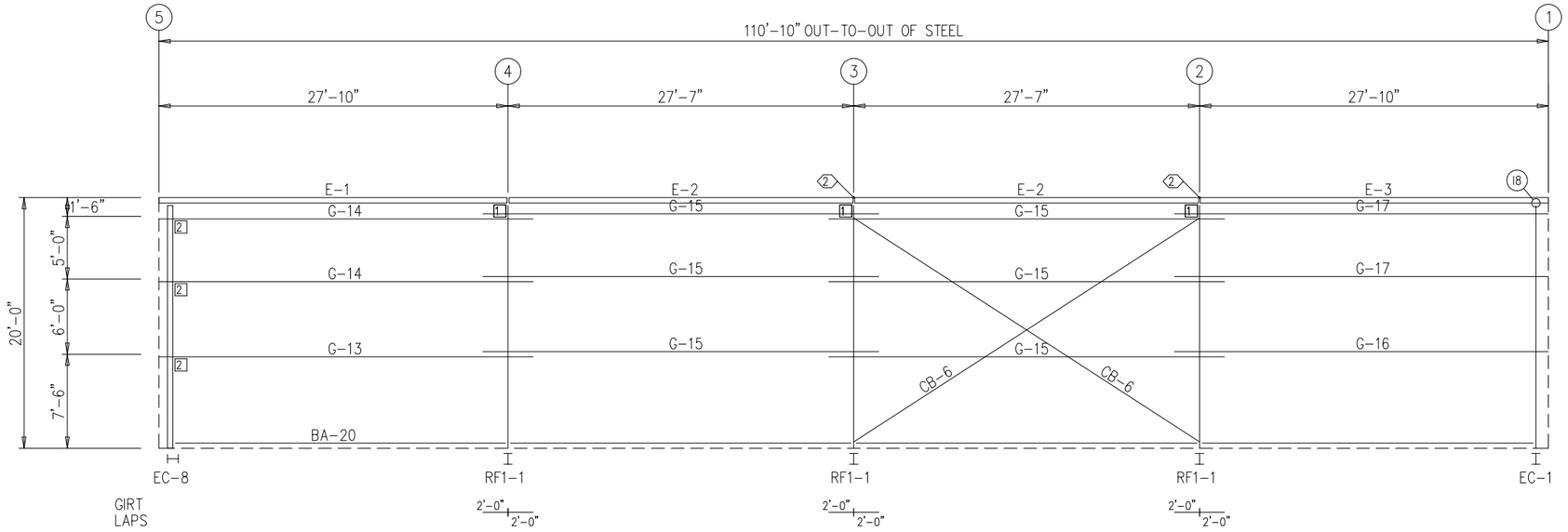
PROJECT NAME
NIELL'S CREEK CHURCH GYM
ANGIER, NC 27501

CUSTOMER NAME
ALEXANDER DESIGN BUILD. LLC
CLAYTON, NC 27520

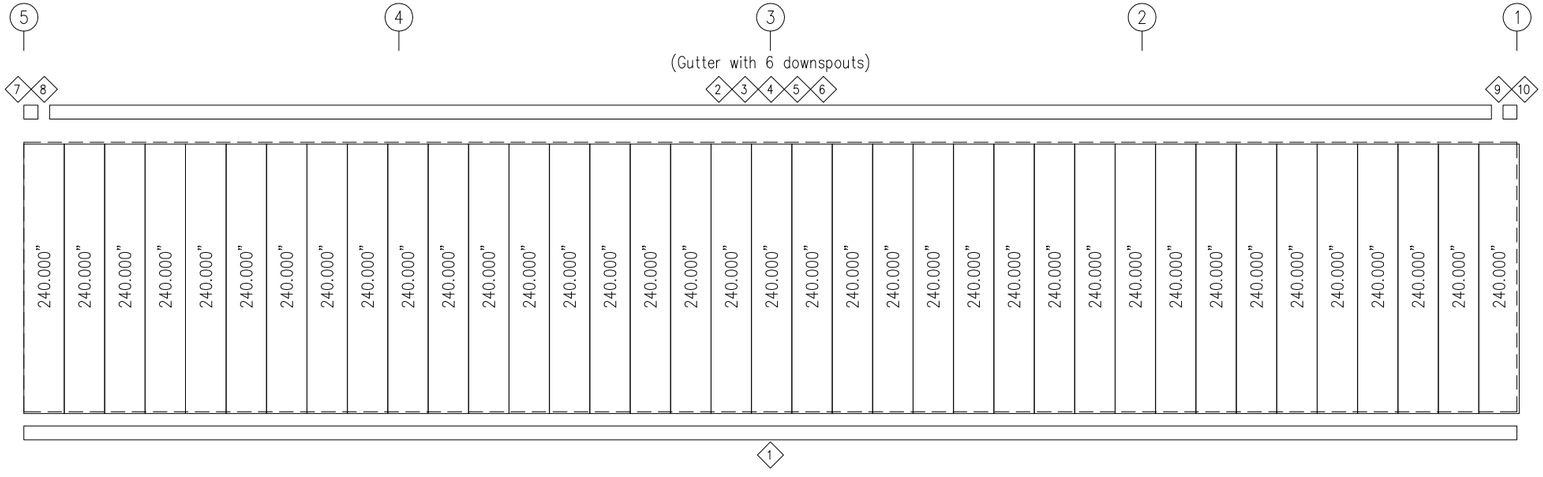
JOB NUMBER
A20B0267A

SHEET TITLE
SWDWG-F

SHEET
E-06



SIDEWALL FRAMING: FRAME LINE A



SIDEWALL SHEETING & TRIM: FRAME LINE A

PANELS: 26 Ga. A3P – GALVALUME PLUS

TRIM TABLE FRAME LINE A		
◇ID	PART	LENGTH
1	BA-20	240.000
2	TGT1	182.000
3	TFEC4	182.000
4	CGB4	7.310
5	GC-A	9.940
6	TFSET	122.000
7	GE1R	9.250
8	TCB4R	15.940
9	GE1L	9.250
10	TCB4L	15.940

SPECIAL BOLTS					
◇ ID	QUAN	TYPE	DIA	LENGTH	WASH
2	4	A325	1/2"	2"	1

MEMBER TABLE FRAME LINE A		
MARK	PART	LENGTH
E-1	95E3060	333.625
E-2	95E3060	330.750
E-3	95E3060	333.625
G-13	95Z067	357.750
G-14	95Z060	357.750
G-15	95Z060	379.000
G-16	95Z067	357.750
G-17	95Z060	357.750
CB-6	RD05-	408.000

CONNECTION PLATES FRAME LINE A	
□ID	MARK/PART
1	ESC02
2	GCC03

NORTH CAROLINA PROFESSIONAL SEAL 24064 ENGINEER RAJESH H. BHAGTANI 3/20/20

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ISSUE	DWN	CHK	ENG	PE	DATE
RELEASE FOR PERMIT	BR	TR	JRF	RHB	3/17/20

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JOB NUMBER
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SHEET TITLE
SWDWG-B

SHEET
E-07