

PROPOSED SOLAR POWER SITES:
 BUIES CREEK SOLAR PROJECT
 1887 LESLIE CAMPBELL AVE.
 LILLINGTON, NC 27546

PREPARED FOR:
 PURE POWER CONTRACTORS
 2812 GRAY FOX RD.
 MONROE, NC 28110

PREPARED BY:
 SOLAR FLEXRACK
 A DIVISION OF NORTHERN STATES METALS
 3207 INNOVATION PLACE
 YOUNGSTOWN, OHIO 44509
 PHONE: 1-888-380-8138

GENERAL NOTES:

- 1. CODES AND STANDARDS**
 IBC 2015
 NEC 2017
 AISC 360-10
 AISI S100-12
 ASCE 7-10
- 2. WIND DESIGN PARAMETERS:**
 DESIGN WIND SPEED, V - 107 MPH
 RISK CATEGORY - I
 WIND EXPOSURE, Kz - 0.85
 TOPOGRAPHIC FACTOR, Kzt - 1.00
 WIND DIRECTIONALITY FACTOR, Kd - 0.85
 GUST FACTOR & NET PRESSURE COEFFICIENT, GCN (BASED ON WIND TUNNEL STUDY)
 UPWARD - 1.10
 DOWNWARD - 0.93
- 3. SNOW DESIGN PARAMETERS:**
 GROUND SNOW LOAD, Pg - 15 PSF
 SNOW EXPOSURE CATEGORY, Ce - 1.0
 SNOW IMPORTANCE FACTOR, I - 0.8
 SNOW THERMAL FACTOR, Ct - 1.2
 SNOW REDUCTION FACTOR SLIPPERY SURFACES, Cs - 0.82
- 4. EARTHQUAKE DESIGN PARAMETERS:**
 RISK CATEGORY - I
 SEISMIC IMPORTANCE FACTOR, Ie - 1.0
 MAPPED SPECTRAL RESPONSE ACCELERATION PARAMETERS,
 Ss - 0.178g
 S1 - 0.084g
 SITE CLASS - D
 DESIGN SPECTRAL RESPONSE ACCELERATION PARAMETERS,
 SDS - 0.190g
 SD1 - 0.135g
 SEISMIC DESIGN CATEGORY - C
 BASIC SEISMIC FORCE-RESISTING SYSTEM - SOLAR RACKING SYSTEM
 SEISMIC RESPONSE COEFFICIENT, Cs - 0.095
 RESPONSE MODIFICATION COEFFICIENT, R - 2
 ANALYSIS PROCEDURE- EQUIVALENT LATERAL FORCE PROCEDURE
- 5. FOUNDATION DESIGN PARAMETERS:**
 FOUNDATION DESIGN DERIVED FROM GEOTECHNICAL REPORT PROVIDED BY MOTT MACDONALD DATED MAY 13, 2019 (DOCUMENT REFERENCE: 406775 | GIR | 1)
- 6. POST INSTALLATION TOLERANCES AT TOP OF POSTS:**
 VERTICAL: +/-1" (SEE NOTE BELOW)
 MAX TWIST: +/-2"
 EAST/WEST: +/-2.5"
 NORTH SOUTH: +/-1"
 MAX OUT OF PLUMB: +/- 1"

(POST INSTALLATION TOLERANCES ACCOUNT FOR STACKED MANUFACTURING TOLERANCES)

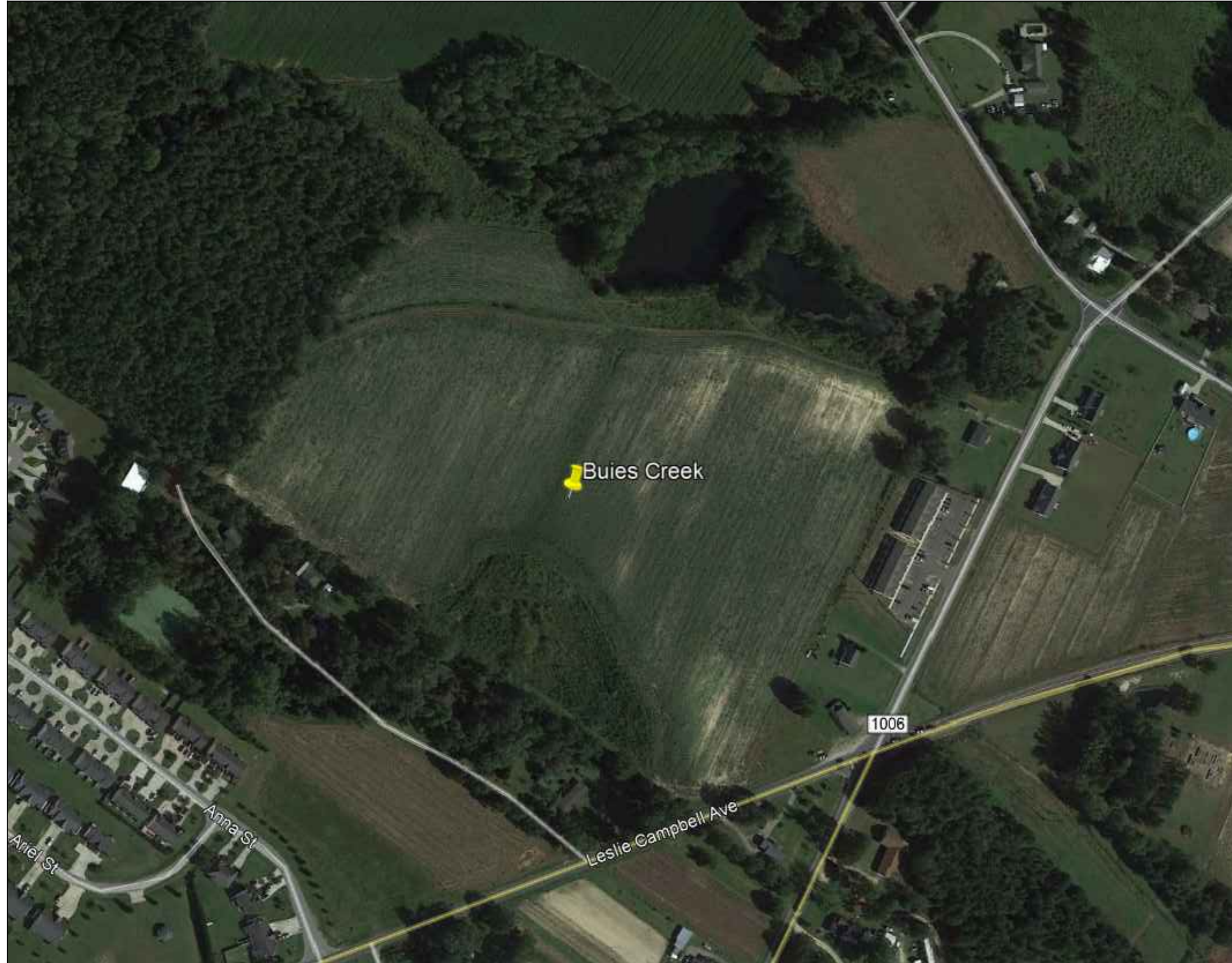
- RECOMMENDATIONS FOR SETTING POST ELEVATIONS:
- FIRST POST OF RACK AND FIRST POST OF NEXT RACK (CONTROL POSTS) SHALL BE PLACED SO THAT THE TOP OF POST IS EQUAL TO THE "POST ABOVE GRADE" DIMENSION. MINIMUM EMBEDMENT DEPTH REQUIRED AT ALL CONTROL POSTS.
 - THE CONTROL POST DOESN'T NECESSARILY HAVE TO BE THE 1ST POST WITHIN EACH RACK
 - A TAUT STRING-LINE SHALL THEN BE PLACED FROM TOP CENTER OF FIRST CONTROL POST TO TOP CENTER OF NEXT CONTROL POST.
 - THE MAXIMUM SLOPE OF THE STRING-LINE SHALL BE LESS THAN OR EQUAL TO +/- 20%.
 - THE REMAINING POSTS TO BE INSTALLED WITHIN +/- 1" FROM STRING LINE.
 - EMBEDMENT DEPTH FOR POSTS BETWEEN CONTROL POSTS MUST BE INSTALLED TO WITHIN +/- 6" OF REQUIRED EMBEDMENT DEPTH.

MINIMUM RECOMMENDED CLEARANCE BETWEEN ADJACENT RACKS TO BE NO LESS THAN 4".

- 7. DESIGN ASSUMPTION:**
 SNOW BANKING AT THE FRONT OF RACK WAS NOT ACCOUNTED FOR IN RACK DESIGN. ANY ADVERSE EFFECTS DUE TO SNOW BANKING, SUCH AS SHADING OR STRUCTURAL CONSIDERATIONS, ARE BEYOND SOLAR FLEXRACK'S SCOPE.
- 8. CONNECTIONS:**
 - SNUG TIGHT:** ALL CONNECTIONS TO BE SNUG TIGHT PER THE RESEARCH COUNCIL OF STRUCTURAL CONNECTIONS (AISC RCSC) UNLESS OTHERWISE NOTED. PERFORM VISUAL INSPECTION TO ENSURE PLIES IN THE CONNECTION HAVE BEEN PULLED INTO FIRM CONTACT.
 - TURN-OF-NUT:** SOME 1/2"-13 ASTM A325T BOLTS MUST BE FASTENED BY TURN OF THE NUT METHOD PER THE RESEARCH COUNCIL OF STRUCTURAL CONNECTIONS (AISC RCSC). FIRST ENSURE FASTENER IS SNUG TIGHT. THEN TURN NUT TO ANGLE BEYOND INITIAL TORQUE MARK AS CALLED OUT IN RESPECTIVE CONNECTION DETAIL(S).
- 9. PV MODULE INFORMATION:**
 NAME/MODEL: FIRST SOLAR SERIES 6 FS-6435 435W
 DIMENSIONS: 79.094" LONG X 48.504" WIDE X 1.929" TALL
 WEIGHT: 79.37 LBS
- 10. MATERIALS AND COATINGS:**
 - PILES:**
 - W-SECTIONS: A992 STEEL HOT DIPPED GALVANIZED PER ASTM A123.
 - COLD FORMED BALLAST CHANNELS - A36 STEEL HOT DIPPED GALVANIZED PER ASTM A123.
 - SMART POST - A653 GRADE 50 STEEL HOT DIPPED GALVANIZED PER ASTM A123
 - HARDWARE:**
 - 1/2"Ø TO BE A325 HOT DIPPED GALVANIZED PER ASTM A153.
 - M8Ø TO BE 18-8 TYPE 304 STAINLESS STEEL.
 - COLD FORMED STEEL:**
 - ALL COLD FORM STEEL TO BE PRE GALVANIZED PER A653 UNLESS OTHERWISE NOTED. SEE S1-S3 FOR THE GRADE OF STEEL FOR EACH MEMBER.
- 11. SPECIAL INSPECTIONS:**
 THE FOLLOWING SPECIAL INSPECTIONS MAY BE REQUIRED PER IBC CHAPTER 17. CHECK WITH LOCAL BUILDING OFFICIAL FOR APPLICABILITY.
 DRIVEN PILES.....(CONTINUOUS)
 -SEE IBC 2015, TABLE 1705.7, ITEMS 1-5
 ASTM A325 BOLTS AND FASTENERS.....(PERIODIC)
 -SEE AISC 360-10, SECTION N5.6
 ASTM A307 BOLTS AND FASTENERS
 -NOT REQUIRED

ABBREVIATIONS:

INT	INTERIOR	C-C	CENTER TO CENTER
MIN	MINIMUM	CL	CENTERLINE
MAX	MAXIMUM	CD	CRITICAL DIMENSION
PLCS	PLACES	DIA	DIAMETER
PAG	POST ABOVE GRADE	DIM	DIMENSION
REF	REFERENCE	EOP	END OF PANEL
TB	TILT BRACKET	EXT	EXTERIOR
TYP	TYPICAL	HORIZ	HORIZONTAL
VERT	VERTICAL	HDG	HOT DIPPED GALVANIZED



PROJECT SITES

NORTH

DRAWING INDEX: G3P-X RACK

ISSUANCE/REVISION	REVISION	DATE	DESCRIPTION
	●	MAR. 10, 2020	SIGN-OFF
	○	MAR. 19, 2020	REVIEW SET
S1	●		2X12 RACK PLAN VIEW, ELEVATIONS, AND NOTES
S2	○		TILT BRACKET COMPONENTS, CONNECTIONS & FOUNDATION DETAILS
S3	○		CONNECTIONS
S4	○		HARDWARE
S5	●		ALTERNATE FOUNDATION DETAILS

LEGEND:
 ● ISSUED
 ○ REVISED, BUT NOT ISSUED

CUSTOMER: PURE POWER CONTRACTORS
 BUIES CREEK SOLAR PROJECT
 LILLINGTON, NC 27546

DATE: 3/19/2020
 DRAWN BY: JS
 CHECK BY: DK
 JOB #: 11442
 SHEET: S0 of S5

COVER SHEET

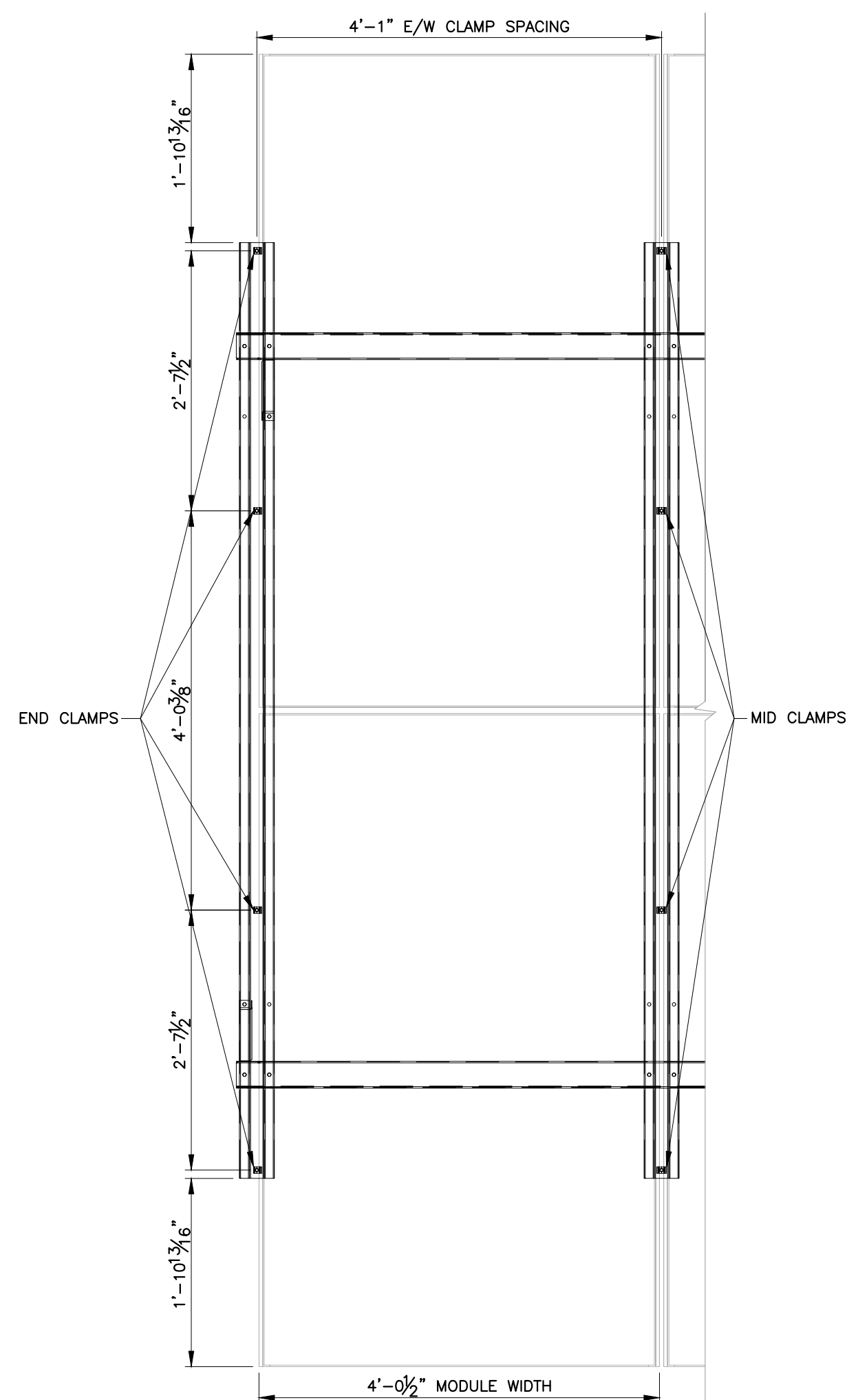
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REV #	DESCRIPTION	DATE
1	UPDATED SITE ADDRESS AND MODULE WATTAGE	3/20/2020

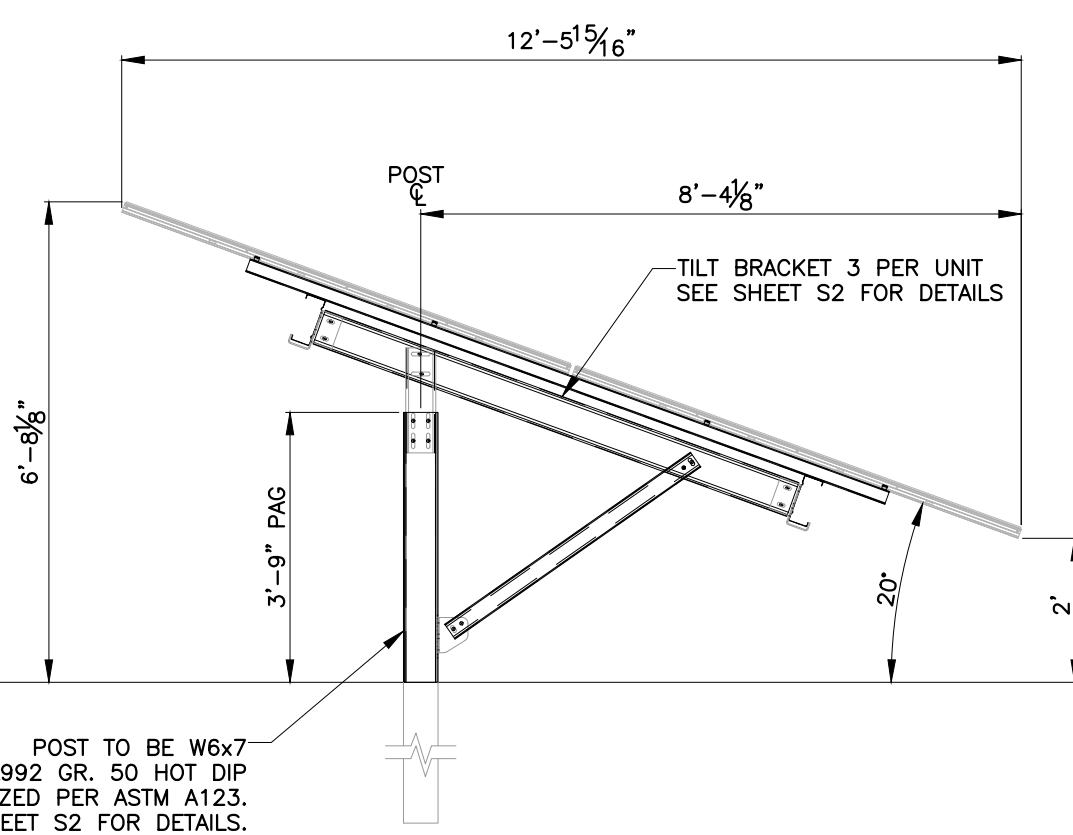
HORIZONTAL BRACING	
1-1/2" DEEP X 3/4" LEGS X 14 GA WALL ASTM A653 SS GRADE 50 G90 GALVANIZED COATING	
DESIGNATION	LENGTH
A	9'-8"
B	9'-3"
C	10'-10"



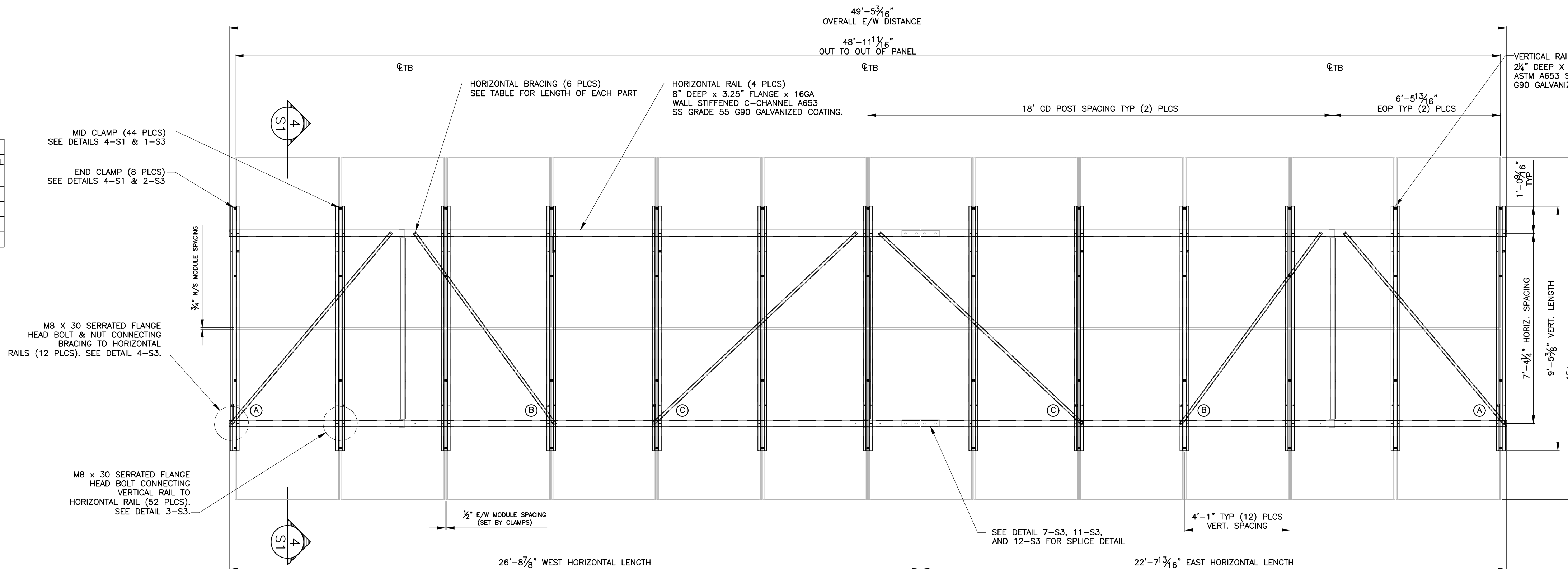
4
S1
MODULE INSTALLATION
SCALE: 3/4" = 1'-0"

MODULE INSTALLATION NOTES:

1. MINIMUM MODULE SPACING TO BE NO LESS THAN 1/4" IN ALL DIRECTIONS.
2. ALL MODULE TOLERANCES PER FIRST SOLAR SERIES 6 MODULE MOUNTING INSTALLATION MANUAL
3. TORQUE M8 BOLTS TO 8-12 FT-LBS

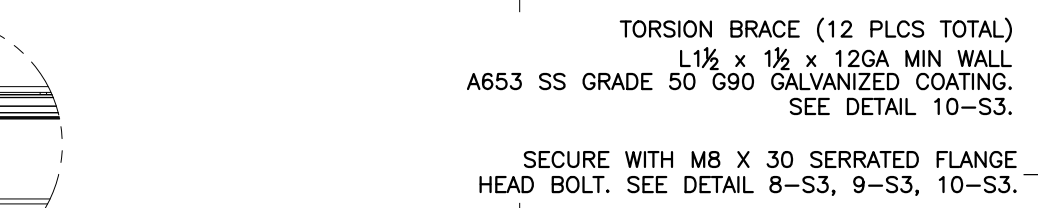
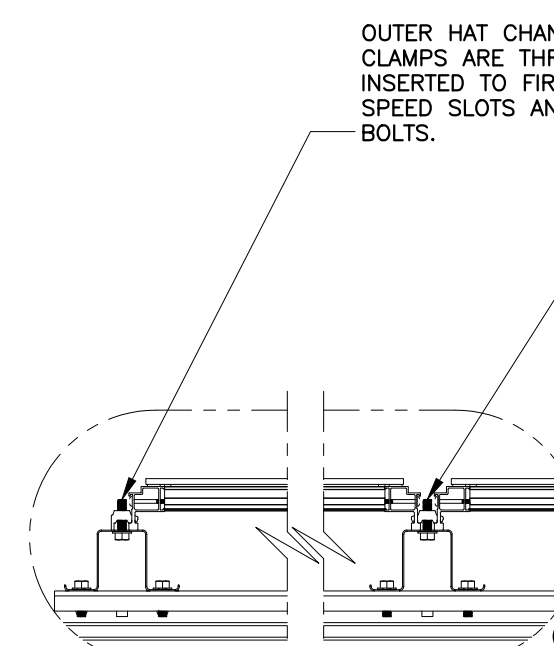


5
S1
DETAIL 5-S1
SCALE: 3/8" = 1'-0"

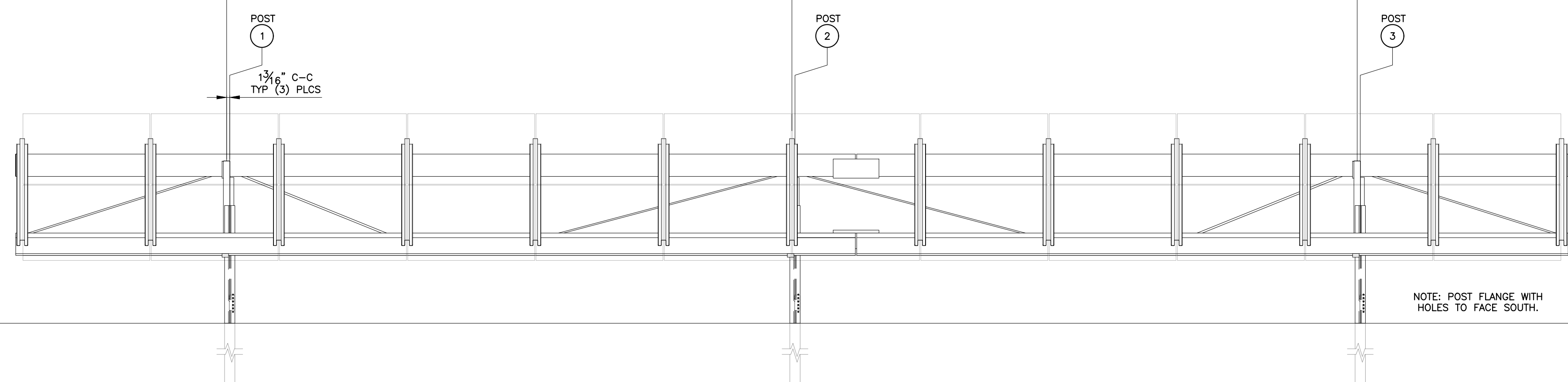


1
S1
PROJECTED TOP PLAN VIEW
SCALE: 3/8" = 1'-0"

PROJECT NORTH



2
S1
VIEW 2-S1
SCALE: 3/8" = 1'-0"



3
S1
FRONT ELEVATION VIEW
SCALE: 3/8" = 1'-0"

CUSTOMER: **PURE POWER CONTRACTORS**
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DATE: 3/19/2020
JOB #: 11442
DRAWN BY: JS
CHECK BY: DK
PAGE: S1 of S5

FIRST SOLAR SERIES 6 FS-6435 435W
2X12 SOLAR FLEXRACK G3P-X

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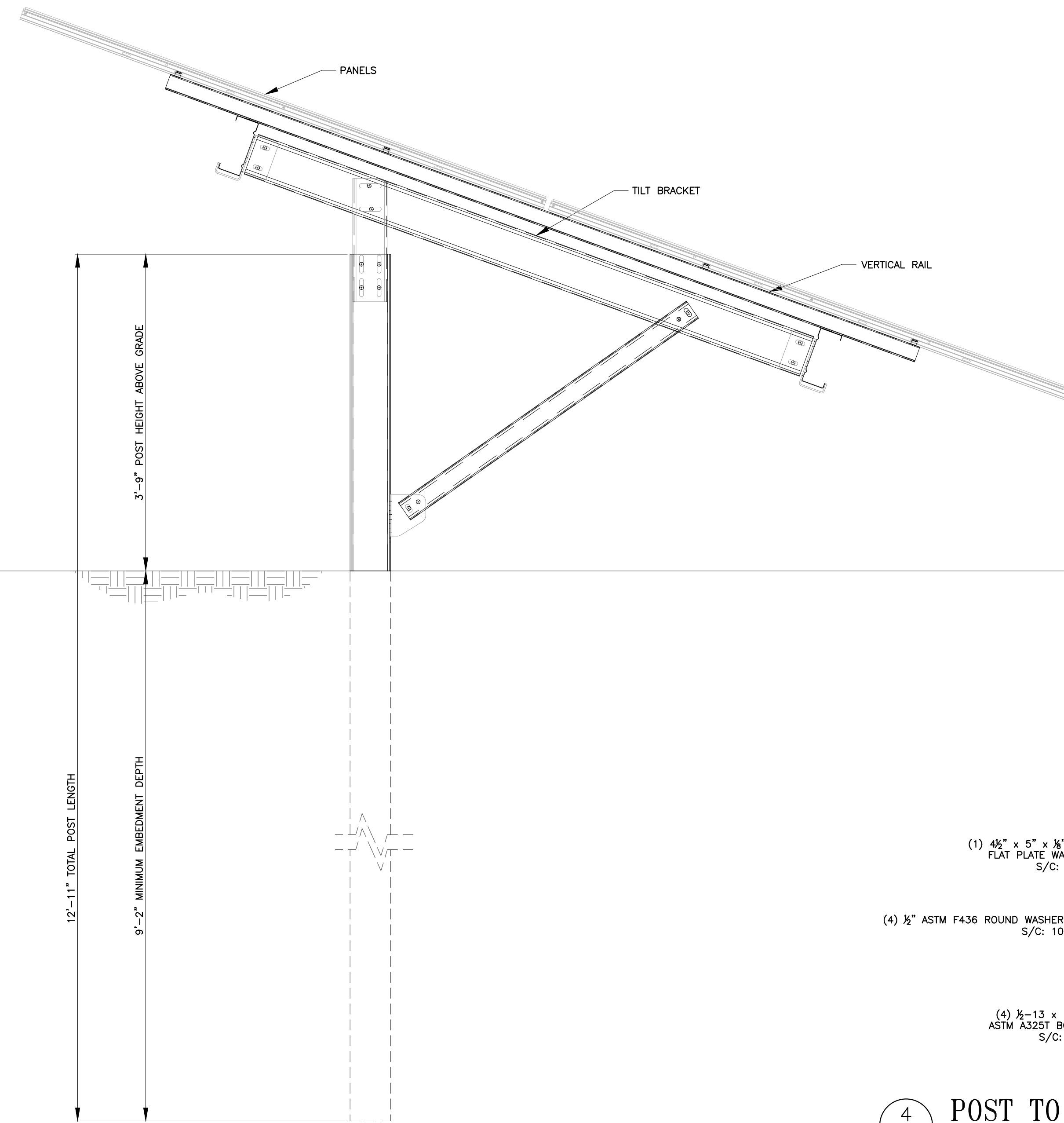
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REV #	DESCRIPTION	DATE
1	UPDATED SITE ADDRESS AND MODULE WATTAGE	2/20/2020

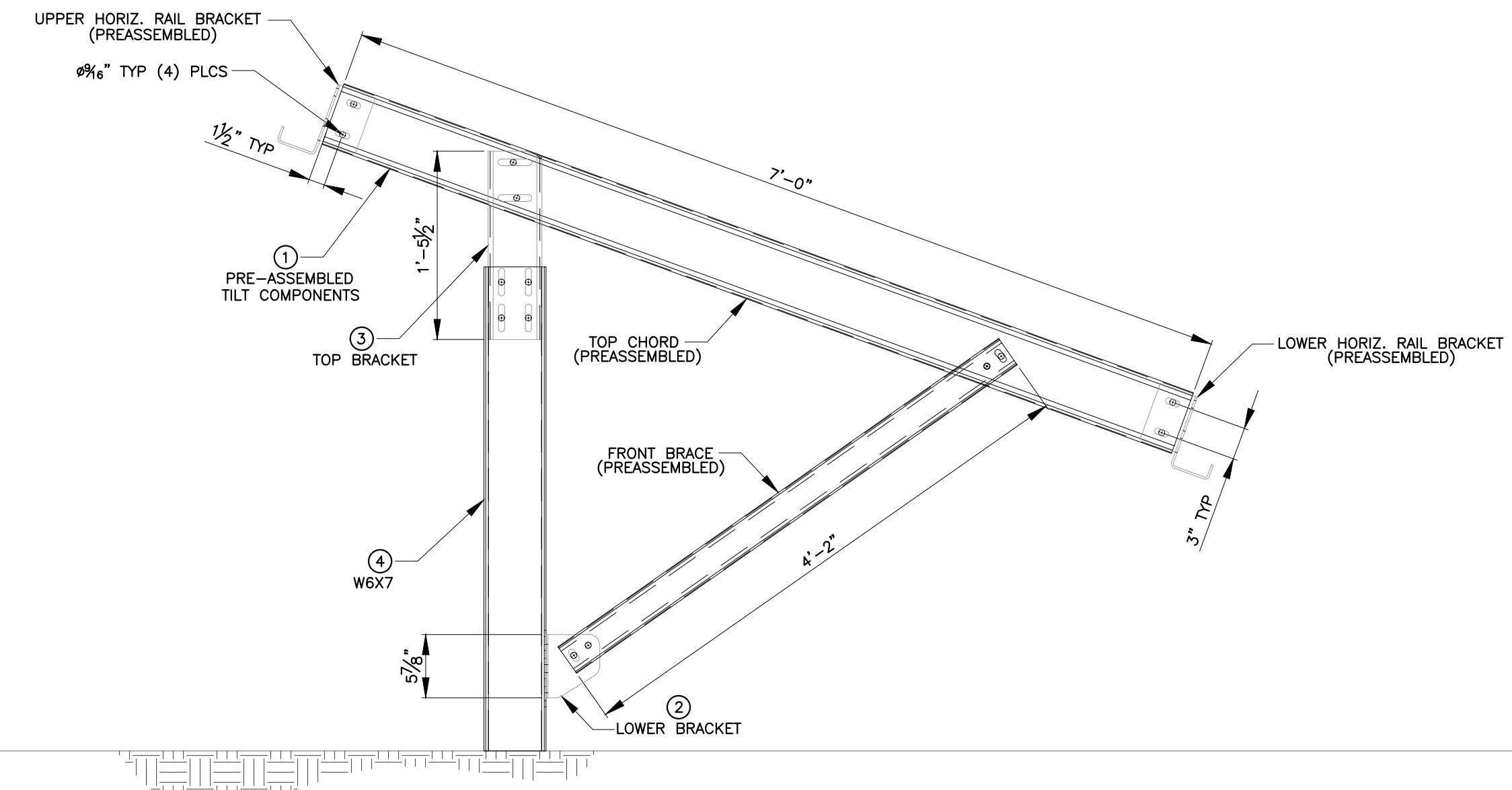
S1

ITEM	DESCRIPTION	MATERIAL	QTY
	TOP CHORD 6 DEEP x 2.5 LEG x 14GA THK C	ASTM A653 HSLAS GRADE 60 G90	
①	FRONT BRACE 3.0 DEEP x 1.75 LEG x 14GA THK C	ASTM A653 SS GRADE 50 G90	1
	LOWER HORIZ. RAIL BRACKET 6.957 x 3.68 x 0.188 THK	ASTM A36 G90	
	UPPER HORIZ. RAIL BRACKET 6.957 x 3.68 x 0.188 THK	ASTM A36 G90	
②	LOWER BRACKET 5 x 2.5 x 0.180 BENT PLATE	ASTM A36 G90	1
③	TOP POST BRACKET 5 x 1.5 x 0.188 BENT PLATE	ASTM A36 G90	1
④	W6X7	ASTM A992 GR. 50 HOT-DIP GALVANIZED	1

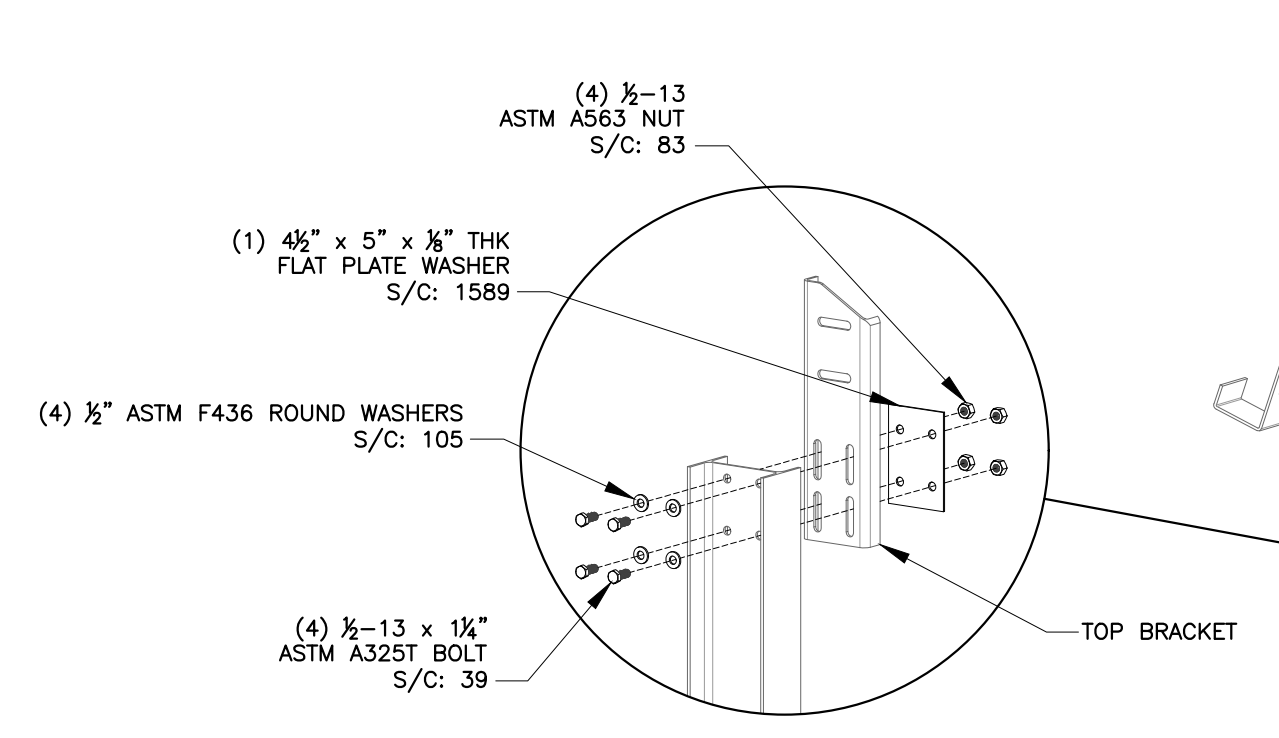
- * ALL QUANTITIES ABOVE ARE FOR ONE (1) TILT BRACKET ASSEMBLY
- * (1) TILT BRACKET ASSEMBLY PER POST
- * SEE 2-S2 THRU 5-S2 FOR ALL TILT BRACKET CONNECTION DETAILS.



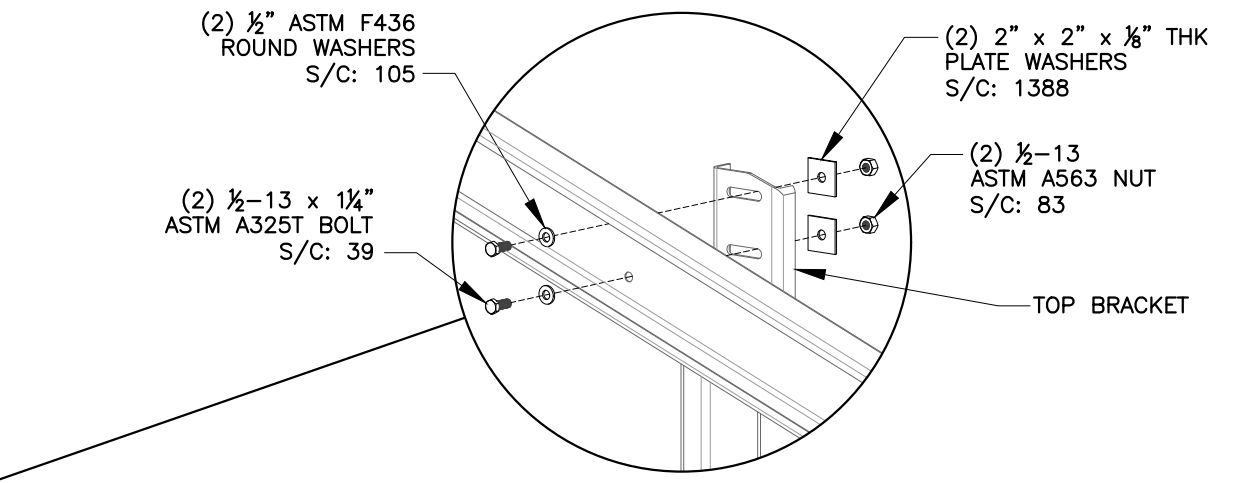
6 FOUNDATION DETAIL
SCALE: 1" = 1'-0"



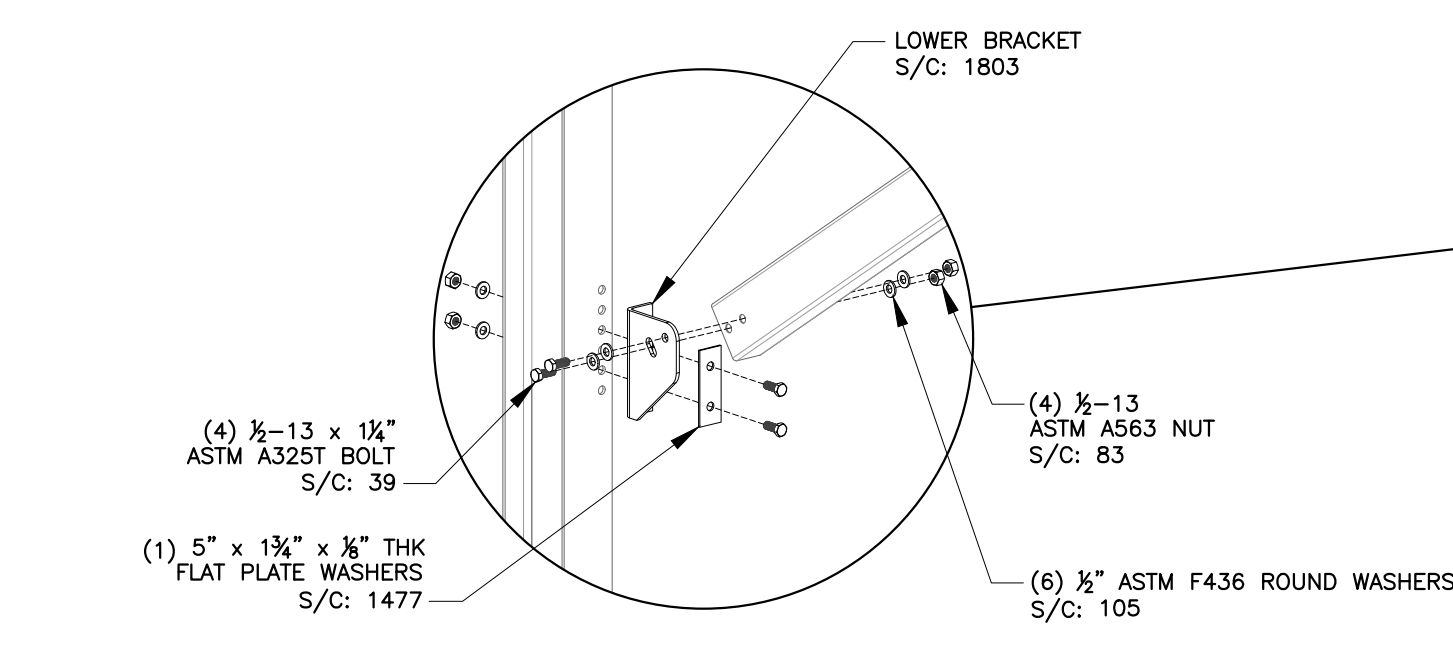
1 TILT BRACKET SIDE ELEVATION
SCALE: 1" = 1'-0"



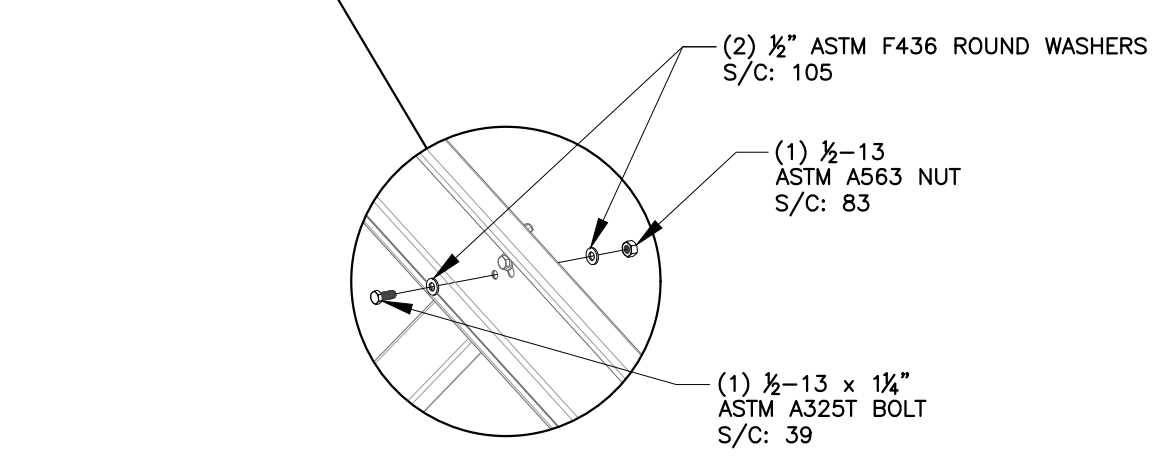
4 POST TO TOP BRACKET CONNECTION
SCALE: NTS
1/2"-13 ASTM A325T BOLTS CONNECTING TOP BRACKET TO POST SHALL BE FASTENED BY TURN OF THE NUT METHOD. (3/8 TURN PAST SNUG TIGHT)



2 TOP CHORD TO TOP BRACKET CONNECTION
SCALE: NTS
1/2"-13 ASTM A325T BOLTS CONNECTING TILT BRACKET TOP CHORD TO TOP BRACKET SHALL BE FASTENED BY TURN OF THE NUT METHOD. (3/8 TURN PAST SNUG TIGHT)



5 BRACE TO LOWER BRACKET CONNECTION
SCALE: NTS
1/2"-13 ASTM A325T BOLTS CONNECTING LOWER BRACKET TO POST SHALL BE FASTENED BY TURN OF THE NUT METHOD. (3/8 TURN PAST SNUG TIGHT)

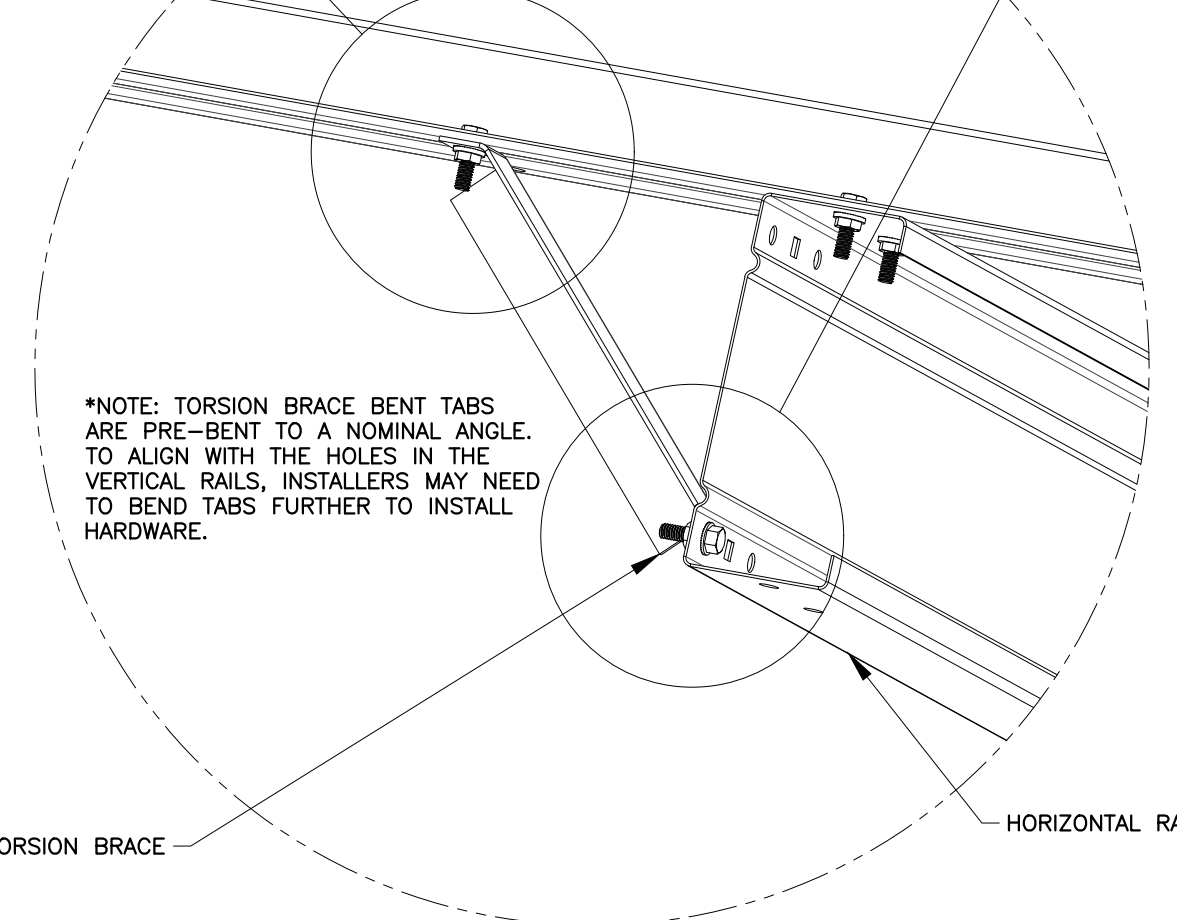
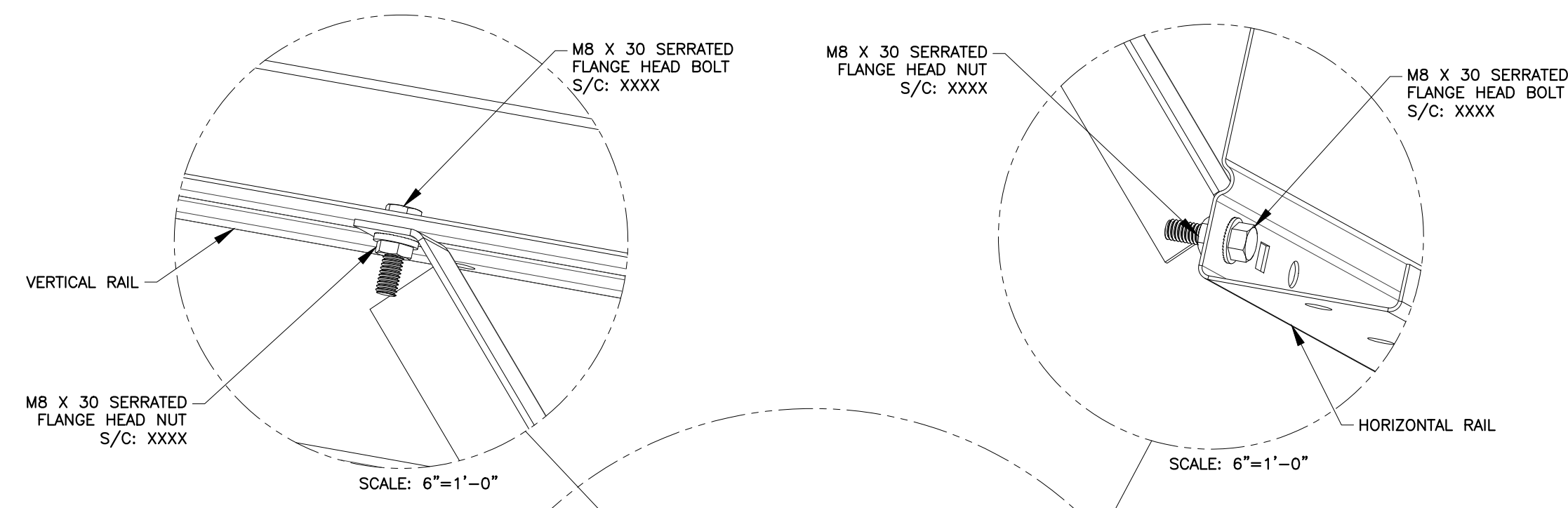


3 TOP CHORD TO BRACE CONNECTION
SCALE: NTS

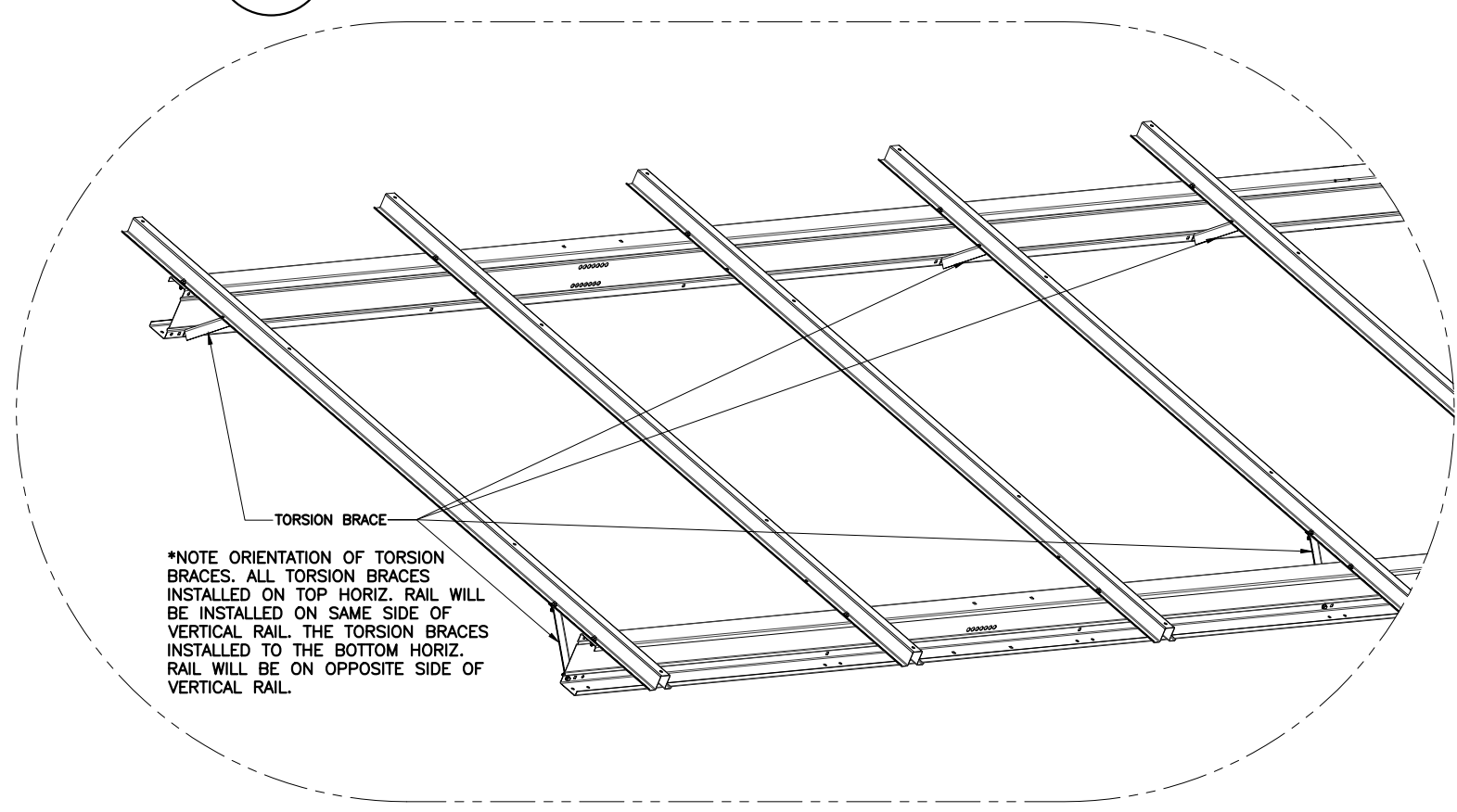
- * TOP CHORD, FRONT BRACE, AND SEAT BRACKETS WILL BE DELIVERED PREINSTALLED AND FOLDED.
- * BOLTS THAT HAVE BEEN PREINSTALLED WILL HAVE SILICONE ON THEM FOR TRANSPORTING PURPOSES, BOLTS WILL HAVE TO BE TIGHTENED.

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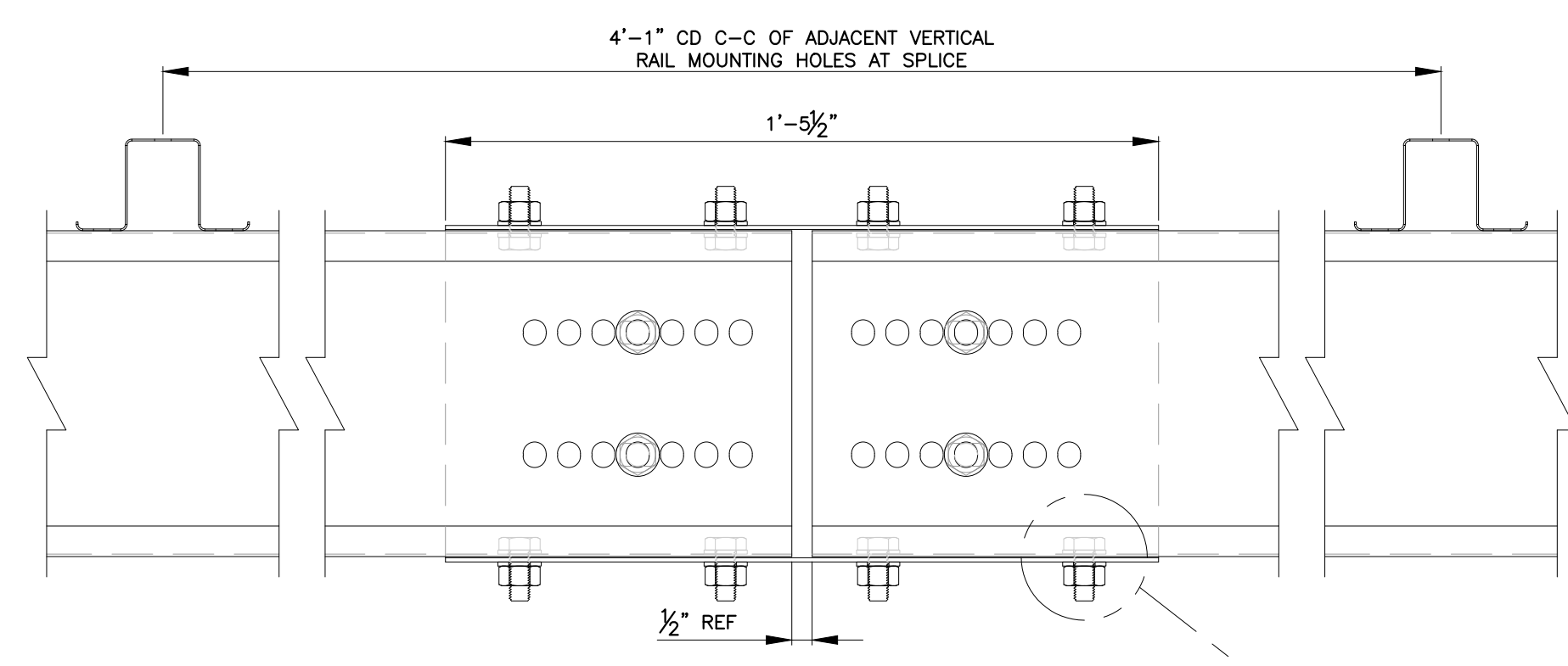
REV #	DESCRIPTION	DATE
1	UPDATED SITE ADDRESS AND MODULE WATTAGE	2/20/2020



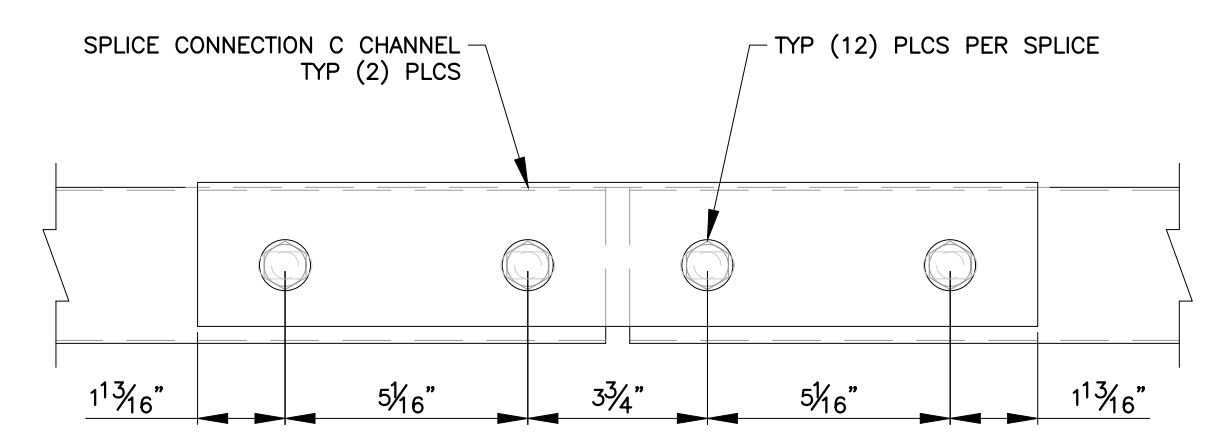
8 TORSION BRACE CONNECTIONS
SCALE: 3" = 1'-0"



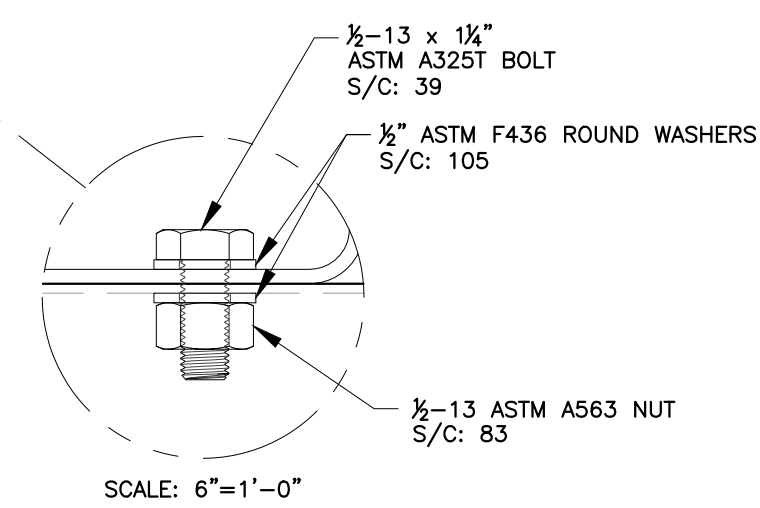
9 TORSION BRACE CONNECTIONS ISO VIEW
SCALE: NTS



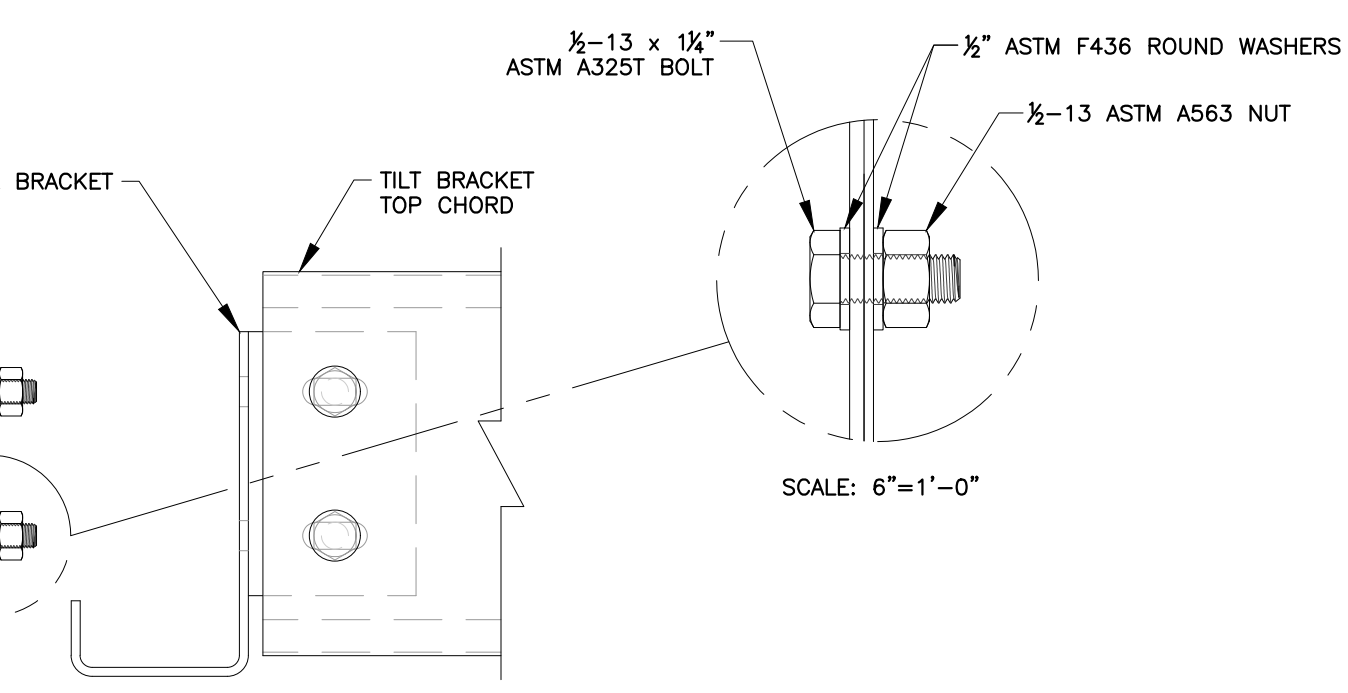
10 SPLICE CONN. ELEVATION
SCALE: 3" = 1'-0"



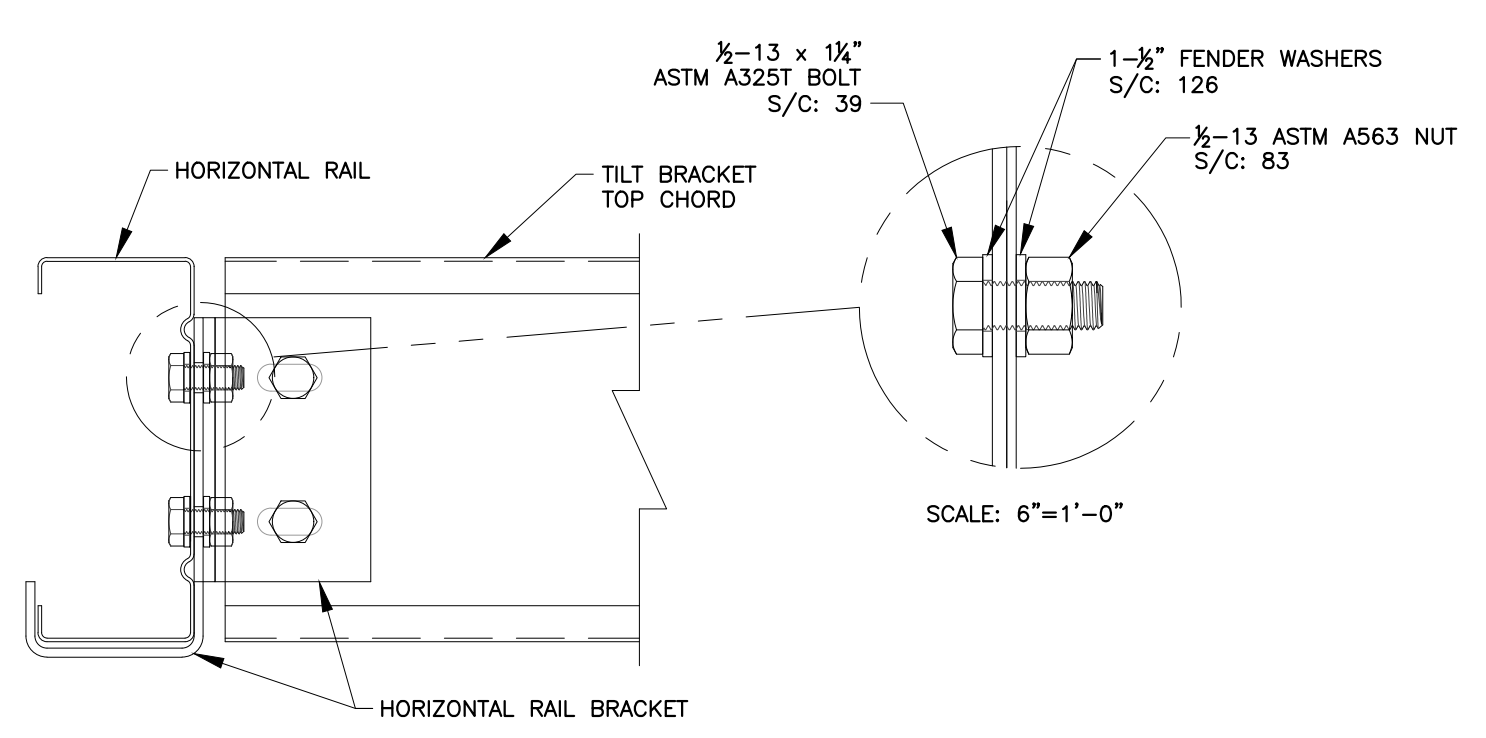
11 SPLICE CONN. FLANGE
SCALE: 3" = 1'-0"



4 HORIZ. RAIL TO HORIZ. BRACING CONN.
SCALE: 3" = 1'-0"

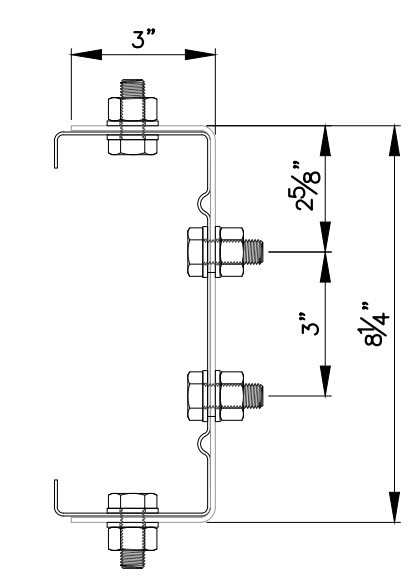


5 HORIZ. RAIL BRACKET TO TILT BRACKET CONN.
SCALE: 3" = 1'-0" (PRE-ASSEMBLED NEEDS TIGHTENED)

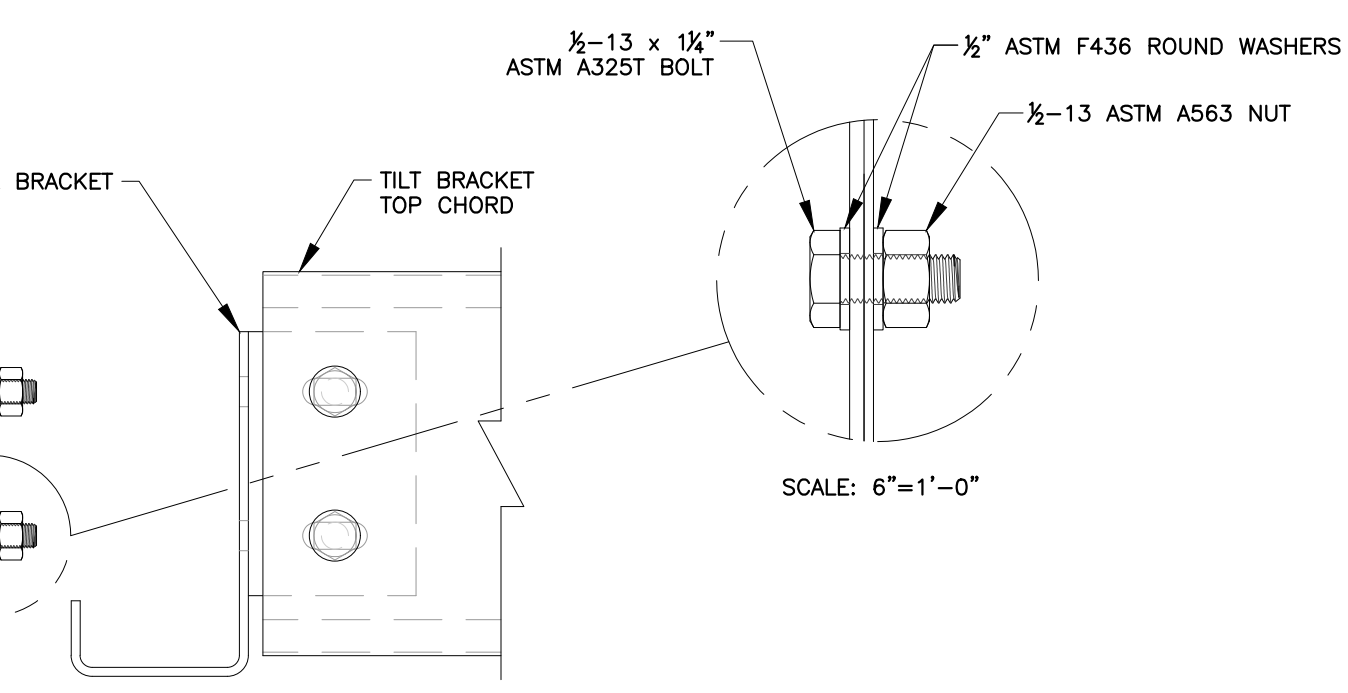


6 TILT BRACKET TO HORIZ. CONN.
SCALE: 3" = 1'-0"

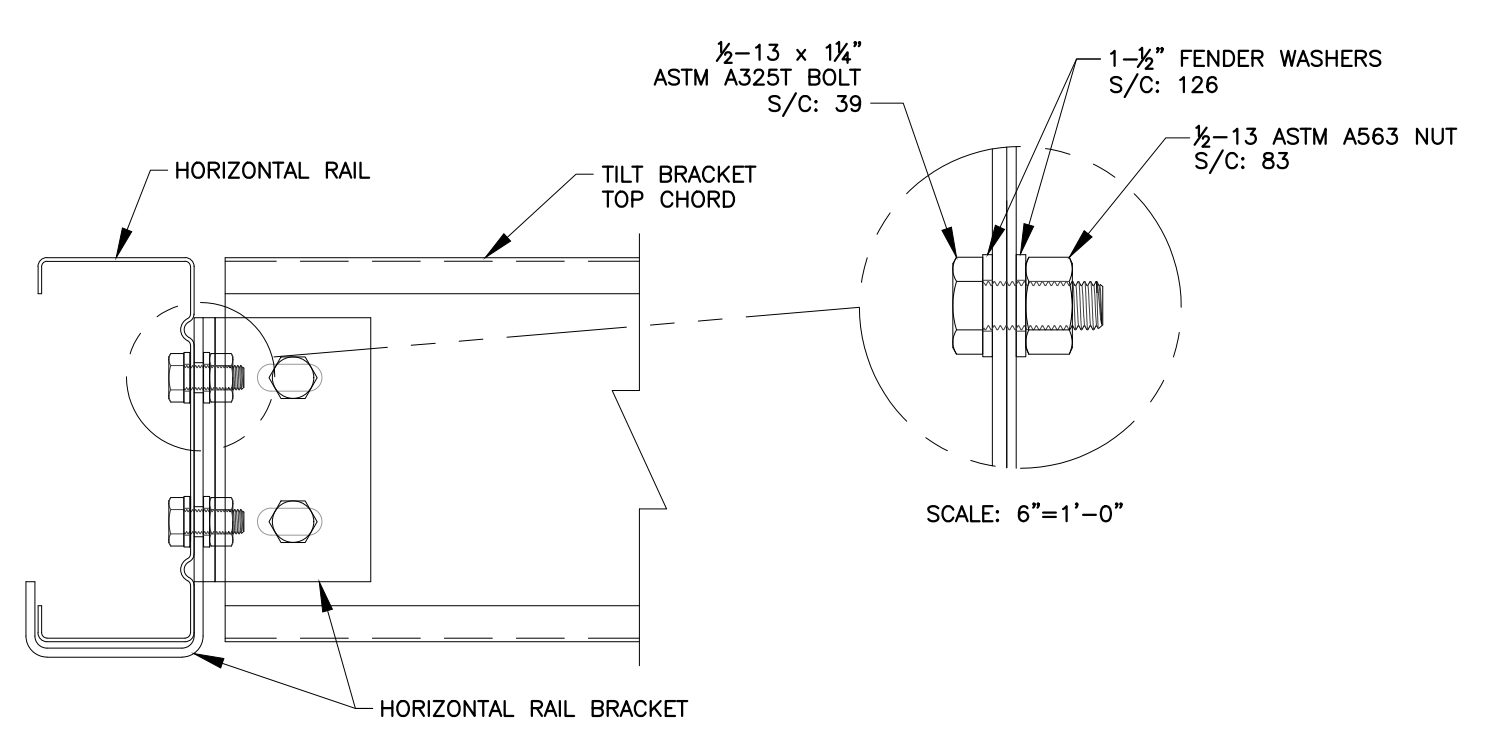
7 SPLICE PROFILE
SCALE: 3" = 1'-0"



4 HORIZ. RAIL TO HORIZ. BRACING CONN.
SCALE: 3" = 1'-0"

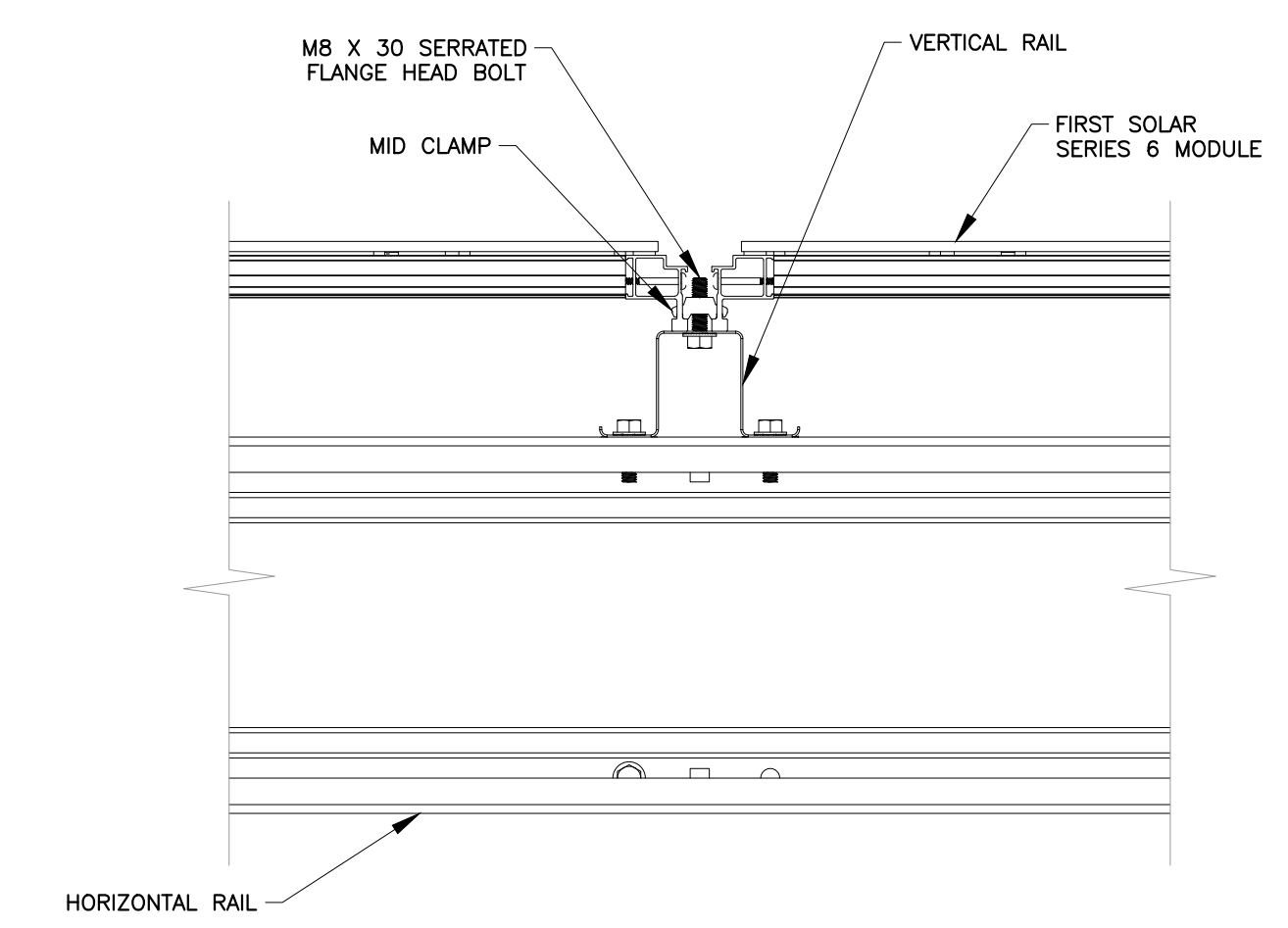
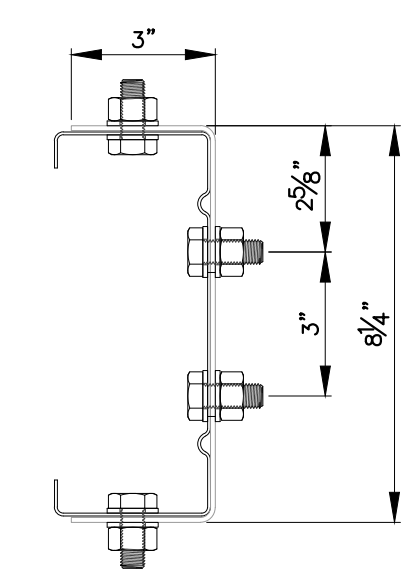


5 HORIZ. RAIL BRACKET TO TILT BRACKET CONN.
SCALE: 3" = 1'-0" (PRE-ASSEMBLED NEEDS TIGHTENED)

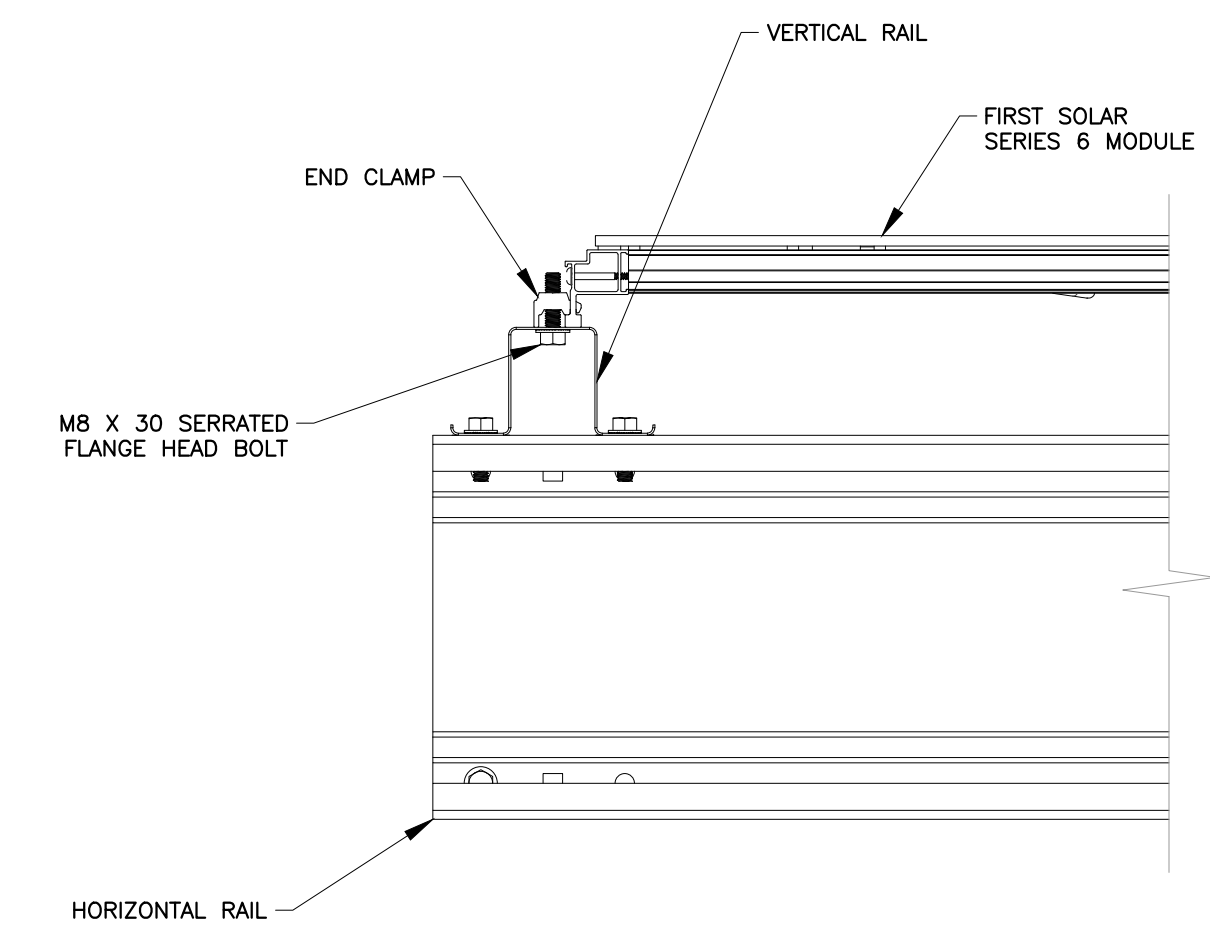


6 TILT BRACKET TO HORIZ. CONN.
SCALE: 3" = 1'-0"

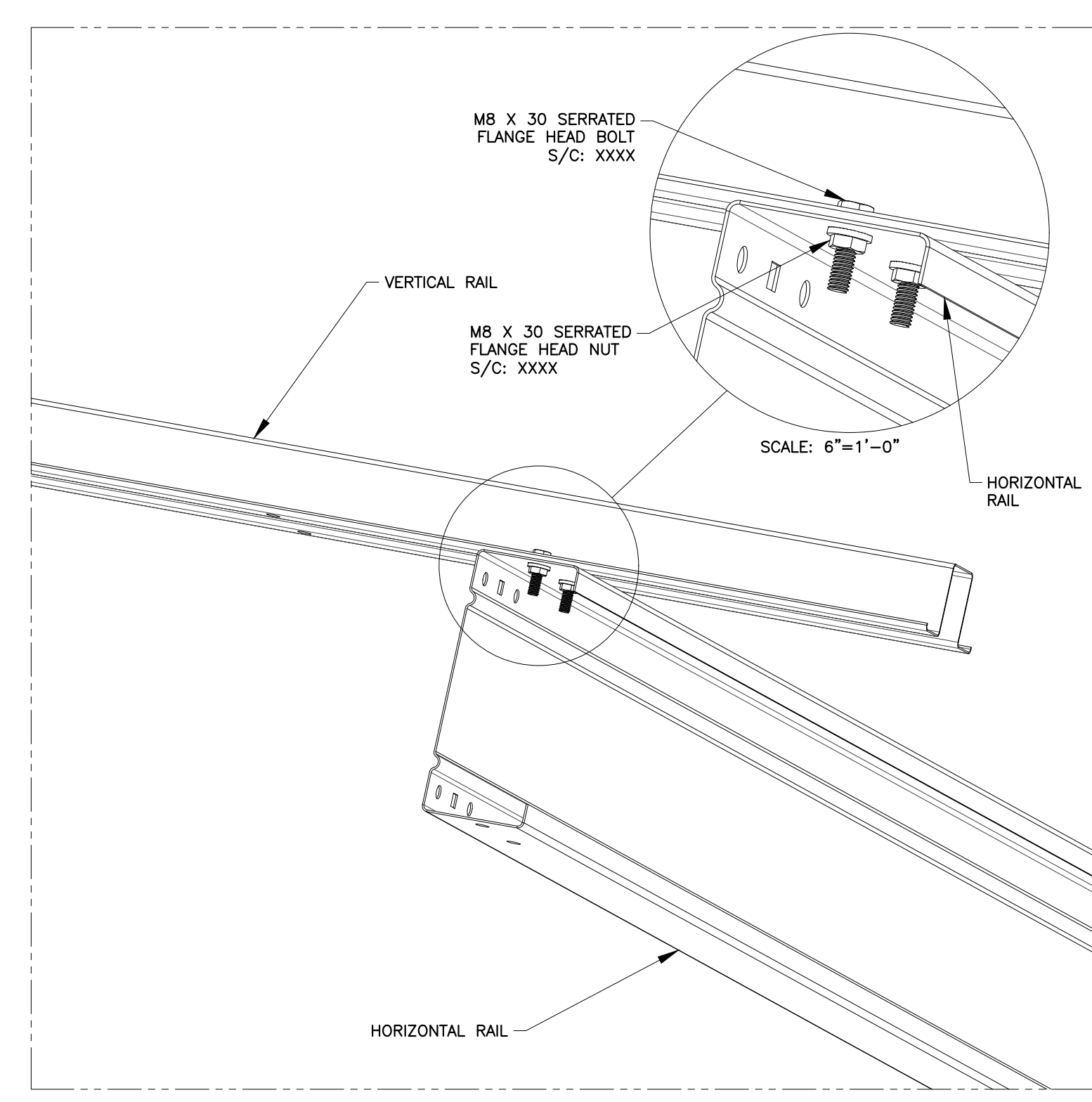
7 SPLICE PROFILE
SCALE: 3" = 1'-0"



1 MID CLAMP DETAIL
SCALE: 3" = 1'-0"



2 END CLAMP DETAIL
SCALE: 3" = 1'-0"



3 HAT CHANNEL TO HORIZ. RAIL CONN.
SCALE: 3" = 1'-0"

PURE POWER CONTRACTORS
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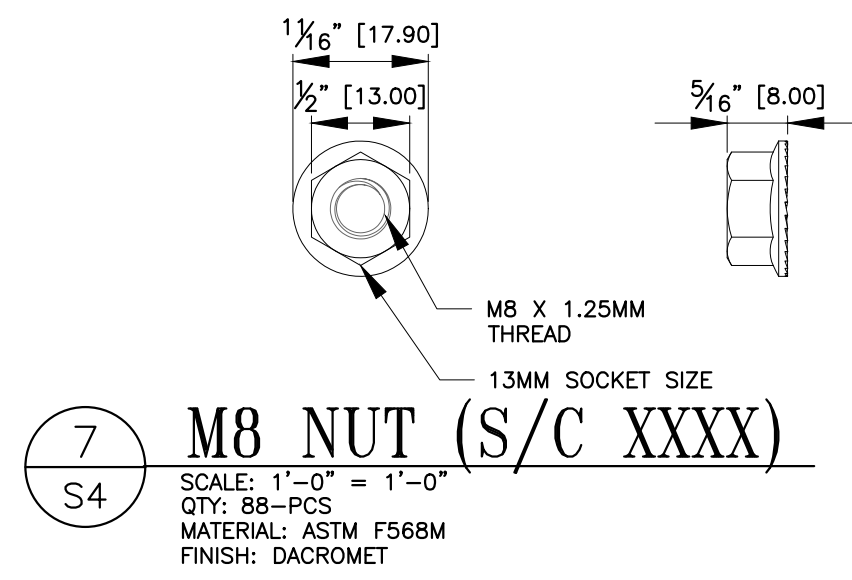
CONNECTIONS

SOLAR FLEX RACK
A Division of Northern States Metals

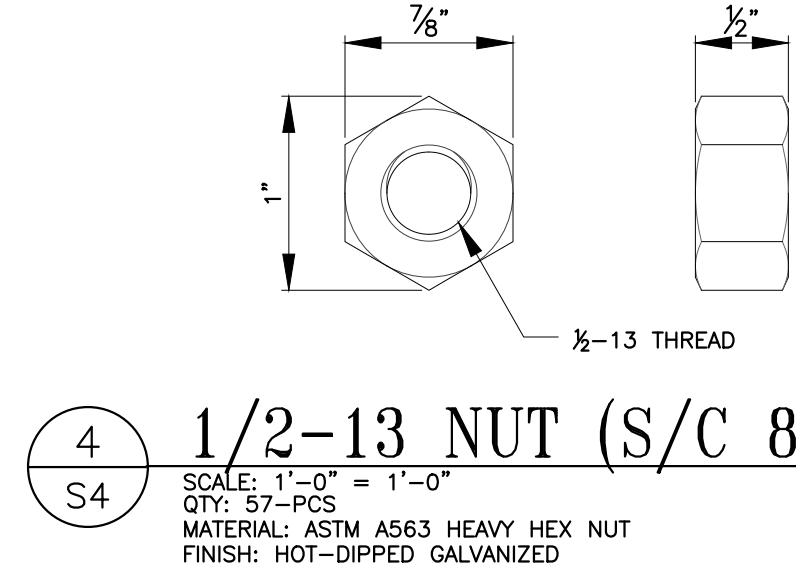
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Phone (888) 380-8138

DATE	11/4/20	JOB #	11442	DRAWN BY	DK	CHECK BY	S3	S3	S5
DATE	3/19/2020	JOB #	11442	DRAWN BY	JS	CHECK BY	DK	S3	S5
REV #	1	DESCRIPTION	UPDATED SITE ADDRESS AND MODULE WATTAGE	DATE	JS	DATE	2/20/2020		

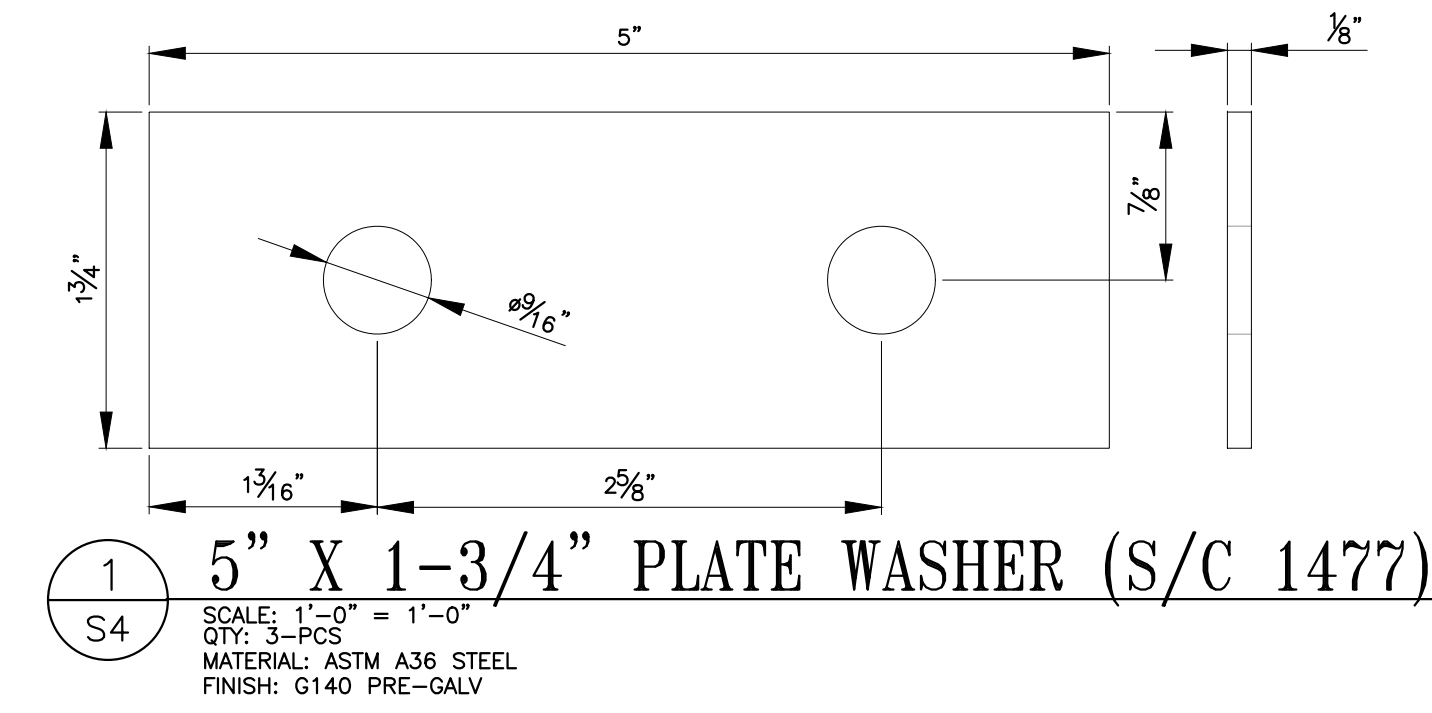
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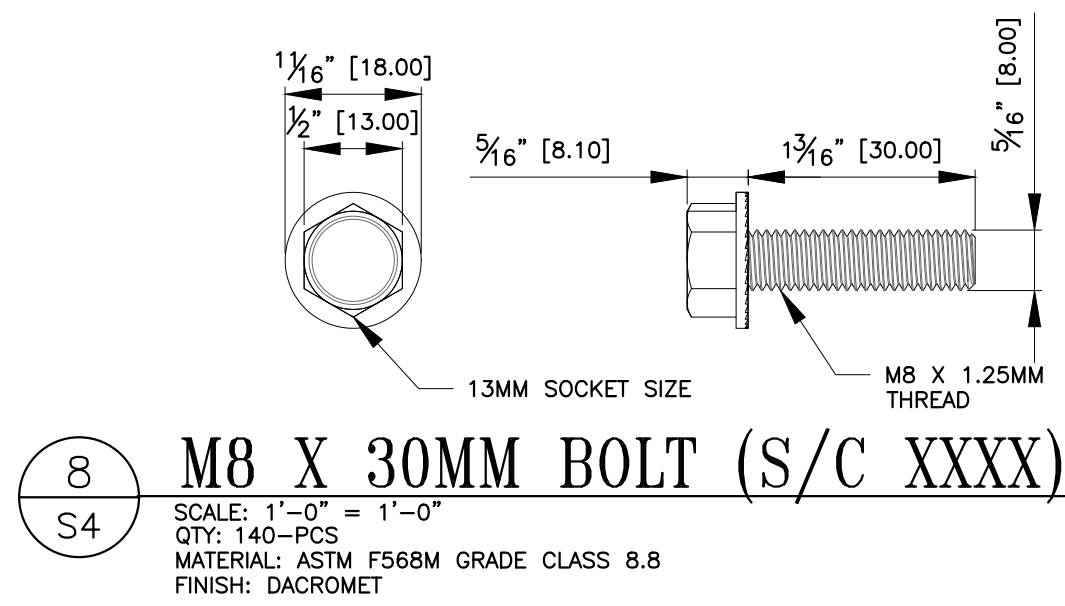
7
S4
M8 NUT (S/C XXXX)
SCALE: 1'-0" = 1'-0"
QTY: 88-PCS
MATERIAL: ASTM F568M
FINISH: DACROMET



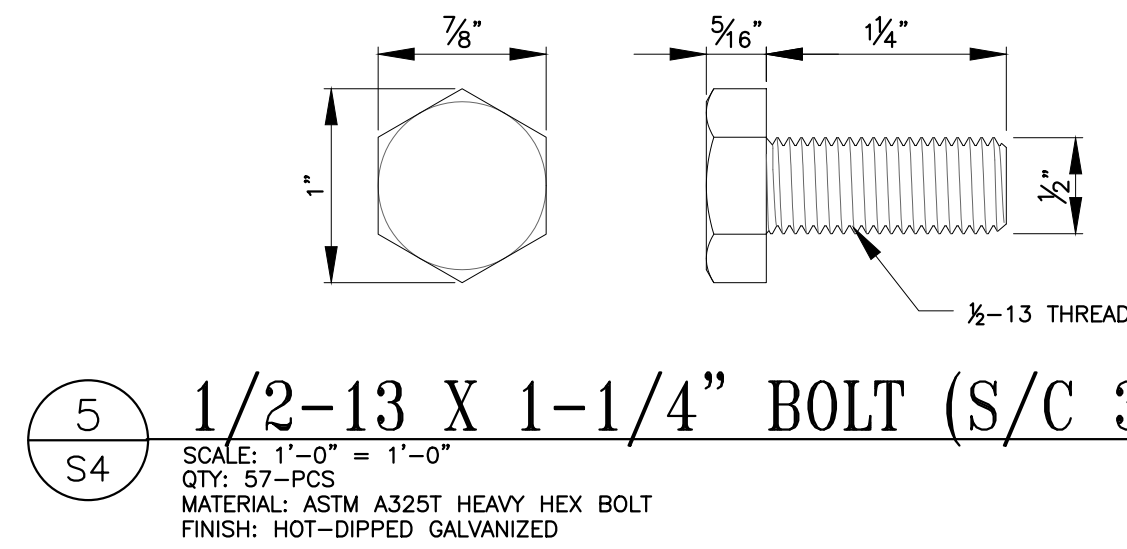
4
S4
1/2-13 NUT (S/C 83)
SCALE: 1'-0" = 1'-0"
QTY: 57-PCS
MATERIAL: ASTM A563 HEAVY HEX NUT
FINISH: HOT-DIPPED GALVANIZED



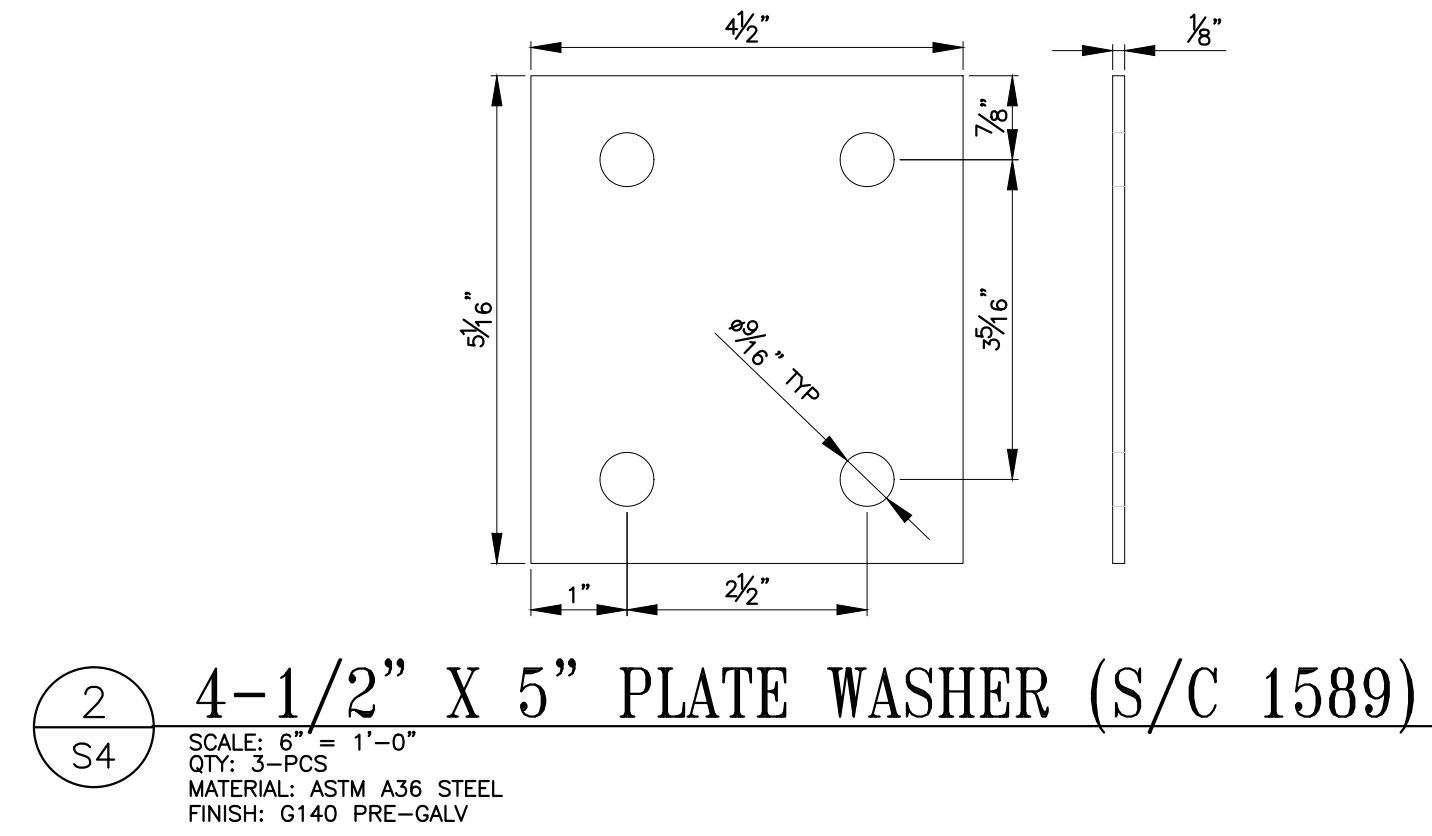
1
S4
5" X 1-3/4" PLATE WASHER (S/C 1477)
SCALE: 1'-0" = 1'-0"
QTY: 3-PCS
MATERIAL: ASTM A36 STEEL
FINISH: G140 PRE-GALV



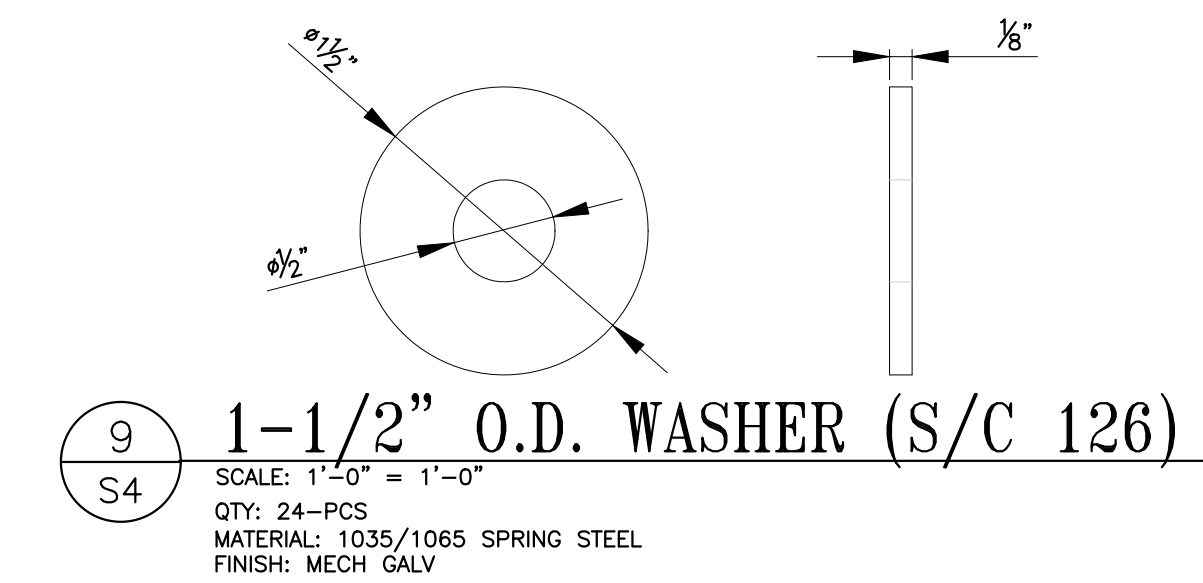
8
S4
M8 X 30MM BOLT (S/C XXXX)
SCALE: 1'-0" = 1'-0"
QTY: 140-PCS
MATERIAL: ASTM F568M GRADE CLASS 8.8
FINISH: DACROMET



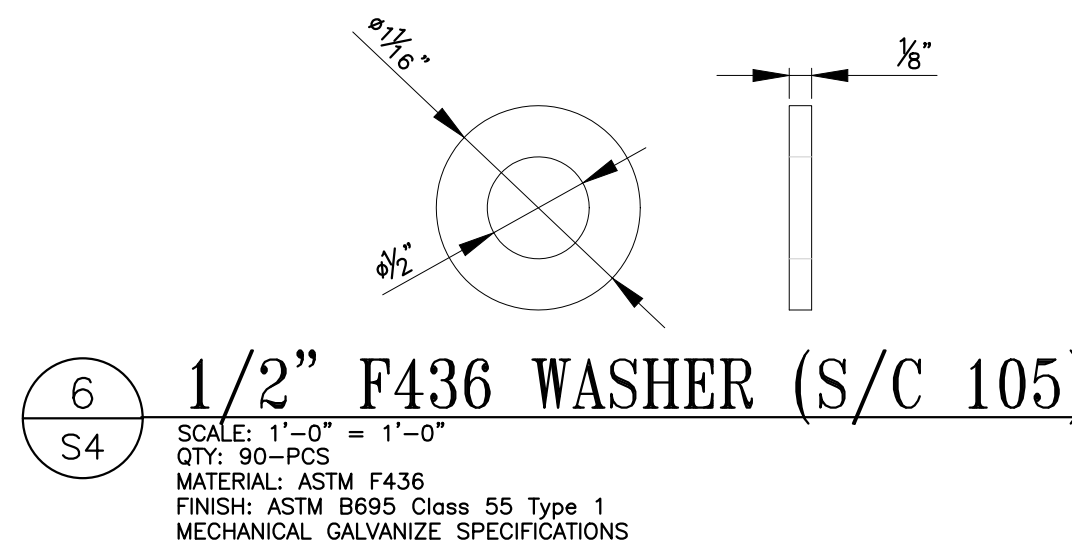
5
S4
1/2-13 X 1-1/4" BOLT (S/C 39)
SCALE: 1'-0" = 1'-0"
QTY: 57-PCS
MATERIAL: ASTM A325T HEAVY HEX BOLT
FINISH: HOT-DIPPED GALVANIZED



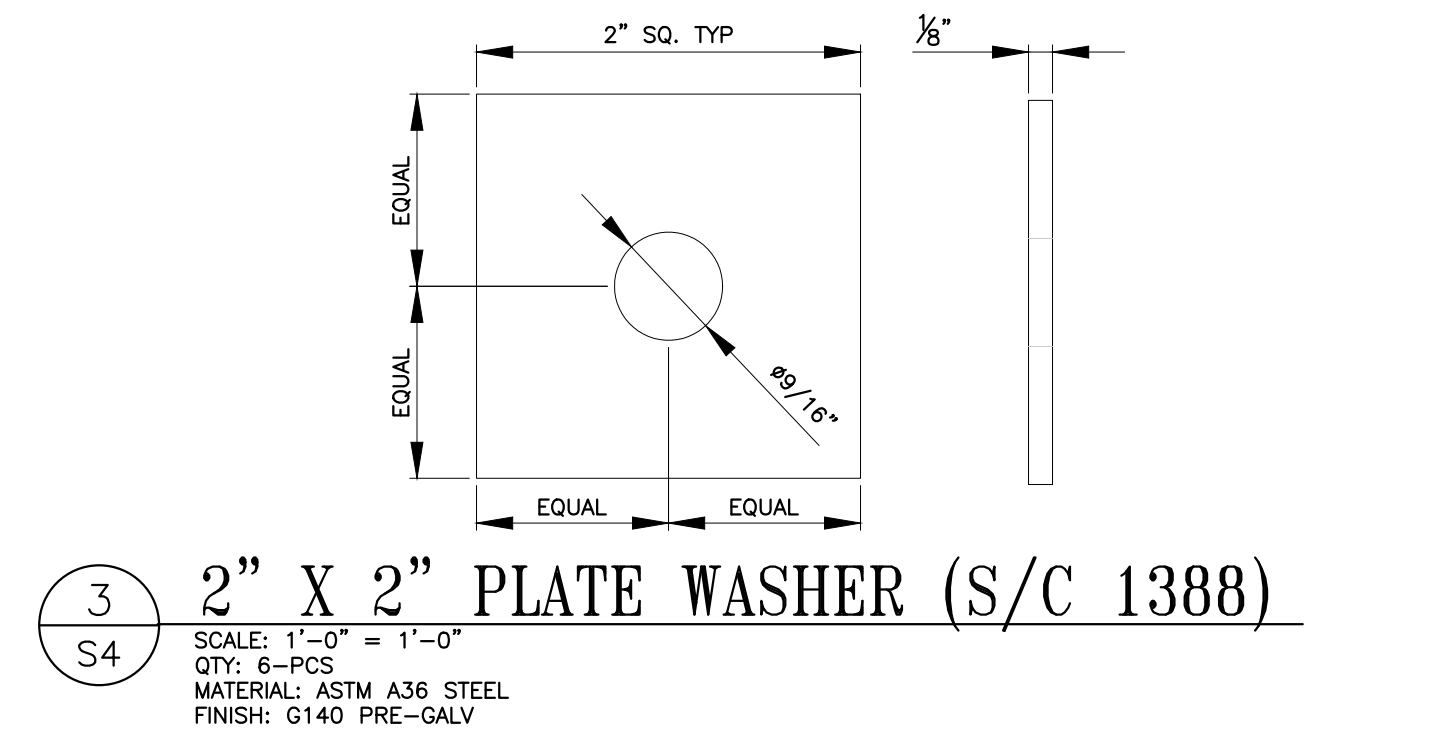
2
S4
4-1/2" X 5" PLATE WASHER (S/C 1589)
SCALE: 6" = 1'-0"
QTY: 3-PCS
MATERIAL: ASTM A36 STEEL
FINISH: G140 PRE-GALV



9
S4
1-1/2" O.D. WASHER (S/C 126)
SCALE: 1'-0" = 1'-0"
QTY: 24-PCS
MATERIAL: 1035/1065 SPRING STEEL
FINISH: MECH GALV



6
S4
1/2" F436 WASHER (S/C 105)
SCALE: 1'-0" = 1'-0"
QTY: 90-PCS
MATERIAL: ASTM F436
FINISH: ASTM B695 Class 55 Type 1 MECHANICAL GALVANIZE SPECIFICATIONS



3
S4
2" X 2" PLATE WASHER (S/C 1388)
SCALE: 1'-0" = 1'-0"
QTY: 6-PCS
MATERIAL: ASTM A36 STEEL
FINISH: G140 PRE-GALV

CUSTOMER: **PURE POWER CONTRACTORS**
BUJES CREEK SOLAR PROJECT
LILLINGTON, NC 27546

DATE: 3/19/2020
JOB #: 11442
DRAWN BY: JS
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PAGE: S4 of S5

Sheet **S4**

HARDWARE

SOLAR FLEX RACK
A Division of Northern States Metals

3207 Innovation Place
Youngstown, OH 44509-4023
Phone (888) 380-8138

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1	UPDATED SITE ADDRESS AND MODULE MATTAGE	2/20/2020

VALIDATION LOAD TESTING PROCEDURE:

1. ALTERNATE FOUNDATION SOLUTIONS PROVIDED DUE TO POTENTIAL REFUSALS IN THE FIELD.
2. FOR POSTS THAT REFUSE BETWEEN 0 FEET AND 4 FEET, REMOVE THE OBSTRUCTION, REPLACE AND COMPACT NATIVE SOIL AND ATTEMPT TO RE-DRIVE TO MINIMUM DESIGN EMBEDMENT DEPTH.
3. POSTS THAT MEET REFUSAL PRIOR TO ACHIEVING THE MINIMUM DESIGN EMBEDMENT DEPTH BUT ACHIEVING THE MINIMUM ALTERNATE EMBEDMENT DEPTH SHALL BE PULL TESTED TO THE LOADS AS SPECIFIED WITHIN THE PROJECT SPECIFIC INFORMATION TABLE. LATERAL LOAD TO BE APPLIED HORIZONTALLY AT 4 FEET ABOVE THE GROUND SURFACE IN THE DIRECTION OF THE STRONG AXIS. AXIAL AND LATERAL LOADS TO BE SUSTAINED FOR A DURATION OF AT LEAST 4 MINUTES. IF THE POSTS RESIST THE LOADS WITHIN THE ALLOWABLE TOLERANCES OF UP TO 1 INCH AXIAL DISPLACEMENT AND UP TO 0.75 INCH LATERAL DEFLECTION AT GRADE, IT WILL BE DEEMED ACCEPTABLE. THE POST SHALL BE CUT, RE-DRILLED, SPRAY GALVANIZED, AND PUT INTO SERVICE.
4. FOR POSTS THAT FAIL THE AXIAL OR LATERAL VALIDATION LOAD TEST, OR POSTS THAT DO NOT ACHIEVE A MINIMUM ALTERNATE EMBEDMENT DEPTH REQUIRED FOR LOAD TESTING, ONE OF THE ALTERNATE FOUNDATIONS SHALL BE USED.
5. PLEASE CONFIRM MINIMUM ALTERNATE EMBEDMENT DEPTH AND TEST LOADS AND PROCEDURES WITH GEOTECHNICAL ENGINEER.

ALTERNATE FOUNDATION INSTALLATION – OPTION A DRILLED SHAFT (1-S5 & 2-S5):

1. EXTRACT THE REFUSED POST.
2. DRILL A SHAFT TO THE DIMENSIONS AS SPECIFIED IN PROJECT SPECIFIC INFORMATION TABLE SHOWN BELOW. DE-WATERING MAY BE REQUIRED.
3. MAINTAINING THE POST PLUMB TO THE INSTALLATION TOLERANCES AS SPECIFIED, BACKFILL DRILLED SHAFT WITH CONCRETE TO THE ELEVATION SHOWN.
4. WHEN CONCRETE IS SET, BACKFILL FROM BOTTOM OF FROST DEPTH TO GRADE WITH PEA GRAVEL. NO SAND OR FINES.

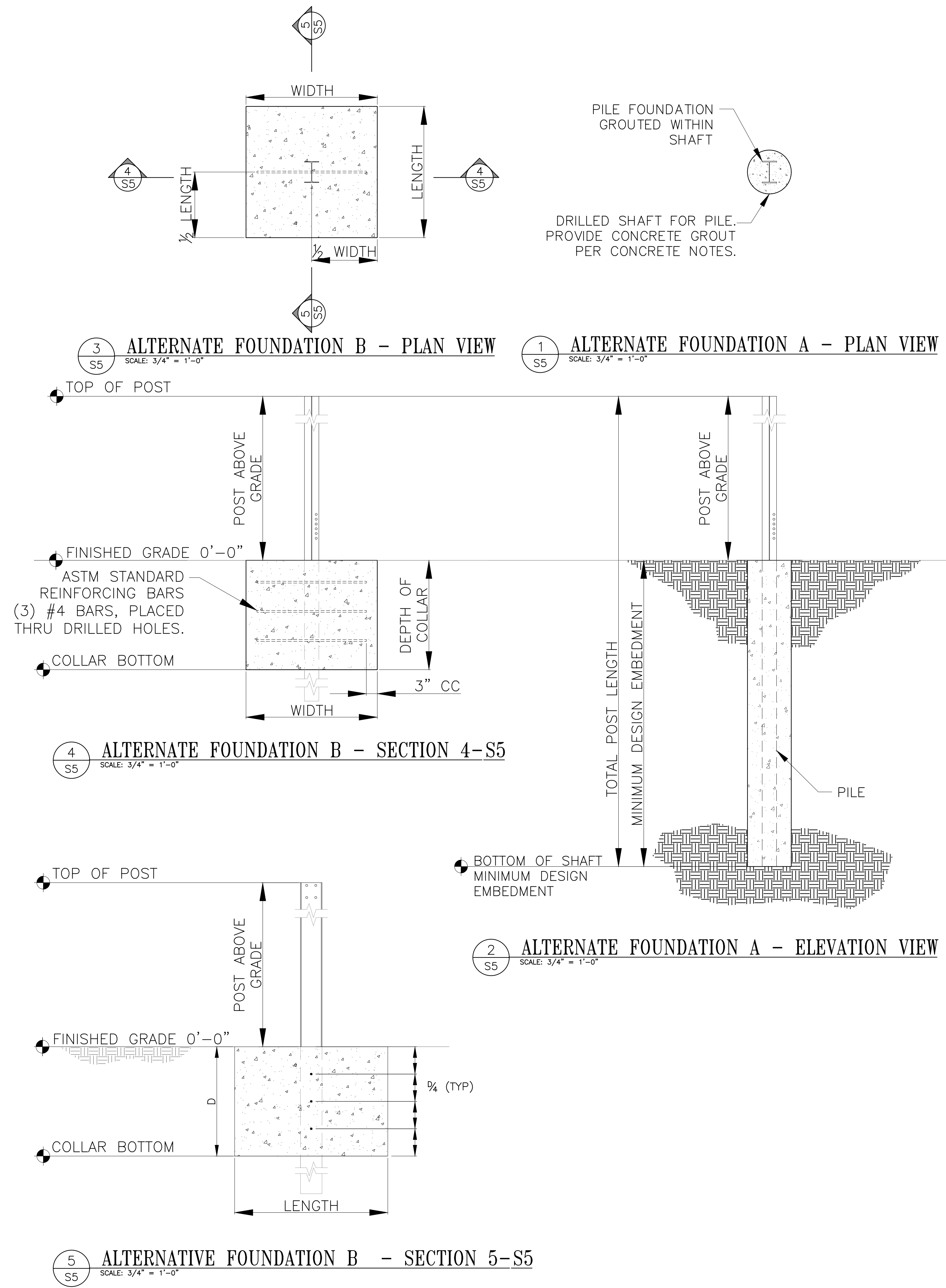
ALTERNATE FOUNDATION INSTALLATION – OPTION B CONCRETE COLLAR (3-S5 THRU 5-S5):

1. WITH THE REFUSED POST REMAINING IN PLACE, EXCAVATE A PIT TO AT LEAST THE DIMENSIONS SPECIFIED IN THE PROJECT SPECIFIC INFORMATION TABLE. DE-WATERING MAY BE REQUIRED.
2. COMPACT THE BASE OF THE COMPLETED EXCAVATION AS REQUIRED TO PRODUCE A FIRM NON-YIELDING SURFACE.
3. DEPENDING ON THE SITE LOCATION, CONCRETE FORMS MAY BE REQUIRED. SEE PROJECT SPECIFIC INFORMATION TABLE BELOW. PLEASE CONFIRM WITH THE GEOTECHNICAL ENGINEER.
4. PLACE FORMS AND POUR CONCRETE. FOLLOW THE CONCRETE NOTES BELOW.
5. WHEN CONCRETE IS SET, REMOVE FORMS, AND WRAP COLLAR 5 TIMES WITH 8 MIL POLYETHYLENE PLASTIC SHEETING. BACKFILL OVER COLLAR TO GRADE AS REQUIRED WITH COMPACT, CLEAN NATIVE MINERAL SOIL OR IMPORTED STRUCTURAL FILL.

CONCRETE NOTES:

1. ALL CONCRETE SHALL BE 3500 PSI @ 28 DAYS.
2. PLEASE VERIFY ACCEPTANCE OF CONCRETE WITH ACI 318-11 (SECTION 5.6)
 - 2.1. PER ACI 5.6.2.3, WHEN TOTAL QUANTITY OF CONCRETE USED ON A PROJECT IS LESS THAN 50 CY, STRENGTH TESTS ARE NOT REQUIRED WHEN EVIDENCE OF SATISFACTORY STRENGTH IS SUBMITTED TO AND APPROVED BY THE BUILDING OFFICIAL.
 - 2.2. CONCRETE TEST CYLINDERS SHALL BE TAKEN IN ACCORDANCE WITH THE ACI CODE. ONE SET OF FIVE CYLINDERS SHALL BE TAKEN FOR EACH DAYS POUR, OR EACH 20 CY OF CONCRETE POURED. ONE SLUMP TEST SHALL BE TAKEN FOR EACH SET OF CYLINDERS. TWO CYLINDERS PER SET SHALL BE TESTED AT SEVEN DAYS AND ANOTHER TWO CYLINDERS PER SET TESTED AT 28 DAYS. SUBMIT TEST REPORTS TO THE ENGINEER.
3. CONCRETE SUBJECT TO FREEZE-THAW CONDITIONS SHALL BE AIR ENTRAINED AT 5-8% AND HAVE A MAXIMUM SLUMP OF 3 INCHES.
4. COLD WEATHER CONCRETING PRACTICES IN ACCORDANCE WITH THE ACI CODE SHALL BE FOLLOWED WHEN THE OUTDOOR AIR TEMPERATURE IS PREDICTED TO BE 40 DEGREE F AND FALLING. NO CALCIUM CHLORIDE SHALL BE ADDED TO ANY CONCRETE.
5. NORMAL WEIGHT CONCRETE, 145 PCF, TO BE USED UNLESS OTHERWISE NOTED
6. MAXIMUM WATER TO CEMENT RATIO OF 0.50 MUST BE USED UNLESS OTHERWISE NOTED.

PROJECT SPECIFIC INFORMATION			
1	VALIDATION TEST LOADS	AXIAL 3,200 LB	LATERAL 2,200 LB
2	MINIMUM ALTERNATE EMBEDMENT (FOR TESTING)	7'-6"	
3	OPTION A: DRILLED SHAFT DIMENSIONS	8'-11"–DESIGN EMBEDMENT, 12" DIAMETER	
4	OPTION B: RECTANGULAR COLLAR DIMENSIONS	3'-9" WIDTH BY 5'-9" LENGTH BY 2'-0" DEPTH	
5	FROST DEPTH	0'-0"	
6	ARE CONCRETE FORMS REQUIRED?	NO	



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ALTERNATE FOUNDATION DETAILS

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